City of Albuquerque
P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

February 7, 1992

Steve Crawford, P.E.
BPLW Architects & Engineers
2400 Louisiana Boulevard, NE Suite 400
Albuquerque, New Mexico 87110

RE: DRAINAGE PLAN FOR FHP MEDICAL CLINIC
(G-20/D4A) ENGINEER'S STAMP DATED JANUARY 9, 1992

Dear Mr. Crawford:

Based on the information provided on the referenced submittal received January 10, 1991, the plan is approved for Certificate of Occupancy release.

If you should have any questions, please do not hesitate to call me at 768-2650.

Cordially,

Gilbert Aldaz, P.E. & P.S.
Civil Engineer/Hydrology

GA
(WP+84)

for

FHP HEALTH CARE

MARCH, 1991

Prepared by: BPLW Architects & Engineers, Inc.
2400 Louisiana Blvd. NE
AFC #5, Suite 400
Albuquerque, NM 87110

Author: Stephen L. Crawford, PE #10274

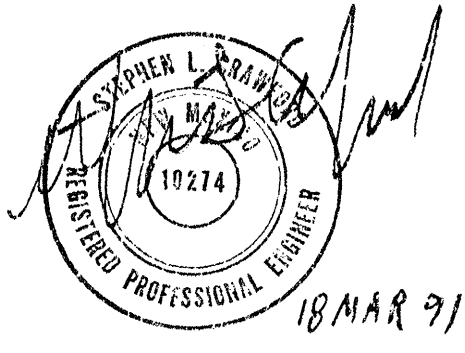


TABLE OF CONTENTS

1. GENERAL INFORMATION:

The project site is located at the southeast corner of Wyoming and General Chenault, NE. The project is a medical office complex situated on Tract A-1 (formerly the westerly 2.67 acres of Tract A, Montgomery Crossing Phase III). Montgomery Blvd. to the north of this site is designated as a 100 year floodway by the FEMA floodway maps. The site's soils consist of Embudo-Tijeras Complex (EtC) and Tijeras gravelly loam (TgB) soil associations, both of which are designated as hydrologic soil group 'B' by the SCS. Because Montgomery Blvd. currently carries significant surface flows in the 100 year storm, previous drainage studies have limited the peak developed discharge from the entire 6.52 acres of former Tract A (now Tracts A-1 & A-2).

2. PROJECT DEVELOPMENT BACKGROUND:

A Conceptual Grading and Drainage Plan was prepared for Tract A (before it was subdivided) in December of 1990. During this time period, the EPC site plan for Tract A was administratively amended to accomodate the proposed medical office uses on the west half of the site (now Tract A-1). The Conceptual Grading & Drainage Plan (Engineer's stamp dated 16 Dec, 1990) was based directly on the original site plan drainage scheme as prepared by Bohannan/Huston in April of 1987. This drainage scheme combined the public offsite storm waters from Hendrix Road to the east with the onsite developed flows and routed them through a series of detention ponds in order to mitigate the sites runoff to an average of 0.5 cfs/acre. These ponds were located in the center of former Tract A. Tract A was subdivided just west of the ponds and the westerly portion was sold to FHP, Inc. based on the site plan as modified. This effectively gave Tract A-1 free discharge at the expense of Tract A-2 and the offsite flows to the east being greatly restricted by the detention ponds. The December Conceptual Grading & Drainage Plan was approved in January of 1991 after Tract A-1 was sold to FHP, Inc.

In February, 1991 the design Grading & Drainage Plan was prepared along with Public Infrastructure Design Plans for the ponds. By the design of the approved Conceptual Grading and Drainage Plan, the ponds were proposed to be public since they carried street runoff from Hendrix Road to the east.

During the DRC review of the Public Infrastructure Design for the ponds, numerous issues pertaining to the City's liabilty concerning the public nature of the detention ponds were raised. Discussions were held with the City Engineer and his staff at which time it was agreed that the public interest would be best served if the public nature of the detention ponds could be eliminated. The developers of Tracts A-1 and A-2 were both agreeable to diverting the Hendrix Road storm waters directly north in a public surface conveyance channel and converting the detention pond to exclusively non-public storm flows. For practical reasons (the sale of Tract A-1 to FHP, Inc. with a free discharge understanding), the discharge rate for Tract A-2 only will be limited to the 0.5 cfs/acre discharge rate commonly applied along this portion of Montgomery Blvd. This modification of the average discharge rate for former Tract A is allowable in lieu of the reduction in liability to the City which is affected by changing the detention ponding area from a public to a private concern. This design change will also improve the visual appearance of the site from Montgomery Blvd. by reducing the area devoted to detention ponding by about 40%.

3. EXISTING CONDITIONS:

The site currently receives offsite flows from the east from two offsite flow basins.

