

City of Albuquerque

Planning Department nt & Building Services Division

n: Tract A		lodgin Elem			
n: Tract A, Hodgin Elem. School, Albuquerque, NM	EPC#: Work Order#:	Iodgin Elementary School Building Permit #: City Drainage #:	(REV 02/2013)	DRAINAGE AND TRANSPORTATION INFORMATION SHEET	Development & pariame ou vices privision
			G17000	j	

WAS A PRE-DESIGN CONFERENCE ATTENDED: DATE SUBMITTED: May 31, 2013	X DRAINAGE REPORT X DRAINAGE PLAN 1st SUBMITTAL DRAINAGE PLAN RESUBMITTAL CONCEPTUAL G & D PLAN GRADING PLAN EROSION & SEDIMENT CONTROL PLAN (ESC) ENGINEER'S CERT (HYDROLOGY) CLOMR/LOMR TRAFFIC CIRCULATION LAYOUT (TCL) ENGINEER'S CERT (DRB SITE PLAN) ENGINEER'S CERT (ESC) SO-19 OTHER (SPECIFY)	Phone#: Fax#:	Contractor:Address:	Phone#: Fax#:	Surveyor: Address:	Phone#: 243-7492 Fax#:	اğ۱.	Gregory T. Hicks & Associates	Address: Fax#:Fax#:	Owner: APS	ľ	Engineering Firm: Isaacson & Arfman, P.A.	City Address: 3801 Morningside Drive NE, 87110	J.	Project Title: Hodgin Elementary School
Yes No Copy Provided By: Isaacson & Arfman PA	CHECK TYPE OF APPROVAL/ACCEPTANCE SOUGHT: SIA/FINANCIAL GUARANTEE RELEASE PRELIMINARY PLAT APPROVAL S. DEV. PLAN FOR SUB'D APPROVAL S. DEV. FOR BLDG. PERMIT APPROVAL SECTOR PLAN APPROVAL FINAL PLAT APPROVAL CERTIFICATE OF OCCUPANCY (PERM) CERTIFICATE OF OCCUPANCY (TCL TEMP) COUNDATION PERMIT APPROVAL BUILDING PERMIT APPROVAL GRADING PERMIT APPROVAL PAVING PERMIT APPROVAL PAVING PERMIT APPROVAL GRADING CERTIFICATION CESC CERT. ACCEPTANCE OTHER (SPECIFY)	E-mail:	Contact:	E-mail:	Comaci.		ا اد	Contact: Greg Hicks, AIA	E-mail:	Contact:		Je, NM 87108 Contact: Fred C. Arfman, PE	riquei que, ener	Work Order#:	Building Permit #: City Diamage #:

Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a drainage submittal. The particular nature, location, and scope to the proposed development defines the degree of drainage detail. One or more of the following levels of submittal may be required based on the following 1. Conceptual Grading and Drainage Plans: Required for approval of Site Development Plans greater than five (5) acres and Sector Plans 2. Drainage Report: Required for building permits, parding permits and site plans less than five (5) acres 3. Drainage Report: Required for subdivision containing more than ten (10) lots or constituting five (5) acres or more 4. Erosion and Sediment Control Plan: Required for any new development and redevelopment site with 1-acre or more of land disturbing area, including project less than 1-acre than are part of a larger common plan of development



Isaacson & Arfman, P.A.

Consulting Engineering Associates

Thomas O. Isaacson, PE (Ret.) & LS (Ret.) * Fred C. Arfman, PE * Åsa Nilsson-Weber, PE

Hodgin Elementary School by Isaacson & Arfman, PA May 20, 2013

The majority of the Hodgin Elementary School property drains to an existing sports field / detention pond constructed as part of 2001 site improvements. This pond includes outlet control structures to limit release of discharge to Hermosa Street. The proposed improvements consist of two separate areas of the school. Each proposed development will result in an insignificant change to the 100-year 6-hour storm discharge to the pond.

Kindergarten Addition:

The proposed kindergarten building addition (CG-101) will continue to discharge to historic flowpath(s). Resulting increase of 0.06 cfs to the existing pond. See Sheet CG-101 for calculations.

Courtyard Drainage Improvements:

The proposed courtyard regrading and storm drain improvements are intended to protect the existing buildings from future flooding.

In the existing condition, the upper and lower courtyard walks and landscaping are graded high in the center and drain towards the buildings. The existing curbed planters exacerbate an already problematic area. In the past, numerous localized 'fixes' have been constructed but none have worked sufficiently to prevent flooding and water damage. Currently, sandbags are kept on-site to reduce stormwater passing into the building during rainstorms.

