



City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

October 30, 1995

Richard Hall
Hall Engineering Co., Inc.
6840 2nd St. NW
Albuquerque, NM 87107

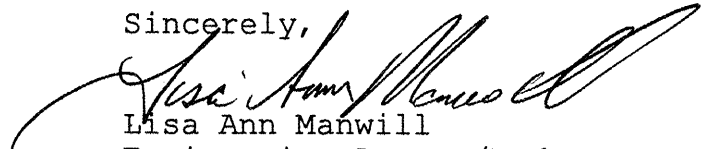
**RE: CLEVELAND MIDDLE SCHOOL (G18-D36) ENGINEER'S CERTIFICATION
FOR CERTIFICATE OF OCCUPANCY. ENGINEER'S CERTIFICATION
DATED OCTOBER 25, 1995.**

Dear Mr. Sexton:

Based on your submittal dated October 26, 1995, the above
referenced project is approved for Certificate of Occupancy.

If I can be of further assistance, please feel free to contact me
at 768-3622.

Sincerely,



Lisa Ann Manwill
Engineering Assoc./Hyd.

c: Pat McMurray - APS
Andrew Garcia
File



City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

June 16, 1994

Christopher L. Weiss
C.L. Weiss Engineering Inc.
P.O. Box 97
Sandia Park, NM 87047

RE: DRAINAGE PLAN FOR AN ADDITION & GRADING/PAVING @ A.P.S.
CLEVELAND MIDDLE SCHOOL (G18-D36) ENGINEER'S STAMP DATED 5/26/94.

Dear Mr. Weiss:

Based on the information provided on your June 2, 1994 submittal, the above referenced site is approved for Building Permit.

Please attach a copy of this approved plan to the construction sets prior to sign-off by Hydrology.

Also, prior to Certificate of Occupancy release, the following will be required:

1. Engineer Certification per the D.P.M. Checklist will be required.
2. Concurrence for Work Order release.

If I can be of further assistance, please feel free to contact me at 768-2667.

Sincerely,

Bernie J. Montoya, CE
Engineering Associate

BJM/dl/WPHYD/8474

c: Andrew Garcia
File

AHYMO SUMMARY TABLE (AHYMO392) - AMAFCA VERSION OF HYMO - MARCH, 1992
 RUN DATE (MON/DAY/YR) =05/26/1994
 INPUT FILE = clev.dat
 USER NO.= C_WEISS_.S92

TIME TO	CFS	PAGE = 1	FROM	TO	AREA	PEAK	RUNOFF
RUNOFF	PEAK	HYDROGRAPH	ID	ID		DISCHARGE	VOLUME
COMMAND	PER	IDENTIFICATION	NO.	NO.	(SQ MI)	(CFS)	(AC-
FT)	(INCHES)	(HOURS)	ACRE	NOTATION			
START							
TIME=	.00						
RAINFALL	TYPE= 1						
RAIN6=	2.450						
COMPUTE NM HYD		101.00	-	1	.01840	54.73	2.069
2.10794	1.500	4.648	PER IMP=	90.00			
COMPUTE NM HYD		102.00	-	2	.01750	40.50	1.460
1.56378	1.500	3.616	PER IMP=	57.00			
ROUTE		101.10	1	3	.01840	48.49	2.069
2.10795	1.533	4.118					
ADD HYD		102.10	2& 3	4	.03590	87.51	3.528
1.84267	1.533	3.809					
COMPUTE NM HYD		102.20	-	5	.01270	27.42	.954
1.40803	1.500	3.373	PER IMP=	44.00			
ADD HYD		102.30	4& 5	6	.04860	114.06	4.482
1.72908	1.533	3.667					
COMPUTE NM HYD		103.00	-	7	.00880	19.00	.661
1.40803	1.500	3.374	PER IMP=	44.00			
ROUTE		102.40	6	8	.04860	113.40	4.482
1.72909	1.533	3.646					
ADD HYD		103.10	7& 8	9	.05740	131.81	5.143
1.67986	1.533	3.588					
FINISH							

AHYMO PROGRAM (AHYMO392) - AMAFCA VERSION OF HYMO - MARCH, 1992
 RUN DATE (MON/DAY/YR) = 05/26/1994
 START TIME (HR:MIN:SEC) = 07:20:07 USER NO.= C_WEISS_.S92
 INPUT FILE = clev.dat

*APS Cleveland Middle School

*

START Time=0.0

*

*File: D:\Storm\Hydro\Clev.dat

*

*City of Albuquerque Criteria, January, 1993

*Re-run of AYHMO, per City review comments (G18-D36)

*

*Basin 1, Area to Natalie & Georgis Street to AP100

*Commercial Density and Cleveland School Buildings

*Basin 1.....to AP100

*

RAINFALL Type=1 Rain Qutr=0.0 Rain one=2.05 Rain six=2.45
 Rain day=0.0 DT=0.033333

COMPUTED 6-HOUR RAINFALL DISTRIBUTION BASED ON NOAA ATLAS 2 -

PEAK AT 1.40 HR.

DT = .033333 HOURS			END TIME = 5.999940 HOURS		
.0000	.0022	.0044	.0066	.0090	.0113
.0162	.0187	.0213	.0240	.0267	.0295
.0354	.0384	.0416	.0448	.0482	.0516
.0589	.0628	.0668	.0710	.0753	.0799
.0897	.0950	.1006	.1063	.1124	.1188
.1639	.2119	.2807	.3747	.4981	.6555
1.0907	1.3120	1.4046	1.4827	1.5523	1.6156
1.7279	1.7784	1.8258	1.8704	1.9125	1.9522
2.0254	2.0592	2.0913	2.1217	2.1506	2.1576
2.1704	2.1763	2.1820	2.1875	2.1927	2.1978
2.2074	2.2120	2.2164	2.2208	2.2250	2.2290
2.2369	2.2407	2.2444	2.2480	2.2515	2.2550
2.2617	2.2650	2.2682	2.2713	2.2744	2.2775
2.2834	2.2863	2.2891	2.2919	2.2947	2.2974
2.3027	2.3053	2.3078	2.3104	2.3129	2.3153
2.3202	2.3225	2.3249	2.3272	2.3295	2.3317
2.3362	2.3384	2.3405	2.3427	2.3448	2.3469
2.3510	2.3531	2.3551	2.3571	2.3591	2.3610
2.3649	2.3668	2.3687	2.3706	2.3724	2.3743
2.3779	2.3797	2.3815	2.3832	2.3850	2.3867
2.3902	2.3919	2.3936	2.3952	2.3969	2.3985
2.4018	2.4034	2.4050	2.4066	2.4082	2.4098
2.4129	2.4144	2.4160	2.4175	2.4190	2.4205
2.4235	2.4249	2.4264	2.4278	2.4293	2.4307
2.4336	2.4350	2.4364	2.4378	2.4392	2.4406
2.4433	2.4446	2.4460	2.4473	2.4487	2.4500

COMPUTE NM HYD ID=1 Hyd No=101.0 DA=0.0184 sqmi Per A=0 Per B=0
 Per C=10 Per D=90 TP=-0.1333 hr Rain=-1

K = .072649HR TP = .133300HR K/TP RATIO = .545000 SHAPE
 CONSTANT, N = 7.106420
 UNIT PEAK = 65.380 CFS UNIT VOLUME = .9992 B = 526.28
 P60 = 2.0500
 AREA = .016560 SQ MI IA = .10000 INCHES INF = .04000
 INCHES PER HOUR
 RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
 .033333

K = .107897HR TP = .133300HR K/TP RATIO = .809431 SHAPE
 CONSTANT, N = 4.419714
 UNIT PEAK = 5.2763 CFS UNIT VOLUME = .9980 B = 382.25
 P60 = 2.0500

AREA = .001840 SQ MI IA = .35000 INCHES INF = .83000
 INCHES PER HOUR
 RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
 .033333

PRINT HYD ID=1 Code=1

PARTIAL HYDROGRAPH 101.00

RUNOFF VOLUME = 2.10794 INCHES = 2.0686 ACRE-FEET
 PEAK DISCHARGE RATE = 54.73 CFS AT 1.500 HOURS BASIN AREA =
 .0184 SQ. MI.

