

City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

October 31, 1988

Jeff Mortensen, P.E. Jeff Mortensen & Associates, Inc. 811 Dallas, NE Albuquerque, New Mexico 87110

RE: DRAINAGE PLAN FOR AN ADDITION TO PARADISE VILLAGE, INC. (G-17/D34) ENGINEER'S STAMP DATED OCTOBER 8, 1988

Dear Mr. Mortensen:

Based on the information provided on your submittal of October 10, 1988, the above referenced plan is approved for Building Permit.

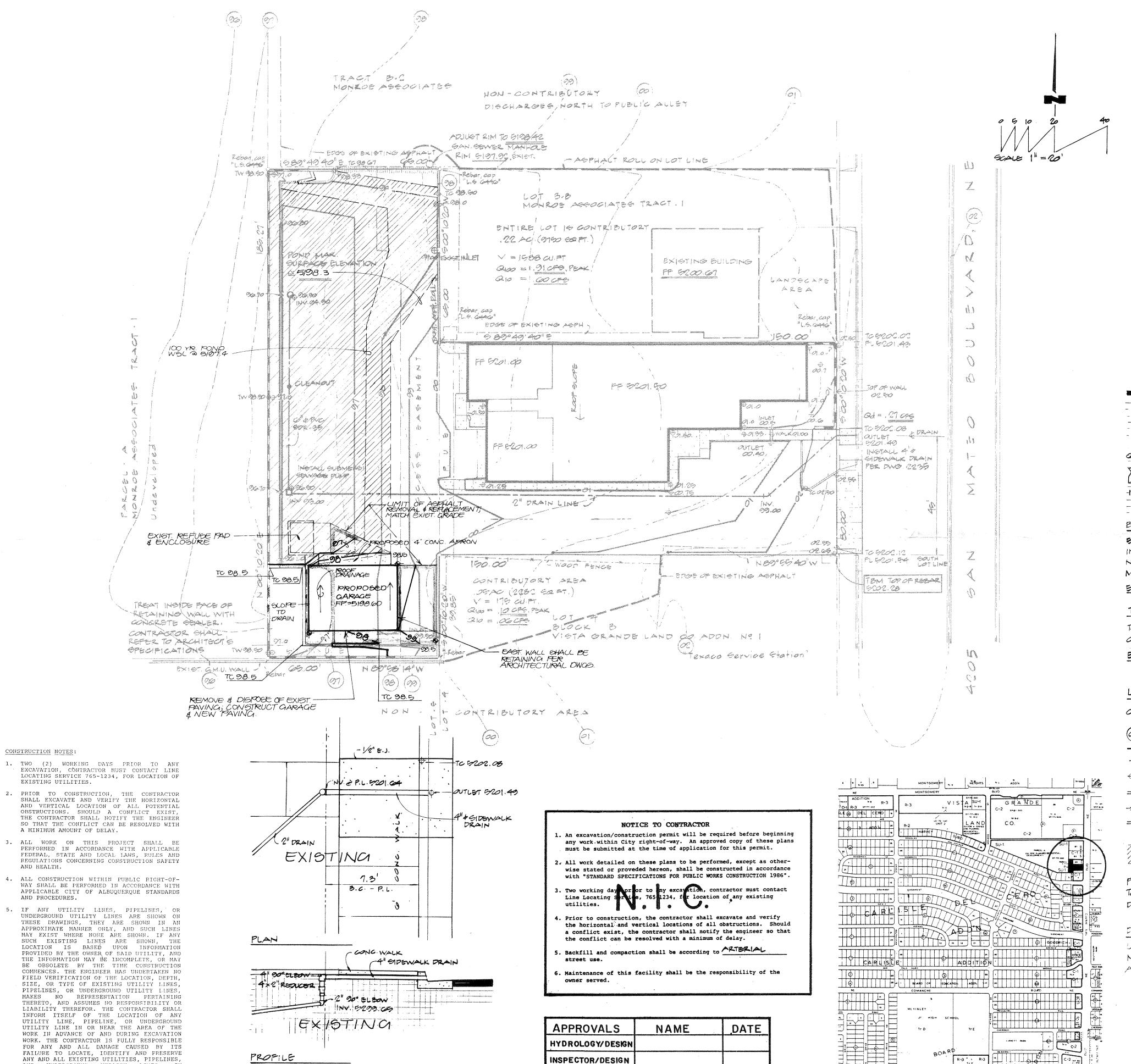
Please attach a copy of this plan to the construction sets prior to sign-off by Hydrology.