The historic area draining into the courtyards is estimated conservatively at 0.9 acres. This area, at the land treatments shown (see attached calculations), will generate approximately 3.4 cfs during a 100-year 6-hour storm event or approximately 1.7 cfs to each courtyard.

Document10

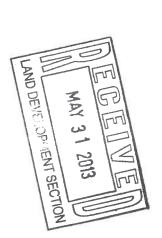
Page **1** of 2

LAND DEVESSIBLE SECTION 5/31/2013

The stormwater enters the courtyard via numerous roof drain downspouts located on all sides. The majority of these downspouts will be extended directly to the proposed storm drain system. In addition, the removal and replacement of concrete walks in the upper courtyard will allow for sloping away from existing doors and overflows to the surrounding landscaping. Trench drains and surface drains will be installed to collect surface flow. The discharge will drain to the existing regional pond as it is historically intended to do.

A water quality manhole (ADS Nyloplast inlet with 'snout') will be installed in the lower courtyard before flow passes beneath the building to the exterior system.





1773 - COURTYARD - DPM Calculations - 100 yr 6 hr.xlsm

								AREA OF SITE:		Based on Drainage Design Criteria f	CALCULATIONS:	
Total Area =	Area D =	Area C =	Area B =	Area A =		STORM DISCHARGE:	100-year, 6-hour	40000 SF	ON-SITE	Based on Drainage Design Criteria for City of Albuquerque Section 22.2, DPM, Vol 2, dated Jan., 1993	CALCULATIONS: Hodgin Elementary School Courtyard Drainage Solution:	
40000	20000	12000	8000	0	Treatment SF			II		PM, Vol 2, dated J	ard Drainage Solu	
100%	$E_D = 2.12$	$E_C = 1.13$	$E_{\rm B} = 0.78$	$E_{A} = 0.53$	% Precip. Zone	EXCESS PRECIP		0.9		Jan., 1993	ition: May 31, 2012	
	2	3	•	3	2	P:						

On-Site Weighted Excess Precipitation (100-Year, 6-Hour Storm)

Weighted $E = \frac{E_A A_A + E_B A_B + E_C A_C + E_D A_D}{A_A + A_B + A_C + A_D}$ [Developed $E = \frac{E_A A_A + E_B A_B + E_C A_C + E_D A_D}{E_A + E_B A_B + E_C A_C + E_D A_D}$

E*A / 12 1.56 in.

On-Site Peak Discharge Rate: $Qp = Q_{pA}A_A + Q_{pB}A_B + Q_{pC}A_C + Q_{pD}A_D / 43,560$ For Precipitation Zone 2 $Q_{pA} = 1.56$ $Q_{pC} =$

Developed V₃₆₀

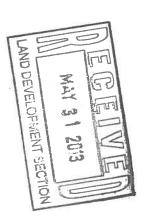
11

5183 CF

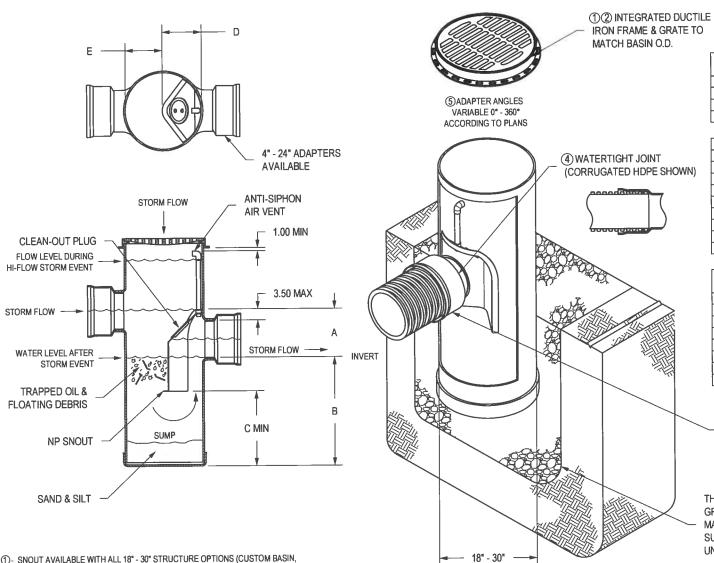
On-Site Volume of Runoff: V360 =

 Q_{pB} Н 1.56 2.28

Developed Q_p Q_{pD} II II 3.14 4.70 3.4 CFS



NYLOPLAST DRAIN BASIN WITH SNOUT



SNOUT OPEN AREA OPEN AREA BASIN W/SNOUT PART# W/O SNOUT SIZE 5618AG041 254 07 9.45 8.52 18' 12.24 11.59 290.50 5624AG0418 446 00 5630AG0418 15.25 15.50 509.95 742.40 21 75 8.99 354.63 5630AG24 742.40