*
 *Basin 2, Natalie Ave to AP200
 *R-1 Density, 30% C-1
 *
 *Basin 2.....to AP 200
 *

COMPUTE NM HYD ID=2 Hyd No=102.0 DA=0.0175 sqmi Per A=22 Per B=18
 Per C=3 Per D=57 TP=-0.1333 hr Rain=-1

K = .072649HR TP = .133300HR K/TP RATIO = .545000 SHAPE
 CONSTANT, N = 7.106420
 UNIT PEAK = 39.382 CFS UNIT VOLUME = .9991 B = 526.28
 P60 = 2.0500
 AREA = .009975 SQ MI IA = .10000 INCHES INF = .04000
 INCHES PER HOUR
 RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
 .033333

K = .144446HR TP = .133300HR K/TP RATIO = 1.083614 SHAPE
 CONSTANT, N = 3.260430
 UNIT PEAK = 17.071 CFS UNIT VOLUME = .9993 B = 302.41
 P60 = 2.0500
 AREA = .007525 SQ MI IA = .56628 INCHES INF = 1.43558
 INCHES PER HOUR
 RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
 .033333

PRINT HYD ID=2 Code=1

PARTIAL HYDROGRAPH 102.00

RUNOFF VOLUME = 1.56378 INCHES = 1.4595 ACRE-FEET
 PEAK DISCHARGE RATE = 40.50 CFS AT 1.500 HOURS BASIN AREA =
 .0175 SQ. MI.

*
 *Route Basin 1 to AP200
 *Natalie Ave, Residential Street Section
 *

COMPUTE RATING CURVE CID=1 VS No=1 No Seg=3 Emin=0 Emax=1.5 Ch Slp=0.0203
 Fp Slp=0.0203 N=0.05 Dist=34ft N=-0.017 Dist=66ft
 N=0.05 Dist=100ft

Dist	Elev	Dist	Elev	Dist	Elev
0.0	2.0	25.0	1.0	33.5	0.7
34.0	0.0	50.0	0.3	66.0	0.0
66.5	0.7	75.0	1.0	100.0	2.0

RATING CURVE VALLEY SECTION 1.0

WATER	FLOW	FLOW
SURFACE	AREA	RATE
ELEV	SQ FT	CFS
.00	.0	.0

.08	.3	.5
.16	1.3	3.1
.24	3.0	9.0
.32	5.4	20.0
.39	7.9	38.3
.47	10.5	61.0
.55	13.1	87.8
.63	15.7	118.3
.71	18.3	152.3
.79	21.1	189.7
.87	24.3	231.0
.95	27.8	276.0
1.03	31.7	324.8
1.11	35.9	377.6
1.18	40.5	434.3
1.26	45.3	494.9
1.34	50.4	559.6
1.42	55.9	628.2
1.50	61.6	701.0

*
 COMPUTE TRAVEL TIME ID=3 Reach=1 No VS=1
 Length=1380ft Slp=0.0203

TRAVEL TIME TABLE
 REACH= 1.0

WATER DEPTH FEET	AVERAGE AREA SQ.FT.	FLOW RATE CFS	TRAVEL TIME HRS
.079	.337	.48	.2682
.158	1.347	3.06	.1690
.237	3.032	9.01	.1290
.316	5.376	20.00	.1031
.395	7.943	38.27	.0796
.474	10.518	60.99	.0661
.553	13.102	87.76	.0572
.632	15.695	118.29	.0509
.711	18.301	152.30	.0461
.789	21.129	189.74	.0427
.868	24.312	230.98	.0403
.947	27.847	275.98	.0387
1.026	31.733	324.84	.0374
1.105	35.940	377.61	.0365
1.184	40.459	434.28	.0357
1.263	45.289	494.92	.0351
1.342	50.431	559.55	.0345
1.421	55.885	628.24	.0341
1.500	61.650	701.05	.0337

ROUTE ID=3 Hyd No=101.1
 Inflow ID=1 DT=0.0

*
 *Combine Basin 2 with Routed Flow From Basin 1
 *

ADD HYD ID=4 Hyd No=102.1 ID I=2 ID II=3
 PRINT HYD ID=4 Code=1

PARTIAL HYDROGRAPH 102.10

RUNOFF VOLUME = 1.84267 INCHES = 3.5281 ACRE-FEET
 PEAK DISCHARGE RATE = 87.51 CFS AT 1.533 HOURS BASIN AREA =
 .0359 SQ. MI.

*
 *Basin 3, Dodd Place to AP200
 *R-1 Density
 *
 *Basin 3.....to AP 200
 *

COMPUTE NM HYD ID=5 Hyd No=102.2 DA=0.01270sqmi Per A=23 Per B=20

Per C=13 Per D=44 TP=-0.1333hr Rain=-1

K = .072649HR TP = .133300HR K/TP RATIO = .545000 SHAPE
CONSTANT, N = 7.106420
UNIT PEAK = 22.062 CFS UNIT VOLUME = .9989 B = 526.28
P60 = 2.0500
AREA = .005588 SQ MI IA = .10000 INCHES INF = .04000
INCHES PER HOUR
RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
.033333

K = .137758HR TP = .133300HR K/TP RATIO = 1.033440 SHAPE
CONSTANT, N = 3.415798
UNIT PEAK = 16.760 CFS UNIT VOLUME = .9993 B = 314.13
P60 = 2.0500
AREA = .007112 SQ MI IA = .52679 INCHES INF = 1.32500
INCHES PER HOUR
RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
.033333

PRINT HYD

ID=5 Code=1

PARTIAL HYDROGRAPH 102.20

RUNOFF VOLUME = 1.40803 INCHES = .9537 ACRE-FEET
PEAK DISCHARGE RATE = 27.42 CFS AT 1.500 HOURS BASIN AREA =
.0127 SQ. MI.

*

*Combine Basin 3 with Routed Flows from Basins 1-2

*

ADD HYD ID=6 Hyd No=102.3 ID I=4 ID II=5

PRINT HYD

ID=6 Code=1

PARTIAL HYDROGRAPH 102.30

RUNOFF VOLUME = 1.72908 INCHES = 4.4818 ACRE-FEET
PEAK DISCHARGE RATE = 114.06 CFS AT 1.533 HOURS BASIN AREA =
.0486 SQ. MI.

*

*Basin 4, Natalie Ave to San Pedro to AP300

*R-1 Density

*

*Basin 4.....to AP300

*

COMPUTE NM HYD ID=7 Hyd No=103.0 DA=0.0088 sqmi Per A=23 Per B=20
Per C=13 Per D=44 TP=-0.1333hr Rain=-1

K = .072649HR TP = .133300HR K/TP RATIO = .545000 SHAPE
CONSTANT, N = 7.106420
UNIT PEAK = 15.287 CFS UNIT VOLUME = .9987 B = 526.28
P60 = 2.0500
AREA = .003872 SQ MI IA = .10000 INCHES INF = .04000
INCHES PER HOUR
RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
.033333

K = .137758HR TP = .133300HR K/TP RATIO = 1.033440 SHAPE
CONSTANT, N = 3.415798
UNIT PEAK = 11.613 CFS UNIT VOLUME = .9990 B = 314.13
P60 = 2.0500
AREA = .004928 SQ MI IA = .52679 INCHES INF = 1.32500
INCHES PER HOUR

RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
.033333

PRINT HYD

ID=7 Code=1

PARTIAL HYDROGRAPH 103.00

RUNOFF VOLUME = 1.40803 INCHES = .6608 ACRE-FEET
PEAK DISCHARGE RATE = 19.00 CFS AT 1.500 HOURS BASIN AREA =
.0088 SQ. MI.

