DRAINAGE INFOR	MATION SHEET $G-16/D1337$
SMITH ENGINEERING CO. APPLICANT'S NAME: DOUGLAS L. ANDREWS	ZONE ATLAS/DRNG FILE #: (51 - 1/0
DRB #: N/A EPC #: N/A	WORK ORDER #:
LEGAL DESCRIPTION: COMANCHE PARK	· · · · · · · · · · · · · · · · · · ·
CITY ADDRESS: NIEKSECTION OF COMAUCHE K	D. AND BEYN MAWE DR. NE
ENGINEERING FIRM: SMITH ENGINEERING CO.	CONTACT: DOUGLAS L. ANDREWS
ADDRESS: 6400 UPTOWN BLUD. NE 87110 COA-DEPT. OF MUNICIPAL DEVELOPMENT OWNER: PARK & MEDIAN DESIGN DIVISION	PHONE: 505-884-0700
OWNER: PARK MEDIAN DESIGN DIVISION	CONTACT: DAVID FLORES
ADDRESS: 1801 4-51 NW 8710Z	PHONE: 505-76R-5300
ARCHITECT: CONSENSUS PLANNING	CONTACT: CHEIS GIREEN
ADDRESS: 924 PARK AVE SW 87107	PHONE: 505 -764-9801
SURVEYOR:	CONTACT:
ADDRESS:	PHONE:
CONTRACTOR:	CONTACT:
ADDRESS:	PHONE:
TYPE OF SUBMITTAL:  DRAINAGE REPORT  DRAINAGE PLAN  CONCEPTUAL GRADING & DRAINAGE PLAN  GRADING PLAN  EROSION CONTROL PLAN  ENGINEER'S CERTIFICATION  OTHER  PRE-DESIGN MEETING:  YES  NO  COPY PROVIDED	CHECK TYPE OF APPROVAL SOUGHT:  SKETCH PLAT APPROVAL  PRELIMINARY PLAT APPROVAL  S. DEV. PLAN FOR SUB'D APPROVAL  S. DEV PLAN FOR BLDG. PERMIT APPROVAL  SECTOR PLAN APPROVAL  FINAL PLAT APPROVAL  FOUNDATION PERMIT APPROVAL  CERTIFICATE OF OCCUPANCY APPROVAL  CERTIFICATE OF OCCUPANCY APPROVAL  PAVING PERMIT APPROVAL  S.A.D. DRAINAGE REPORT  DRAINAGE REQUIREMENTS  SUBDIVISION CERTIFICATION  OTHER  (SPECIFY)
DATE SUBMITTED: 17-79-03  BY: DOUGUAS L. ANDER	DEC 2 9 2003

Revised 02/98



# City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

February 16, 2004

Doug Andrews, PE Smith Engineering Co. 6400 Uptown Blvd. Ste 500E Albuquerque, NM 87110

Re: Comanche Park Drainage Report

Engineer's Stamp dated 12-29-03, (G16/D133)

Dear Mr. Andrews,

Based on your information provided in your submittal dated 12-29-03, the above referenced report is approved for Grading Permit. Please submit a copy of the plan, with AMAFCA's signature denoting their approval to grade in their R/W, for my signature in order to obtain the Permit. This project requires a National Pollutant Discharge Elimination System (NPDES) permit. Refer to the attachment that is provided with this letter for details. If you have any questions please feel free to call the Municipal Development Department, Hydrology section at 768-3654 (Charles Caruso) or 768-3645 (Bryan Wolfe).

If you have any questions, you can contact me at 924-3986.

Sincerely,
Brady S. Brunn

Bradley L. Bingham, PE

Principal Engineer, Planning Dept. Development and Building Services

C: Chuck Caruso, CoA Lynn Mazur, AMAFCA file

# DRAINAGE REPORT

for

# COMANCHE PARK – PHASE 1

# CITY OF ALBUQUERQUE, NEW MEXICO

# Prepared for:

# CITY OF ALBUQUERQUE DEPARTMENT OF MUNICIPAL DEVELOPMENT PARK AND MEDIAN DESIGN DIVISION

December, 2003

December, 2003

DECEMBER 11771



Smith Engineering Company

A Full-Service Engineering Company

Prepared by:

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#### **APPENDICIES**

Land Treatment Summary
AHYMO Basin Parameter Worksheets, Peak Basin Flows, and
Volumes
AHYMO Summary Tables

## MAPS – in back pockets of Report

Map No. 1	Off-Site Drainage Area Map
Map No. 2	On-Site Drainage Area Map – Existing Condition
Map No. 3	On-Site Drainage Area Map - Proposed Condition

# CIVIL CONSTRUCTION DRAWINGS – in back pockets of Report

Sheet 6 of 13	Civil Site Plan
Sheet 7 of 13	Grading Plan
Sheet 8 of 13	Civil Details
Sheet 9 of 13	Retaining Wall Detail
Sheet 10 of 13	Field Retaining Wall Detail

#### I. INTRODUCTION

Comanche Park is located just east of the North Diversion Channel. The park is actually two parks (north and south) separated by Comanche Road. The primary purpose of the park or parks is to serve as a detention pond facility for storm water runoff. The southern portion of the park currently drains to the northern portion via an existing 36" storm drain pipe under Comanche Road. The southern portion is also the home of Thunderbird Little League while the northern portion is strictly an earthen detention pond facility. A lift/pump station currently lifts the storm water from the northern park/ponding area to the North Diversion Channel.

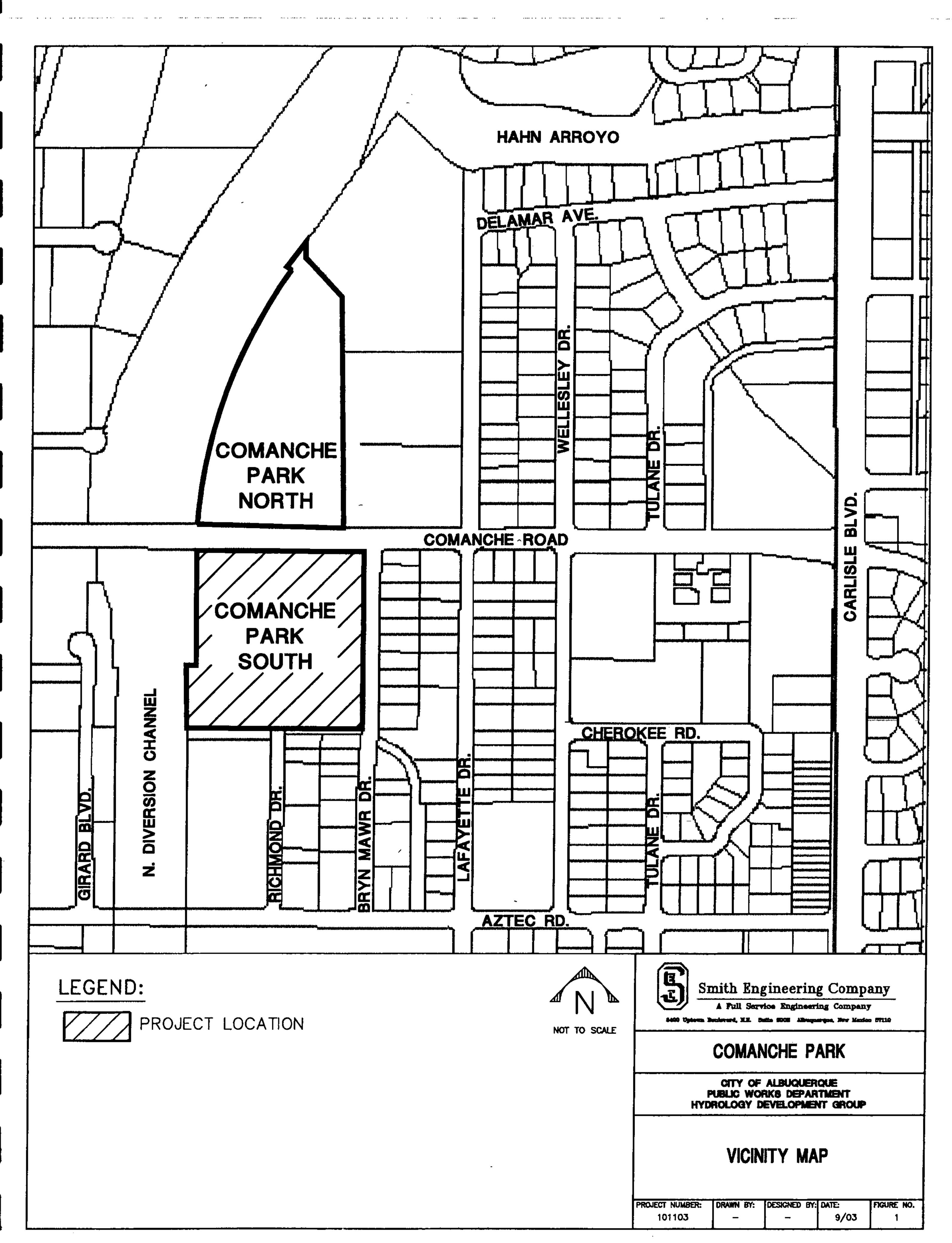
The City of Albuquerque Park and Median Design Division is planning to renovate the Thunderbird Little League baseball fields that lie within the southern portion of Comanche Park. The southern park consists of approximately 8.5 acres. Figure No. 1 shows the location of both parks.

Smith Engineering Company (SEC) has been retained by Consensus Planning Inc. to provide civil engineering services for the proposed park improvements. Included in the civil engineering scope of work is to provide drainage engineering services for the redevelopment of the site.

This drainage report will analyze both the existing hydraulic capacity of the south and north ponds as well as the impact the proposed improvements have on both the south and north detention pond facilities. The main purpose of this report is to show the proposed improvements to the south park will not negatively impact the pond volume of either pond.

#### II. DRAINAGE AREA BOUNDARIES

The off-site drainage basin that contributes to the Comanche Park detention facilities is generally bounded on the east by Carlisle Blvd., the south by Candelaria Road, the west by the North Diversion Channel and the north by the Hanh Arroyo. A small portion of the contributing drainage basin is located north of the Hahn Arroyo with these storm water flows routed to the north park via a 36" storm drain pipe under the existing concrete lined Hahn Arroyo. The drainage area boundary described above can be seen on Map No. 1 located in the back pocket of this report.



#### III. DRAINAGE CHARACTERISTICS

#### A. Existing Topography

The off-site contributing drainage basin generally slopes from the east to the west with a surface gradient varying from two to three percent with most of the storm water flows directed to public right of ways.

The southern park/pond is currently graded relatively flat (with fairly steep slopes at the parks outer boundaries) with the general grade from the south and southeast to the north to the existing 36-inch inlet structure connecting the south park/pond to the north park/pond. An existing small earthen channel currently exists within the south park transporting storm water through the park from the southeast corner of the park to the 36-inch concrete inlet structure mentioned above.

The northern park/pond is also currently graded relatively flat (with fairly steep slopes at the parks outer boundaries) from the east to the west and ultimately to the existing lift/pump station located in the north park/pond just east of the North Diversion Channel. The topography of both the off-site basins and the on-site basins are shown on Maps 1 through 3 located in the back pockets of this report

#### B. Existing Vegetation

The off-site drainage basin is approximately 95% developed. The analysis included in this report assumes the off-site drainage basin is 100% developed. The south park currently consists of five grass baseball fields and small sporadic dirt parking areas. The proposed south park improvements will consist of five dirt baseball fields, a concrete low flow channel and an unpaved (base course) parking area. The north pond is currently undeveloped and consists of native grass vegetation. No new proposed improvements to the north park will be analyzed as part of this report.

#### C. Land Use

Developed condition land uses were determined from City of Albuquerque Zone Atlas Maps as well as 1999 Aerial Photographs. Various land uses exist within the off-site basin including commercial, office and various types of residential. The north and south park sites are currently zoned residential which is typical for park zoning. Figure No. 2 shows current City of Albuquerque zoning. Map No. 1, located in the back pocket shows the 1999 Aerial Photograph used to verify the zoning and aid in the determining land use. Appendix A displays the various zoning designations and their associated land treatments used in the hydrologic analysis.

#### IV. EXISTING DRAINAGE FACILITIES

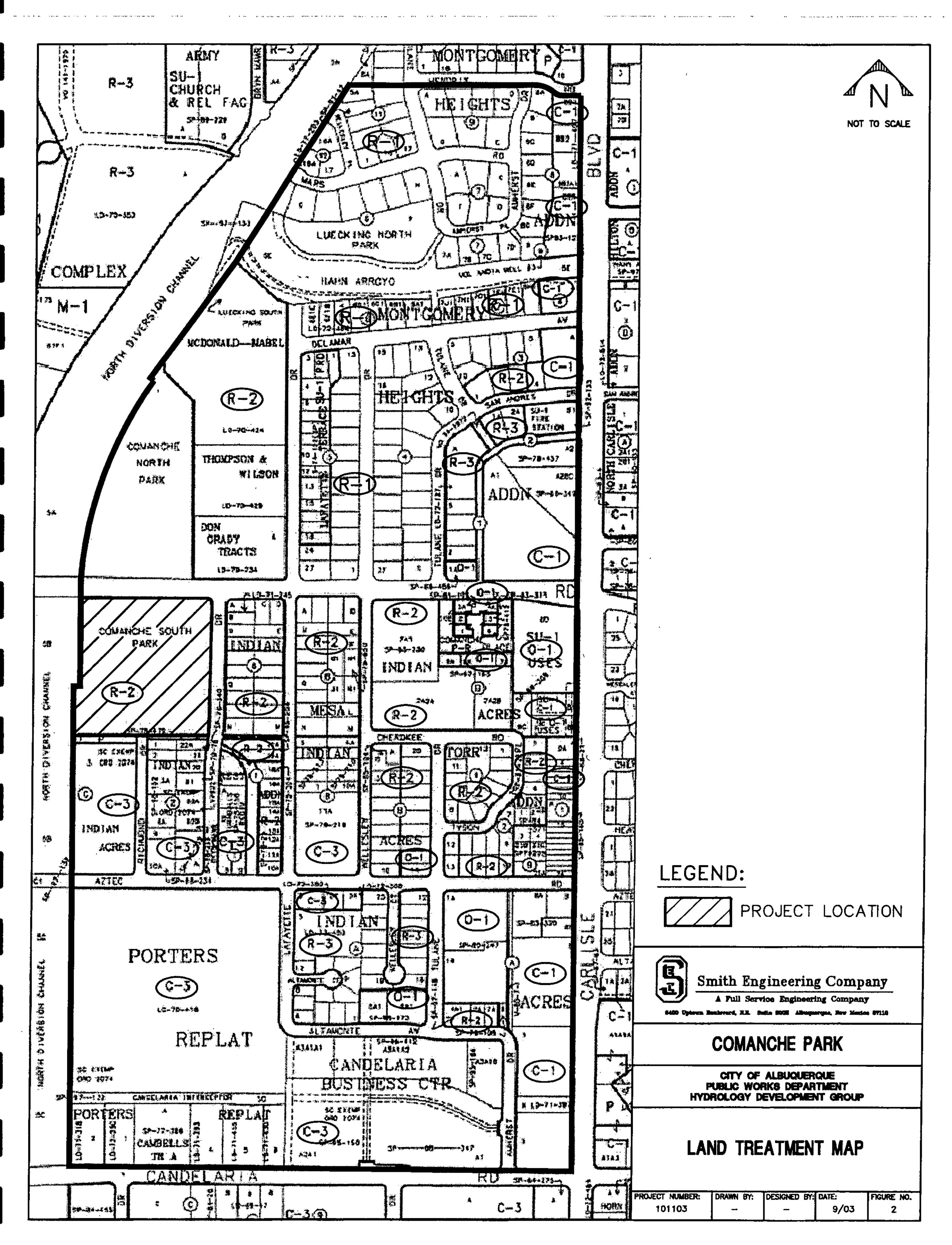
The primary source of storm water entering the south park/pond is via an unlined/earthen arroyo/channel located southeast of the south park. The open channel crosses Bryn Mawr Dr. and enters the park from the southeast corner of the park. It should be noted that there is no drainage crossing structure in/under Bryn Mawr Dr. to

convey the off-site flow under Bryn Mawr Dr. The storm water currently drains over the paved roadway and into the south park/pond. As stated above, an existing 36-inch storm drain pipe connects the south park/pond to the north park/pond. The 36-inch pipe drains water from the south pond to the north pond.