A) OS-1 is a City Well site and has the following hydrologic properties:

A = 0.34 acres
P100 = 2.4 inches

Tc < 10 minutes
I100 = 5.07 in/hr
CC = 0.53
12%(0.90)+13%(0.95)+75%(0.40)
Q100 = 0.91 cfs

%Imp = 25
Perv CN = 79 (poor range)
CCN = 83
RO = 1.0 inches
V100 = 1234 cf

B) OS-2 is a developed (medium density residential) site and has the following hydrologic properties:

A = 2.65 acres

Tc < 10 minutes
I100 = 5.07 in/hr
C = 0.79
Q100 = 10.61 cfs

%Imp = 65
CN = 85
RO = 1.2 inches
V100 = 11,540 cf

C) The onsite existing hydrologic conditions for basin B-1 (the project site) are as follows:

A = 2.67 acres

Tc < 10 minutes
I100 = 5.07 in/hr
C = 0.40
Q100 = 5.41 cfs

CN = 70 (fair range)
RO = 0.5 inches
V100 = 4850 cf

D) Basin B-2 (east portion Tract A) has the following existing hydrologic properties:

A = 3.85 acres

Tc < 10 minutes
I100 = 5.07 in/hr
C = 0.40
Q100 = 7.81 cfs

CN = 70 (fair range)
RO = 0.5 inches
V100 = 6990 cf

E) The total existing 100 year flow rate (offsite and onsite) from Tract A into Montgomery Blvd. is 24.74 cfs.

4. PROPOSED DRAINAGE SOLUTION:

To mitigate the effects of developing Tracts A-1 & A-2, onsite detention ponding will be constructed in basin B-2 near the center of former Tract A. Based on the Project Development history recited in section 2 of this plan: the flows from basin B-1 will be allowed to discharge freely; the flows from basin OS-2 will be diverted north to Montgomery via a surface conveyance channel; and the flows from basins OS-1 & B-2 will be routed through an on-site detention pond. This will reduce the cumulative developed peak flows from Tract A to an acceptable level and will allow Tract A-1 to be developed as indicated on the site plan without adversely impacting the drainage conditions in Montgomery Blvd. Basin B-1 can be developed with a free discharge condition provided that the construction of the detention ponding and offsite water block (at NE corner of OS-1) are done concurrently. See sheet C2.3 in the map pocket and the calculations in Appendix 'A'.

5. DEVELOPED CONDITIONS:

A) The site/grading plan (see sheet C2.1 in map pocket) for basin B-1 indicates that it will have the following hydrologic properties after development:

A = 2.67 acres

Tc < 10 minutes	%Imp = 78
I100 = 5.07 in/hr	Perv CN = 61 (good lawn)
CC = 0.79	CCN = 88
21%(0.90)+22%(0.25)+57%(0.95)	RO = 1.4 inches
Q100 = 10.69 cfs	V100 = 13,570 cf

B) Tract A-2 (basin B-2) is assumed to be developed with commercial uses and will have the following developed hydrologic properties:

A = 3.85 acres

Tc < 10 minutes	%Imp = 85
I100 = 5.07 in/hr	Perv CN = 61 (good lawn)
C = 0.85	CCN = 90
Q100 = 16.59 cfs	RO = 1.5 inches
	V100 = 20,960 cf

6. DETENTION POND DESIGN:

The total peak discharge from Tract A-2 after development must be limited to:

0.91	+	1.93	=	2.84 cfs
(OS-1)		(6.52 x 0.5cfs/acre)		

The cummulative hydrologic properties of the basins tributary to the ponding area are as follows:

A = 4.19 acres
P100 = 2.4 inches

Tc < 10 minutes
I100 = 5.07 in/hr
CC = 0.82
Q100 = 17.41 cfs

CCN = 89
RO = 1.4 inches
V100 = 21,290 cf

Assuming a pond depth of about 5 feet and given the allowable discharge of 2.84 cfs, a single 7 inch outlet orifice will control the discharge adequately (inlet control). The pond hydrograph (per DPM chapter 22.2, part F) shows that a total of 17,610 cf of detention volume is required to satisfy the discharge requirement and that the pond will be drained in about 4 hours after the 100-year event. A pond with vertical sides, a commercial area guardrail, 3900 sq ft of bottom area, and 4.52 feet deep will be needed. The hydrograph is located in Appendix 'B'.

The 10 inch PVC outlet pipe (n=0.010) at a slope of 1.5% has a capacity of 3.49 cfs (Mannings). OK

Check 12' wide inlet weir, $17.41=2.65(12)H^{1.5}$; H = 0.67'.
The opening sides will be one CMU block high (0.67'). OK

7. SUMMARY:

The development of Tracts A-1 & A-2 will result in a slight reduction of peak flows into Montgomery Blvd due to the construction of the detention ponding. The peak flows will be changed from 24.74 cfs (before) to 23.95 cfs (after) which represents a 3% reduction. The project will also redirect Hendrix Road public storm flows, which currently sheet flow across private property, to a new public surface channel north to Montgomery Blvd.