*

*Route Basins 1-3 thru Basin 4 to AP300

*Natalie Ave, Residential Street Section

*

COMPUTE RATING CURVE CID=1 VS No=1 No Seg=3 Emin=0 Emax=1.5 Ch Slp=0.0171
Fp Slp=0.0174 N=0.05 Dist=34ft N=-0.017 Dist=66ft
N=0.05 Dist=100ft

Dist	Elev	Dist	Elev	Dist	Elev
0.0	2.0	25.0	1.0	33.5	0.7
34.0	0.0	50.0	0.3	66.0	0.0
66.5	0.7	75.0	1.0	100.0	2.0

RATING CURVE VALLEY SECTION 1.0

WATER SURFACE ELEV	FLOW AREA SQ FT	FLOW RATE CFS
.00	.0	.0
.08	.3	.4
.16	1.3	2.8
.24	3.0	8.3
.32	5.4	18.4
.39	7.9	35.1
.47	10.5	56.0
.55	13.1	80.5
.63	15.7	108.6
.71	18.3	139.8
.79	21.1	174.2
.87	24.3	212.0
.95	27.8	253.3
1.03	31.7	298.2
1.11	35.9	346.6
1.18	40.5	398.7
1.26	45.3	454.4
1.34	50.4	513.8
1.42	55.9	576.9
1.50	61.6	643.8

*

COMPUTE TRAVEL TIME ID=8 Reach=1 No VS=1
Length=700ft Slp=0.0171

TRAVEL TIME TABLE

REACH= 1.0

WATER DEPTH FEET	AVERAGE AREA SQ.FT.	FLOW RATE CFS	TRAVEL TIME HRS
.079	.337	.44	.1482
.158	1.347	2.81	.0934
.237	3.032	8.27	.0713
.316	5.376	18.36	.0570
.395	7.943	35.12	.0440
.474	10.518	55.98	.0365
.553	13.102	80.55	.0316
.632	15.695	108.57	.0281
.711	18.301	139.78	.0255
.789	21.129	174.15	.0236
.868	24.312	212.01	.0223
.947	27.847	253.32	.0214

1.026	31.733	298.18	.0207
1.105	35.940	346.64	.0202
1.184	40.459	398.69	.0197
1.263	45.289	454.38	.0194
1.342	50.431	513.76	.0191
1.421	55.885	576.88	.0188
1.500	61.650	643.78	.0186

ROUTE ID=8 Hyd No=102.4
Inflow ID=6 DT=0.0

*

*Combine Basin 4 with Routed Flow From Bains 1-3 @ AP300

*

ADD HYD ID=9 Hyd No=103.1 ID I=7 ID II=8
PRINT HYD ID=9 Code=1

PARTIAL HYDROGRAPH 103.10

RUNOFF VOLUME = 1.67986 INCHES = 5.1426 ACRE-FEET
PEAK DISCHARGE RATE = 131.81 CFS AT 1.533 HOURS BASIN AREA =
.0574 SQ. MI.

*

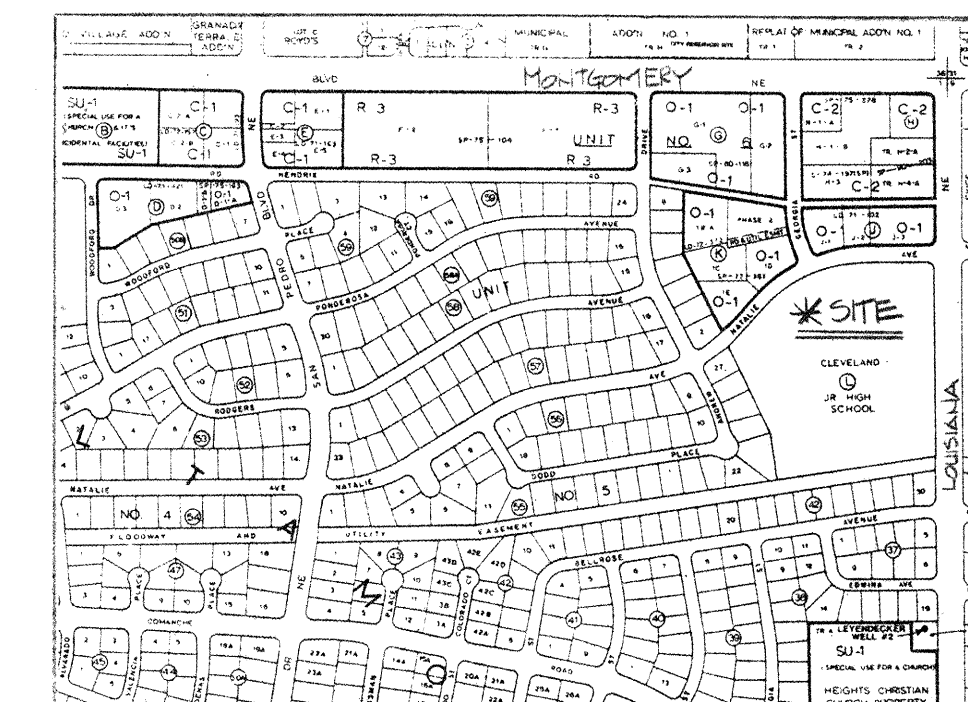
FINISH

NORMAL PROGRAM FINISH

END TIME (HR:MIN:SEC) = 07:20:08

KEYNOTES:

1. CONSTRUCT 52'-WIDE (B/D) COMBINATION BUS/CAR DRIVEWAY ENTRANCE. PROVIDE SIDEWALK RAMP AS SHOWN. REFER TO C.O.A. STD.DWG. 2426, 2440 AND 2441.
2. CONSTRUCT 53.7'-WIDE (B/D) COMBINATION BUS/CAR DRIVEWAY ENTRANCE. PROVIDE SIDEWALK RAMP AS SHOWN. REFER TO C.O.A. STD.DWG. 2426, 2440 AND 2441.
3. MATCH ELEVATIONS OF EXISTING CURB/GUTTER AND WALK, THIS AREA.
4. GRADE THIS AREA TO DRAIN AS SHOWN.
5. 6" MEDIAN CURB (TYP) SEE C.O.A. DWG. 2415.
6. INSTALL SINGLE 'D' DROP INLET WITH 24" STORM DRAIN PIPES AS SHOWN. REFER TO STORM DRAIN PLANS FOR MORE INFORMATION.
7. SEE STORM DRAIN PLANS FOR MORE INFORMATION.
8. REGRADE INTERIOR OF TRACK TO DRAIN TO NEW DROP INLET.



LEGEND

APPLIES TO ALL SHEETS

- SIDEWALK, CURB AND GUTTER (EXISTING, PROPOSED)
- PROPOSED ASPHALT
- BUILDING (EXISTING, PROPOSED)
- PROPERTY LINE
- EXISTING SPOT ELEVATION
- EXISTING CONTOUR
- PROPOSED SPOT ELEVATION
- PROPOSED CONTOUR
- SURFACE FLOW DIRECTION (EXISTING, PROPOSED)
- LANDSCAPED AREA
- LA
- TC
- FL
- TA
- TG
- TW
- FF
- R/W



C.L. WEISS ENGINEERING, INC.
ALBUQUERQUE, NEW MEXICO

Revisions



Weller Architects P.C.