If I can be of further assistance, please feel free to call me at 768-2650.

Cordially,

Dennu & Modage Bernie J. Montoya, C.E. Engineering Assistant

BJM/bsj (WP+691)



CONST/ACPT.

VICINITY MAP

CONSTRUCTION NOTES:

AND HEALTH.

AND PROCEDURES.

FACILITIES.

- AND UNDERGROUND UTILITY LINES. IN

PLANNING AND CONDUCTING EXCAVATION, THE

STATUTES, MUNICIPAL AND LOCAL ORDINANCES, RULES AND REGULATIONS, IF ANY, PERTAINING

CONTRACTOR SHALL COMPLY WITH STATE

TO THE LOCATION OF THESE LINES AND

SIDEWALK DRAIN DETAILS 14" = 1'.0'

DRAINAGE PLAN The following items concerning the Paradise Village Drainage Plan are contained hereon: Vicinity Map

Grading Plan 3. Calculations

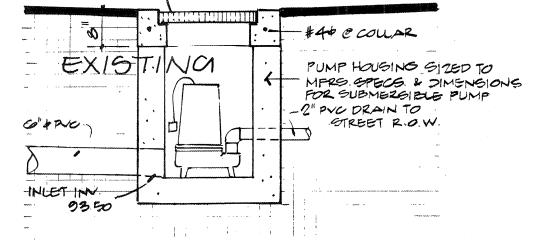
As shown by the Vicinity Map, the site is located on the west side of San Mateo Boulevard N.E. between Montgomery Boulevard N.E. and Douglas MacArthur N.E. At present, the site is developed with a building, paving and landscaping. The surrounding area is mostly developed. The proposed improvements consist of the addition of a 2car garage and minor repaving.

As shown by Plate G-17 of the Albuquerque Master Drainage Study, (AMDS), this site does not lie within a designated Flood Hazard Zone, however, downstream flooding is noted at the intersection of Montgomery Boulevard N.E. and San Mateo Boulevard N.E. Because of this, this site detains its runoff with an existing submersible pump which discharges to San Mateo Boulevard N.E.

The Grading Plan shows 1) existing and proposed grades indicated by spot elevations and contours at 1'-0" intervals, 2) continuity between existing and proposed grades, 3) the limit and character of the existing improvements, and 4) the limit and character of the proposed improvements. At present, the site drains from east to west to an existing detention pond at the west side of the site. From this point, the ponded runoff is pumped east onto San Mateo Boulevard N.E. and released at a controlled rate of 0.3 cfs. No offsite flows enter the site from the east because of the presence of the adjacent developed street. Offsite flows enter the site from the north and south and are included in the pond storage volumes. No offsite flows enter the site from the west, as the adjacent property is topographically lower than the project site.

The Calculations which appear hereon analyze both the existing and developed conditions for the 100-year, 6-hour rainfall event. The Rational Method has been used to quantify the peak rate of discharge and the SCS Method has been used to quantify the volume of runoff. However, the SCS Method was not employed in the previous submittal for this project site dated 3-1-88, 6-1-88, and 7-6-88. Therefore, any differences in the volume quantities from the previous submittals are due to this. Both Methods have been used in accordance with the City of Albuquerque Development Process Manual, Volume II, and the Mayor's Emergency Rule adopted January 14, 1986. As shown by these calculations, the proposed improvements will have a negligible affect on the runoff generated by the site.

24" × 24" GRATING IN CONC. COLLIR



5. FIBA STANDARD ACG BRAGS TABLET SET IN MEDIAN STRIP OF MONTCOMERY BOULEVARD, he east side of intersection with san MATED BUYD.

ELEY. 5201.83

T.B.M.

TOP OF REBAR FOUND AT GOUTHEAST PROPERTY CORNER OF GITE (ALONG GAN MATEO BLVD.)