The on-site pond will have a maximum water surface elevation of 78.15 during the 100-year storm. A twenty foot section of the pond wall with top elevation of 78.33 (at the northwest corner of the pond) will serve as the pond's overflow spillway.

17.41 cfs = 2.65(20 lf)H^1.5 ; H = 0.67'.

The opening sides will be one CMU block high (0.67'). OK

APPENDIX A

BPLW

Architects & Engineers, Inc.

2400 Louisiana Blvd. NE
AFC #5 Suite 400
Albuquerque, NM 87110
(505) 881-2759

63 East Main Street
Suite 602
Mesa, AZ 85201
(602) 827-2759

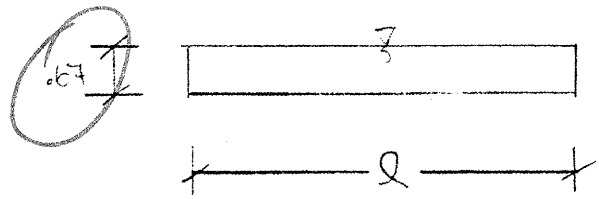
Project FHP

Subject [illegible]

Project No. 9101 Date 2-6-91 By [illegible]

- ☐ Memorandum
- ☐ Telephone record
- ☐ Note to the file
- ☐ Minutes of meeting
- ☐ To be typed
- ☐ _____

CHANNEL INLET AT HENDRIX CUL-DE-SAC



SINCE CHANNEL @ ADVERSE DIRECTION TO FLOW, RECOMMEND THROAT TO BE EAST R. RESULTING IN L = 25', NEED GATES @ BOTH ENDS

$Q_{ACT} = 11 CFS$

$Q = 2.65 Q L h^{1.5}$

$Q = \frac{11}{2.65 \cdot 6.67^{1.5}} = 7.63 FT$

* JUST MAKE OPENING 10 FT

$2.65(10)(.67)^{1.5} = 14.42 CFS > 11 CFS$ OK

$Q = C L H^{3/2}$

$H = \left(\frac{Q}{CL}\right)^{2/3} = \left(\frac{11 CFS}{2.65 \times 25}\right)^{2/3} = 0.30'$

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(602) 827-2759

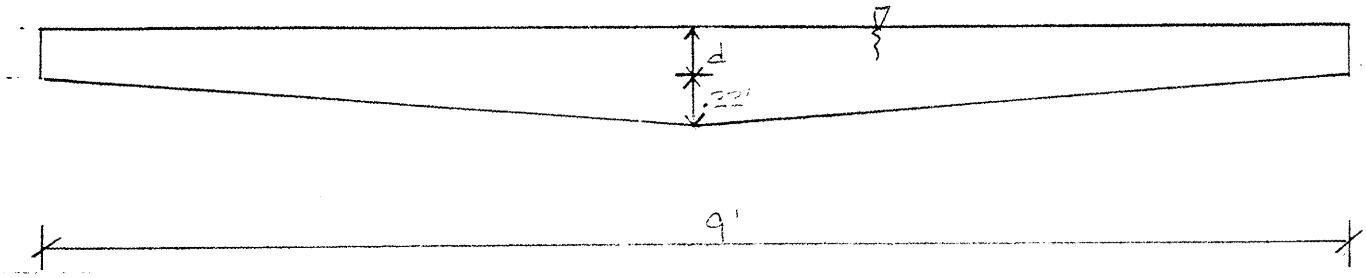
Project FHP

Subject FLOOD ROUTING THRU CHANNEL

Project No. 91021 Date 3-91 By LGH

- ☐ Memorandum
- ☐ Telephone record
- ☐ Note to the file
- ☐ Minutes of meeting
- ☐ To be typed
- ☐ _____

STANDARD COA DRAINAGE CHANNEL



$Q_{ACT} = 11 CFS$ 11cfs

$Q = Q_{ACT}$

$n = .014$ .014

$S_o = .005$ .005

$$Q = \frac{1.49 A R^{2/3} S^{1/2}}{n}$$

$A = 9d + 1.5$

$P = 2d + 9.02$

$R = A/P$

$$Q = \frac{1.49 [9d + 1.5]}{.014} \left[\frac{9d+1.5}{2d+9.02}\right]^{2/3} [.005]^{1/2}$$

$$Q = 7.52 (9d+1.5) \left(\frac{9d+1.5}{2d+9.02}\right)^{2/3}$$

LET $d = 4'' = .33'$ (HEIGHT OF SIDEWALLS)