A storm drain system also exists at the intersection of Comanche Road and Bryn Mawr Drive. The system consists of seven catch basins, 18-inch to 30-inch connector pipes and a 36-inch storm drain pipe that empties into the northeast corner of the north park/pond. This storm drain system collects storm water runoff at low point at the intersection. This existing storm drain system is limited by the capacity of the 36" pipe emptying into the north park/pond (approximately 94 cfs) thus; the remaining flow overtops the curb and enters the south park/pond (at the northeast corner of the south park/pond) as overland flow.

Another existing 36" storm drain line also exists north of the North Park/Pond that currently drains water into the north pond. This is the same pipe discussed in a previous section that drains water from the north under the Hahn Arroyo and south to the north park/pond. Eventually, an existing lift/pump station located in the north park/pond pumps the storm water from the north park/pond into the North Diversion Channel.

Map No. 1, 2 and 3 located in the back pockets of the report, show all of the existing storm drain facilities impacting the Comanche Park detention pond facilities.



#### V. FLOOD PLAINS

After reviewing the Flood Insurance Rate Maps for Bernalillo County and Incorporated Areas (Panel 138 of 825 Map No. 35001C0138 D and Panel 351 of 825 Map No. 35001C0351 D) both dated September 20, 1996, flood plains do exist within both the south park/pond and the north park/pond. Figure 3 shows the existing flood plains that lie within the two parks. Since the proposed improvements to the south park will maintain the site as a detention pond facility with no increase in the High Water Level (HWL), no CLOMR or LOMR will be required.

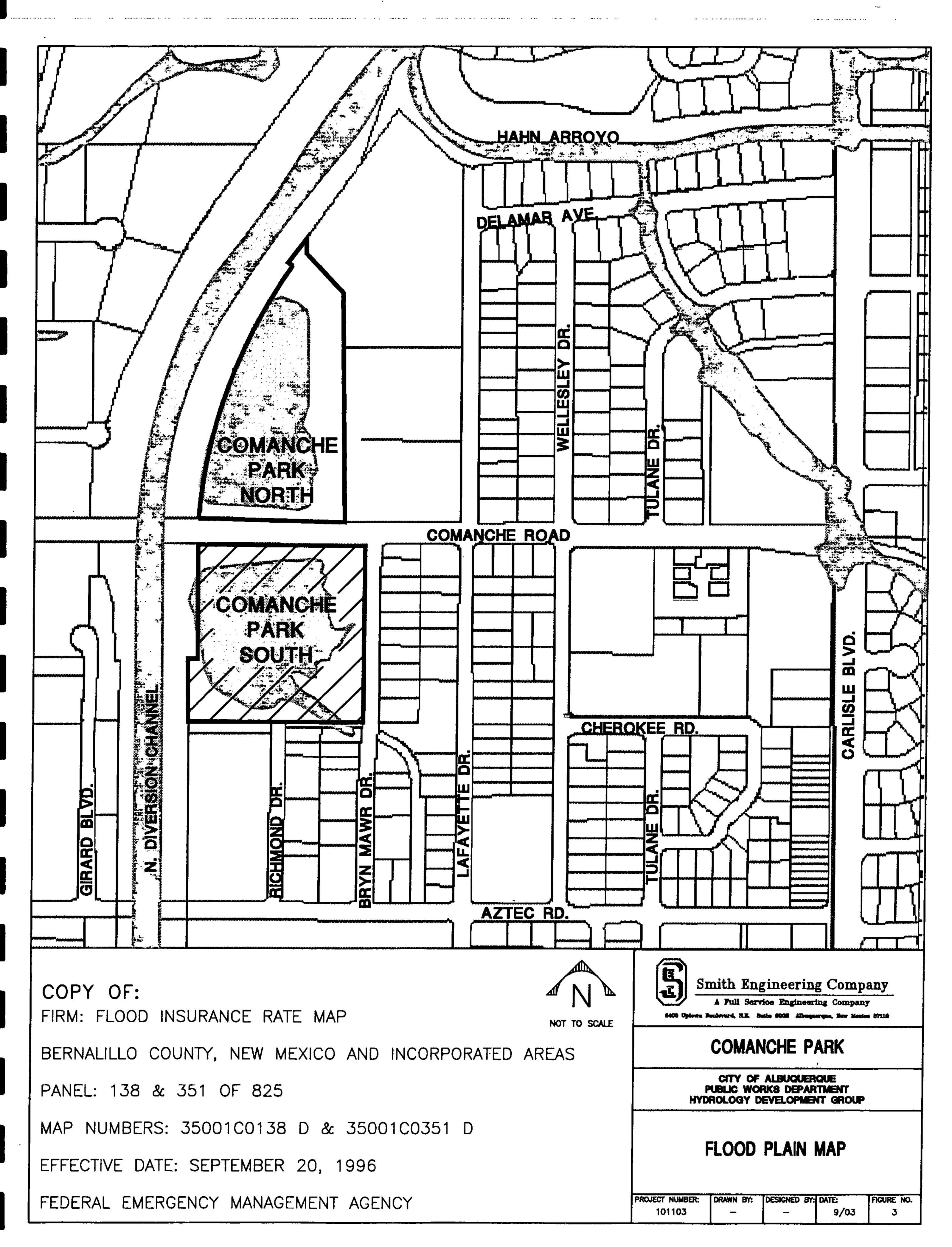
#### VI. HYDROLOGY

Frequency flows were quantified using the AHYMO computer program according to "Section 22.2 Hydrology of the Development Process Manual, Design Criteria for the City of Albuquerque, New Mexico". Mapping for the off-site hydrologic analysis utilized the orthophotography and vector contour composite images (part of the Bernalillo County Digital Mapping) obtained from the Albuquerque Metropolitan Arroyo and Flood Control Authority (AMAFCA). The south park/pond utilized a topographic/boundary survey performed by Albuquerque Surveying Company, Inc. dated January, 1997. The north park/pond utilized a topographic survey performed by Community Sciences Corporation dated December 2000.

Rainfall amounts for the frequency events were derived from the NOAA Atlas show in "Section 22.2 Hydrology of the Development Process Manual", figures C-1, C-2 and C-3. Developed condition sub-basins were generally broken down into platted lands while topographic features were utilized to determine existing condition drainage sub-basins.

Existing condition land treatments were determined utilizing the Bernalillo County mapping mentioned above. Developed condition land treatments were determined by correlating the proposed zoning mentioned above to the appropriate land treatments as shown in "Section 22.2 Hydrology of the Development Process Manual", tables A-4 and A-5. Appendix "A" links the zoning used for developed conditions to the appropriate land treatments used in the AHYMO computer models. A summary of Land Treatments used in the hydrologic analysis are shown in Appendix "A".

Time of concentration calculations utilized the SCS Upland Method as described in "Section 22.2 Hydrology of the Development Process Manual". Appendix "B" contains AHYMO basin parameter worksheets together with peak flows and volumes for each sub-basin. AHYMO output summary tables are included in Appendix "C". Detailed AHYMO computer model input and output files are available for observation at the offices of Smith Engineering Company. 2-year, 5-year, 10-year and 100-year peak flows and volumes at designated analysis points are shown on Map No. 1 located in the back pockets of this report.



#### VII. PROPOSED IMPROVEMENTS

All of the improvements relative to the Comanche Park – Phase 1 Improvements project will take place within the south park/pond. The improvements include the construction of five new dirt baseball fields for the Thunderbird Little League, an aggregate base course parking lot along with a low flow concrete channel to direct flows through the site. Retaining walls will also be constructed at the southwest corner of the south park. Some concrete valley gutter will be constructed directing flow from the parking area to either the pond outlet structure or the concrete low flow channel. Civil Construction Drawings (Sheets 6 through 10 of 13) located in the back pockets of the report show the phase 1 improvements.

#### VIII. ANALYSIS

The analysis of the park site is simple. The only hydraulic characteristic of importance in this analysis is pond volume. The first step in the analysis is to determine the existing capacity of each pond and the second is to determine if the existing ponds contain or hold the 100% developed conditions flow that currently impacts the ponds. In other words, determine the HWL in each pond and check it verses the allowable HWL. Steps three and four are similar to the first two steps except the proposed improvements to the south park will be incorporated into the analysis. It is important to remember that the two ponds are connected by a 36" reinforced concrete pipe. With this in mind, the two ponds work together as one, meaning the north pond utilizes some of the south pond volume. Any change to one of the ponds effect the HWL and volume of the other pond. Below is a summary of the results of the analysis.

#### A. South Park – Existing Condition

The existing allowable volume of the south pond is 7.84 Ac.Ft. The existing allowable HWL in the south pond is 5082.00 feet. With the 100% developed condition 100-year, 24-hour storm event flow impacting the site, the existing HWL in the south pond is 5081.79 feet. Thus, the existing south pond contains the 100-year, 24-hour storm event.

The existing allowable volume of the north pond (including a portion of the south pond) is 61.44 Ac.Ft. The existing allowable HWL in the north pond is 5086.00 feet. With the 100% developed condition 100-year, 24-hour storm event flow impacting the site, the existing HWL in the north pond is 5079.06 feet. Thus, the existing north pond easily contains the 100-year, 24-hour storm event.

Table 1 shows the existing condition stage/storage discharge table for the south pond. Table 2 shows the existing condition stage/storage/discharge for the north detention pond. This information was utilized in the pond routing routines in the AHYMO computer model to determine high water levels in each pond. AHYMO summary tables can be found in Appendix C.

#### B. South Park - Proposed Condition

The proposed allowable volume of the south pond is 12.16 Ac.Ft. The proposed allowable HWL in the south park remains the same as the existing condition analysis at 5082.00 feet. The increase in allowable volume between the proposed condition and the existing condition is attributed to the regarding of the south park site. With the 100% developed condition 100-year, 24-hour storm event flow impacting the site, the proposed HWL in the south pond is 5081.11 feet. Thus, the proposed south park contains the storm water runoff from the 100-year, 24-hour storm event.

Due to the proposed improvements to the south park, the allowable volume of the north pond (including a portion of the south pond) increases from 61.44 Ac.Ft. to 65.77 Ac.Ft. The allowable HWL in the north pond remains the same as the existing condition at 5086.00 feet. With the 100% developed condition 100-year, 24-hour storm event flow impacting the site, the new HWL in the north pond is 5078.45 feet. Once again, the north park easily contains the runoff from the 100-year, 24-hour storm event.

Table 3 shows the proposed condition stage/storage discharge table for the south pond. Table 4 shows the proposed condition stage/storage/discharge for the north detention pond. This information was utilized in the pond routing routines in the AHYMO computer model to determine high water levels in each pond. AHYMO summary tables can be found in Appendix C.

# COMANCHE PARK SOUTH DETENTION POND

#### **EXISTING CONDITION**

## STAGE/STORAGE/DISCHARGE TABLE

	<del></del>	<del></del>	<del></del>	
		INCREMENTAL	TOTAL	
ELEVATION	AREA	VOLUME	VOLUME	DISCHARGE
(FT.)	(SQ.FT.)	(AC.FT.)	(AC.FT.)	(CFS)
5076.37	0		0.00	0
		0.00		
5077.00	381		0.003	0
		0.02		
5078.00	1,190		0.02	0
		0.06		
5079.00	3,700		0.08	0
		0.75		
5080.00	61,863		0.83	28
		2.56		
5081.00	161,521		3.39	42
		4.45		
5082.00	225,820		7.84	53
		XX		
5083.00*	XX		XX	XX
		XX		
5084.00*	XX		XX	XX

<sup>\*</sup> Elev. 5083 and 5084 extend outside the Comanche Park South area and into Bryn Mawr Dr. and into the channel southeast of the South Pond.

Discharge values obtained from Culvert Master for 36" RCP L = 229 ft., S = 0.7074% and a tailwater elevation of 5079.25 ft. in the North Pond.

# COMANCHE PARK NORTH DETENTION POND

#### **EXISTING CONDITION**

#### STAGE/STORAGE/DISCHARGE TABLE

#### STAGE/STORAGE/DISCHARGE TABLE

(VOLUME COMBINED WITH SOUTH DETENTION POND)

#### (NORTH DETENTION POND VOLUME ONLY)

		INCREMENTAL	TOTAL	
ELEVATION	AREA	VOLUME		DISCHARGE
(FT.)	(SQ.FT.)		(AC.FT.)	(CFS)
5070.00	0		0.00	0
		0.01		
5071.00	652		0.01	0.23
		0.02		
5072.00	826		0.02	0.23
<u> </u>		0.02		
5073.00	1,041		0.05	0.23
		0.06		
5074.00	4,284		0.11	8.03
		0.86		
5075.00	70,792		0.97	15.83
		2.70		
5076.00	164,326		3.67	15.83
	400.000	4.06		45.00
5077.00	189,662		7.73	15.83
5070.00	400 400	4.46	40.40	45.00
5078.00	199,122	4 6 4	12.19	15.83
5070.00	005 440	4.64	40.04	45.00
5079.00	205,416	4 70	16.84	15.83
5000.00	244 669	4.79	24.62	45.92
5080.00	211,668	4.02	21.62	15.83
5081.00	217,939	4.93	26.56	15.83
5061.00	217,939	5.08	20.50	15.65
5082.00	224,683	3.00	31.64	15.83
3002.00	- '	5.24	<u> </u>	10.00
5083.00	232,228	٠,٣-	36.88	15.83
. 5555.00	202,220	5.41	-00.00	10.00
5084.00	239,218	VT !	42.29	15.83
		5.57		
5085.00	246,236		47.86	15.83
		5.74		
5086.00	253,697		53.60	15.83
		I		

		INCORPAGNICAL.	TOTAL	
	4054	INCREMENTAL		
ELEVATION		VOLUME		DISCHARGE
(FT.)	(SQ.FT.)	(AC.FT.)	(AC.FT.)	(CFS)
5070.00				
5070.00	0	·	0.00	0
	·	0.01		
5071.00	652	i	0.01	0.23
		0.02		
5072.00	826		0.02	0.23
		0.02		
5073.00	1,041		0.05	0.23
		0.06		
5074.00	4,284		0.11	8.03
		0.86		
5075.00	70,792		0.97	15.83
		2.70		
5076.00	164,326		3.67	15.83
		4.06		<u>-                                    </u>
5077.00	189,662		7.73	15.83
		4.46		
5078.00	199,122	]	12.21	15.83
		4.64		
5079.00	205,416		16.91	15.83
		4.79		
5080.00	211,668		22.45	15.83
		4.93		
5081.00	217,939	<del></del>	29.95	15.83
	·····	5.08	<del></del>	· <del>····································</del>
5082.00	224,683		39.48	15.83
		5.24		<del></del>
5083.00	232,228		44.72	15.83
		5.41		
5084.00	239,218		50.13	15.83
		5.57		
5085.00	246,236	0.0.	55.70	15.83
		5.74	<del></del>	10.00
5086.00	253,697	<u> </u>	61.44	15.83
_ 0000.00	200,007		U1.77	10.00

Discharge values based upon "Final Report - Volume Two for Albuquerque Storm Water Pumping Stations Rehabilitation Study - Phase I Stations 26, 27, 32, 34, 35, 36, 37, and Alcalde (Proposed)", A/E Sercices Agreement 81-26 for Municipal Development Department City of Albuquerque, BovayEngineers, Inc.

Pump No. 1 - Sump Pump, Pumps at 103 gpm = 0.23 cfs.

Pump No. 2 - Storm Drain Pump, Pumps at 3500 gpm = 7.8 cfs at Elev. = 5074.00.

Pump No. 3 - Storm Drain Pump, Pumps at 3500 gpm = 7.8 cfs at Elev. = 5075.00.

<sup>\*</sup> Storage from Elev. = 5077 to Elev. = 5082.00 uses the volume from the Comanche Park South Pond.

