CITY OF ALBUQUERQUE



March 6, 2009

Tucker H. Green, P.E.

Per Se Engineering

2116 Lead Ave SE

Albuquerque, NM 87106

Re: Sandia Plaza Improvements, 3301 Juan Tabo NE,

Approval of Permanent Certificate of Occupancy (C.O.)

Engineer's Stamp dated 2/21/08 (G-21/D038)

Certification dated 3-04-09

Dear Mr. Green,

PO Box 1293

Based upon the information provided in your submittal received 3-04-09, the above referenced certification is approved for release of Permanent Certificate of Occupancy by Hydrology.

Albuquerque

If you have any questions, you can contact me at 924-3982.

NM 87103

Timothy E. Sims

Sincerely

Plan Checker—Hydrology,

www.cabq.gov

Development and Building Services

C: CO Clerk-Katrina Sigala

File

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levels of submitted may be required based on the following:

Sector Plans. 1. Conceptual Crading and Drainage Plan: Required for approval of Site Development Plans greater than five (5) acres and

2. Drainage Plans: Required for building permits, grading permits, paving permits and site plans less than five (5) acres.

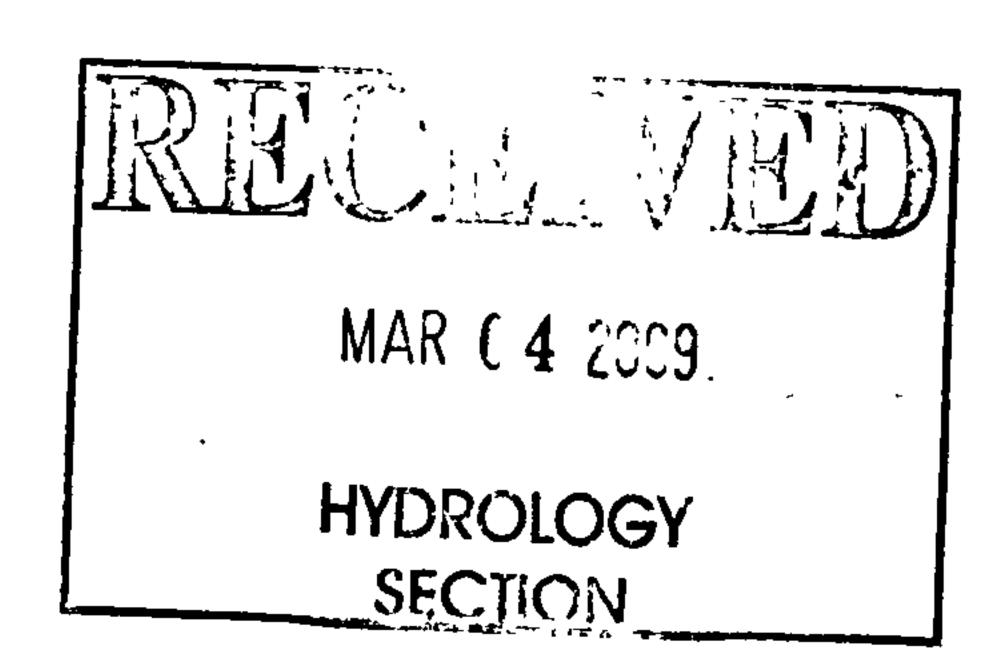
3. Drainage Report: Required for subdivision containing more than ten (19) lots or constituing five (3) acres or more.

Per Se Engineering Drainage, Utilities, & Site Design 2116 Lead Ave. SE - Albuquerque NM 87106 - 505.232-9394 - Telephone 1980.

Monday, February 2, 2009

Traci Wolf, Defined Fitness, 3301 Juan Tabo NE Albuquerque NM 87111

Paul Blanchard Sandia Plaza 3301 Juan Tabo NE / 5850 Eubank Blvd. NE, Suite B52 Albuquerque NM 87111



Re: "As Built" Certification for Sandia Plaza / Defined Fitness Improvements at Sandia Plaza, Albuquerque, NM

This letter concerns construction which was not performed according to the approved sitework plans that I prepared, as noted below. If these items are acceptable to each of you, separately, they will be acceptable to me and I will be able to certify to the City of Albuquerque that my part of the project has been constructed sufficiently well.