$Q = 20.31 CFS > 11 CFS$ OK

* FOR CONSTRUCTION EASE, USE 8" SIDEWALLS

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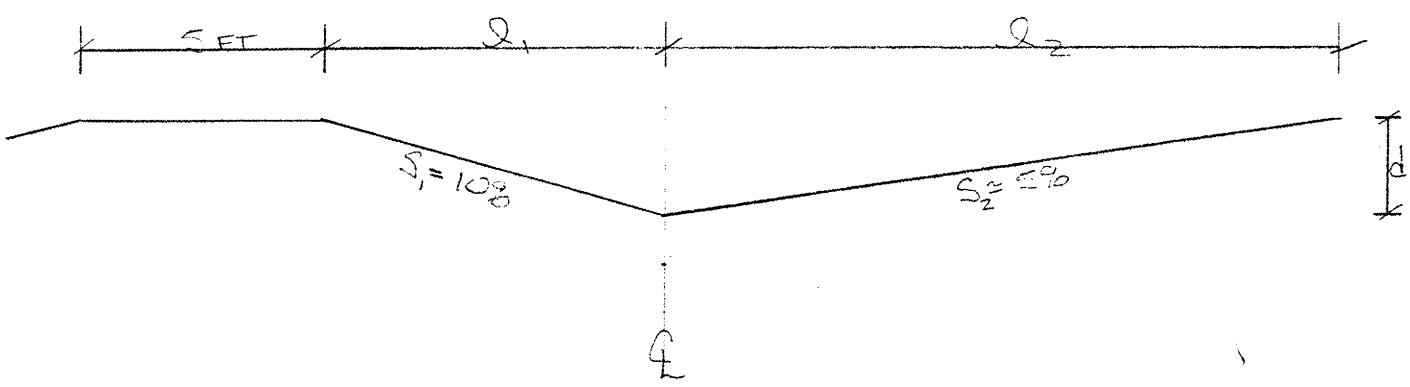
Project EHP

Subject FLOOD CALCS

Project No. 91001 Date 3-91 By LAH

- ☐ Memorandum
- ☐ Telephone record
- ☐ Note to the file
- ☐ Minutes of meeting
- ☐ To be typed
- ☐ _____

[illegible] FLOOD SPLIT - ADJACENT TO WELL SITE DRIVEWAY



$n = .017$

$S_o = .5\% = .005$

$$Q = \frac{1.49 A R^{2/3} S^{1/2}}{n} \geq 11 \text{ CFS}$$

LET $d = .45'$

$Q_1 = 4.5$ FT

$Q_2 = 9$ FT

$A = \frac{1}{2}(.45)(4.5 + 9) = 3.04 \text{ FT}^2$

$$R = \frac{A}{P} = \frac{3.04}{\sqrt{.45^2 + 4.5^2} + \sqrt{.45^2 + 9^2}} = \frac{3.04}{13.53} = .024 \text{ FT}$$

$Q = 1.49(3.04)(.024)^{2/3}(.005)^{1/2}(1/.017) = 6.95 \text{ CFS} < 11 \text{ CFS}$ NG

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Project EHC

Subject FLOW CALCS

Project No. 91001 Date 3-91 By LAH

- ☐ Memorandum
- ☐ Telephone record
- ☐ Note to the file
- ☐ Minutes of meeting
- ☐ To be typed
- ☐ _____

TRY $d = .55$ FT
$l_1 = 5.5$ FT
$l_2 = 11$ FT

$A = 4.54$ FT^2

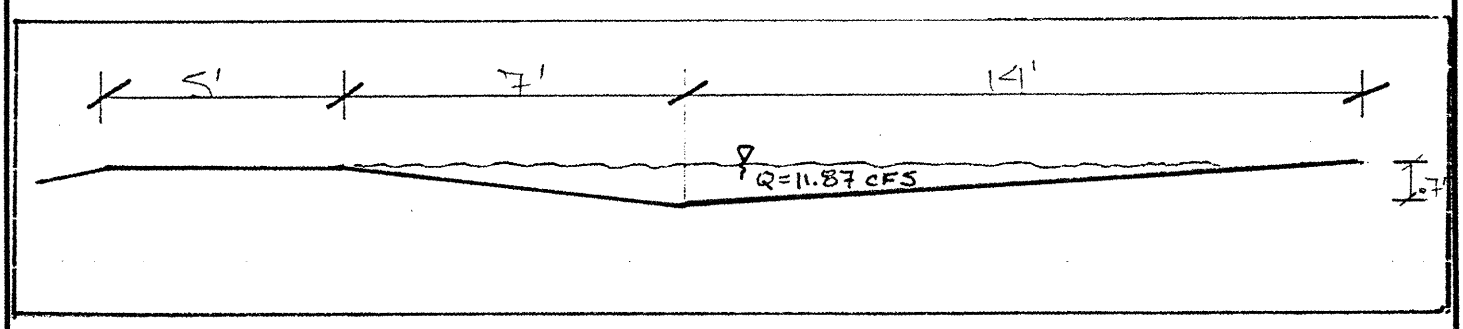
$P = \{(.55)^2 + (5.5)^2\}^{1/2} + \{(.55)^2 + (11)^2\}^{1/2} = 16.54$ FT

$R = A/P = .27$ FT

$Q = 1.49 (4.54)(.27)^{2/3} (.003)^{1/2} (1/.017) = 11.87$ CFS > 11 CFS OK

ADD .15 FT FOR FREE BOARD.