# COMANCHE PARK SOUTH DETENTION POND

#### PROPOSED CONDITION

#### STAGE/STORAGE/DISCHARGE TABLE

		INCREMENTAL	TOTAL	
ELEVATION	AREA	VOLUME	VOLUME	DISCHARGE
(FT.)	(SQ.FT.)	(AC.FT.)	(AC.FT.)	(CFS)
5076.37	0		0.00	0
		0.01		
5077.00	1,640		0.012	0
		0.34		
5078.00	28,105		0.35	0
		1.16		
5079.00	73,165		1.52	0
		2.31		
5080.00	128,151		3.83	28
		3.55		
5081.00	181,042		7.38	42
		4.79		
5082.00	236,000		12.16	53
		XX		
5083.00*	XX		XX	XX
•	1	XX		
5084.00*	XX		XX	XX

<sup>\*</sup> Elev. 5083 and 5084 extend outside the Comanche Park South area and into Bryn Mawr Dr. and into the channel southeast of the South Pond.

Discharge values obtained from Culvert Master for 36" RCP L = 229 ft., S = 0.7074% and a tailwater elevation of 5079.25 ft. in the North Pond.

# COMANCHE PARK NORTH DETENTION POND

#### PROPOSED CONDITION

#### STAGE/STORAGE/DISCHARGE TABLE

#### STAGE/STORAGE/DISCHARGE TABLE

(VOLUME COMBINED WITH SOUTH DETENTION POND)

#### (NORTH DETENTION POND VOLUME ONLY)

		INCREMENTAL	TOTAL	
ELEVATION	AREA	VOLUME	VOLUME	DISCHARGE
(FT.)	(SQ.FT.)	(AC.FT.)	(AC.FT.)	(CFS)
			,	, , , , , ,
5070.00	0		0.00	0
		0.01		
5071.00	652		0.01	0.23
		0.02		
5072.00	826		0.02	0.23
		0.02		
5073.00	1,041		0.05	0.23
· · · · · · · · · · · · · · · · · · ·		0.06		
5074.00	4,284		0.11	8.03
		0.86		
5075.00	70,792		0.97	15.83
		2.70		
5076.00	164,326		3.67	15.83
		4.06		
5077.00	189,662		7.73	15.83
		4.46		
5078.00	199,122		12.19	15.83
5000.00		4.64		
5079.00	205,416		16.84	15.83
5000.00	044 000	4.79		
5080.00	211,668	1 00	21.62	15.83
5004.00	047 000	4.93		45.00
5081.00	217,939		26.56	15.83
E000.00	204 000	5.08	24.04	45.00
5082.00	224,683	E 04	31.64	15.83
E002.00	222 222	5.24	20.00	45.00
5083.00	232,228	EAA	36.88	15.83
5004.00	220 249	5.41	40.00	45.00
5084.00	239,218	E E7	42.29	15.83
5005 00	246 226	5.57	47.00	45.02
5085.00	246,236	5 74	47.86	15.83
5086 00	253,697	5.74	52.60	15.02
5086.00	200,097	<u></u>	53.60	15.83

		INCREMENTAL	TOTAL	
ELEVATION		VOLUME	VOLUME	DISCHARGE
(FT.)	(SQ.FT.)	(AC.FT.)	(AC.FT.)	(CFS)
5070.00	0		0.00	0
		0.01		ļ
5071.00	652		0.01	0.23
		0.02		
5072.00	826		0.02	0.23
		0.02		<del> </del>
5073.00	1,041		0.05	0.23
		0.06		
5074.00	4,284	······································	0.11	8.03
		0.86		
5075.00	70,792		0.97	15.83
		2.70		
5076.00	164,326		3.67	15.83
		4.06		
5077.00	189,662		7.74	15.83
		4.46		
5078.00	199,122		12.55	15.83
		4.64		
5079.00	205,416		18.35	15.83
		4.79		
5080.00	211,668		25.45	15.83
		4.93		
5081.00	217,939		33.93	15.83
		5.08		
5082.00	224,683	·	43.80	15.83
		5.24		
5083.00	232,228	771.21.21.21.2	49.04	15.83
		5.41		
5084.00	239,218		54.45	15.83
		5.57		
5085.00	246,236		60.03	15.83
		5.74		
5086.00	253,697		65.77	15.83

Discharge values based upon "Final Report - Volume Two for Albuquerque Storm Water Pumping Stations Rehabilitation Study - Phase I Stations 26, 27, 32, 34, 35, 36, 37, and Alcalde (Proposed)", A/E Sercices Agreement 81-26 for Municipal Development Department City of Albuquerque, BovayEngineers, Inc.

Pump No. 1 - Sump Pump, Pumps at 103 gpm = 0.23 cfs.

Pump No. 2 - Storm Drain Pump, Pumps at 3500 gpm = 7.8 cfs at Elev. = 5074.00.

Pump No. 3 - Storm Drain Pump, Pumps at 3500 gpm = 7.8 cfs at Elev. = 5075.00.

<sup>\*</sup> Storage from Elev. = 5077 to Elev. = 5082.00 uses the volume from the Comanche Park South Pond.

#### **COMANCHE PARK**

#### **DETENTION POND RESULTS**

#### SOUTH PARK EXISTING CONDITIONS

	TOP OF POND	2-YEAR	5-YEAR	10-YEAR	100-YEAR
	ELEVATION	24-HOUR	24-HOUR	24-HOUR	24-HOUR
	WITHIN	HWL	HWL	HWL	HWL
DETENTION	PROPERTY	ELEVATION	ELEVATION	ELEVATION	ELEVATION
POND	(FT.)	(FT.)	(FT.)	(FT.)	(FT.)
South Park Detention Pond	5,082.00	5,080.24	5,080.60	5,080.92	5,081.79
North Park Detention Pond	5,087.00	5,076.25	5076.91 *	5077.40 *	5079.06 *

<sup>\*</sup> HWL's in the North Park Detention Pond above Elevation 5076.37' will "Back Up" into the South Park Detention Pond

#### SOUTH PARK PROPOSED CONDITIONS

	TOP OF POND	2-YEAR	5-YEAR	10-YEAR	100-YEAR
	ELEVATION	24-HOUR	24-HOUR	24-HOUR	24-HOUR
	WITHIN	HWL	HWL	HWL	HWL
DETENTION	PROPERTY	ELEVATION	ELEVATION	ELEVATION	ELEVATION
POND	(FT.)	(FT.)	(FT.)	(FT.)	(FT.)
South Park Detention Pond	5,082.00	5,079.49	. 5,079.85	5,080.12	5,081.11
North Park Detention Pond	5,087.00	5,075.72	5076.45 *	5076.97 *	5078.45 *

<sup>\*</sup> HWL's in the North Park Detention Pond above Elevation 5076.37' will "Back Up" into the South Park Detention Pond

#### IX. CONCLUSIONS

The existing condition analysis shows that both the south park/pond and the north park/pond contain the 100% developed condition 100-year, 24-hour storm event water volume. The proposed condition analysis shows that both the south park/pond and the north park/pond also contains the 100-year, 24-hour storm water volume. In fact, the proposed improvements to the south park/pond increase overall volume in both ponds thus, improving the hydraulic capacity of both ponds. Table 5 of this report displays the pond volume results for both the existing and proposed ponds.

#### X. REFERENCES

Anderson-Hydro, August 1997, "AHYMO Computer Program User's Manual"

City of Albuquerque, July 1997, "Development Process Manual, Section 22.2, Hydrology".

Haestad Methods Inc., August 2000, "FlowMaster PE Version 6.0 Computer Software".

Haestad Methods Inc., November 2000, "CulvertMaster Computer Software".

Bovay Engineers, Inc., June 1983, "Final Report – Volume 2 for Albuquerque Storm Water Pumping Stations Rehabilitation Study – Phase 1 Stations: 26, 27, 32, 34, 35, 36, 37, and Alcalde (Proposed)".

# APPENDIX A

# LAND TREATMENT SUMMARY

# COMANCHE PARK

#### LAND TREATMENT SUMMARY

		LAND TR	REATMENT	S
ZONE DESIGNATION	"A"	"B"	"C"	"D"
RESIDENTIAL				
R-1 *		22	21	57
R-2 *		15	15	70
R-3 *		10	10	80
COMMERCIAL				
C-1		5	5	90
C-3		5	5	90
<u>OFFICE</u>			_	
O-1		5	5	90
COA DICHT OF WAVE				
CARLISE BLVD		_		00
CARLISE BLVD		5	5	90
COMANCHE RD.		5	5	90
PARKS **		84	9	7

Percent Land Treatment "D" show in table above taken from Table A-5, DPM Section 22.2.

<sup>\*</sup> Land Treatments for Residential Areas used in this study were determined from 1999 Aerial Photography.

<sup>\*\*</sup> Land Treatments for Parks used in this study were determined from 1999 Aerial Photography and proposed land uses.

## APPENDIX B

AHYMO BASIN PARAMETER WORKSHEETS,
PEAK BASIN FLOWS, AND
VOLUMES

#### **COMANCHE PARK**

#### OFF-SITE ANALYSIS

# AHYMO BASIN PARAMETER WORKSHEET PEAK BASIN FLOWS AND VOLUMES

#### **DEVELOPED CONDITIONS \***

													2 Y	EAR	5 `	YEAR	10 Y	EAR	100	YEAR
<del> </del>	<del></del>	·····		<b>7</b>	<del></del>								PEAK	RUNOFF	PEAK	RUNOFF	PEAK	RUNOFF	PEAK	RUNOFF
			ELEV.						. —	•			FLOW	VOLUME	FLOW	VOLUME	FLOW	VOLUME	FLOW	VOLUME
54611	AREA	LENGTH	DIFF.	SLOPE		VEL	T(c)	T(p)	LAN	D TREA	ATMEN	T (%)	(24hr.)	(24hr.)	(24hr.)	(24hr.)	(24hr.)	(24hr.)	(24hr.)	(24hr.)
BASIN	(sq.ml.)	(ft.)	(ft.)	(%)	K	(fps)	(hr.)	(hr.)	Α	В	U	٥	(cfs)	(ac.ft.)	(cfs)	(ac.ft.)	(cfs)	(ac.ft.)	(cfs)	(ac.ft.)
PS-1	0.0777	250	2.0	4.00					İ				•							
F3-1	0.0777	250 150	3.0	1.20	1.0	1.10	0.06	0.04	:											•
	Ì	160	2.0 6.0	1.33	1.0	1.15	0.04	0.02												•
		200	4.0	3.75 2.00	2.0	3.87	0.01	0.01												
		100	5.0	5.00	2.0 2.0	2.83 4.47	0.02	0.01									-			
		850	25.0	2.94	3.0	5.14	0.01	0.00			!									
	TOTAL =		20.0	2.54	3.0	3.14	<u>0.05</u> 0.18	<u>0.03</u> 0.12		40	40	20	70	0.477						
<del></del>					ļ		0.16	0.12	0	10	10	80	79	3.177	109	4.467	133	5.472	209	8.866
PS-2	0.0112	130	2.0	1.54	1.0	1.24	0.03	0.02						1 I						
	1	700	7.5	1.07	3.0	3.11	0.06	0.04												
	TOTAL =						0.09	0.06	0	15	15	70	10	0.406	15	0.500	4.0	0.747	00	
										,,,			<del></del>	0.400	13	0.580	18	0.717	29	1.186
PS-3	0.0061	400	3.0	0.75	1.0	0.87	0.13	0.09	]											
		380	8.0	2.11	2.0	2.90	<u>0.04</u>	0.02												
	TOTAL =		···			ļ	0.16	0.11	0	5	5	90	7	0.278	9	0.385	11	0.469	17	0.746
50.4#									ï									<u> </u>	17	0.740
PS-4 *		700	6.0	0.86	3.0	2.78	0.07	0.05						<u> </u>				:		
······································	TOTAL =					<u></u>	0.07	0.05	0	60	40	0	2	0.051	7	0.171	12	0.279	25	0.713
DN 4	0.0202	000																		
PN-1	0.0323	200	2.0	1.00	1.0	1.00	0.06	0.04						l i						
	1	800 630	21.0	2.63	3.0	4.86	0.05	0.03										1		
_	TOTAL =	630	1.5	0.24	3.0	1.46	<u>0.12</u>	0.08	_					[ <b> </b>						
	TOTAL -		, <u>-</u> u				0.22	0.15	0	13	13	74	29	1.231	40	1.747	49	2.151	79	3.526
PN-2	0.0062	975	20.0	2.05	3.0	4 30	0.06		,											
	TOTAL =		20.0	2.03	3.0	4.30	<u>0.06</u> 0.06	0.04	<b> </b>	40	40	~.			_					
							0.08	0.04	0	13	13	74	- 6	0.236	8	0.335	10	0.413	16	0.677
PN-3	0.0074	130	4.0	3.08	1.0	1.75	0.02	0.01												
		100	0.5	0.50	1.0	0.71	0.04	0.03	•											
		170	5.0	2.94	1.0	1.71	0.03	0.02												
		650	9.0	1.38	3.0	3.53	<u>0.05</u>	0.03			:									
	TOTAL =			<u></u>			0.14	0.09	0	13	13	74	7	0.282	10	0.400	12	0.493	10	0.000
																0.400	۱۷	0.453	19	0.808
PN-4	0.0041	275	8.0	2.91	1.0	1.71	0.04	0.03												
		300	10.0	3.33	3.0	5.48	<u>0.02</u>	0.01												
	TOTAL =		<del></del>				0.06	0.04	0	20	20	60	3	0.130	5	0.189	6	0.236	10	0.401
<b>5</b> 21 -													<u>.</u>							0.401
PN-5	0.0067	300	14.0	4.67	1.0	2.16	0.04	0.03												
	TOTAL	550	3.0	0.55	3.0	2.22	<u>0.07</u>	<u>0.05</u>												
	TOTAL =						0.11	0.07	0	15	15	70	6	0.243	9	0.347	11	0.429	17	0.710
PN-6	0.0080	650	7.0	4.00			<b>A</b>													
1 14-0	TOTAL =		7.0	1.08	3.0	3.11	<u>0.06</u>	<u>0.04</u>	_											
<del></del>	TO TALL			L	<u> </u>		0.06	0.04	0	15	15	70	7	0.290	10	0.415	13	0.512	21	0.847