My understanding is that your interests in part overlap and in part are different. I am sending the same letter to each of your and will need from each of you a copy with your signature of approval. I will include copies of the signed letters with my certification submittal. You can send .pdf files of the signed letters to me at TuckGreen@Netzero.com.

First I reproduce several General Notes from the approved plan set:

- 3. CONTRACTOR'S GOOD SENSE: THE CONTRACTOR IS EXPECTED TO USE HIS GOOD SENSE, EXPERIENCE, AND JUDGMENT.
- 4. IF UNUSUAL, CONFLICTING, OR EVEN APPARENTLY <u>ODD CIRCUMSTANCES</u> ARISE THE CONTRACTOR IS TO CONSULT WITH THE ENGINEER FOR A RESOLUTION BEFORE PROCEEDING WITH FINAL LAYOUT OR CONSTRUCTION.
- 5. CHANGES & DEVIATIONS ARE TO BE APPROVED IN WRITING IN ADVANCE OF CONSTRUCTION, CONTRACTOR IS TO KEEP A MARKUP COPY OF THE PLANS AND TO NOTE THEREON ALL CHANGES AND DEVIATIONS FROM THE PLANS.
- 6. ALL WORK TO BE PERFORMED IN A <u>WORKMANLIKE MANNER</u>.

I was not contacted about the changes and deviations noted herein, either before or during construction. I recently asked the Contractor's current superintendent, Hugh Robinson, for the mark-up plan set showing changes and was told it did not exist. Design addressed both practical and regulatory concerns.

Areas that were constructed less well than designed impair my ability to point to this project with pride as indicative of my work.

Items to be noted here:

In the following, "main entrance area" refers to the area of the east side of the building from the 2-ft change in floor. elevation on the north over to the corner by the bike rack and the old "double door flush with the wall" main entrance, roughly 144 ft south. The change in floor elevation is about 23 ft north of the designed location of the north side of the new entrance vestibule. Somewhere between the south side of the design location of the vestibule and change in floor elevation was (and is) a single door opening outward.

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Page 1 of 3

Per Se Engineering Drainage, Utilities, & Site Design

- 1. The landscape islands in the north parking area are much deeper than per plan. From a drainage point of view they will work even better than planned. The question is whether they look good enough.
- 12. The plans call for rebuilding 4 landscape islands along the east side of the north-south drive aisle in front of the building. These are the two islands flanking the main entrance from Juan Tabo and the two immediately north. The islands needed to be rebuilt so that the paving in the north-south driveway would not have an unpleasant side slope. None of the islands was rebuilt and the paving has more sideslope than I think is desirable or appropriate. There does not appear to be a practical problem with drivability. Again the question is one of looks, and whether "bad" looks detract from the value of the project.
- 3. Where the new paving meets the old, the plans show neat, straight lines for joining. Here, particularly, General Note 6 ("WORKMANLIKE MANNER") is relevant. Originally all the joins were constructed messy; none was sawcut or otherwise made neat. Recently the join at the main entrance drive was improved somewhat but not, in my opinion, to the extent of looking good or workmanlike. All the other joins remain unattractive, in my judgement, but present no drivability problems.
- 4. The parking at the front of the building was designed with a HC access ramp into the curb at the middle of a large (18-ft wide) access aisle right in front the new main entrance. The new entrance had a "bumped-out" vestibule with double set of double doors opening eastward toward parking. A key feature of grading design provided about a foot of fall from finished floor elevation at the entrance down to the low point of the swale opposite the ramp. Maintaining a minimum 1/2 percent slope in this swale was a main reason repaving had to extend so far south.

Without any notice to me, most of the parking layout along the east side of the building was changed, including many of the HC accessible spaces and aisles. None of the HC spaces or aisles looks excessively steep but without a revised plan to go with the as-built grade it is impossible to be completely sure. Also, the vestibule was constructed differing in size, location, and design from the approved plans; with exterior sliding doors on the north and south but no doors on the east. A single door swings out eastward about 7 ft east north of the vestibule. The HC access ramp is roughly opposite the center of the east face of the vestibule (no openings).

In terms of drainage, having the HC ramp away from the main door openings is probably good, but the hinged door and the sliding door on the north side of the vestibule may present a practical problem, in being more vulnerable to runoff coming down the main drive from Juan Tabo. The affected parties must judge for themselves about the attractiveness of the revisions, particularly as to the concrete drainage swale, which is a change that impairs my ability to point to this project as an example of my work.