$d = .55' + .15' = .70$
$l_1 = 7$ FT
$l_2 = 14$ FT



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Mesa, AZ 85201
(602) 827-2759

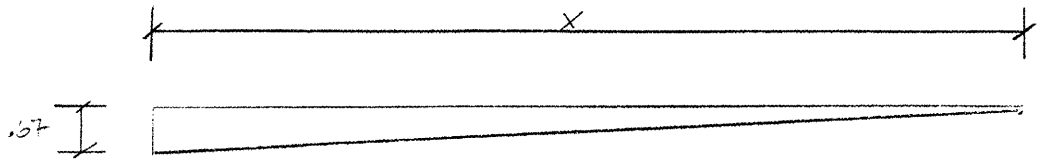
Project FLP

Subject FLOOD CALCS

Project No. 91001 Date 3-91 By LAH

- ☐ Memorandum
- ☐ Telephone record
- ☐ Note to the file
- ☐ Minutes of meeting
- ☐ To be typed
- ☐ _____

AT GUTTER BY EXISTING WATER METER JUST SOUTH OF MONTGOMERY



$Q_{ACT} = 11 CFS$

$n = .017$

$S_o = .005$

TRY $x = 18$ FT

$A = \frac{1}{2}(.67)(18) = 6 FT^2$

$P = 18.01$ FT

$R = A/P = .333$ FT

$R^{2/3} = .48$

$Q = \frac{1.49 A R^{2/3} S^{1/2}}{n} = 1.49(6)(.48)(.005)^{1/2}(1/.017)$

$Q = 17.87 CFS > 11 CFS$ OK

Copies to:

APPENDIX B

DIMENSIONLESS UNIT HYDROGRAPH TRACT A-2 POND
(City of Albuquerque)

A = 4.19 P100 = 2.40 RO = 1.40
Tc = 10 CN = 89

Point	t/Tp	t(min)	y	In Q(cfs)	In V (cf)	Out Q(cfs)	Out V(cf)	Total Net V	Pond Depth
1	0.0	0.00	0.000	0.00	0	0.00	0	0	0.00
2	0.1	1.00	0.030	0.80	48	0.00	0	48	0.01
3	0.2	2.00	0.100	2.66	160	0.00	0	208	0.05
4	0.3	3.00	0.190	5.06	304	0.01	0	511	0.13
5	0.4	4.00	0.310	8.26	495	0.05	3	1,003	0.26
6	0.5	5.00	0.470	12.52	751	0.28	0	1,754	0.45
7	0.6	6.00	0.660	17.58	1,055	0.57	34	2,774	0.71
8	0.7	7.00	0.820	21.84	1,310	0.87	52	4,032	1.03
9	0.8	8.00	0.930	24.77	1,486	1.14	68	5,450	1.40
10	0.9	9.00	0.990	26.37	1,582	1.38	83	6,950	1.78
11	1.0	10.00	1.000	26.63	1,598	1.59	95	8,452	2.17
12	1.1	11.00	0.990	26.37	1,582	1.78	107	9,927	2.55
13	1.2	12.00	0.930	24.77	1,486	1.95	117	11,297	2.90
14	1.3	13.00	0.860	22.90	1,374	2.09	125	12,546	3.22
15	1.4	14.00	0.780	20.77	1,246	2.21	133	13,659	3.50
16	1.5	15.00	0.680	18.11	1,087	2.32	139	14,607	3.75
17	1.6	16.00	0.560	14.91	895	2.40	144	15,358	3.94
18	1.7	17.00	0.460	12.25	735	2.47	148	15,945	4.09
19	1.8	18.00	0.390	10.39	623	2.52	151	16,417	4.21
20	1.9	19.00	0.330	8.79	527	2.55	153	16,791	4.31
21	2.0	20.00	0.280	7.46	447	2.59	155	17,083	4.38
22	2.2	22.00	0.207	5.51	662	2.61	313	17,432	4.47
23	2.4	24.00	0.147	3.91	470	2.64	317	17,585	4.51
24	2.6	26.00	0.107	2.85	342	2.65	318	17,609	4.52
25	2.8	28.00	0.077	2.05	246	2.65	318	17,537	4.50
26	3.0	30.00	0.055	1.46	176	2.65	318	17,395	4.46
27	3.2	32.00	0.040	1.07	128	2.63	316	17,207	4.41
28	3.4	34.00	0.029	0.77	93	2.62	314	16,985	4.36
29	3.6	36.00	0.021	0.56	67	2.60	312	16,740	4.29
30	3.8	38.00	0.015	0.40	48	2.58	310	16,478	4.23
31	4.0	40.00	0.011	0.29	35	2.56	307	16,206	4.16
32	4.5	45.00	0.005	0.13	40	2.54	761	15,485	3.97
33	5.0	50.00	0.000	0.00	0	2.48	743	14,742	3.78
-	-	60.00	-	-	0.00	2.41	1,447	13,294	3.41
-	-	80.00	-	-	0.00	2.28	2,738	10,556	2.71
-	-	100.00	-	-	0.00	2.01	2,415	8,141	2.09
-	-	120.00	-	-	0.00	1.74	2,089	6,052	1.55
-	-	140.00	-	-	0.00	1.47	1,758	4,294	1.10
-	-	160.00	-	-	0.00	1.18	1,421	2,873	0.74
-	-	180.00	-	-	0.00	0.90	1,075	1,798	0.46
-	-	200.00	-	-	0.00	0.59	708	1,090	0.28
-	-	220.00	-	-	0.00	0.22	265	825	0.21
-	-	240.00	-	-	0.00	0.18	216	609	0.16
			TOTAL VOLUME =		21,097				