#### **COMANCHE PARK**

#### OFF-SITE ANALYSIS

# AHYMO BASIN PARAMETER WORKSHEET PEAK BASIN FLOWS AND VOLUMES

#### DEVELOPED CONDITIONS \*

													2 Y	EAR	5`	YEAR	10 YI	EAR	100	YEAR
			<del></del>		<del></del>					_			PEAK	RUNOFF	PEAK	RUNOFF	PEAK	RUNOFF	PEAK	RUNOFF
		• ••• · · · · · · · · · · · · · · · · ·	ELEV.			<u> </u>						·	FLOW	VOLUME	FLOW	VOLUME	FLOW	VOLUME	FLOW	VOLUME
	AREA	LENGTH	DIFF.	SLOPE		VEL	T(c)	T(p)	LAN	D TRE	ATMEN	T (%)	(24hr.)	(24hr.)	(24hr.)	(24hr.)	(24hr.)	(24hr.)	(24hr.)	(24hr.)
BASIN	(sq.mi.)	(ft.)	(ft.)	(%)	К	(fps)	(hr.)	(hr.)	Α	В	С	D	(cfs)	(ac.ft.)	(cfs)	(ac.ft.)	(cfs)	(ac.ft.)	(cfs)	(ac.ft.)
					1															
PN-7	0.0009	330	4.0	1.21	3.0	3.30	0.03	<u>0.02</u>								<u> </u>				
	TOTAL =					ļ	0.03	0.02	0	5	5	90	1	0.041	1	0.057	2	0.069	3	0.110
DN 9	0.0024	750	00.0					1		ł									<u></u>	
PN-8	0.0024	750	23.0	3.07	3.0	5.25	0.04	0.03			Ì						Ì			
* ***	TOTAL =						0.04	0.03	0	5	5	90	3	0.109	4	0.152	4	0.184	7	0.294
PN-9	0.0297	450	4.0											:						<del></del>
FN-5	0.0297	150	1.0	0.67	0.7	0.57	0.07	0.05		İ								]		
	]	470	4.5	0.96	3.0	2.94	0.04	0.03												
		300	2.5	0.83	3.0	2.74	0.03	0.02										1		
		230	5.0	2.17	3.0	4.42	0.01	0.01						<u> </u>						
		100	10.0	10.00	3.0	9.49	0.00	0.00						}						
	TOTAL	380	4.0	1.05	3.0	3.08	<u>0.03</u>	<u>0.02</u>												
	TOTAL =						0.20	0.13	0	30	30	40	18	0.666	29	1.034	37	1.333	65	2.413
PN-10	0.0024	220	<b>5</b> 0	4.50																<del></del>
FIN-10	0.0024	330	5.0	1.52	0.7	0.86	<u>0.11</u>	<u>0.07</u>		]										
	TOTAL =				· <del></del> · <u> </u>		0.11	0.07	0	67	26	7	0.5	0.014	1	0.034	2	0.052	4	0.122
PN-11	0.0184	150	4.5	4.00	,															· · · · · · · · · · · · · · · · · · ·
r IN-11	0.0104	150	1.5	1.00	0.7	0.70	0.06	0.04	<u>'</u>											
		825	12.0	1.45	3.0	3.62	0.06	0.04												
		315	2.0	0.63	3.0	2.39	0.04	0.02										;		
	TOTAL -	170	1.0	0.59	3.0	2.30	<u>0.02</u>	<u>0.01</u>												
	TOTAL =		· <del>· · · · · ·</del>				0.18	0.12	0	19	19	62	15	0.599	22	0.870	28	1.084	45	1.828
PN-12	0.0142	200		4.50	4.5												, ,			······································
F14-12	0.0142	200	3.0	1.50	1.0	1.22	0.05	0.03												
		400	1.0	0.25	2.0	1.00	0.11	0.07	}											
	TOTAL -	400	8.0	2.00	2.0	2.83	<u>0.04</u>	<u>0.03</u>												
	TOTAL =			<del></del>			0.20	0.13	0	15	15	70	13	0.515	19	0.736	23	0.909	36	1.504
PN-13 *	0.0046	100	400	40.00		,		_											,	
1 14-13	0.0015	100	19.0	19.00			<u>0.01</u>	<u>0.00</u>							İ					
	TOTAL -	400	1.0	0.25	2.0	1.00	<u>0.11</u>	<u>0.07</u>												
	TOTAL ≃			<u></u>			0.12	0.08	50	0	50	0	1	0.021	5	0.113	8	0.210	21	0.589

<sup>\*</sup> LAND TREATMENTS FOR COMANCHE PARK SOUTH SHOWN IN THIS WORKSHEET INDICATE EXISTING CONDITIONS

										T	<del> </del>	<u> </u>	<u></u>	<del></del>	<del></del>	<del></del>	<del> </del>	<del></del>	<del></del>	····
PS-4 **	0.0153	1,100	5.4	0.49	4.0	2.80	0.11	0.07		<b></b>			<u> </u>			<u> </u>			<del> </del>	<del> </del>
	TOTAL =						0.11	0.07	0	0	94	6	7	0.161	13	0.318	17	0.452	31	0.960

<sup>\*\*</sup> LAND TREATMENTS FOR COMANCHE PARK SOUTH SHOWN DIRECTLY ABOVE INDICATE DEVELOPED CONDITIONS

# APPENDIX C

# AHYMO SUMMARY TABLES

COMAN2.SUM

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994 INPUT FILE = COMAN2.IN PEAK RUNOFF FROM HYDROGRAPH AREA DISCHARGE VOLUME IDENTIFICATION NO. NO. (CFS) (SQ MI) (AC-FT)

RUN DATE (MON/DAY/YR) =12/18/2003 USER NO. = C\_ANDRSN.IO1

TIME TO PAGE = CFS RUNOFF PEAK PER COMMAND (HOURS) ACRE NOTATION (INCHES) START TIME= .00 COMMANCHE PARK CONSISTS OF TWO AREAS. ONE IS SOUTH OF COMMANCHE RD. EAST OF THE NORTH DIVERSION CHANNEL CONSISTING OF THE THUNDERBIRD LITTLE LEAGUE BASEBALL FIELDS. THE SECOND AREA IS NORTH OF COMANCHE BLVD. BOTH AREAS ACT AS DETENTION POND FACILITIES. THE TWO AREAS ARE CONNECTED BY AN EXISTING 36" CULVERT UNDER COMANCHE BLVD. THE RUNOFF FLOWS IN THE NORTH POND ARE PUMPED UP TO THE NORTH DIVERSION CHANNEL. THE CONTRIBUTING DRAINAGE BASIN IS APPROXIMATELY 95% DEVELOPED. THIS ANAYLSIS ASSUMES 100% DEVELOPED CONDITIONS WITH THE EXCEPTION OF THE COMANCHE PARKS. THE EXISTING LAND TREATMENTS WERE UTILIZED FOR THE NORTH AND SOUTH PARK AREAS. ALL OTHER LAND TREATEMENTS WERE DETERMINED BY USING THE LATEST ARERIAL PHOTO OF THE AREA AND TABLE A-5 OF SECTION 22 OF THE CITY OF ALBUQUERQUE DEVELOPMENT PROCESS MANUAL (DPM). 2-YEAR, 24-HOUR STORM EVENT DATE: September 15, 2003 \*\$ FILENAME - input : Coman2.IN FILENAME - output : Coman2.OUT FILENAME - summary table : Coman2.SUM PROJECT TITLE: DRAINAGE REPORT FOR COMANCHE PARK \*\$ SEC PROJECT NUMBER: 101103C \*S CONSULTANT: SMITH ENGINEERING COMPANY \*S CLIENT: CONSENSUS PLANNING INC. \*\$ \*5 COMMANCHE PARK (North and South) DRAINAGE ANALYSIS: 2-YR, 24 HOUR STORM EVENT RAINFALL TYPE= 2 RAIN24= 1.160 THIS SECTION WILL ROUTE AND ADD THE BASINS \*\$ THAT FLOW INTO SOUTH POND, BASINS PS-1 TO PS-4. BASIN PS-4 IS THE SOUTH POND/BASEBALL FIELDS COMPUTE NM HYD PS-1&AP-1 - 3 .07770 3.177 79.09 .76664 1.500 1.590 PER IMP= 80.00 ROUTE AP-1R .07770 75.17 3.177 .76665 1.550 1.512 COMPUTE NM HYD PS-2 .01120 10.31 .406 .68008 1.500 1.438 PER IMP= 70.00 ADD HYD AP-2 2& 3 10 .08890 84.08 3.583 .75573 1.550 1.478 PS-3&AP-3 -COMPUTE NM HYD .00610 6.92 .278 .85320 1.500 1.772 PER IMP= 90.00 PART\_AP-4 10& 3 10 ADD HYD .09500 90.98 3.861 .76199 1.500 1.496 COMPUTE NM HYD PS-4 - 3 .01530 2.27 .051 .06240 1.550 .232 PER IMP= TOTAL FLOW FROM PS BASIN INTO SOUTH POND A PORTION OF THE FLOW FROM AP-7 WILL ENTER THE SOUTH POND DUE TO FLOW OVERTOPPING THE CURB AT COMANCHE RD. AND BRYN MAWR DR. \*\$ SEE NOTES BELOW RELATIVE TO AP-7. FROM TO PEAK RUNOFF TIME TO PAGE = 2CFS HYDROGRAPH ID ID DISCHARGE VOLUME PEAK AREA RUNOFF PER COMMAND IDENTIFICATION NO. NO. (CFS) (SQ MI) (AC-FT) (INCHES) NOTATION (HOURS) ACRE ADD HYD PART\_AP-4 10& 3 30 .11030 93.17 3.912 .66494 1.320 1.500 **\*S** THIS SECTION WILL DETERMINE FLOWS ENTERING COMANCHE PARK NORTH. BASINS PN-1 TO PN-13. COMPUTE NM HYD PN-1 -.03230 28.78 1.231 .71471 1.500 1,392 PER IMP= 74,00 COMPUTE NM HYD PN-2 .00620 **5.97** .236 .71471 1.505 PER IMP= 74.00 1.500 ADD HYD AP-5 3& 4 10 .03850 34.75 1.467 .71469 1.410 1.500 ROUTE 10 AP-5R .03850 34.33 1.468 .71471 1.393 1.550 COMPUTE NM HYD PN-3 .00740 7.13 .282 .71471 1,505 PER IMP= 74.00 1,500 COMPUTE NM HYD PN-4 .00410 3.34 . 130 .59352 1.500 1.273 PER IMP= 60.00 PART\_AP-6 3& 4 10 ADD HYD .01150 10.47 .412 .67145 1.422 1.500 AP-6 2&10 10 ADD HYD .05000 43.79 1.879 1.368 .70474 1.500 AP-6R 10 ROUTE .05000 44.66 1.879 .70476 1.550 1.396 COMPUTE NM HYD PN-5 .00670 6.17 .243 .68008 1.500 1.439 PER IMP= 70.00 COMPUTE NM HYD PN-6 .00800 7.36 .290 .68008 1.438 PER IMP= 70.00 1.500 ADD HYD PN-5&PN-6 3& 4 .01470 13.53 .533 1.439 .68004 1.500 COMPUTE NM HYD PN-7 .00090 1.02 .041 .85321 1.779 PER IMP= 90.00 1.500 COMPUTE NM HYD PN-8 .00240 2,72 .109 .85320 1.773 PER IMP= 90.00 1.500 ADD HYD PN-7&PN-8 3& 4 .00330 3.75 . 150 .85305 1.775 1.500 ADD HYD PN-7&PN-8&PN 10&20 10 17.28 .01800 .683 .71176 1.500 1.500 ADD HYD AP-7 2&10 91 .06800 59.58 2.563 .70660 1.550 1.369

A PORTION OF THE FLOW FROM AP-7 WILL ENTER THE SOUTH POND DUE Page 1

2.563

.000

.70660

.00000

1.550

-.050

1.369

.000

59.58

.00

.06800

,00000

AP-7NORTH

91

AP-7SOUTH AND 99

TOTAL FLOW FROM PS BASIN INTO SOUTH POND

DIVIDE HYD

\*\$

**\***S

COMAN2.SUM

*S TO	FLOW OVERTOPPING	THE C	IIDD AT	COMANCHE DO	AND DOVE MAWD						
_	E NOTES BELOW REL				MIND DITTE MANNET	DR.					
ADD HYD		30499		. 11030	93.17	3.912	.66494	1.500	1.320		
	WE WILL NOW						100101	11000	11020		
*S				ERT PIPE CONN							
*S											
*S											
*S	THE OUTF	LOW VA	LUES US	ED IN THIS RE	SERVOIR ROUTE	ASSUME A TAI					
*S	WATER EL	EV OF	5079.25	' IN THE COMA	NCHE NORTH PAR	RK POND.					
ROUTE RESERV	VOIR AP-4 POND	10	90	.11030	31.34	3.883	.66010	1.850	. 444	AC-FT=	1,442
COMPUTE NM I	HYD PN-98AP-8	-	3	.02970	17.83	.666	.42040	1.500		PER IMP=	40.00
ROUTE RESERV	VOIR AP-8 POND	3	15	.02970	7.62	.666	. 42039	1.700		AC-FT=	.221
COMPUTE NM I	HYD PN-10	۱ -	3	.00240	. 46	.014	.11246	1.500		PER IMP=	7.00
ADD HYD	AP-9	15& 3	10	.03210	7.81	. 680	.39693	1.700	.380		
ROUTE	AP-9R	10	2	.03210	7.80	.680	.39695	1.700	.380		
COMPUTE NM I	HYD PN-11&AP-10	•	3	.01840	15.37	.599	.61083	1.500		PER IMP=	62.00
ROUTE	AP-10R	3	12	.01840	12.59	.599	.61086	1.550	1.069		
COMPUTE NM H	HYD PN-12	-	3	.01420	13.07	.515	.68008	1.500		PER IMP=	70.00
ADD HYD	AP-10R&PN-12	12& 3	10	.03260	23.89	1.114	.64098	1.550	1.145		
ADD HYD	AP-11	2&10	10	.06470	29.72	1.794	.51990	1.550	.718		
ROUTE RESERV	VOIR AP-11_POND	10	92	.06470	17.67	1.794	.51991	1.850		AC-FT=	.229
<b>*</b> \$ %%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%	<del>\%%%%</del> %%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%	<del>ጜጜጜጜጜ</del> ጜ	<mark></mark> ቴ <b>ጓ</b> ጓጓጓጓ	<b>%%%%%%%%%%%%</b> %%%%%%%%%%%%%%%%%%%%%%%%	<b>Ŀ</b> %%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%	<b>*********</b>					
*S TH	HIS SECTION ADDS	ALL THE	FLOW	GOING INTO THE	NORTH COMANC	HE POND					
*S	<b>ኑ%<b>%%%%%%%%%%</b>%%%%%%%%%%%%%%%%%%%%%%%%%</b>	<del>ጜጜጜጜጜጜ</del>	<b>688888</b> 8	<del></del> <del>8</del> 8888888888	<del>՟</del> %%%%%%%%%%%%%%%%	<del>%%</del> %%%%%%%%%%%					
COMPUTE NM F	1YD PN-13	-	3	.01470	.87	.021	.02620	1.550	.092	PER IMP=	.00
		FROM	TO		PEAK	RUNOFF		TIME TO	CFS	PAGE =	3
	HYDROGRAPH	ID	ID	AREA	DISCHARGE	VOLUME	RUNOFF	PEAK	PER		
COMMAND	IDENTIFICATION	NO.	NO.	(SQ MI)	(CFS)	(AC-FT)	(INCHES)	(HOURS)	ACRE	NOTATIO	ON
ADD HYD	PN-13&AP-4R	3&90	10	. 12500	31.47	3.904	.58555	1.850	.393		
ADD HYD	PN-13&AP-4R&	98&10	10	. 19300	88.72	6.466	.62820	1.550	.718		
ADD HYD	AP-12	92&10	10	. 25770	105.91	8.260	.60101	1.550	.642		
*S	<b>᠄%%%%%%%%%%%%%%%</b>	<del></del> ጜጜጜጜጜጜ	<i>?</i> \$%%%%	<del></del> ቔቔቔቔቔቔቔቔቔቔቔቔቔቔቔቔቔቔቔቔቔቔቔቔቔቔቔቔቔቔቔቔቔቔቔቔ	<del></del> የጜጜጜጜጜጜጜጜጜጜጜጜ	<b>%%%%%%%%%%%%</b>					
	IIS SECTION WILL										
	THE NORTH DIVERS										
	<b>ֈ</b> ֍֍֍֍֍֍֍֍֍֍֍֍֍֍֍֍			<mark>ቔጜጜጜጜጜጜጜጜጜጜጜ</mark> ጜጜጜጜጜ	<b>%%%%%%%%%%%%%</b>	<del>የ</del> 88888888888					
ROUTE RESERV FINISH	OIR AP-12-POND	10	93	.25770	15.83	8.255	.60063	2.700	.096	\C-FT=	4.685