ELEV. 5202.28

LEGEND

CONTOUR INTERVAL ONE FOOT (1.0')

(01) --- EXISTING CONTOUR PROPOSED CONTOUR existing spot elevation 01.00 PROPOSED SPOT ELEVATION

EXISTING CMU WALL ---- EXISTING FENCE

PROPOSED CMU WALL "T.W." INDICATES LEVEL OF CONCRETE FILL

extent of petention bagin

PROPOSED SPOT ELEVATIONS AND CONTOURS ARE TO FINISHED GURFACES, CONTRACTOR SHALL DETERMINE APPROPRIATE GUBGRADES

DEGCRIPTION OF PROPERTY

LOT B-A

MONROE AGGOCIATES - TRACT ! ALBUQUERQUE, NEW MEXICO

4005 SAN MATEO BOULEN AZO, NE



CALCULATIONS

Ground Cover Information From SCS Bernalillo County Soil Survey, Plate 21: TgB - Tijeras Hydrologic Soil Group: B Existing Pervious CN = 61 (DPM Plate 22.2 C-2 Open Spaces: good condition) Developed Pervious CN = 61 (DPM Plate 22.2 C-2 Open Spaces: Good Condition)

Time of Concentration/Time to Peak $T_C = 0.0078 L^{0.77}/S^{0.385}$ (Kirpich Equation)

 $T_p = T_c = 10 \text{ min.}$ Point Rainfall $P_6 = 2.3 \text{ in. (DPM Plate 22.2 D-1)}$

Rational Method Discharge: Q = CiA where C varies $i = P_6 (6.84) T_C^{-0.51} = 4.86 in/hr$ $P_6 = 2.20 in (DPM Plate 22.2D-1)$

A = area, acres SCS Method

Volume: V = 3630(DRO) AWhere DRO = Direct runoff in inches A = area, acres

 $T_C = 10 \text{ min (minimum)}$

Existing Condition

Atotal = 24,045 sf = 0.55 Ac Roof area = 530 sf (0.22) Paved area = 16,905 sf (0.70)Landscaped area = 1,830 sf (0.08)C = 0.88 (Weighted average per Emergency Rule, 1/14/86) $Q_{100} = CiA = 0.88(4.86)(0.55) = 2.4 cfs$ A_{imp} = 17,435 sf; % impervious = 73 % Composite CN = 88 (DPM Plate 22.2 C-3) DRO = 1.3 in (DPM Plate 22.2 C-4) $V_{100} = 3630 \text{ (DRO)A} = 2,600 \text{ cf}$

<u>Developed Condition</u>

Atotal = 24,045 sf = 0.55 Ac Roof area = 6,110 sf (0.25) Paved area = 16,885 sf (0.70)Landscaped area = 1,050 sf (0.04)C = 0.90 (Weighted average per Emergency Rule, 1/14/86) $Q_{100} = CiA = 0.90(4.86)(0.55) = 2.4$ cfs Aimp = 22,995 sf; % impervious = 95 % Composite CN = 96 (DPM Plate 22.2 C-3) DRO = 1.9 in (DPM Plate 22.2 C-4)

 $\frac{\text{Comparison}}{\Delta Q_{100} = 2.4 - 2.4 = 0 \text{ cfs (Unchanged)}}$ $\Delta V_{100} = 3,800 - 2,600 = 1,200 cf (Increase)$

V100 = 3630 (DRO)A = 3,800 cf

Offsite Contributing Flows

Qoffsite = 1.0 cfs Voffsite = 1,760 cf

Pond Calculations

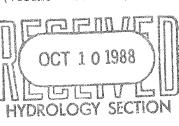
Qtotal = 2.4 + 1.0 = 3.4 cfs Vtotal = 3800 + 1730 = 5530 cf VTotal = VRequired (Required on Site Volume)

Volume Provided

- 96.5 to 97.0 Area @ 97.0 contour = 4215 sf Volume = 1/2(4215+0)(97-96.5) = 1055 cf

- 97 to 98 Area @ 98 contour = Volume = $1/2(7100+4215)(98-97) = \underline{5660}$ Total Volume

100 year WSL @ Approx. 5197.4 (Volume = 5500 cf)



SHEET

OF