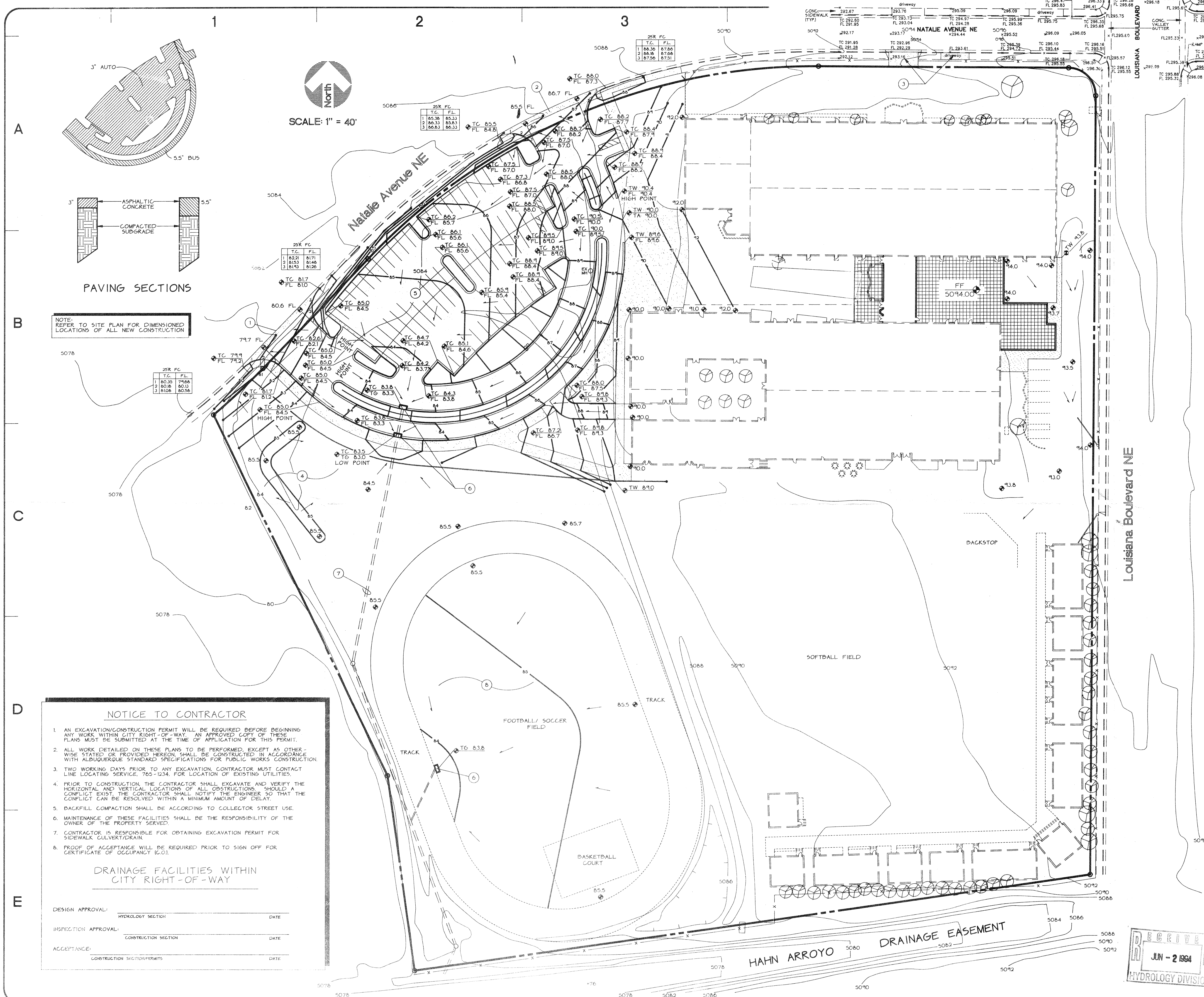
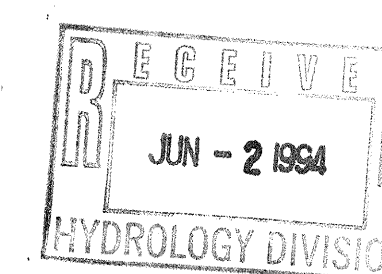
401 Alvarado Dr. S.E. Suite D
Albuquerque, New Mexico 87108
Tel (505)255-8270 Fax (505)255-8830