#### COMAN5.SUM

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994 INPUT FILE = COMAN5.IN

RUN DATE (MON/DAY/YR) =12/18/2003 USER NO.= C\_ANDRSN.IO1

								USER R	O C_ANDI	134.10 f	
	HYDROGRAPH	FROM ID	TO ID	AREA	PEAK Discharge	RUNOFF	DUNGEE	TIME TO	CFS	PAGE	= 1
COMMAND	IDENTIFICATION	NO.	NO.	(SQ MI)	(CFS)	VOLUME (AC-FT)	RUNOFF (INCHES)	PEAK (HOURS)	PER ACRE	NOTA	TION
START						•	•	,			
	CHE PARK CONSISTS	OF TWO	AREAS.	ONE IS SOUT	H OF COMMANCH	E RD.				TIME=	.00
*S EAST O	F THE NORTH DIVERS	SION CH	IANNEL C	ONSISTING OF	THE THUNDERB	IRD					
	LEAGUE BASEBALL F					ANCHE					
*S BLVD. *S THE TW	BOTH AREAS ACT AS O AREAS ARE CONNEC							· · · · · · · · · · · · · · · · · · ·	;		
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*S PROJECT	TITLE: DRAINAGE	KEPUK	I FUR C	UMANCHE PARK							
*S SEC PROJE	ECT NUMBER: 101103	C									
	HT: SMITH ENGINEER		MPANY								
*S CLIENT; C	CONSENSUS PLANNING	ING.									
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	MMANCHE PARK (Nor		-	DRAINAGE AN	ALYSIS:						
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	HAT FLOW INTO SOU				S-4.						
*S BA	SIN PS-4 IS THE SO		•								
*\$ *C &&&&&&&&	0.0.0.0.0.0.0.0.0.0.0.0.0.0										
COMPUTE NM H	የ የ	<del>******</del>	3	************ . 07770	<del>677777777777</del> 109.43	ያቴቴቴ 4.467	1.07797	1.500	2.201 P	ED TMD=	80.00
ROUTE	AP-1R	3	2	.07770	103.72	4.467	1.07798	1.500	2.086	- tr - 141	00.00
COMPUTE NM H		-	3	.01120	14.62	.580	.97172	1.500	2.040 PI	ER IMP=	70.00
ADD HYD COMPUTE NM H	AP-2 YD PS-3&AP-3	26.3	10 3	.08890 .00610	118.34 9.32	5.048 .385	1.06458 1.18422	1.500 1.500	2.080 2.387 PI	ED 7MD-	90.00
ADD HYD	PART_AP~4 1	104 3	_	.09500	127.66	5.433	1.07226	1.500	2.307 FI	in imp-	90.00
COMPUTE NM H	· - ·	-	3	.01530	7.34	. 171	.20922	1.500		ER IMP=	.00
	AL FLOW FROM PS BA ORTION OF THE FLOW			· - · · -	SUITH BUND D	IIE					
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		FROM	TO		PEAK	DUNAFF		TTHE TO	A-CA	B14-	^
	HYDROGRAPH	_	ID	AREA	DISCHARGE	RUNOFF Volume	RUNOFF	TIME TO PEAK	CFS Per	PAGE =	= 2
COMMAND	IDENTIFICATION		NO.	(SQ MI)	(CFS)	(AC-FT)	(INCHES)	(HOURS)	ACRE	NOTATI	ION
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ADD HYD *S %%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%	PART_AP-4 1 የየየየዮዮዮዮዮዮዮዮዮዮዮዮዮዮ			. 11030 ፠፠፠፠፠፠፠፠፠	135.01 *********	5.604 ******	. 95255	1.500	1.912		
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" S%ጜጜጜጜጜጜጜጜ COMPUTE NM H\	ኔ <b>ጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜ</b> ጜ YD PN - 1	የተዋልጿት -	****** 3	********* . 03230	%%%%%%%%%%%%%%% 40.40			1 500	1 OEA DE	O THO	74.00
COMPUTE NM HY		-	4	.00620	8.37	1.747 .335	1.01422 1.01422	1.500 1.500	1.954 PE 2.110 PE		
ADD HYD	AP-5	3& 4	10	.03850	48.77	2.083	1.01420	1.500	1.979		
ROUTE Compute NM Hy	AP-5R		2	.03850	47.99	2.083	1.01422	1.550	1.947	,	<b>4</b>
COMPUTE NM HY	<b>15</b>	-	3 4	.00740 .00410	9.9 <del>9</del> 4.90	. 400 . 189	1.01422 .86547	1.500 1.500	2.109 PE 1.868 PE		<b>-</b>
ADD HYD	PART_AP-6		10	.01150	14.89	.589	.96113	1.500	2.023	" TWL-	JV. VV
ADD HYD			10	.05000	62.01	2.672	1.00200	1.500	1.938		
ROUTE COMPUTE NM HY	AP-6R (D PN-5	10 -	2	. 05000 . 00679	62.80 8.75	2.672 347	1.00201	1.550	1.963	D THE	70 00
COMPUTE NM HY		-	4	.00800	10.44	.347 .415	.97172 .97172	1.500 1.500	2.040 PE 2.040 PE		70.00 70.00
ADD HYD	PN-5&PN-6	3& 4	10	.01470	19.19	.762	.97168	1.500	2.040	· • • • • • • • • • • • • • • • • • • •	
COMPUTE NM HY		-	3	.00090	1.38	.057	1.18422	1.500	2.399 PE		90.00
COMPUTE NM HY NDD HYD		- 3& 4 2	4 20	.00240 .00330	3.67 5.05	. 152 . 208	1.18422 1.18407	1.500 1.500	2.390 PE	R IMP=	90.00
NDD HYD	PN-7&PN-8&PN 10		10	.01800	24.25	.970	1.01062	1.500	2.392		
ADD HYD			91	.06800	83.75	3.642	1.00428	1.550	1.924		
DIVIDE HYD	AP-7NORTH AP-7SOUTH		98 99	.06800 .00000	83.75	3.642	1.00428	1,550	1.924		
S TOTA	L FLOW FROM PS BAS		-		.00	.000	. 00000	050	.000		
•	RTION OF THE FLOW				SOUTH POND DU	Ε					
					Page 1						

COMAN5.SUM

*S T0	FLOW OVERTOPPING	THE C	URR AT	COMANCHE RD.	AND BRYN MAWR	,,,					
	E NOTES BELOW REL										
ADD HYD	AP-4	30499	10	.11030	135.01	5.604	.95255	1.500	1.912		
*S	WE WILL NOW	ROUTE	THE TOT	AL FLOW INTO	THE SOUTH PONE	THROUGH					
*S	THE EXIS	TING 3	6" CULV	ERT PIPE CONN	ECTING THE SOL	JTH POND TO					
<b>*</b> \$	THE NORT	H POND									
*S											
*S	THE OUTF	LOW VA	LUES US	ED IN THIS RE	SERVOIR ROUTE	ASSUME A TAI					
*S	WATER EL	EV OF	5079.25	' IN THE COMA	NCHE NORTH PAR	K POND.					
ROUTE RESER	VOIR AP-4_POND	10	90	. 11030	36.42	5.574	.94755	1.950	.516	AC-FT=	2.369
COMPUTE NM	HYD PN-9&AP-8	-	3	.02970	28.86	1.034	.65297	1.500	1.518	PER IMP=	40.00
ROUTE RESER	VOIR AP-8_POND	3	15	.02970	11.99	1.034	.65296	1.700	.631	AC-FT=	. 366
COMPUTE NM	HYD PN-10	-	3	.00240	1.23	.034	.26520	1.500	.803	PER IMP=	7.00
ADD HYD	AP-9	15& 3	10	. 03210	12.46	1.068	. 62376	1.650	.606		
ROUTE	AP-9R	10	2	.03210	12.46	1.068	.62378	1.700	.607		
COMPUTE NM	HYD PN-11&AP-10	-	3	.01840	22.38	.870	. 88672	1.500	1.900	PER IMP=	62.00
ROUTE	AP - 10R	3	12	.01840	19.10	. 870	.88674	1.550	1.622		
COMPUTE NM	HYD PN-12	-	3	.01420	18.53	. 736	.97172	1.500	2.039	PER IMP=	70.00
ADD HYD	AP-10R&PN-12	128 3	10	. 03260	35.14	1.606	. 92373	1.550	1.684		
ADD HYD	AP-11	2&10	10	.06470	44.80	2.674	.77491	1.550	1.082		
ROUTE RESER	VOIR AP-11_POND	10	92	.06470	19.78	2.684	.77792	2.000	.478	AC-FT=	.640
<b>*</b> \$ %%%%%%%%%	<del>ጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜ</del>	<b></b> 88888	<mark></mark> ሄ%የ%የ	<b>%%%%%%%%%%%</b> %%%%%%%%%%%%%%%%%%%%%%%%%	፟ጜጜጜጜጜጜጜጜጜጜጜጜጜጜ	<b>*********</b>					
•S T	HIS SECTION ADDS	ALL THI	E FLOW	GOING INTO THE	E NORTH COMANC	HE POND					
<b>*</b> \$ %%%%%%%%	<del>ጜጜጜ</del> ጜጜጜጜጜጜጜጜጜጜጜጜጜ	<b>ጜጜጜጜጜ</b> ጜ	<b>\%%%%%</b> %	<del>ቕ</del> ጜጜጜጜጜጜጜጜጜጜጜጜ	፟ጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜ	<del>የ</del> ጜጜጜጜጜጜጜጜጜጜ					
COMPUTE NM	HYD PN-13	-	3	.01470	4.62	.113	.14444	1.500	.491	PER IMP=	.00
		FROM	TO		PEAK	RUNOFF		TIME TO	CFS	PAGE =	3
	HYDROGRAPH	ID	ID	AREA	DISCHARGE	VOLUME	RUNOFF	PEAK	PER		
COMMAND	IDENTIFICATION	NO.	NO.	(SQ MI)	(CFS)	(AC-FT)	(INCHES)	(HOURS)	ACRE	ITATON	ON
ADD HYD	PN-13&AP-4R	3&90	10	. 12500	36.88	5.687	.85310	1.800	. 461		
ADD HYD	PN-13&AP-4R&	98&10	10	. 19300	119.11	9.330	.90637	1.550	.964		
DYH DOJ	AP-12	92&10	10	.25770	136.81	12.014	.87412	1.550	.829		
S	፟ጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜ	<mark>ቔጜጜጜጜጜ</mark> ጜ	6%%%%%%	<del>ጜጜጜጜጜጜጜጜጜጜጜጜጜጜ</del>	<b>6886888888888</b>	<del>%%%%%%%%%%%</del>					
'S T	HIS SECTION WILL I	ROUTE A	NP-12 T	HROUGH THE COM	MANCHE NORTH P	OND.					
	O THE NORTH DIVERS				•						
S 88888888	<sup>გ</sup> ጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜ	<del></del> ሄጜጜጜጜጜ	\$\$\$\$\$\$\$	<del></del> <del></del>	<del>፧ጜጜጜጜጜጜጜጜጜጜጜ</del> ጜጜ	<b>%%%%%%%</b> %%%%%					
ROUTE RESERY	VOIR AP-12-POND	10	93	.25770	15.83	12.009	.87374	3.050	.096	AC-FT=	7.377

#### COMAN10.SUM

AHYMO SUMMARY TABLE (AHYM0194) - AMAFCA Hydrologic Model - January, 1994 INPUT FILE = COMAN10.IN

RUN DATE (MON/DAY/YR) =12/18/2003 USER NO. = C\_ANDRSN.IO1

	111111111111111111111111111111111111111	FROM			PEAK	RUNOFF		TIME TO	CFS	PAGE	= 1
COMMAND	HYDROGRAPH IDENTIFICATION	ID NO.	ID NO.	AREA (SQ MI)	DISCHARGE (CFS)	VOLUME (AC-FT)	RUNOFF (INCHES)	PEAK (HOURS)	PER ACRE	NOTA	FION
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_	September 15, 200	3 .						September 1	100		louz
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	IAME - summary table						The same of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the later of the la		Acros San		
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ADD HYD COMPUTE NM	AP-2 HYD PS-3&AP-3	2& 3	10 3	.08890 .00610	144.54 11.15	6.189 .468	1.30528 1.44003	1.500 1.500	2.540	PER IMP=	90.00
ADD HYD	PART_AP-4	10& 3	_	.09500	155.69	6.657	1.31392	1.500	2.561	Len YWL-	30.00
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	OTAL FLOW FROM PS 8/ PORTION OF THE FLOW	_				UE	. 54150	1.500		PER IMP=	.00
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	WE WILL NOW										
*S	THE EXIS	STING 3	e. CAF/	/ERT PIPE CONN	ECTING THE SO	UTH POND TO		•			
*S	THE NORT	TH POND									
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<b>*</b> \$	THE OUTF	LOW VA	LUES US	SED IN THIS RE	SERVOIR ROUTE	ASSUME A TAI					
<b>*</b> S	WATER EL	EV OF	5079.25	5' IN THE COMA	NCHE NORTH PAI	RK POND.					
ROUTE RESE	RVOIR AP-4_POND	10	90	.11131	40.87	6.973	1.17459	2.000	.574 A	.C-FT=	3.183
COMPUTE NM	HYD PN-9&AP-8	-	3	.02970	37.34	1.333	.84166	1.500	1.964 P	ER IMP=	40.00
ROUTE RESES	RVOIR AP-8_POND	3	15	.02970	15.53	1.333	.84165	1700	.817 A	C-FT=	. 484
COMPUTE NM	HYD PN-10	-	3	.00240	1.90	. 052	.40419	1.500	1.238 P	ER IMP=	7.00
ADD HYD	<b>AP-</b> 9	15& 3	10	.03210	16.28	1.385	.80892	1.650	.793		
ROUTE	AP-9R	10	2	.03210	16.28	1.385	. 80895	1.700	.792		
COMPUTE NM	HYD PN-11&AP-10	-	3	.01840	27.74	1.084	1.10494	1.500		ER IMP=	62.00
ROUTE	AP-10R	3	12	.01840	24.38	1.084	1.10497	1.550	2.070		
COMPUTE NM	HYD PN-12	-	3	.01420	22.70	.909	1.20068	1.500	2.498 P	ER IMP=	70.00
ADD HYD	AP-10R&PN-12	12& 3	10	.03260	44.06	1.994	1.14663	1.550	2.112		
ADD HYD	AP-11	2&10	10	.06470	56.88	3.378	.97908	1.550	1.374		
ROUTE RESER	RVOIR AP-11_POND	10	92	.06470	23.72	3.384	.98054	2.000	.573 A	C-FT=	.944
*S	ዸጜጜጜጜጜጜጜጜጜጜጜጜጜ <sup>፟</sup> ጜጜጜጜ	<del>%</del> %%%%%	<del></del> ፟፟፟፟፟፟፟፟፟፟፟፟፟፟፟፟፟፟፟፟፟፟፟፟፟፟፟፟፟፟፟፟፟፟፟፟	<del>፞ጜጜጜጜጜጜጜጜጜጜጜጜ</del>	፟ጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜ	<b>\$</b> \$\$\$\$\$\$\$\$\$\$\$					
*S T	HIS SECTION ADDS	ALL THI	E FLOW	GOING INTO THE	E NORTH COMANO	HE POND					
*\$ <b>%%%%%%%</b> %	<del>ዸ</del> ጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜ	<b>%%%%%%</b> %	<del>፟</del> ፟ጜጜጜጜጜጜ	<del>ጜጜጜጜጜጜጜጜጜጜጜጜ</del> ጜ	<mark>፟ጜጜጜጜጜጜጜጜጜጜጜጜጜጜ</mark>	<b></b> \$\$\$\$\$\$\$\$\$\$\$\$\$					
COMPUTE NM	HYD PN-13	-	3	.01470	8.32	.207	.26340	1.500	.884 P	ER IMP=	.00
		FROM	TO		PEAK	RUNOFF		TIME TO	CFS	PAGE =	3
	HYDROGRAPH	ID	ID	AREA	DISCHARGE	VOLUME	RUNOFF	PEAK	PER		_
COMMAND	IDENTIFICATION	NO.	NO.	(SQ MI)	(CFS)	(AC-FT)	(INCHES)	(HOURS)	ACRE	NOTATIO	ON
				•		,	,	<b>(</b> ,			
ADD HYD	PN-13&AP-4R	3&90	10	. 12601	42.09	7.180	1.06830	1.650	.522		
ADD HYD	PN-13&AP-4R&	98&10	10	. 19300	135.23	11.600	1,12697	1.550	1.095		
ADD HYD	AP-12	92&10	10	.25770	153.39	14.984	1.09021	1.550	.930		
*S	<del>፧ጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜ</del>	<del>%%%%%</del> %	<sup>ֈ</sup> 88888	ጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜ	<del></del> 68888888888888	<b>%%%%%%%%%%%%%</b>					
*S T	HIS SECTION WILL	ROUTE A	P-12 T	HROUGH THE COM	MANCHE NORTH P	OND.					
*S T	O THE NORTH DIVERS	SION CH	IANNEL	VIA THE EXIST	NG PUMP/LIFT	STATION.					
	<sup>1</sup> %%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%				• • • • • • • • • • • • • • • • • • • •						
ROUTE RESER	VOIR AP-12-POND	10	93	.25770	15.83	14.979	1.08982	3.300	.096 A	C-FT=	9.532
FINISH				<del>_</del>	, <b></b>	<del>-</del>					- ·