PERCENTAGE OF SNOUT OPEN AREA VS OUTLET PIPE OPEN AREA													
PIPE SIZE	5618AG0412	5624AG0418	5630AG0418	5630AG24									
4"	551%	893%	1395%	2469%									
6"	257%	417%	651%	1153%									
8"	145%	235%	366%	649%									
10"	93%	150%	235%	415%									
12"	63%	102%	159%	281%									
15"	N/A	67%	104%	184%									
18"	N/A	46%	72%	127%									
24"	N/A	N/A	N/A	72%									

		В	В	В	
PIPE SIZE	Α	18" BASIN	24" BASIN	30" BASIN	С
4"	7.51	24.18	30.05	36.87	12.00
6"	9.25	22.44	28.31	35,13	12.00
8"	11.29	24.40	30.27	37.09	16.00
10"	13.21	26.48	32.35	39.17	20.00
12"	15.05	28.64	34.51	41.33	24.00
15"	17.78	N/A	37.78	44 60	30 00
18°	20.98	N/A	40.58	47 40	36.00
24°	27.79	N/A	N/A	58 96	48 00

4 VARIOUS TYPES OF INLET & OUTLET ADAPTERS AVAILABLE: 4" - 24" FOR CORRUGATED HDPE (ADS N-12, ADS SINGLE WALL, HANCOR DUAL WALL), SDR 35, SCH 40 DWV, CORRUGATED & RIBBED PVC

THE BACKFILL MATERIAL SHALL BE CRUSHED STONE OR OTHER GRANULAR MATERIAL MEETING THE REQUIREMENTS OF CLASS II MATERIAL AS DEFINED IN ASTM D2321. BEDDING & BACKFILL FOR SURFACE DRAINAGE INLETS SHALL BE PLACED & COMPACTED UNIFORMLY IN ACCORDANCE WITH ASTM D2321.

①- SNOUT AVAILABLE WITH ALL 18" - 30" STRUCTURE OPTIONS (CUSTOM BASIN, ROAD & HIGHWAY, & CURB INLET)
 ②- FRAMES, GRATES, HOODS, & BASE PLATES SHALL BE DUCTILE IRON PER

ASTM A536 GRADE 70-50-05

3 - DRAIN BASIN TO BE CUSTOM MANUFACTURED ACCORDING TO PLAN DETAILS RISERS ARE NEEDED FOR BASINS OVER 84" DUE TO SHIPPING RESTRICTIONS SEE DRAWING NO. 7001-110-065