5. This item is possibly the most important and is related to possible problems with runoff reaching the building: The sidewalk in the main entrance area was NOT rebuilt per the approved plans. A strip of sidewalk roughly 7 ft wide adjacent to the building was not rebuilt at all, at least south of the vestibule.

The plans show some of the existing spot elevations shot by the surveyor as well as the designed new spot elevations. (The symbol for existing elevations is a circle with an X. The symbol for new spot elevations is a spot, or dot.) The existing spot elevations show that near the south side of the main entrance area (near the bicycle rack and the old main entrance) the finished floor is lower than the sidewalk at the top of the nearby parking spaces. We took care in our plans to have the sidewalk rebuilt so that it would drain away from the building (rather than toward it, as previously). This is shown on Sheet D2 by Key Note F, which says "REMOVE EXIST. SIDEWALK & REPLACE TO GRADES SHOWN", and also by the new spot elevations.

Per the plan received from Garlan Bryan prior to our design, the only openings in the remodeled main entrance area were to be the double doors at the entrance vestibule. As constructed, it appears that the vestibule entrance has been added and that both old doors remain, one of them the old main entrance.

Page 2 of 3

Per Se Engineering Drainage, Utilities; & Site Design

2116 Lead Ave. SE - Albuquerque NM 87106 - 505.232-9394 - Colores Colo

Apparently, in the past, the strip of sidewalk not rebuilt was covered with tiles over a thin layer (roughly 1/2 inch) of concrete over older sidewalk. The tiles are gone and the thin layer is broken up and broken out in several places, presenting an uneven surface that may be a safety hazard as well as unattractive. The drainage issue is the possibility of trapping water by the building.

6. The as-built survey by Harris Surveying indicates that the paving and/or concrete contractors frequently did not get grades within 3 or 4 inches of design. It is hard to think that a competent crew attempting to do a good job could have been so far off so often. My visual site inspection did not reveal any glaring problems, however. In areas that are flatter than design — especially where concrete slope is less than 0.5% or where pavement slope is less than 1.2% – I would anticipate minor puddles ("bird baths") along with shortened pavement life.

If runoff reaching the building is a problem in the future, there are several possible solutions. An obvious one that I have mentioned on various occasions would be to force a "water block" at the main entrance from Juan Tabo, right near Juan Tabo. This would require some repaving and would function to traffic like a low, broad, speed bump.

Sincerely,

Tucker Green, PE

I accept all of the above "Items to be noted" as they have been constructed. I have the authority to accept them.

Date

I accept all of the above "Items to be noted" as they have been constructed. I have the authority to accept them.

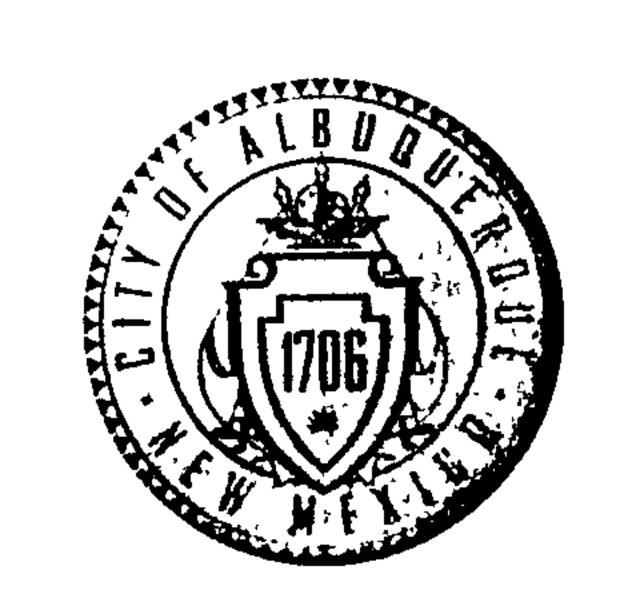
Paul Blanchard, Sandia Plaza owners

Traci Wolf, Defined Fitness

Date

Page 3 of 3

CITY OF ALBUQUERQUE



Planning Department Transportation Development Services Section

March 4, 2009

Garlan D. Bryan, Registered Architect 2403 San Mateo NE, Ste. W-1 Albuquerque, NM 87110

Re: Certification Submittal for Final Building Certificate of Occupancy for

Defined Fitness Expansion and Renovation, [G-21 / D038]

3301 Juan Tabo NE

Architect's Stamp Dated 02/26/09

Dear Mr. Bryan:

Sincerely,

PO Box 1293

The TCL / Letter of Certification submitted on March 4, 2009 is sufficient for acceptance by this office for final Certificate of Occupancy (C.O.). Notification has been made to the Building and Safety Section.