#### COMAN100.SUM

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994
INPUT FILE = COMAN100.IN

RUN DATE (MON/DAY/YR) = 12/18/2003 USER NO. = C\_ANDRSN.IO1

FROM PEAK RUNOFF PAGE = 1TIME TO CFS HYDROGRAPH **AREA** DISCHARGE VOLUME ID PEAK RUNOFF PER IDENTIFICATION COMMAND NO. (CFS) (SQ MI) (AC-FT) (INCHES) (HOURS) ACRE NOTATION START TIME= .00 COMMANCHE PARK CONSISTS OF TWO AREAS. ONE IS SOUTH OF COMMANCHE RD. EAST OF THE NORTH DIVERSION CHANNEL CONSISTING OF THE THUNDERBIRD LITTLE LEAGUE BASEBALL FIELDS. THE SECOND AREA IS NORTH OF COMANCHE BOTH AREAS ACT AS DETENTION POND FACILITIES. JOHN MEK-EXISTING
CONDITIONS THE TWO AREAS ARE CONNECTED BY AN EXISTING 36" CULVERT UNDER COMANCHE BLVD. THE RUNOFF FLOWS IN THE NORTH POND ARE PUMPED UP TO THE NORTH DIVERSION CHANNEL. THE CONTRIBUTING DRAINAGE BASIN IS APPROXIMATELY 95% DEVELOPED. THIS ANAYLSIS ASSUMES 100% DEVELOPED WITH THE EXCEPTION OF THE COMANCHE PARKS. THE EXISTING LAND TREATMENTS WERE UTILIZED FOR THE NORTH AND SOUTH PARK AREAS. ALL OTHER LAND TREATEMENTS WERE DETERMINED BY USING THE LATEST ARERIAL PHOTO OF THE AREA AND TABLE A-5 OF SECTION 22 OF THE CITY OF ALBUQUERQUE DEVELOPMENT PROCESS MANUAL (DPM). 100-YEAR, 24-HOUR STORM EVENT DATE: September 15, 2003 FILENAME - input : Coman100.IN FILENAME - output : Coman100.0UT FILENAME - summary table : Coman100.SUM Make and the second of the second second second second second second second second second second second second PROJECT TITLE: DRAINAGE REPORT FOR COMANCHE PARK \*S SEC PROJECT NUMBER: 101103C \*S CONSULTANT: SMITH ENGINEERING COMPANY \*S CLIENT: CONSENSUS PLANNING INC. \*S COMMANCHE PARK (North and South) DRAINAGE ANALYSIS: 100-YR, 24 HOUR STORM EVENT RAINFALL TYPE= 2 RAIN24= 2.680 THIS SECTION WILL ROUTE AND ADD THE BASINS THAT FLOW INTO SOUTH POND, BASINS PS-1 TO PS-4. BASIN PS-4 IS THE SOUTH POND/BASEBALL FIELDS COMPUTE NM HYD PS-1&AP-1 .07770 8.866 2.13945 208.97 1.500 4.202 PER IMP= 80.00 ROUTE .07770 AP-1R 202.00 8.866 2.13945 4.062 1.500 COMPUTE NM HYD PS-2 .01120 28.78 1.186 1.98545 4.015 PER IMP= 70.00 1.500 ADD HYD AP-2 2& 3 .08890 230.78 10.052 2.12004 4.056 1.500 COMPUTE NM HYD PS-3&AP-3 .00610 17.17 .746 2.29343 4.399 PER IMP= 90.00 1.500 PART\_AP-4 10& 3 10 ADD HYD .09500 247.95 10.798 2.13117 1.500 4.078 COMPUTE NM HYD PS-4 -.01530 25.42 .713 .87418 1.500 2.596 PER IMP= TOTAL FLOW FROM PS BASIN INTO SOUTH POND A PORTION OF THE FLOW FROM AP-7 WILL ENTER THE SOUTH POND DUE TO FLOW OVERTOPPING THE CURB AT COMANCHE RD. AND BRYN MAWR DR. SEE NOTES BELOW RELATIVE TO AP-7. FROM TO PEAK RUNOFF TIME TO **CFS** PAGE = 2HYDROGRAPH ID ID DISCHARGE VOLUME AREA RUNOFF PEAK PER COMMAND IDENTIFICATION NO. NO. (INCHES) (SQ MI) (CFS) (AC-FT) NOTATION (HOURS) ACRE ADD HYD PART\_AP-4 10& 3 30 .11030 273.37 11.511 1.95681 1.500 3.873 \*S THIS SECTION WILL DETERMINE FLOWS ENTERING COMANCHE PARK NORTH. BASINS PN-1 TO PN-13. PN-1 -COMPUTE NM HYD .03230 78,67 3.806 PER IMP= 74.00 3.526 2.04705 1.500 COMPUTE NM HYD PN-2 .00620 16.24 .677 2.04705 4.093 PER IMP= 74.00 1.500 ADD HYD AP-5 38 4 .03850 94.91 4.203 2.04703 1.500 3.852 ROUTE AP-5R 10 92.82 .03850 4.203 2.04706 1.500 3.767 COMPUTE NM HYD PN-3 .00740 19.38 .808 2.04705 1.500 4.092 PER IMP= 74.00 COMPUTE NM HYD PN-4 .00410 10.04 , 400 1.83147 1.500 3.827 PER IMP= 60.00 ADD HYD PART\_AP-6 3& 4 10 .01150 29.42 1.208 1.97014 1.500 3.998 ADD HYD AP-6 2&10 10 .05000 5.412 122.24 2.02935 1.500 3.820 ROUTE AP-6R 10 .05000 122.12 5.412 2.02937 1.550 3.816 COMPUTE NM HYD .00670 17.22 .709 1.98546 1.500 4.016 PER IMP= 70.00 COMPUTE NM HYD PN-6 20.56 .00800 .847 1.98546 4.015 PER IMP= 70.00 1.500 ADD HYD PN-5&PN-6 3& 4 .01470 37.78 1.557 1.98541 4.016 1.500 COMPUTE NM HYD PN-7 4.424 PER IMP= 90.00 .00090 2.55 . 110 2,29344 1.500 COMPUTE NM HYD PN-8 .00240 6.77 .294 2.29344 4.405 PER IMP= 1.500 ADD HYD PN-7&PN-8 3& 4 20 .00330 9.31 2.29326 . 404 1.500 4.410 ADD HYD PN-7&PN-8&PN 10&20 10 .01800 47.09 1.960 2.04185 4.088 1.500 ADD HYD AP-7 2&10 91 .06800 165.20 7.372 2.03266 1.500 3.796 DIVIDE HYD AP-7NORTH .05922 94.00 6.420 2.03266 1,450 2.480 AP-7SOUTH AND .00878 71.20 . 952 2.03266 1.500 12.669

Page 1

TOTAL FLOW FROM PS BASIN INTO SOUTH POND

A PORTION OF THE FLOW FROM AP-7 WILL ENTER THE SOUTH POND DUE

\*\$

COMAN100.SUM

*S T0	FLOW OVERTOPPING	THE C	URB AT	COMANCHE RD. A	ND BRYN MAWR	DR.					
	E NOTES BELOW REL				244 57	10 469	1.96240	1.500	4.521		
ADD HYD	AP-4 WE WILL NOW	30&99			344.57 Je golitu dono		1.50240	1.500	4,321		
*\$		-•		VERT PIPE CONNEC							
_						III FOND IV					
*\$ *\$	INE NON!	n runu									
-s *S	TUE AUTE	IOW VA	ure no	SED IN THIS RESI	::::::::::::::::::::::::::::::::::::::	ACCIIME A TAT					
-5 *S				5' IN THE COMANG							
_				.11908	50.73	12.432	1.95749	2.100	222	AC-FT=	6.922
ROUTE RESERV					65.37 ·	2.413	1.52349	1.500	·	PER IMP=	40.00
COMPUTE NM (				.02970	24.81		1.52349	1.750		AC+FT=	.941
ROUTE RESERV		-		.02970	4.08		.94922	1.500		PER IMP=	7.00
COMPUTE NM I		15& 3		.00240 .03210	26.47	2.535	1.48052	1.700	1.288	ren imr-	7.00
ADD HYD		-	2	.03210	26.47	2.535	1.48055	1.700	1.287		
ROUTE	AP-9R		2	.03210	45.46	1.828	1.86226	1.500		PER IMP=	62.00
COMPUTE NM I			3	.01840	42.33	1.828	1.86229	1.550	3.595	PER IMP	02.00
ROUTE	AP-10R HYD PN-12		12	.01420		1.504	1.98546	1.500		PER IMP=	70.00
COMPUTE NM I			3		36.48 75.40	3.331	1.91592	1.500	3.614	FER IMF-	70.00
ADD HYD	AP-10R&PN-12			.03260	75.40 06.60						
ADD HYD	AP-11			.06470	96.69	5.866	1.69990	1.550	2.335	AC ET-	4 047
ROUTE RESERV	_		92	.06470	36.74	5.873	1.70199	2.050	.00/	AC-FT=	1.947
	<b>₺</b> ጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜ ዸጜዹዹ <u>ዹ</u> ፟ዹጜጜዹዹዹቔቜዹ			• •							
-	IS SECTION ADDS										
	Ŀ <b>%%%%%%%%%%%%%%%</b> **********************						75000	4 500	2 225	DED THO-	00
COMPUTE NM F	IYD PN-13	•	3	.01470	21.03	. 589	.75080	1.500	2.235	PER IMP=	.00
		FROM	TO		PEAK	RUNOFF		TIME TO	CFS	PAGE =	3
	HYDROGRAPH	IĐ	ID	AREA	DISCHARGE	VOLUME	RUNOFF	PEAK	PER		
COMMAND	IDENTIFICATION	NO.	NO.	(SQ MI)	(CFS)	(AC-FT)	(INCHES)	(HOURS)	ACRE	ITATON	ON
ADD HYD	PN-13&AP-4R	3&90	10	.13378	62.50	13.021	1.82490	1.550	.730		
ADD HYD	PN-13&AP-4R&	98&10	10	.19300	156.50	19.440	1.88865	1.550	1.267		
ADD HYD	AP-12	92&10	10	.25770	182.01	25.314	1.84179	1.650	1.104		
*S %%%%%%%%	<del>ֈ</del> ֍֍֍֍֍֍֍֍֍֍֍֍֍֍֍֍֍֍	<del>የ</del> ያያያያያያ	<sub>የ</sub> &&&&&&	ጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜ	<del>ֈ</del> ֏ <del>Ց</del> ֏ <del>ՑՑՑՑ</del> ՑՑ	<del>%%%%%%%%%%%</del> %					
*S TH	IS SECTION WILL I	ROUTE A	P-12 T	HROUGH THE COMA	NCHE NORTH P	OND.					
*S TO	THE NORTH DIVERS	SION CH	IANNEL	VIA THE EXISTIN	G PUMP/LIFT	STATION.					
*\$ <b>%%%%%%%%</b> %	<b>:%%%%%%%%%%%%%%%%</b> %%%%%%%%%%%%%%%%%%%%	<b>&amp;&amp;&amp;&amp;</b> &&	<b>፡</b> የተለያ የተ	<b>፞</b> ጜጜጜጜጜጜጜጜጜጜጜጜጜ	<b>:%%%%%%%%%%%%%%</b>	<del></del> 8%%%%%&%%&%					
ROUTE RESERV FINISH	OIR AP-12-POND	10	93	.25770	15.83	25.308	1.84139	4.400	.096	AC-FT=	17.243

PRO2.SUM

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994
INPUT FILE = Pro2.IN

RUN DATE (MON/DAY/YR) =12/18/2003 USER NO. = C\_ANDRSN.IO1

FROM TO PEAK RUNOFF TIME TO PAGE = CFS **HYDROGRAPH** ID AREA VOLUME DISCHARGE RUNOFF **PEAK** PER COMMAND IDENTIFICATION NO. NO. (AC-FT) (SQ MI) (CFS) (INCHES) NOTATION (HOURS) ACRE START TIME= .00 COMMANCHE PARK CONSISTS OF TWO AREAS. ONE IS SOUTH OF COMMANCHE RD. EAST OF THE NORTH DIVERSION CHANNEL CONSISTING OF THE THUNDERBIRD LITTLE LEAGUE BASEBALL FIELDS. THE SECOND AREA IS NORTH OF COMANCHE BOTH AREAS ACT AS DETENTION POND FACILITIES. THE TWO AREAS ARE CONNECTED BY AN EXISTING 36" CULVERT UNDER COMANCHE BLVD. THE RUNOFF FLOWS IN THE NORTH POND ARE PUMPED UP TO THE NORTH DIVERSION CHANNEL. THE CONTRIBUTING DRAINAGE BASIN IS APPROXIMATELY 95% DEVELOPED. THIS ANAYLSIS ASSUMES 100% DEVELOPMENT. PROPOSED LAND TREATMENTS WERE UTILIZED FOR THE SOUTH PARK AREA WHILE EXISTING LAND TREATEMENTS WERE UTILIZED FOR THE NORTH PARK. ALL OTHER LAND TREATEMENTS WERE DETERMINED BY USING THE LATEST ARERIAL PHOTO OF THE AREA AND TABLE A-5 OF SECTION 22 OF THE CITY OF ALBUQUERQUE DEVELOPMENT PROCESS MANUAL (DPM). CHEAR ZA-HOUR \*\$ DATE: December 2, 2003 FILENAME - input : Pro2.IN FILENAME - output : Pro2.OUT FILENAME - summary table : Pro2.SUM PROJECT TITLE: DRAINAGE REPORT FOR COMANCHE PARK \*S SEC PROJECT NUMBER: 101103C \*S CONSULTANT: SMITH ENGINEERING COMPANY \*S CLIENT: CONSENSUS PLANNING INC. \*\$ COMMANCHE PARK (North and South) DRAINAGE ANALYSIS: \*S 2-YR, 24 HOUR STORM EVENT RAINFALL TYPE= 2 RAIN24= 1.160 \*S \*\$ THIS SECTION WILL ROUTE AND ADD THE BASINS THAT FLOW INTO SOUTH POND, BASINS PS-1 TO PS-4. BASIN PS-4 IS THE SOUTH POND/BASEBALL FIELDS COMPUTE NM HYD PS-18AP-1 - 3 .07770 3.177 .76664 79.09 1.500 1.590 PER IMP= 80.00 ROUTE AP-1R .07770 75.17 3.177 .76665 1.550 1.512 COMPUTE NM HYD PS-2 .01120 10.31 .406 .68008 1.438 PER IMP= 70.00 1.500 ADD HYD AP-2 2& 3 10 .08890 84.08 3.583 .75573 1.550 1.478 COMPUTE NM HYD PS-3&AP-3 -.00610 6.92 .278 .85320 1.500 1.772 PER IMP= 90.00 PART\_AP-4 10& 3 10 ADD HYD .09500 90.98 3.861 .76199 1.500 1.496 COMPUTE NM HYD PS-4 -.01530 6.61 . 161 . 19772 1.500 .675 PER IMP= 6.00 TOTAL FLOW FROM PS BASIN INTO SOUTH POND A PORTION OF THE FLOW FROM AP-7 WILL ENTER THE SOUTH POND DUE TO FLOW OVERTOPPING THE CURB AT COMANCHE RD. AND BRYN MAWR DR. SEE NOTES BELOW RELATIVE TO AP-7. \*S FROM TO PEAK RUNOFF TIME TO CFS PAGE = 2HYDROGRAPH ID DISCHARGE ΙD AREA VOLUME PEAK RUNOFF PER COMMAND IDENTIFICATION NO. NO. (CFS) (SQ MI) (AC-FT) (INCHES) (HOURS) ACRE NOTATION ADD HYD PART\_AP-4 10& 3 30 . 11030 97.59 4.022 . 68371 1.500 1.382 \*\$ THIS SECTION WILL DETERMINE FLOWS ENTERING COMANCHE PARK NORTH. BASINS PN-1 TO PN-13. COMPUTE NM HYD PN-1 -.03230 28.78 1.231 .71471 1.392 PER IMP= 74.00 1.500 COMPUTE NM HYD PN-2 .00620 5.97 .236 .71471 1.500 1.505 PER IMP= 74.00 ADD HYD AP-5 38 4 .03850 34.75 1.467 .71469 1.410 1.500 ROUTE AP-5R 10 .03850 34.33 1.468 .71471 1.393 1.550 COMPUTE NM HYD PN-3 .00740 7.13 .282 .71471 1.500 1.505 PER IMP= 74.00 COMPUTE NM HYD PN-4 .00410 3.34 . 130 .59352 1.273 PER IMP= 60.00 1.500 PART\_AP-6 3& 4 ADD HYD .01150 10.47 .412 .67145 1.500 1.422 ADD HYD AP-6 2&10 10 .05000 43.79 1.879 .70474 1.500 1.368 AP-6R 10 ROUTE .05000 44.66 1.879 .70476 1.550 1.396 COMPUTE NM HYD .00670 6.17 .243 1.500 1.439 PER IMP= 70.00 .68008 COMPUTE NM HYD PN-6 .00800 7.36 .290 .68008 1.438 PER IMP= 70.00 1.500 ADD HYD PN-5&PN-6 3& 4 .01470 13,53 . 533 .68004 1.439 1.500 COMPUTE NM HYD PN-7 .00090 1.02 .041 .85321 1.779 PER IMP= 90.00 1.500 COMPUTE NM HYD PN-8 .00240 2.72 . 109 .85320 1.773 PER IMP= 90.00 1,500 ADD HYD PN-7&PN-8 3& 4 .00330 3.75 .150 .85305 1.500 1.775 ADD HYD PN-7&PN-8&PN 10&20 .01800 17.28 .683 .71176 1.500 1.500 ADD HYD AP-7 2&10 .06800 59.58 2.563 .70660 1.550 1.369 DIVIDE HYD AP-7NORTH .06800 59.58 2.563 .70660 1.550 1.369 AP-7SOUTH AND -00000 .00 .000 .00000 -.050 .000 TOTAL FLOW FROM PS BASIN INTO SOUTH POND **\*S \***\$ A PORTION OF THE FLOW FROM AP-7 WILL ENTER THE SOUTH POND DUE