ADDITIONS AND REMODEL TO
CLEVELAND MIDDLE SCHOOL
ALBUQUERQUE PUBLIC SCHOOLS
Albuquerque, New Mexico

Scale: 1" = 40' Drawn By: BJB Checked By: CLW Job Number: 9105.01 Date: / /

DRAINAGE AND
GRADING PLAN

C11
SH. OF



PAVING SECTIONS

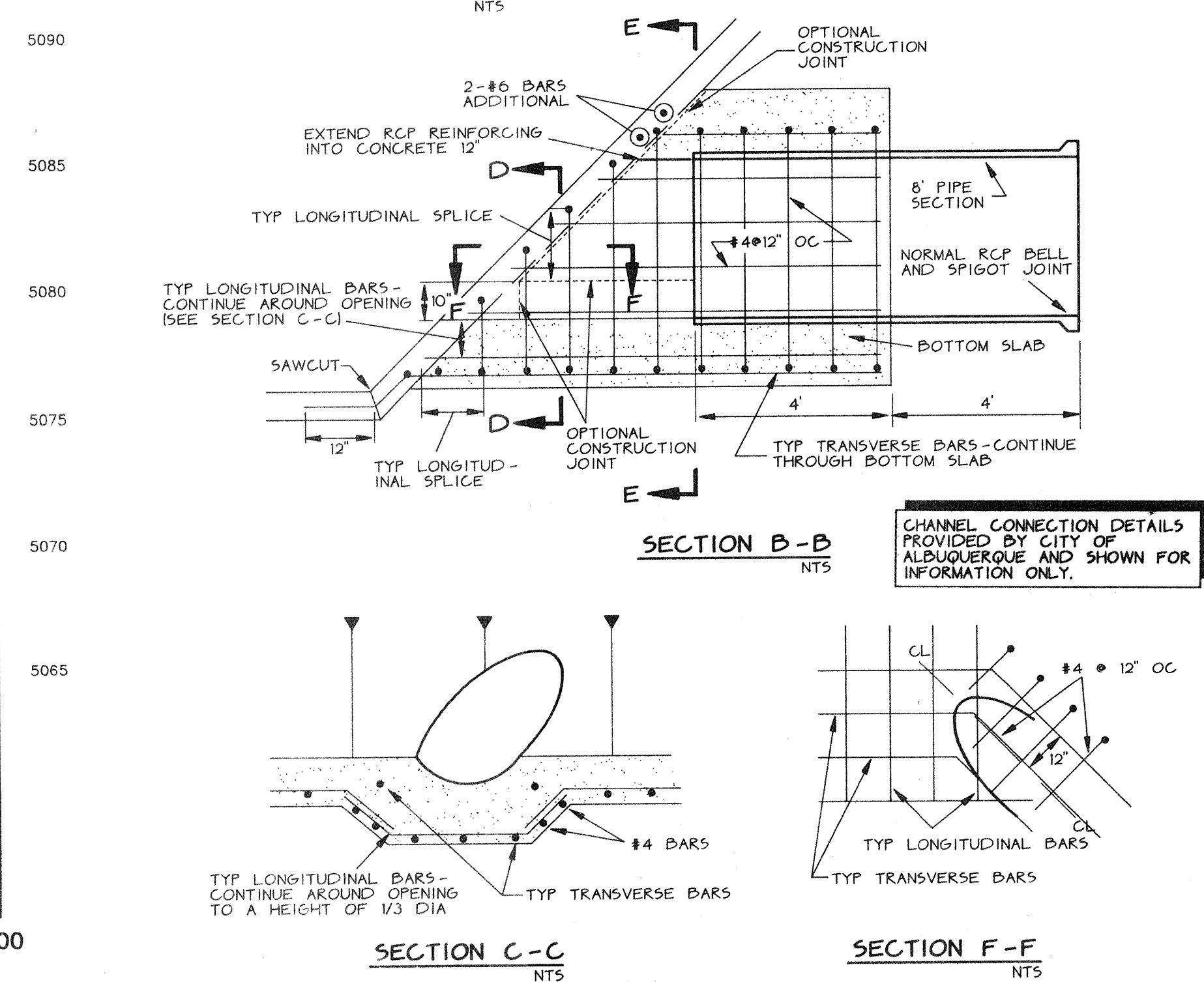