Albuquerque

NM 87103

www.cabq.gov

Nilo E. Salgado-Fernandez, P.E.

Senior Traffic Engineer

Development and Building Services

Planning Department

C:

Engineer
Hydrology file
CO Clerk

Albuquerque - Making History 1706-2006

DRAINAGE AND TRANSPORTATION INFORMATION SHEET

(REV 12/2005)

G-21/D038

PROJECT TITLE: DEFINED FINES EXPANSION! REMOVATION	
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LEGAL DESCRIPTION: TRACT H-3-A LA REINA DE LOS A	CTOSIUNII NOLLISECTION 9 TOWNSH
CITY ADDRESS: 3301 JUAN MABO NE ALBUQUERQUE, N.M.	BILL NORTH, DANGE GEAST
ENGINEERING FIRM: MCCRNACK ENGINEERING	CONTACT: JM MCCOLNACK
ADDRESS: 2001 CARLISLE BLYD NE	PHONE: 505-881-0570
CITY, STATE: ALBUGUERUUE, NM 87110	ZIP CODE:
OWNER: SANDIA PLAZA PARTNERS LLC	CONTACT:
ADDRESS: 5850 EUBANK BLUD NE SUITE B-6	2 PHONE: \$05-275-0000
CITY, STATE: ALBUGUERRUE, NM	ZIP CODE: \$7111
	_
ARCHITECT: GARLAN BLYAN ARCHITECT	CONTACT: GARLAN BRYAN
ADDRESS: 2403 SAN MATEO BLVD SUITE W-1	PHONE: 505-884-9694
CITY, STATE: ALBUGUER QUE, N.M.	ZIP CODE: <u>8 7 1 1 0</u>
SURVEYOR: PER SE ENGINEERING	CONTACT:
ADDRESS: 2116 LEAD AVE. SE	PHONE: 505 - 232 - 9394
CITY, STATE: ALBYGILEN WIE, NM	ZIP CODE: <u>87106</u>
CONTRACTOR: BLANCHARD CONSTRUCTION	CONTACT:
ADDRESS: 5850 EUBANK BLUB. NE SUITE B-1	
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Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a drainage submittal. The particular nature, location, and scope to the proposed development defines the degree of drainage detail. One or more of the following levels of submittal may be required based on the following:

- 1. Conceptual Grading and Drainage Plan: Required for approval of Site Development Plans greater than five (5) acres and Sector Plans.
- 2. Drainage Plans: Required for building permits, grading permits, paving permits and site plans less than five (5) acres.
- 3. Drainage Report: Required for subdivision containing more than ten (10) lots or constituting five (5) acres or more.

Garlan Bryan Architect

2403 San Mateo Blvd. NE Suite W-1 Albuquerque, N.M. 87110 Phone: 505.884.9694

Fax: 505.872.0437

OF NEW

U. BRIAN

ERED ARCY

February 26, 2009

Project: 3301 Juan Tabo,

Defined Fitness Expansion and Renovation

Phase 1 and 2

TRAFFIC CERTIFICATION

I, Garlan Bryan, Architect, of the firm Garlan Bryan Architect, hereby certify that this project is in substantial compliance with and in accordance with the design intent of the DRB, AA, or TCL approved plan dated 12/29/2007. The record information edited onto the original design document has been obtained by Garlan Bryan Architect. I further certify that I have personally visited the project site on 2/24/2009 and have determined by visual inspection that the survey data provided is representative of actual site conditions and is true and correct to the best of my knowledge and belief. This certification is submitted in support of a request for a final certificate of occupancy.

actual site conditions and is true and corre submitted in support of a request for a final	ct to the best of my knowledge and belief. Thi certificate of occupancy.
Exceptions:	
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HONE

Deficiencies:

The record information presented hereon is not necessarily complete and intended only to verify substantial compliance of the traffic aspects of this project. Those relying on the record document are advised to obtain independent verification of its accuracy before using it for any other purpose.