(4)- DRAINAGE CONNECTION STUB JOINT TIGHTNESS SHALL CONFORM TO ASTM D3212 FOR CORRUGATED HDPE (ADS & HANCOR DUAL WALL) &

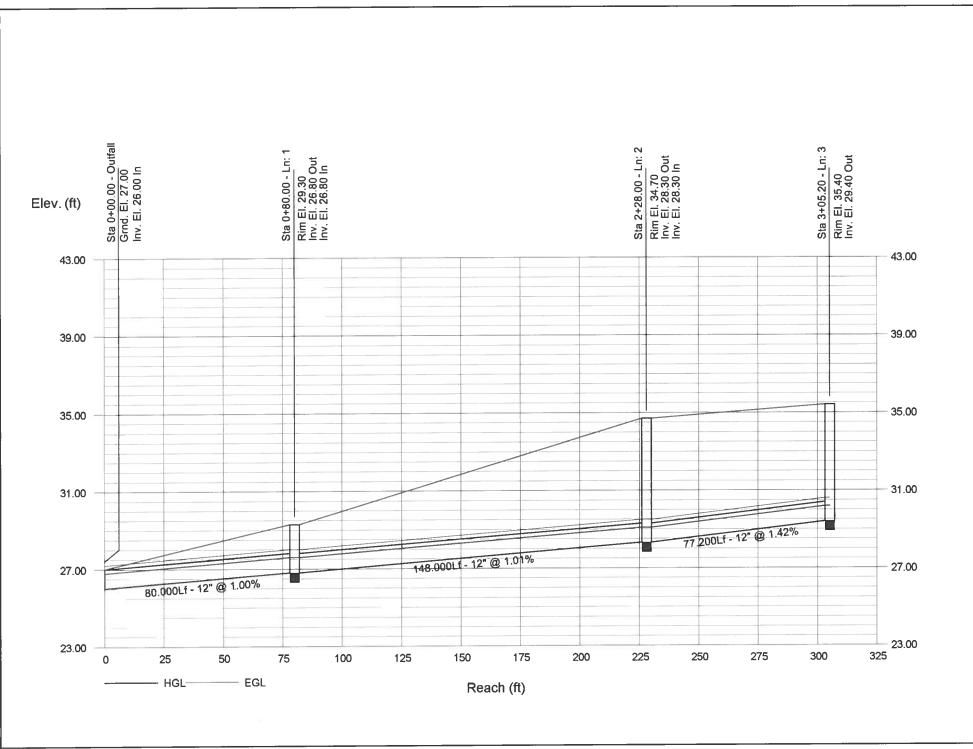
ADAPTERS CAN BE MOUNTED ON ANY ANGLE 0° TO 360°. TO DETERMINE MINIMUM ANGLE BETWEEN ADAPTERS SEE DRAWING NO. 7004-110-043, 7004-110-044, & 7004-110-045

THIS PRINT DISCLOSES SUBJECT MATTER IN WHICH NYLOPLAST HAS PROPRIETARY RIGHTS. THE RECEIPT OR POSSESSION OF THIS PRINT DOES NOT CONFER, TRANSFER, OR LICENSE THE USE OF THE DESIGN OR TECHNICAL INFORMATION SHOWN HEREIN REPRODUCTION OF THIS PRINT OR ANY INFORMATION CONTAINED HEREIN, OR MANUFACTURE OF ANY ARTICLE HEREFROM, FOR THE DISCLOSURE TO OTHERS IS FORBIDDEN, EXCEPT BY SPECIFIC WRITTEN PERMISSION FROM NYLOPLAST

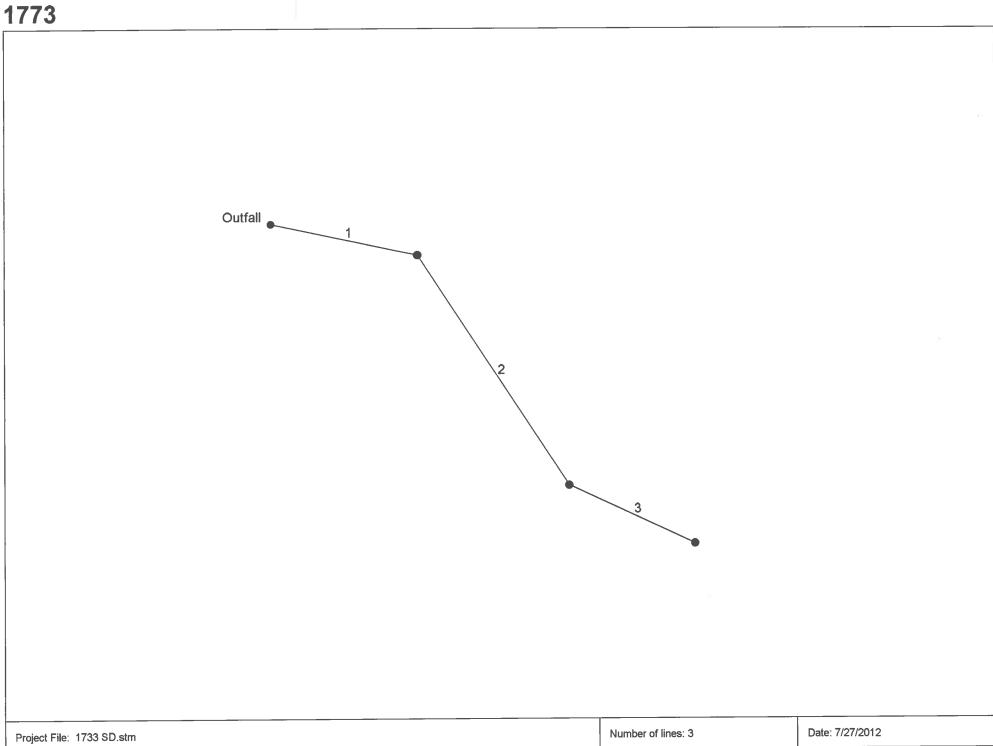
ORAWN BY	EBC	MATERIA	L			1 4		BUI
DATE	4-27-06					Nyl	oplast	PHI FAX
APPD BY	CJA	PROJEC1	NO./NA	AE .		TITLE	opza.o.	ww
DATE	4-27-06						DRAIN BASIN WIT	
NWG SIZE	Δ	SCALE	1:30	SHEET	1 OF 1	DWG NO.	7004-110-050)