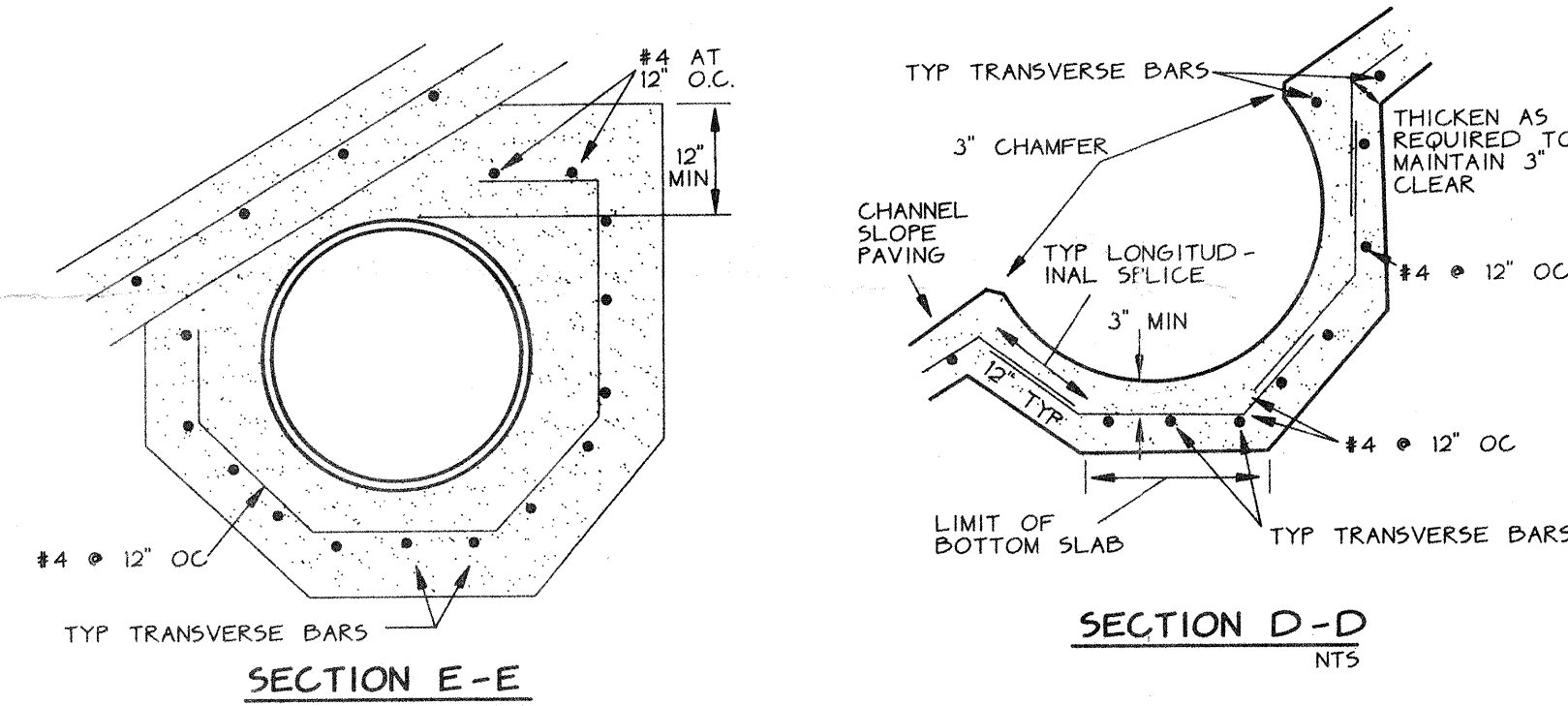
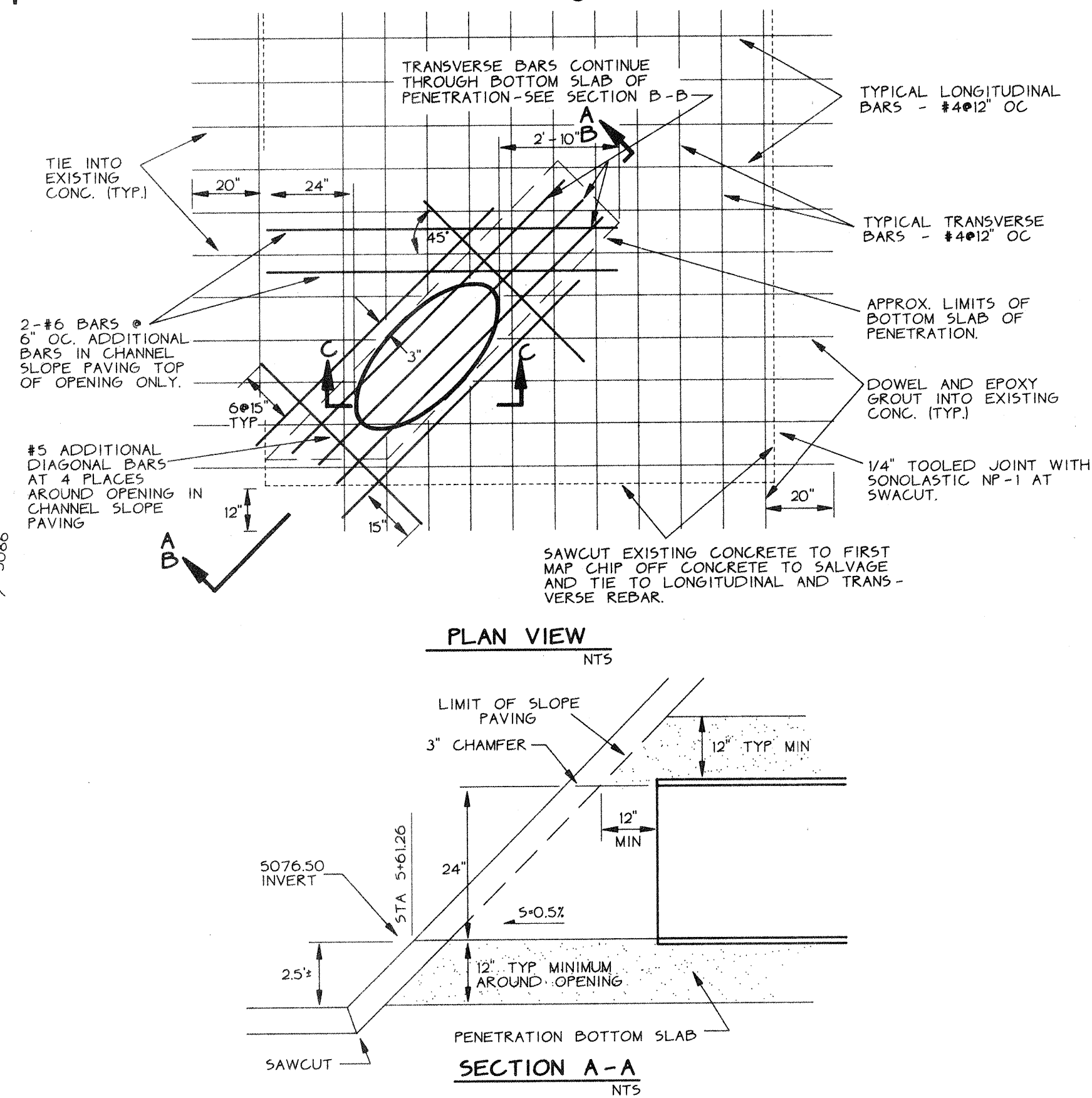
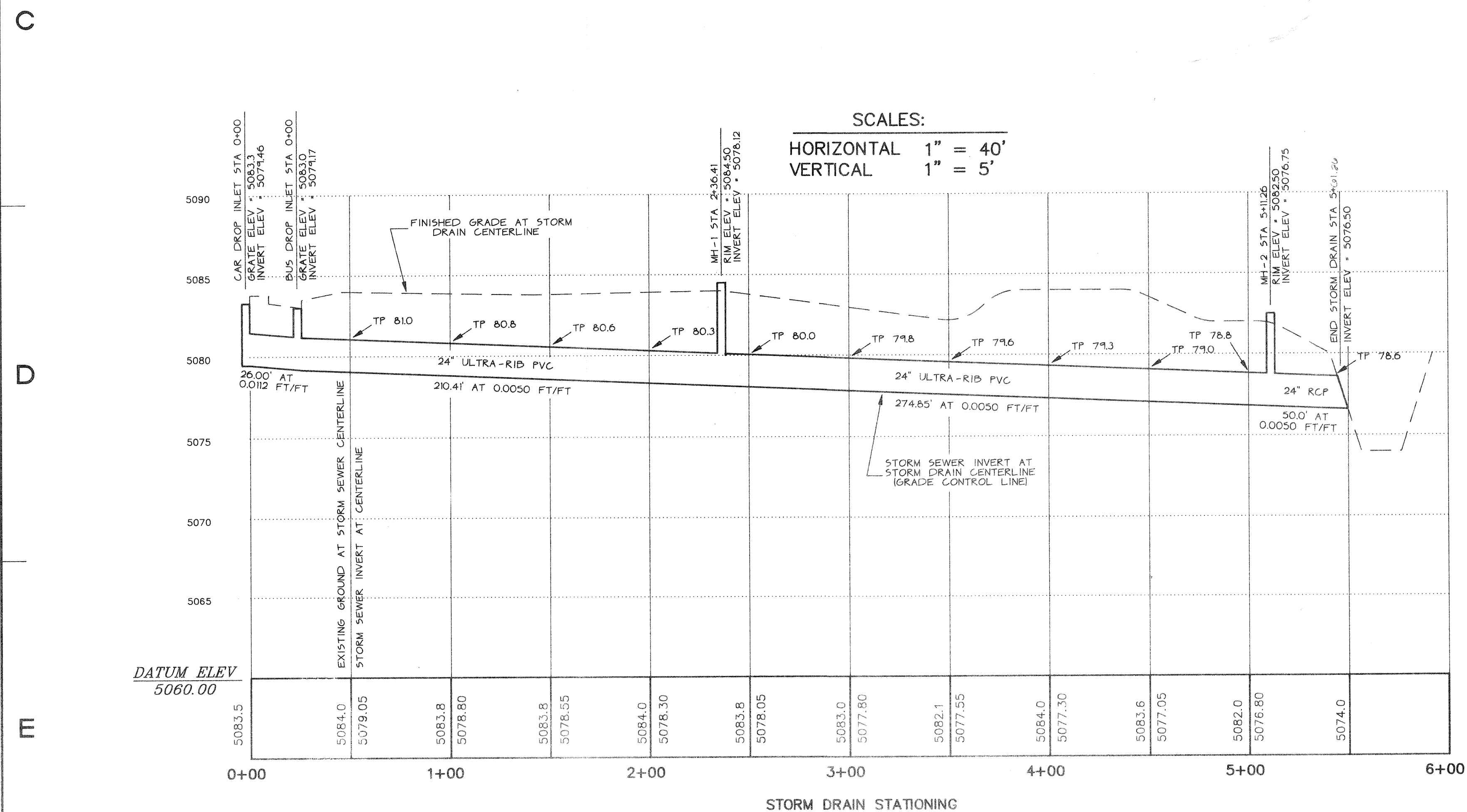
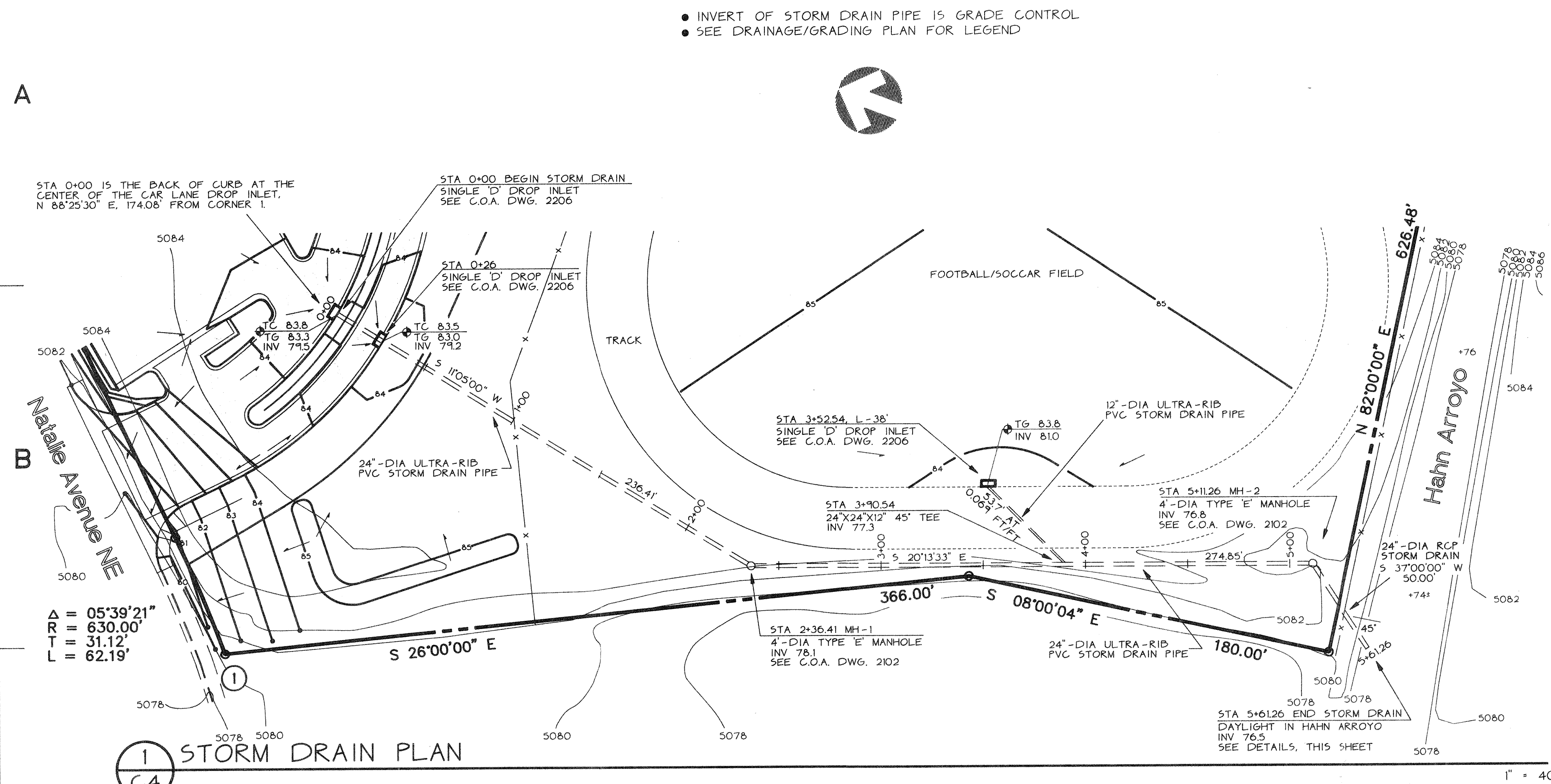
NOTE: REFER TO SITE PLAN FOR DIMENSIONED LOCATIONS OF ALL NEW CONSTRUCTION