NOTES: 1) Hydrograph assumes outlet is one 7 inch diameter orifice
(Q = 0.6 * A * (2gH)^0.5)
2) Pond depths based on 3900 sf, vertical wall design

City of Albuquerque
P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

January 8, 1992

PROJECT ACCEPTANCE LETTER

Mr. Carlos Spiess
Sundance Mechanical & Utility Contractors
5920 Midway Park Boulevard N.E.
Albuquerque, NM 87109

RE: F.H.P. MEDICAL CLINIC, PROJECT NO. 3803.91

Dear Mr. Spiess:

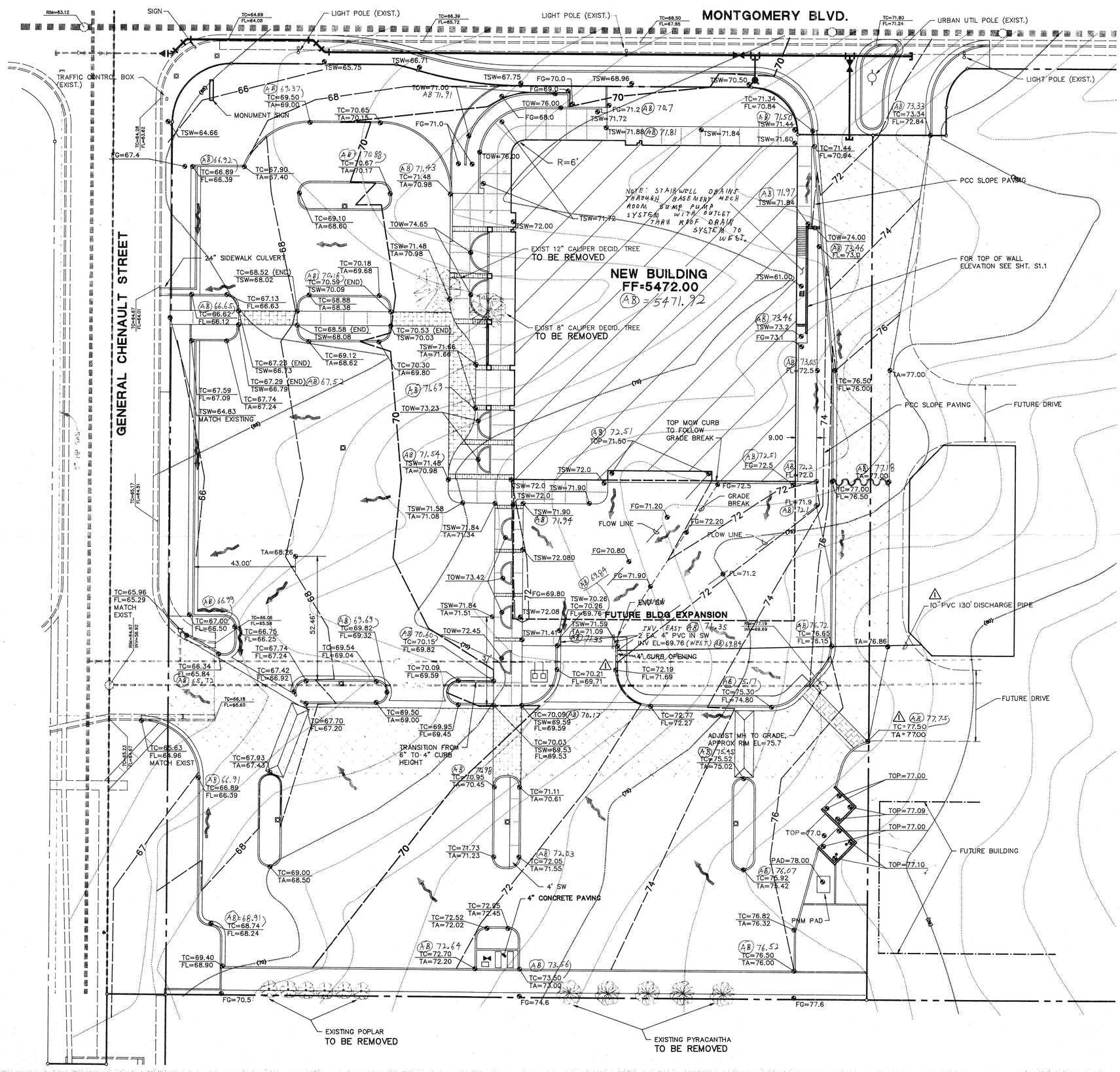
The above referenced project has been completed according to the plans and specifications. The project consisted of installation of an 8" fire line and a 4' bury fire hydrant on Montgomery. Constructed two (2) turnbays off Montgomery for access to the F.H.P. Building. Also constructed a drainage channel and paved alley east of F.H.P. Building.