(Garlan Bryan, Architect)

2-26-08

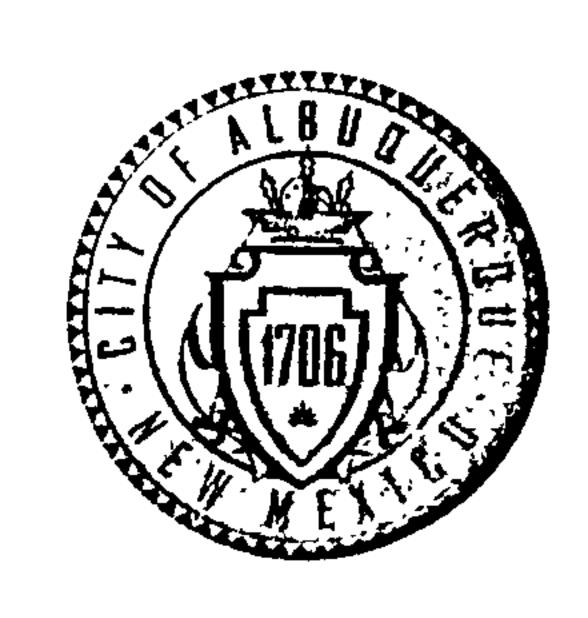
(Date)

MAR C 4 2009

HYDROLOGY

SECTION

CITY OF ALBUQUERQUE



March 27, 2008

Tucker Green, PE
Per Se Engineering
2116 Lead Ave SE
Albuquerque, NM 87106

Re: Sandia Plaza – Defined Fitness Grading and Drainage Plan Engineer's Stamp dated 2-21-08 (G21/D38)

Dear Mr. Green,

Based upon the information provided in your submittal dated 2-21-08, the above referenced plan is approved as amended. This is now the plan that must be certified for Certificate of Occupancy.

If you have any questions, you can contact me at 924-3986

Sincerely,

Bradley L. Bingham, PE
Principal Engineer, Planning Dept

Development and Building Services

NM 87103

PO Box 1293

Albuquerque

www.cabq.gov

DRAINAGE REPORT FOR SANDIA PLAZA IMPROVEMENTS: DEFINED FITNESS EXPANSION & IMPROVEMENTS

PROJECT LOCATION: 3301 JUAN TABO BLVD. NE, ALBUQUERQUE, NM, (APX NE CORNER OF CLAIM NEBOS)
CANDELARIA, OPPOSITE LOWE'S HOME IMPROVEMENT CENTER

LEGAL DESCRIPTION: A PORTION OF TRACT H-3-A, LA REINA DE LOS ALTOS, INIT 2

FEB 21 2008

FLOOD ZONE: NOT IN 100-YEAR FLOOD ZONE. SEE MAP.

HYDROLOGY SECTION

THE SITE HAS BEEN COMPLETELY DEVELOPED SINCE SOME TIME BACK IN THE 1970'S, AT WHICH TIME THE CITY WAS NOT REQUIRING OR KEEPING TRACK OF DRAINAGE REPORTS THE WAY IT DOES NOW. IN 1998 TIERRA WEST PREPARED A REPORT FOR THE ADDITION OF A KRISPY KREME DONUT STORE (NOW OUT OF BUSINESS AND VACANT) NEAR THE NORTHEAST PORTION OF THE SITE (DRAINAGE FILE G21-D38). THAT REPORT ANALYZED THE OVERALL SITE AS 90% LAND CLASS "D" (IMPERVIOUS) AND 10% CLASS "B" (LANDSCAPING).

THE TIERRA WEST REPORT AND SUBSEQUENT WRITTEN DISCUSSION CLARIFIED THAT IT WAS ACCEPTABLE FOR MUCH OF SANDIA PLAZA TO DRAIN ONTO THE APARTMENT COMPLEX ADJACENT ON THE WEST. THE PARKING NORTH OF THE MAIN (NOW DEFINED FITNESS) BUILDING AND MUCH OF THE KRISPY KREME SITE DRAINED TO AN OPENING IN THE PERIMETER WALL NEAR THE NORTHWEST SITE CORNER AND THEN ONTO PAVEMENT AT THE ADJACENT APARTMENT COMPLEX, THEN EVENTUALLY OUT TO CANDELARIA BLVD. DRAINAGE FROM PART OF THE ROOF, PAVEMENT BETWEEN THE MAIN BUILDING AND THE WEST WALL, AND THE PAVEMENT DIRECTLY SOUTH OF THE MAIN BUILDING DRAINED TOWARD THE SOUTHWEST CORNER OF THE SITE AND THEN ONTO THE AUTOZONE SITE. FROM THE AUTOZONE SITE THIS DRAINAGE WENT TO CANDELARIA BLVD, WITH SOME POSSIBLY SPILLING ONTO THE APARTMENT COMPLEX SITE. MOST OF THE SITE, HOWEVER, DRAINED OUT THE WEST DRIVEWAY, EAST OF AUTOZONE, THEN ONTO CANDELARIA BLVD.