NOTICE TO CONTRACTOR

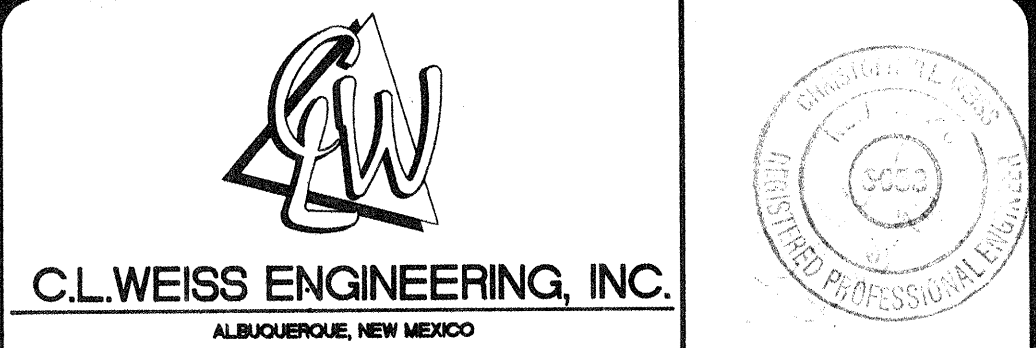
1. AN EXCAVATION/CONSTRUCTION PERMIT WILL BE REQUIRED BEFORE BEGINNING ANY WORK WITHIN CITY RIGHT-OF-WAY. AN APPROVED COPY OF THESE PLANS MUST BE SUBMITTED AT THE TIME OF APPLICATION FOR THIS PERMIT.
2. ALL WORK DETAILED ON THESE PLANS TO BE PERFORMED, EXCEPT AS OTHERWISE STATED OR PROVIDED HEREON, SHALL BE CONSTRUCTED IN ACCORDANCE WITH ALBUQUERQUE STANDARD SPECIFICATIONS FOR PUBLIC WORKS CONSTRUCTION.
3. TWO WORKING DAYS PRIOR TO ANY EXCAVATION, CONTRACTOR MUST CONTACT LINE LOCATING SERVICE, 765-1234, FOR LOCATION OF EXISTING UTILITIES.
4. PRIOR TO CONSTRUCTION, THE CONTRACTOR SHALL EXCAVATE AND VERIFY THE HORIZONTAL AND VERTICAL LOCATIONS OF ALL OBSTRUCTIONS. SHOULD A CONFLICT EXIST, THE CONTRACTOR SHALL NOTIFY THE ENGINEER SO THAT THE CONFLICT CAN BE RESOLVED WITHIN A MINIMUM AMOUNT OF DELAY.
5. BACKFILL COMPACTION SHALL BE ACCORDING TO COLLECTOR STREET USE.
6. MAINTENANCE OF THESE FACILITIES SHALL BE THE RESPONSIBILITY OF THE OWNER OF THE PROPERTY SERVED.
7. CONTRACTOR IS RESPONSIBLE FOR OBTAINING EXCAVATION PERMIT FOR SIDEWALK CULVERT/DRAIN.
8. PROOF OF ACCEPTANCE WILL BE REQUIRED PRIOR TO SIGN OFF FOR CERTIFICATE OF OCCUPANCY (C.O.).

DRAINAGE FACILITIES WITHIN CITY RIGHT-OF-WAY

DESIGN APPROVAL: _____ DATE _____
HYDROLOGY SECTION
INSPECTION APPROVAL: _____ DATE _____
CONSTRUCTION SECTION
ACCEPTANCE: _____ DATE _____
CONSTRUCTION SECTION/PERMITS



3 CHANNEL CONNECTION DETAILS
C4 45° PENETRATION



Revisions



Weller Architects P.C.

401 Alvarado Dr. S.E. Suite D
Albuquerque, New Mexico 87108
Tel (505)255-8270 Fax (505)255-8830

ADDITIONS AND REMODEL TO
CLEVELAND MIDDLE SCHOOL
ALBUQUERQUE PUBLIC SCHOOLS
Albuquerque, New Mexico

Scale: N.T.S. Drawn By: BJB Checked By: CLW Job Number: 9105.01 Date: / /

STORM DRAIN
DETAILS

C4.1
SH. OF

KEYNOTES:

1. CONSTRUCT 52'-WIDE (B/B) COMBINATION BUS/CAR DRIVEWAY ENTRANCE. PROVIDE SIDEWALK RAMP AS SHOWN. REFER TO C.O.A. STD. DUGS. 2426, 2440 AND 2441.
2. CONSTRUCT 53'-WIDE (B/B) COMBINATION BUS/CAR DRIVEWAY ENTRANCE. PROVIDE SIDEWALK RAMP AS SHOWN. REFER TO C.O.A. STD. DUGS. 2426, 2440 AND 2441.
3. MATCH ELEVATIONS OF EXISTING CURB/GUTTER AND WALK, THIS AREA.
4. GRADE THIS AREA TO DRAIN AS SHOWN.
5. 6" MEDIAN CURB (TYP) SEE C.O.A. DUG 2415.
6. INSTALL SINGLE 'D' DROP INLET WITH 24" STORM DRAIN PIPES AS SHOWN. REFER TO STORM DRAIN PLANS FOR MORE INFORMATION.
7. SEE STORM DRAIN PLANS FOR MORE INFORMATION.
8. REGRADE INTERIOR OF TRACK TO DRAIN TO NEW DROP INLET.

REVISED NOV. 15, 1994

SHOWING PROPOSED CHAIN LINK FENCE LOCATIONS.

REVISED 6-9-95

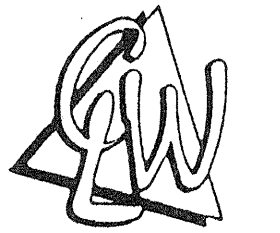
Δ CATCH FIELD INLET / STORM DRAIN DELETED

Δ CHANGES TO PARKING CONFIGURATION / GRACES N. NOTED.