3130 VERONA AVE BUFORD, GA 30518 PHN (770) 932-2443 XX (770) 932-2490

ww.nyloplast-us.com N DETAIL REV B



Storm Sewers



Storm Sewers v9.00

Storm Sewer Inventory Report

.ine		Align	nent			Flow	Data					Line ID					
	Dnstr Line No.	Length	Defl angle (deg)	Junc Type	Known Q (cfs)	Drng Area (ac)	Runoff Coeff (C)	Inlet Time (min)	Invert El Dn (ft)	Line Slope (%)	Invert El Up (ft)	Line Size (in)	Line Shape	N Value (n)	J-Loss Coeff (K)	Inlet/ Rim El (ft)	
1	End	80.000	11.995	мн	0.00	0.00	0.00	0.0	26.00	1.00	26.80	12	Cir	0.012	0.75	29.30	
2	1	148.000	44.937	MH	0.00	0.00	0.00	0.0	26.80	1.01	28.30	12	Cir	0.012	0.58	34.70	
3	2	74.000			3.40	0.00	0.00	0.0	28.30	1.49	29.40	12	Cir	0.012	1.00	35.40	
		!		;			;										
			:										=				
				ì													
						al al	:									:	
							!		:		:			:			
1773												Number o	of lines: 3			Date: 7	/27/2012

C4	Course	Tabulati	0 D
Storm	Sewer	Tabulati	on

Page	•
------	---

Statio	n	Len	Drng A	rea	Rnoff	Area x	С	Тс			Total	Cap full	Vel	Pipe		Invert Ele	overt Elev HGL Elev		Grnd / Ri	m Elev	Line ID	
Line	То		Incr	Total	coeff	Incr	Total	Inlet	Syst	(1)	liow	Tuli		Size	Slope	Dn	Up	Dn	Up	Dn	Up	
	Line	(ft)	(ac)	(ac)	(C)			(min)	(min)	(in/hr)	(cfs)	(cfs)	(ft/s)	(in)	(%)	(ft)	(ft)	(ft)	(ft)	(ft)	(ft)	
1 2 3		80.000 148.000 74.000	0.00	0.00 0.00 0.00	0.00 0.00 0.00	0.00	0.00	0.0 0.0 0.0	(min) 0.9 0.3 0.0	0.0 0.0 0.0	(cfs) 3.40 3.40 3.40	3.86 3.88 4.70	5.16 5.16 5.16	12 12 12	1.00 1.01 1.49	26.00 26.80 28.30	26.80 28.30 29.40	26.78 27.58 29.08	27.58 29.08 30.18	(ft) 27.00 29.30 34.70	29.30 34.70 35.40	
1773	<u> </u>	<u> </u>	L			<u> </u>	<u> </u>									Number	r of lines: 3			Run Da	te: 7/27/20	12

NOTES:Intensity = 69.87 / (Inlet time + 13.10) ^ 0.87 ; Return period =Yrs. 2 ; c = cir e = ellip b = box

Storm Sewers v9 00

Storm Sewer Summary Report

Line No.	Line ID	Flow rate (cfs)	Line Size (in)	Line shape	length	Invert EL Dn (ft)	Invert EL Up (ft)	Line Slope (%)	HGL Down (ft)	HGL Up (ft)	Minor loss (ft)	HGL Junct (ft)	Dns Line No.	Junction Type
1		3.40	12	Cir	80.000	26.00	26.80	1.000	26.78	27.58	0.31	27.58	End	Manhole
2		3.40	12	Cir	148.000	26.80	28.30	1.014	27.58	29.08	0.24	29.08	1	Manhole
3		3.40	12	Cir	77.200	28.30	29.40	1.425	29.08	30.18	0.41	30.18	2	Generic
1773				<u> </u>	<u> </u>		<u> </u>	1	Number o	f lines: 3	J	Run I	Date: 7/27/	2012
NOTES	Return period = 2 Yrs.			<u>. </u>					I					

Storm Sewers v9.00