SINCE THEN PART OF THE MAIN PRE-1998 BUILDING DEVELOPED STRUCTURAL PROBLEMS AND WAS TORN DOWN. AERIAL PHOTOS FROM THE CITY'S GIS WEB SITE SHOW THE OLD BUILDING EXTENDING NORTH PAST THE NORTH SIDE OF THE KRISPY KREME BUILDING, FARTHER THAN THE WORK PROPOSED HERE, AND THE AREA NORTH OF THAT AS BEING PAVING WITHOUT LANDSCAPING.

THE WORK PROPOSED IN THE PRESENT PROJECT BASICALLY REPLACES PART OF THAT FORMER BUILDING WITH NEW TENANT SPACE AND PROVIDES SIGNIFICANTLY MORE LANDSCAPING THAN EXISTED PREVIOUSLY. THE NEW STRUCTURE EXTENDS TO ROUGHLY THE SOUTH SIDE OF THE KRISPY KREME BUILDING. THUS THIS PROJECT WILL REDUCE RUNOFF FROM THAT WHICH WOULD OCCUR UNDER EXISTING, LONG-STANDING, PREVIOUSLY APPROVED CONDITIONS. DRAINAGE PATTERNS ARE ESSENTIALLY UNCHANGED EXCEPT THAT RUNOFF FROM THE KRISPY KREME SITE (AND POSSIBLY ALSO FROM THE MOTORCYLE DEALERSHIP NORTH OF KRISPY KREME) WILL GO OUT THE WEST DRIVEWAY RATHER THAN ONTO THE APARTMENT COMPLEX BY WAY OF THE WALL OPENING IN THE NORTHWEST CORNER.

CALCULATIONS INCLUDED WITH THIS SUBMITTAL INDICATE THAT THE TOTAL LANDSCAPING TO BE PROVIDED ON THE PROJECT SITE (I.E. EXCLUSIVE OF THE KRISPY KREME SITE) IS MORE THAN THE 10% OF THE NET AREA (I.E. EXCLUSIVE OF THE BUILDING AREA) REQUIRED BY THE ZONING ORDINANCE BUT JUST SLIGHTLY LESS (9.62%) THAN THE PREVIOUSLY (AND APPARENTLY INACCURATELY) CLAIMED 10% OF THE TOTAL SITE AREA. SINCE THE LANDSCAPE AREA IS INCREASED AND THE IMPERVIOUS AREA IS DECREASED, RUNOFF WILL ACTUALLY BE REDUCED BOTH IN PEAK RATE AND IN TOTAL VOLUME. PLASTIC WILL NOT BE USED UNDER THE LANDSCAPING 2/21/2008 7:26 AM

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Page 1 of 2

FOR THIS PROJECT. IN KEEPING WITH PREVIOUS REPORTS, LANDSCAPING FOR THIS PROJECT, WHETHER XERIC OR NOT, IS ANALYZED AS TREATMENT "B" EXCEPT FOR THE STEEP AREA BETWEEN THE BUILDING AND THE NORTH PARKING AREA. WHICH IS ANALYZED AS TREATMENT "C".