LEGEND

APPLIES TO ALL SHEETS

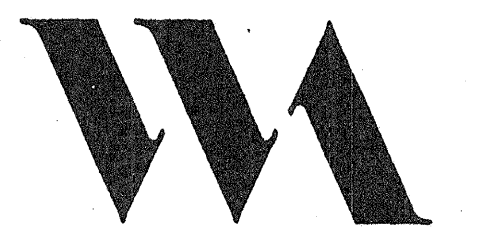
- SIDEWALK, CURB and GUTTER (EXISTING, PROPOSED)
- PROPOSED ASPHALT
- BUILDING (EXISTING, PROPOSED)
- PROPERTY LINE
- EXISTING SPOT ELEVATION
- EXISTING CONTOUR
- PROPOSED SPOT ELEVATION
- PROPOSED CONTOUR
- SURFACE FLOW DIRECTION (EXISTING, PROPOSED)
- LANDSCAPED AREA
- TOP OF CURB
- FLOW LINE
- TOP OF ASPHALT
- TOP OF GRATE
- TOP OF WALK
- FINISHED FLOOR
- RIGHT OF WAY



C.L. WEISS ENGINEERING, INC.

ALBUQUERQUE, NEW MEXICO

Revisions SEE ABOVE



Weller Architects P.C.

401 Alvarado Dr. S.E. Suite D
Albuquerque, New Mexico 87108
Tel (505)255-8270 Fax (505)255-8830

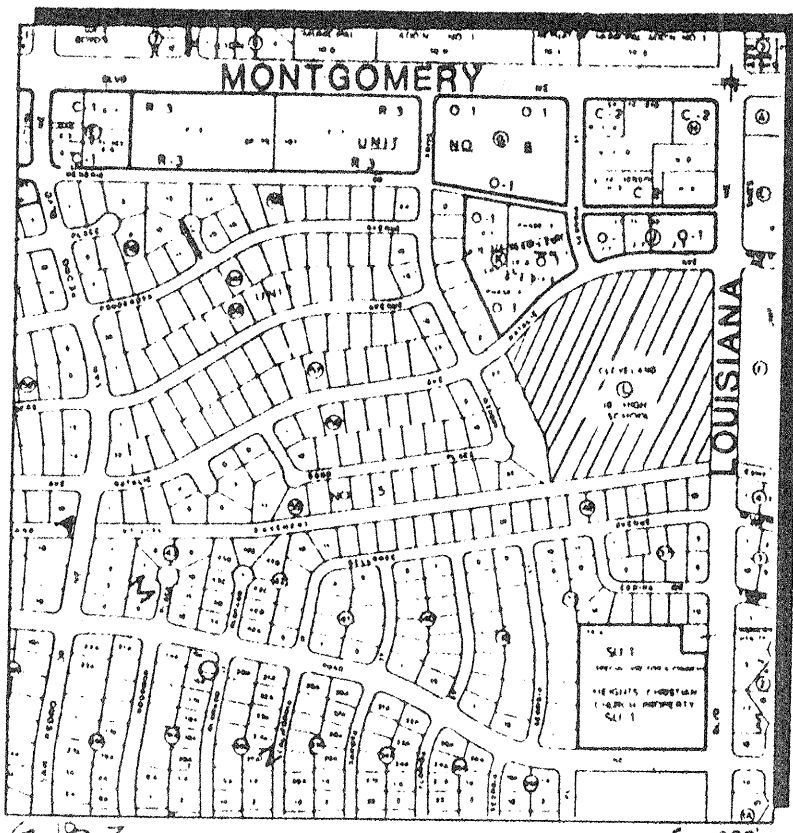
ADDITIONS AND REMODEL TO
CLEVELAND MIDDLE SCHOOL
ALBUQUERQUE PUBLIC SCHOOLS
Albuquerque, New Mexico

Scale: 1" = 40' Drawn By: BJB Checked By: CLW Job Number: 9105.01 Date: July 1994

DRAINAGE AND
GRADING PLAN

C11

SHEET 3 OF 17

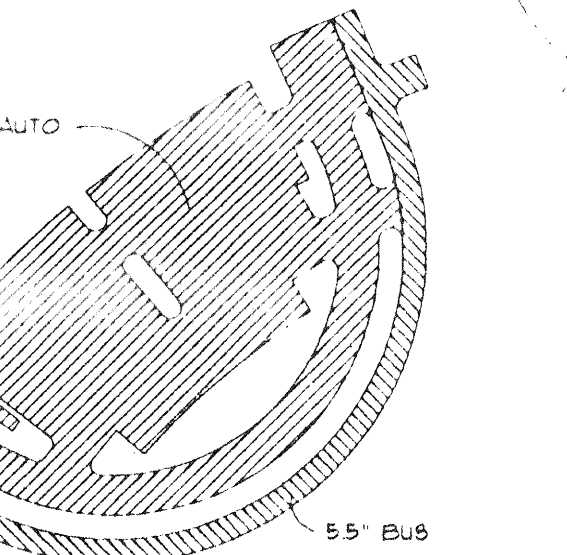


VICINITY MAP

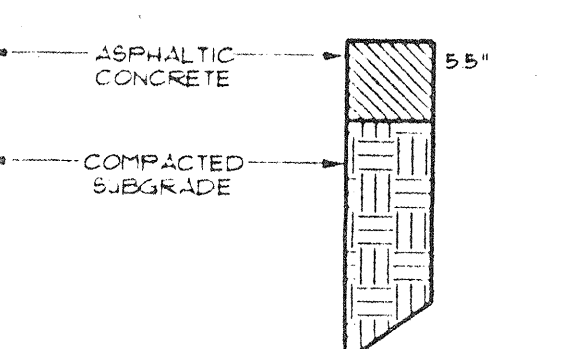
LEGAL TRACT L-UNIT 6 OF ALBUQUERQUE, NEW MEXICO

NEW TERMINATION POST PATCH FABRIC

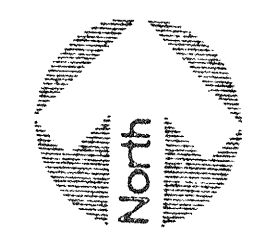
REPAIR EXISTING C.L. FENCE MATCH TO NEW FENCE



PAVING KEY



PAVING SECTIONS



SCALE: 1" = 40'

NOTE: REFER TO SITE PLAN FOR DIMENSIONED LOCATIONS OF ALL NEW CONSTRUCTION

PROVIDE NEW TERMINATION POST PATCH FABRIC TO MATCH

EXISTING FENCE TO REMAIN

Natalie Avenue NE

LC	FL
1 80.30 81.60	
2 80.30 81.60	
3 81.30 81.60	

NEW CHAIN LINK FENCE MATCH EXIST. HT. 4'-0" WIDE GATE

NEW 4'-0" CHAIN LINK FENCE RE USE SALVAGED FABRIC

PAIR OF 4'-0" C.L. GATES

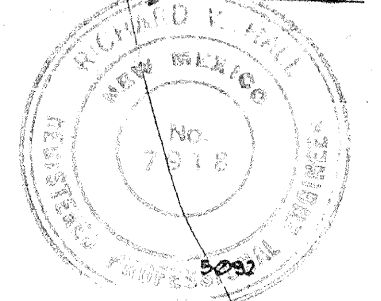
NOTE: ELEVATIONS IN PARENTHESIS () ARE AS-BUILT

AS-BUILT CERTIFICATION

I hereby certify that I have checked the elevations shown on this Grading Plan in the field and that the as-built elevations are in substantial compliance with the design elevations as shown hereon.

Richard V. Hall
Richard V. Hall
N.M.P.E. No. 7918

Date: Oct 25, 1995



SOFTBALL FIELD

TRACK

FOOTBALL / SOCCER FIELD

BASKETBALL COURT

HAHN ARROYO

DRAINAGE EASEMENT

Louisiana Boulevard NE

94-1230