Page 1

PRO2.SUM

*S TO FLOW OVERTOPPING			. AND BRYN MAWR	DR.					
*S SEE NOTES BELOW REI	LAIIVE IU 4 30&99		97.59	4.022	60271	1 500	4 200		
*SWE WILL					.68371	1.500	1.382		
		CULVERT PIPE CO	•						
		ES USED IN THIS F	•						
		079.25' IN THE CO							
ROUTE RESERVOIR AP-4 PONI		90 .11030	13.68	2.549	. 43335	2.150	104	AC-FT=	2.643
COMPUTE NM HYD PN-9&AP-8			17.83	.666	.42040	1.500		PER IMP=	40.00
ROUTE RESERVOIR AP-8 POND		_	7.62	.666	.42039	1.700		AC-FT=	.221
COMPUTE NM HYD PN-10		3 .00240	.46	.014	. 11246	1.500		AC-F1= PER IMP≈	7.00
	_	10 .03210	7.81	.680	.39693	1.700	.380	ren imr~	7.00
ROUTE AP-9F		2 .03210	7.80	.680	.39695	1.700	.380		
COMPUTE NM HYD PN-11&AP-10		3 .01840	15.37	.599	.61083	1.500		PER IMP≃	62 00
ROUTE AP-10F	_	12 .01840	12.59	.599	.61086	1.550	1.069	ren imr-	02,00
COMPUTE NM HYD PN-12	_	3 .01420	13.07	.535 .515	.68008	1.500		DED TWD-	70.00
ADD HYD AP-10R&PN-12	_	10 .03260	23.89	1.114	.64098	1.550	1.436	PER IMP=	70.00
ADD HYD AP-11		10 .05200	29.72	1.794	.51990	1.550	.718		
ROUTE RESERVOIR AP-11 POND		92 .06470	17.67	1.794	.51991	1.850		AC-FT=	220
*S %%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%		·			.51551	1.650	.421	MU-FI-	.229
*S THIS SECTION ADDS									
*S %%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%									
COMPUTE NM HYD PN-13		3 .01470	.87	.021	.02620	1.550	002	PER IMP=	00
- ADD HYD PN-13&AP-4R		10 .12500	13.72	2.570	.38546	2.150	.172	ren imr~	.00
	5430	. 12300	10.72	2.0.0	. 35540	2.150	. 172		
	FROM 1	ro	PEAK	RUNOFF		TIME TO	CFS	PAGE =	3
HYDROGRAPH		ID AREA	DISCHARGE	VOLUME	RUNOFF	PEAK	PER	I AGE	•
COMMAND IDENTIFICATION		NO. (SQ MI)	(CFS)	(AC-FT)	(INCHES)	(HOURS)	ACRE	NOTATIO	ON
ADD HYD PN-13&AP-4R&	QRE10 1	10 . 19300	60.57	5.132	. 49861	1 EEA	400		
	92&10	, , , , , ,	77.75	6.926	.50396	1.550	.490		
*S %%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%					. 50550	1.550	.471		
*S THIS SECTION WILL									
*S TO THE NORTH DIVER		· · · · · · · · · · · · · · · · ·							
*S %%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%									
ROUTE RESERVOIR AP-12-POND		.25770	15.83	6.921	.50354	2.500	noe A	\C-FT=	2.906
FINISH		120770		U,UL!		2.500	.030 /	7 <b>0</b> - [   <b>-</b>	£.300

PRO5.SUM

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994 INPUT FILE = Pro5.IN

#### RUN DATE (MON/DAY/YR) =12/18/2003 USER NO.= C\_ANDRSN.IO1

	HADDUCDVD	FROM		ADEA	PEAK	RUNOFF	DIVIOEE	TIME TO	CFS	PAGE	= 1	
COMMAND	HYDROGRAPH IDENTIFICATION	ID NO.	ID NO.	AREA (SQ MI)	DISCHARGE (CFS)	VOLUME (AC-FT)	RUNOFF (INCHES)	PEAK (HOURS)	PER ACRE	NOTAT	ION	
START										TIME=	.00	
	MANCHE PARK CONSISTS		. — -									
	r of the North Divers Tle League Baseball F											
*S BLV	. BOTH AREAS ACT AS	-						· Maria Caralle	. 4	THE RESERVE OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE PARTY OF THE		
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ADD HYD			10	.03850	48.77	2.083	1.01422	1.500	1.979	EN IMPS	, 4.00	
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PRO5.SUM 74 July 187 TO FLOW OVERTOPPING THE CURB AT COMANCHE RD. AND BRYN MAWR DR. SEE NOTES BELOW RELATIVE TO AP-7. ADD HYD AP-4 30&99 10 .11030 140.29 5.751 .97758 1.500 1.987 -WE WILL NOW ROUTE THE TOTAL FLOW INTO THE SOUTH POND THROUGH THE EXISTING 36" CULVERT PIPE CONNECTING THE SOUTH POND TO THE NORTH POND. -----\*S THE OUTFLOW VALUES USED IN THIS RESERVOIR ROUTE ASSUME A TAI \*\$ WATER ELEV. OF 5079.25' IN THE COMANCHE NORTH PARK POND. ROUTE RESERVOIR AP-4\_POND 10 90 .11030 23.88 4.275 .72676 2.100 .338 AC-FT= 3.489 COMPUTE NM HYD PN-9&AP-8 .02970 28.86 1.034 .65297 1.500 1.518 PER IMP= 40.00 ROUTE RESERVOIR AP-8\_POND .02970 11.99 1.034 .65296 1.700 .631 AC-FT= .366 COMPUTE NM HYD PN-10 .00240 1.23 .034 .26520 1.500 .803 PER IMP= 7.00 ADD HYD AP-9 15& 3 .03210 12.46 1.068 .62376 .606 1.650 ROUTE AP-9R 10 .03210 12.46 1.068 .62378 .607 1.700 COMPUTE NM HYD PN-11&AP-10 .01840 22.38 .870 .88672 1.500 1.900 PER IMP= 62.00 ROUTE AP-10R .01840 19.10 .870 .88674 1.550 1.622 COMPUTE NM HYD PN-12 .01420 18.53 .736 .97172 1.500 2.039 PER IMP= 70.00 ADD HYD AP-10R&PN-12 12& 3 10 .03260 35.14 1.606 .92373 1.550 1.684 AP-11 2&10 10 ADD HYD .06470 44.80 2.674 .77491 1.550 1.082 ROUTE RESERVOIR AP-11\_POND 10 92 .06470 19.78 2.684 .77792 2.000 .478 AC-FT= .640 **\***\$ THIS SECTION ADDS ALL THE FLOW GOING INTO THE NORTH COMANCHE POND COMPUTE NM HYD PN-13 - 3 .01470 4.62 .113 .14444 1.500 .491 PER IMP= ADD HYD PN-13&AP-4R 3&90 10 . 12500 24.16 4.389 .65828 2.100 .302 FROM TO PEAK RUNOFF TIME TO CF\$ PAGE = 3 HYDROGRAPH ID ID AREA DISCHARGE VOLUME RUNOFF PEAK PER COMMAND IDENTIFICATION NO. NO. (SQ MI) (CFS) (AC-FT) (INCHES) (HOURS) ACRE NOTATION ADD HYD PN-13&AP-4R& 98&10 10 . 19300 90.50 8.031 .78018 1.550 .733 ADD HYD AP-12 92&10 10 ,25770 108.20 10.715 .77962 1.550 .656 \*S THIS SECTION WILL ROUTE AP-12 THROUGH THE COMANCHE NORTH POND. TO THE NORTH DIVERSION CHANNEL VIA THE EXISTING PUMP/LIFT STATION. ROUTE RESERVOIR AP-12-POND 10 93 .25770 15.83 10.709 .77917 2.950 .096 AC-FT= 5.480 FINISH

#### PRO10.SUM

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994 INPUT FILE = Pro10.IN

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MAINFALL ITE	E- 2								H.A	AIN24=	1.790
	<b>?%%%%%%%%%%%%%%%</b> %%	<b>6888888</b> 8	<b>%%%%%%%%%</b> %%%%%	<b>%%%%%%%%%</b> %	፟ጜ፠፠፠፠፠፠፠፠ <b>፠</b> ፠፠	<b>.%%</b>					
	S SECTION WILL R										
*S TH	AT FLOW INTO SOU	JTH POND	, BASINS PS-	1 TO PS-4.	•						
	IN PS-4 IS THE S	SOUTH PO	ND/BASEBALL	FIELDS							
*S	0.0.0.0.0.0.0.0.0.0.0.0					••					
COMPUTE NM HY	%%%%%%%%%%%%%%% 'D PS - 1&AP - 1			'' የተመመመ የተመመመ 1770	********** 132.55	76% 5.472	1 22026	1 500	0 666 DE	'n TWO-	00.00
ROUTE	AP-1R	3		7770	126.63	5.472	1.32036 1.32036	1.500 1.500	2.546	R IMP=	80.00
COMPUTE NM HY		•		1120	17.91	.717	1.20068	1.500		R IMP=	70.00
ADD HYD	AP-2	2& 3	10 .0	8890	144.54	6.189	1.30528	1.500	2.540		
COMPUTE NM HY	•	-		0610	11.15	.468	1.44003	1.500	2.855 PE	R IMP=	90.00
ADD HYD	PART_AP-4	10& 3		9500 4500	155.69	6.657	1.31392	1.500	2.561		
COMPUTE NM HY *S TOTA	D PS-4 L <b>flow</b> from PS b	- ASTN TN		1530 n	17.12	. 452	.55387	1.500	1.749 PE	R IMP=	6.00
	RTION OF THE FLO		<del>,</del>		OUTH POND DU	E					
	LOW OVERTOPPING										
*S SEE	NOTES BELOW RELA	TIVE TO	AP-7.								
			<b></b>			<b></b>			<b>.</b> -		
	UVDDAADA		TO		PEAK	RUNOFF		TIME TO	CFS	PAGE =	2
COMMAND	HYDROGRAPH IDENTIFICATION				SCHARGE	VOLUME	RUNOFF	PEAK	PER	NATATIO	<b>A.</b> 1
· CHIMAIN	IDENTIFICATION	NO.	10. (34	MI)	(CFS)	(AC-FT)	(INCHES)	(HOURS)	ACRE	NOTATION	N
ADD HYD	PART AP-4	10& 3	30 .1 <sup>.</sup>	1030	172.81	7.109	1.20849	1.500	2.448		
^S %%%%%%%%%%%%	<del></del> <del>የ</del> %88%8%8%%%%%%%			፟፟፟፟ቔቔቔቔቔቔቔቔቔቔቔቔቔቔቔቔቔቔቔቔቔቔቔቔቔቔቔቔቔቔቔቔቔ							
	S SECTION WILL D		E FLOWS ENTER	RING COMAN	CHE PARK NO	RTH.					
	INS PN-1 TO PN-1										
° ጋጜጜጜጜጜጜጜጜጜጜጜ COMPUTE NM HYI	`\$\$\$\$\$\$\$\$\$\$\$\$\$\$\$\$\$\$\$\$\$\$ ````						1 04055	1 500	0 200 DE	D TMD- ·	74 00
COMPUTE NM HY		-		3230 3620	49.23 10.20	2.151 .413	1.24855 1.24855	1.500 1.500	2.382 PE	-	74.00 74.00
ADD HYD	AP-5			8850	59.43	2.564	1.24854	1.500	2.412	: AMI ~ /	, -, -, -, -, -, -, -, -, -, -, -, -, -,
ROUTE	AP-5R	10		850	58.41	2.564	1.24856	1.550	2.370		
COMPUTE NM HYL		-		740	-12.17	.493	1.24855	1.500	2.570 PE	R IMP= 7	74.00
COMPUTE NM HYE				410	6.10	.236	1.08101	1.500	2.323 PE	R IMP= €	50.00
NDD HYD	<b>—</b>			150	18.27	.729	1.18876	1.500	2.482		
NDD HYD ROUTE	AP-6 AP-6R	2&10 1 10		5000 5000	75.90 76.65	3.293 3.293	1.23479	1.500 1.550	2.372		
COMPUTE NM HYE		10		670	70.05 10.72	3.293 .429	1.23481 1.20068	1.550 1.500	2.395 2.499 PEI	à TMD= 3	70.00
COMPUTE NM HYL		-		800	12.79	.512	1.20068	1.500	2.499 PE		
ADD HYD				470	23.51	.941	1.20065	1.500	2.499		- <del></del>
OMPUTE NM HYD		-	3 .00	090	1.65	.069	1.44003	1.500	2.870 PE	l IMP= 9	0.00
COMPUTE NM HYD		_		240	4.39	. 184	1.44003	1.500	2.858 PE	IMP= 9	0.00
ADD HYD		3& 4 2	·	330	6.04	.253	1.43985	1.500	2.862		
ADD HYD ADD HYD	PN-7&PN-8&PN 1	10&20 1 2&10 9		800 800	29.55 102.23	1.195	1.24450	1.500	2.566		
DIVIDE HYD	AP-7 AP-7NORTH			800 699	102.23 94.00	4.487 4.421	1.23736 1.23736	1.550 1.500	2.349 2.193		
	AP-7SOUTH			101	8.23	.067	1.23736		12.690		
*S TOTAL	. FLOW FROM PS BA				_ <del>_</del>	~ <del>~ •</del>	<del></del>		<del></del>		