The City of Albuquerque accepts the referenced project as a whole and the contractual correction period begins January 8, 1992. The correction period on this project is for one (1) year.

Sincerely,

Brian L. Speicher, P.E.
Chief Construction Engineer
Public Works Department

BLS:kv



ENGINEERS CERTIFICATION:
I hereby certify that the finished construction is in substantial compliance with the approved drainage plan.
By: Stephen L. Crawford
PE # 10274

STEPHEN L. CRAWFORD NEW MEXICO 10274 REGISTERED PROFESSIONAL ENGINEER

12-6-91
1-9-92

SCALE: 1" = 20'

10' 0 20'

04-01-91 MCD

91001

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Architects & Engineers, Inc.

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Designing to Shape the Future

BENCHMARK: M-2A, ACS BRASS CAP AT THE INTERSECTION OF MONTGOMERY AND LA BARRANCA ELEVATION = 5460.382

LEGAL DESCRIPTION: TRACT A-1, MONTGOMERY CROSSING ADDITION, PHASE III

NOTES:

1. THE WORK SHOWN ON THIS SHEET IS **NOT PART OF THE CITY WORK ORDER.**
2. TWO (2) WORKING DAYS PRIOR TO ANY EXCAVATION, CONTRACTOR MUST CONTACT THE LINE LOCATING SERVICE AT 260-1990 FOR LOCATION AND BLUESTAKING OF EXISTING UTILITIES.
3. THE CONTRACTOR MUST SECURE A TOPSOIL DISTURBANCE PERMIT PRIOR TO STARTING CONSTRUCTION.
4. CONTRACTOR SHALL ENSURE THAT NO SOIL ERODES FROM THE SITE INTO PUBLIC ROW OR ONTO PRIVATE PROPERTY. THIS CAN BE ACHIEVED BY CONSTRUCTING TEMPORARY BERMS AT THE PROPERTY LINES AND WETTING THE SOIL TO KEEP IT FROM BLOWING.
5. THE CONTRACTOR SHALL PROMPTLY CLEAN UP ANY MATERIAL EXCAVATED WITHIN THE PUBLIC ROW SO THAT THE EXCAVATED MATERIAL IS NOT SUSCEPTIBLE TO BEING WASHED DOWN THE STREET.
6. SEE DETAIL 25,C5.2 FOR TYPICAL PARKING AREA PAVEMENT SECTION.

LEGEND

(AB) AS-BUILT	TYPICAL MAIN DRIVE PAVING, SEE DETAIL 24, C5.2 FOR SECTION
TC=82.28 FL=81.62	EXIST ELEVATION
TC=82.28 FL=81.62	NEW ELEVATION
	FLOW DIRECTION
	WATER BLOCK
TC	TOP OF CURB
FL	FLOW LINE
TA	TOP OF ASPHALT
FG	FINISH GRADE
TSW	TOP OF SIDEWALK
MH	MANHOLE
TOW	TOP OF WALL
TOP	TOP OF PAVING

REV.	DESCRIPTION	DATE
1	10" PVC PRIVATE DISCHARGE PIPE RELOCATED MIN. 5' NORTH OF 6" WATERLINE. CHANGE TC EL. REMOVE 2' CURB OPENING.	4-17-91