A PORTION OF THE FLOW FROM AP-7 WILL ENTER THE SOUTH POND DUE Page 1

TOTAL FLOW FROM PS BASIN INTO SOUTH POND

\*S

PRO10.SUM

*S	TO FLOW OVERTOPPING	THE C	URB AT C	COMANCHE RD.	AND BRYN MAWR	• - •					
*S ADD HYD	SEE NOTES BELOW REL		· · · •	. ' .11131	180.76	7.176	1.20876	1 500	2 527		
	WE WILL		-				1.20876	1.500	2.537		
*S					ECTING THE SOU						
*S											
*S	•		_		SERVOIR ROUTE						
*S					ANCHE NORTH PA						
ROUTE RES				.11131	29.63	5.699	. 95992	2.100	416	AC-FT=	4.244
COMPUTE				,02970	37.34	1.333	.84166	1.500		PER IMP=	40.00
ROUTE RES				.02970	15.53	1.333	.84165	1.700		AC-FT=	. 484
COMPUTE N	<b>-</b>			.00240	1.90	.052	.40419	1.500		PER IMP=	7.00
ADD HYD		158 3	_	.03210	16.28	1.385	.80892	1,650	.793	200	
ROUTE	AP-9R		2	.03210	16.28	1.385	.80895	1.700	.792		
COMPUTE N			3	.01840	27.74	1.084	1.10494	1.500		PER IMP=	62.00
ROUTE	AP-10R		12	.01840	24.38	1.084	1.10497	1,550	2.070		<b>32.33</b>
COMPUTE N			3	.01420	22.70	.909	1.20068	1.500		PER IMP=	70.00
ADD HYD	AP-10R&PN-12		10	.03260	44.06	1.994	1.14663	1.550	2.112		
ADD HYD	AP-11	_		.06470	56.88	3.378	.97908	1.550	1.374		
ROUTE RES	SERVOIR AP-11 POND	10	92	.06470	23.72	3.384	.98054	2.000		AC-FT=	.944
*S	<sub>፞</sub> ጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜ <sup>፟</sup> ጜጜጜጜ	<b>%%%%%%</b> %	<mark></mark> ነጻጜጜጜጜጜጜ	<b>***********</b>	፟ጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜ	<del>\%</del> %%%%%%%%%%					
*\$	THIS SECTION ADDS	ALL THE	FLOW G	OING INTO TH	E NORTH COMANCI	IE POND					
<b>*</b> \$ %%%%%	<b><sub>6</sub>%%%%%%%%%%</b> %%%%%%%%%%%%%%%%%%%%%%%%%%	<del>%%%%%%</del> %	<sup>ֈ</sup> ጜጜጜጜጜጜ	<b>**********</b>	፟ጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜ	<b>\%%%%%%%%%%%%</b>					
COMPUTE N	IM HYD PN-13	-	3	.01470	8.32	.207	.26340	1.500	.884	PER IMP=	.00
ADD HYD	PN-13&AP-4R	3&90	10	.12601	30.15	5.905	.87867	2.050	.374		
		FROM	TO		PEAK	RUNOFF		TIME TO	CFS	PAGE =	3
	HYDROGRAPH	ID	ĬD	AREA	DISCHARGE	VOLUME	RUNOFF	PEAK	PER		
COMMAND	IDENTIFICATION	NO.	NO.	(SQ MI)	(CFS)	(AC-FT)	(INCHES)	(HOURS)	ACRE	ITATON	ON
ADD HYD	PN-13&AP-4R&	98&10	10	.19300	110.22	10.326	1.00316	1.550	. 892		
ADD HYD	AP-12	92&10	10	.25770	128.37	13.709	.99748	1.550	.778		
*S %%%%%	<b>᠄ጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜ</b>	<del>ኒ</del> ፄ%%%%%	<b>፧ጜጜጜጜጜጜጜ</b>	<del>%%%%%%%%%%%%</del>	፟፟፟፟ጜጜጜጜጜጜጜጜጜጜጜጜጜጜ	<b>688888888</b> 8					
*S	THIS SECTION WILL I	ROUTE A	P-12 TH	ROUGH THE CO	MANCHE NORTH PO	ND.					
<b>*</b> \$	TO THE NORTH DIVERS	SION CH	ANNEL V	IA THE EXIST	ING PUMP/LIFT S	STATION.					
*S <b>ጓጓጓጓ</b> ጓጓ	<del>፧ጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜ</del>	<del>የ</del> ቆ8888	<b>*****</b>	<del>ጜጜጜጜጜጜጜጜጜጜጜጜጜ</del>	<del></del> 18888888888888888	<b>ֈ</b> ֍֍֍֍֍֍֍֍					
ROUTE RES Finish	ERVOIR AP-12-POND	10	93	.25770	15.83	13.703	.99702	3.200	.096 A	C-FT=	7.600

PRO100.SUM RUN DATE (MON/DAY/YR) = 12/18/2003 AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994 USER NO. = C\_ANDRSN.IO1 INPUT FILE = Pro100.IN PAGE = TIME TO CFS RUNOFF PEAK FROM PER PEAK RUNOFF VOLUME DISCHARGE **AREA HYDROGRAPH** ID NOTATION ACRE (HOURS) (INCHES) (AC-FT) (CFS) (SQ MI) NO. NO. IDENTIFICATION COMMAND TIME= .00 START COMMANCHE PARK CONSISTS OF TWO AREAS. ONE IS SOUTH OF COMMANCHE RD. EAST OF THE NORTH DIVERSION CHANNEL CONSISTING OF THE THUNDERBIRD LITTLE LEAGUE BASEBALL FIELDS. THE SECOND AREA IS NORTH OF COMANCHE BOTH AREAS ACT AS DETENTION POND FACILITIES. THE TWO AREAS ARE CONNECTED BY AN EXISTING 36" CULVERT UNDER COMANCHE BLVD. THE RUNOFF FLOWS IN THE NORTH POND ARE PUMPED UP TO THE NORTH DIVERSION CHANNEL. THE CONTRIBUTING DRAINAGE BASIN IS APPROXIMATELY 95% DEVELOPED. THIS ANAYLSIS ASSUMES 100% DEVELOPMENT. Zona Ton PROPOSED LAND TREATMENTS WERE UTILIZED FOR THE SOUTH PARK AREA WHILE EXISTING LAND TREATEMENTS WERE UTILIZED FOR THE NORTH PARK. ALL OTHER LAND TREATEMENTS WERE DETERMINED BY USING THE LATEST ARERIAL PHOTO OF THE AREA AND TABLE A-5 OF SECTION 22 OF THE CITY OF ALBUQUERQUE DEVELOPMENT PROCESS MANUAL (DPM). LOOYEAR, ZA-HOUR \*5 DATE: December 2, 2003 \*5 : Pro100.IN FILENAME - input : Pro100.0UT FILENAME - output The same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the same of the sa FILENAME - summary table : Pro100.SUM PROJECT TITLE: DRAINAGE REPORT FOR COMANCHE PARK \*S \*S SEC PROJECT NUMBER: 101103C \*S CONSULTANT: SMITH ENGINEERING COMPANY \*\$ CLIENT: CONSENSUS PLANNING INC. **\***\$ COMMANCHE PARK (North and South) DRAINAGE ANALYSIS: 100-YR, 24 HOUR STORM EVENT \*S 2.680 RAIN24= RAINFALL TYPE= 2 \*S THIS SECTION WILL ROUTE AND ADD THE BASINS THAT FLOW INTO SOUTH POND, BASINS PS-1 TO PS-4. BASIN PS-4 IS THE SOUTH POND/BASEBALL FIELDS 4.202 PER IMP= 80.00 1.500 2.13945 8.866 208.97 .07770 PS-18AP-1 -COMPUTE NM HYD 4.062 1.500 2.13945 8.866 202.00 .07770 AP-1R 3 ROUTE 4.015 PER IMP= 70.00 1.500 1.98545 1.186 28.78 .01120 PS-2 -COMPUTE NM HYD 4.056 1.500 2.12004 10.052 230.78 .08890 AP-2 2& 3 10 ADD HYD 4.399 PER IMP= 90.00 1.500 2.29343 .746 17.17 .00610 PS-3&AP-3 -COMPUTE NM HYD 4.078 1.500 2.13117 10.798 247.95 PART\_AP-4 108 3 10 ,09500 ADD HYD 3.187 PER IMP= 6.00 1.500 1.17589 .960 31.20 .01530 PS-4 -COMPUTE NM HYD TOTAL FLOW FROM PS BASIN INTO SOUTH POND \*S A PORTION OF THE FLOW FROM AP-7 WILL ENTER THE SOUTH POND DUE \*S TO FLOW OVERTOPPING THE CURB AT COMANCHE RD. AND BRYN MAWR DR. SEE NOTES BELOW RELATIVE TO AP-7. PAGE = 2CFS TIME TO RUNOFF PEAK FROM TO PER PEAK RUNOFF VOLUME DISCHARGE HYDROGRAPH ID ID ARËA NOTATION ACRE (HOURS) (INCHES) (AC-FT) (CFS) IDENTIFICATION NO. NO. (SQ MI) COMMAND 3.954 1.500 1.99866 11.757 279.15 .11030 PART\_AP-4 10& 3 30 ADD HYD THIS SECTION WILL DETERMINE FLOWS ENTERING COMANCHE PARK NORTH. BASINS PN-1 TO PN-13. 3.806 PER IMP= 74.00 1,500 2.04705 3.526 78.67 .03230 COMPUTE NM HYD PN-1 -4.093 PER IMP= 74.00 1.500 2.04705 .677 16.24 .00620 PN-2 -COMPUTE NM HYD 3.852 2.04703 1.500 4.203 94.91 .03850 AP-5 3& 4 10 ADD HYD 3.767 2.04706 1.500 4.203 92.82 .03850 AP-5R 10 ROUTE 4.092 PER IMP= 74.00 2.04705 1.500 19.38 .808 .00740 PN-3 -COMPUTE NM HYD 3.827 PER IMP= 60.00 1.500 1.83147 .400 10.04 .00410 PN-4 -COMPUTE NM HYD 3.998 1.97014 1.500 1.208 29.42 PART\_AP-6 3& 4 10 .01150 ADD HYD 3.820 2.02935 1.500 5.412 122.24 .05000 AP-6 2&10 10 ADD HYD 3.816 1.550 2,02937 5.412 122.12 .05000 AP-6R 10 2 ROUTE 4.016 PER IMP= 70.00 1.500 1.98546 17.22 .709 .00670 PN-5 COMPUTE NM HYD 4.015 PER IMP= 70.00 1.98546 1.500 .847 20.56 .00800 PN-6 COMPUTE NM HYD 4.016 1.500 1.98541 1.557 37.78 .01470 PN-5&PN-6 3& 4 ADD HYD 4,424 PER IMP= 90.00 1.500 2.29344 . 110 2.55 .00090 PN-7 COMPUTE NM HYD 4.405 PER IMP= 90.00 1.500 2.29344 .294 6.77

AP-7NORTH

PN-8

PN-78PN-8 38 4 20

AP-7 2&10 91

PN-7&PN-8&PN 10&20 10

COMPUTE NM HYD

ADD HYD

ADD HYD

ADD HYD

**\*S** 

DIVIDE HYD

.00240

.00330

.01800

.06800

.05922

.00878

9.31

47.09

94.00

71.20

165.20

4.410

4.088

3.796

2,480

12.669

1.500

1.500

1.500

1.450

1.500

2.29326

2.04185

2.03266

2.03266

2.03266

.404

1.960

7.372

6.420

. 952

AP-7SOUTH AND 99 TOTAL FLOW FROM PS BASIN INTO SOUTH POND \*S

A PORTION OF THE FLOW FROM AP-7 WILL ENTER THE SOUTH POND DUE

PRO100.SUM

*S TO	FLOW OVERTOPPING	THE C	URB AT	COMANCHE RD. AN	D BRYN MAWR	DR.					
*S SEE	NOTES BELOW REL	ATIVE	TO AP-7	<b>'.</b>							
ADD HYD	AP-4	30899	10	.11908	350.36	12.709	2.00116	1.500	4.597		
*\$	WE WILL	NOW RO	UTE THE	TOTAL FLOW INT	O THE SOUTH	POND THROUGH					
<b>*</b> \$	THE EXIS	STING 3	6" CULV	ERT PIPE CONNEC	TING THE SO	UTH POND TO					
<b>*</b> \$	THE NORT	TH POND									
*S	THE OUTF	LOW VA	LUES US	ED IN THIS RESE	RVOIR ROUTE	ASSUME A TAI					
*S	WATER EL	EV. OF	5079.2	5' IN THE COMAN	CHE NORTH PA	ARK POND.					
ROUTE RESERVE	DIR AP-4_POND	10	90	. 11908	43.22	11.228	1.76795	2.100	.567	AC-FT=	7.909
COMPUTE NM H	D PN-98AP-8	4	3	.02970	65.37	2.413	1.52349	1.500	3.439	PER IMP=	40.00
ROUTE RESERVE	DIR AP-8_POND	3	15	.02970	24.81	2.413	1.52348	1.750	1.305	AC-FT=	.941
COMPUTE NM HY	/D PN-10	-	3	.00240	4.08	. 122	.94922	1.500	2.657	PER IMP=	7.00
ADD HYD	AP-9	15& 3	10	.03210	26.47	2.535	1.48052	1.700	1.288		
ROUTE	AP-9R	10	2	.03210	26.44	2.535	1.48055	1.700	1.287		
COMPUTE NM HY	D PN-11&AP-10	-	3	.01840	45.46	1.828	1.86226	1.500	3.861	PER IMP=	62.00
ROUTE	AP-10R	3	12	.01840	42.33	1.828	1.86229	1.550	3.595		
COMPUTE NM HY	7D PN-12	<b>:</b> -	3	.01420	36.48	1.504	1.98546	1.500	4.014	PER IMP≔	70.00
ADD HYD	AP-10R&PN-12	12& 3	10	.03260	75.40	3.331	1.91592	1.500	3.614		
ADD HYD	AP-11	2&10	10	-06470	96.69	5.866	1.69990	1.550	2.335		
ROUTE RESERVO	IR AP-11_POND	10	92	.06470	36.74	5.873	1.70199	2.050	.887	AC-FT=	1.947
			·	<b>%%%%%%%%%%%%%%%</b>							
*S TH	S SECTION ADDS	ALL THE	E FLOW	GOING INTO THE I	NORTH COMANO	HE POND					
•		<b>%%%%%%</b> %	<b>ኔ</b> %%%%%%	<del></del> ጻ%ጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜ	<b>ጜጜጜጜጜጜጜጜጜጜጜ</b> ጜ	<b>6</b> 8888888888					
COMPUTE NM HY			3	.01470	21.03	.589	.75080	1.500	2.235	PER IMP=	.00
ADD HYD	PN-13&AP-4R	3&90	10	. 13378	49.77	11.817	1.65619	1.600	.581		
		FROM	TO		PEAK	RUNOFF		TIME TO	CFS	PAGE =	3
	HYDROGRAPH	ID	ID	AREA [	DISCHARGE	VOLUME	RUNOFF	PEAK	PER		
COMMAND	IDENTIFICATION	NO.	NO.	(SQ MI)	(CFS)	(AC-FT)	(INCHES)	(HOURS)	ACRE	NOTATI	ON
ADD HYD	PN-13&AP-4R&	98&10	10	. 19300	143.77	18.237	1.77170	1.600	1.164		
ADD HYD	AP-12	92&10	10	.25770	171.63	24.110	1.75420	1.650	1.041		
*S	<b>%%%%</b> %%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%	<del>የየ</del> የጻጻ	<b>688888</b> 8	ጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜ	<mark>፟፟፟፟ጜጜጜጜጜጜጜጜጜጜጜ</mark> ጜጜጜጜጜጜጜ	<b>የ</b> የያያያያያያያያያ					
*S THI	S SECTION WILL	ROUTE A	P-12 T	HROUGH THE COMAN	NCHE NORTH P	OND.					
*S TO	THE NORTH DIVER	SION CH	IANNEL	VIA THE EXISTING	PUMP/LIFT	STATION.					
<b>*\$</b>	<b>%%%%%%%%%%%%%%%</b>	<del>%%%</del> %%%%	; <b>%%%%</b> %%	ጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜጜ	<mark>ֈ</mark> ֎֍֍֍֍֍֍֍֍֍֍	<b>%%%%%%%%%%%</b> %					
ROUTE RESERVO FINISH	IR AP-12-POND	10	93	.25770	15.83	24.102	1.75364	4.450	.096	AC-FT=	15.135

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