STEPHEN L. CRAWFORD NEW MEXICO 10274 REGISTERED PROFESSIONAL ENGINEER
ENGINEER

STATE OF NEW MEXICO CHARLIE M. OTERO No. 1614 REGISTERED ARCHITECT
ARCHITECT

FHP MEDICAL OFFICES

MONTGOMERY & GENERAL CHENAULT

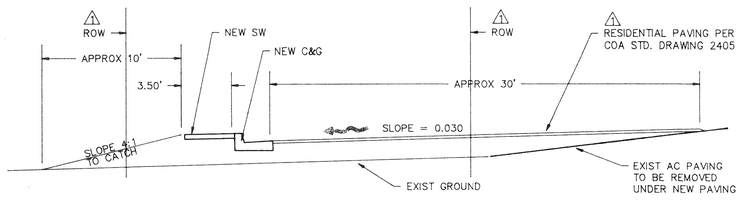
ALBUQUERQUE, NEW MEXICO

PROJECT NO. 91001
CITY: 3803.91

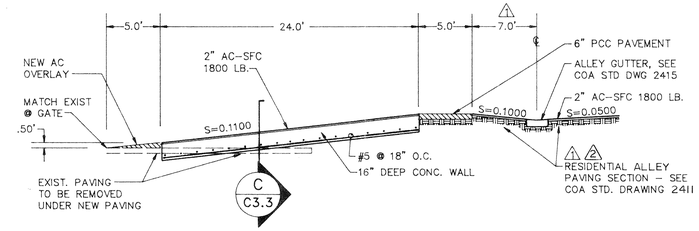
DATE
04-01-91

GRADING & PAVING PLAN

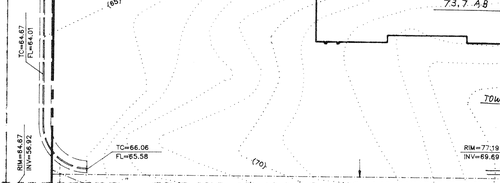
DRAWING NO.
C2.1
SHEET OF



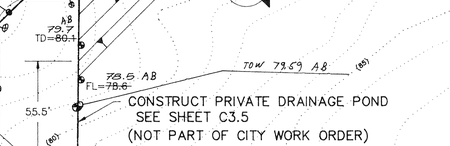
A C3.4 PAVING DETAIL SECTION
SCALE: 1" = 5'



B C3.4 WELLSITE DRIVEWAY RECONSTRUCTION
N.T.S.



C C3.3 WELLSITE DRIVEWAY SECTION
N.T.S.



D C3.4 DRAINAGE CHANNEL LINING
N.T.S.

E C3.3 TRAINER DIKE SECTION
SCALE: 1" = 10'
(NOT PART OF CITY WORK ORDER)

AB = As-built

LEGEND

TA=5491.35	EXIST ELEVATION
TC=5491.28 FL=5490.60	EXIST ELEVATION
	NEW ELEVATION
	WATER BLOCK
	FLOW DIRECTION
MB	MONTGOMERY BLVD.
SW	SIDEWALK
C&G	CURB & GUTTER
TD	TOP OF DIKE
TA	TOP OF ASPHALT
TC	TOP OF CURB
FL	FLOW LINE
INV	INVERT
TOW	TOP OF WALL

JAN 10 1992

(BPLW PROJECT NO. = 91001)

BPLW
Architects & Engineers, Inc.
2400 Louisiana Blvd. NE
AFC #5 Suite 400
Albuquerque, New Mexico 87110
(505) 881-2759
63 East Main Street
Suite 602
Mesa, Arizona 85201
(602) 827-2759
Designing to Shape the Future

NO.	DATE	REVISIONS	BY
1	4-17-91	ADD ROW LINES TO ALLEY SECTIONS, CHANGED TEMPORARY PAVING TO RESIDENTIAL PAVING. CHANGE REFERENCE DRAWING FROM 2405 TO 2411.	
2	4-29-91		

DESIGNED BY BPLW ARCH. & ENGINEERS. DATE 03-06-91
DRAWN BY MICHELE CRAWFORD DRAFTING DATE 03-06-91
CHECKED BY SLC/LAH DATE 03-06-91

CITY OF ALBUQUERQUE
PUBLIC WORKS DEPARTMENT
ENGINEERING GROUP

TITLE: FHP MEDICAL OFFICES
MONTGOMERY & GENERAL CHENAULT
OFFSITE DRAINAGE IMPROVEMENTS

APPROVALS	ENGINEER	DATE	APPROVALS	ENGINEER	DATE
DRC Chairman			Water		5-10-91
Transportation		8-26-91	Waste Water		"
Hydrology		4-25-91			
Parks & Rec.	N/A				

DRAWING NO. 3803.91 | MAP NO. G-20 | SHEET 6 OF 8 C3.3

AS BUILT INFORMATION
CONTRACTOR SUNDANCE MECHANICAL DATE 8-91
WORK COMMUNITY STAKED BY BPLW ARCH & ENGINEERS DATE 11-91
ACCEPTANCE BY " DATE 11-91
VERIFICATION BY " DATE 11-91
DRAWINGS CORRECTED BY " DATE 12-91
MICRO-FILM INFORMATION
RECORDED BY DATE
NO.

BENCH MARKS
BENCHMARK: M-24, ACS BRASS CAP AT THE INTERSECTION OF MONTGOMERY AND LA BARRANCA
ELEVATION = 5460.382

SURVEY INFORMATION
FIELD NOTES BY DATE
NO. THE MANN COMPANY NOV '90
DESIGN

ENGINEER'S SEAL
4-29-91
4-1-91