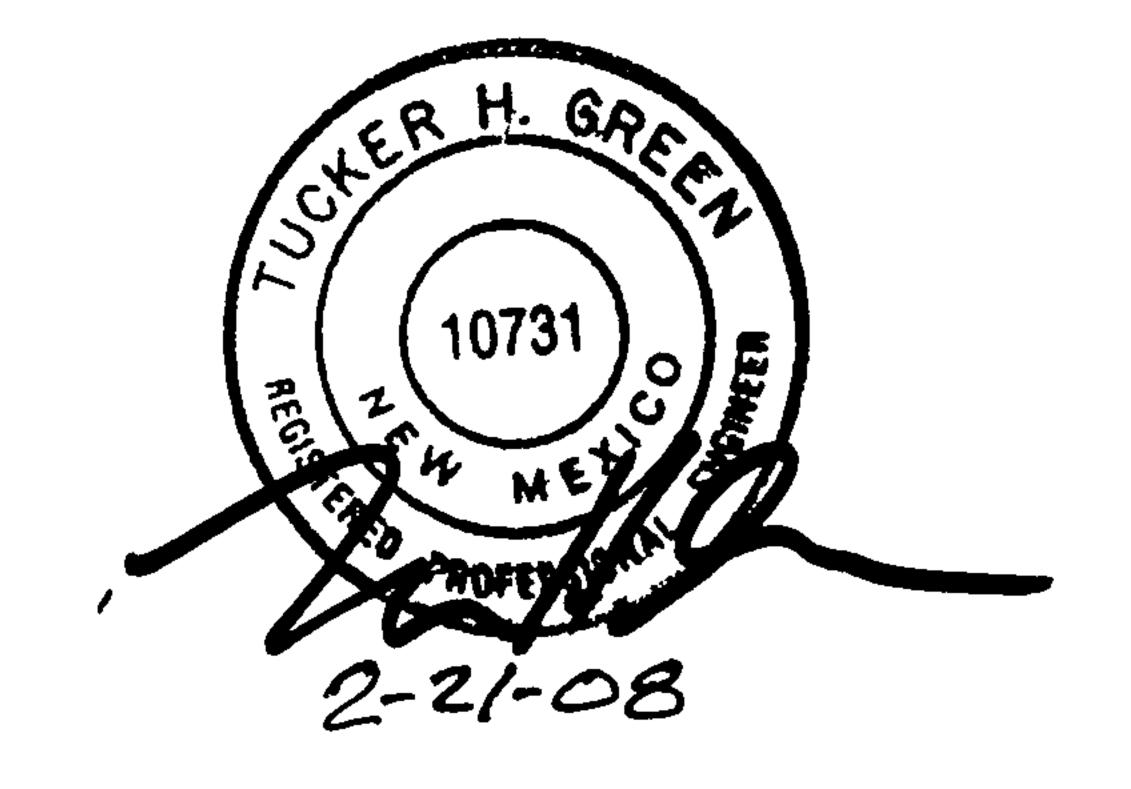
THE LANDSCAPE ISLANDS IN THE NORTH PARKING AREAS ARE DESIGNED TO FUNCTION AS MINI RESERVOIRS, AND THUS TO REDUCE THE PEAK RUNOFF RATE AND PROVIDE A SMALL AMOUNT OF WATER HARVESTING. HOWEVER, THE STORAGE CAPACITIES ARE SO SMALL AND THE HYDRAULIC CHARACTERISTICS ARE SO SUBJECT TO MINOR CHANGES IN CONSTRUCTION THAT THE ISLANDS WERE NOT ANALYZED AS RESERVOIRS. THE EFFECT IS THERE, BUT TO SOME SMALL AND UNKNOWN EXTENT.

THE NEW BUILDING EXTENDS THE EXISTING 5624.33 FINISHED FLOOR ELEVATION APX. 45 FT FARTHER NORTH, WITH THE RESULT THAT THE NORTH PARKING AREA, WHICH DRAINS TO AN EXISTING WALL OPENING NEAR THE NORTHWEST CORNER OF THE SITE, IS ABOUT 2 FEET HIGHER THAN THE FINISHED FLOOR. THE INTERVENING LANDSCAPE AREA RECEIVES RUNOFF FROM A SMALL SERVICE DRIVEWAY AND SOME ADJACENT SIDEWALK IN ADDITION TO THE RAIN THAT FALLS DIRECTLY ON IT. TLANDSCAPE AREA NORMALLY HAS NO OUTFLOW BUT IT DOES HAVE PROVISION FOR OVERFLOW BELOW FINISHED FLOOR ELEVATION. THE STORAGE CAPACITY AT ELEVATION 23.2 (JUST LOWER THAN EDGE OF SIDEWALK) IS 2257 CF (AVERAGE END AREA METHOD) WHILE THE STORAGE NEEDED FOR THE REGULATORY 100-YR, 10-DAY STORM 1S 1835 CF. AT ABOUT ELEV 24.02. OUTFLOW WOULD START SLIGHTLY BELOW THIS, AT THE INVERT OF THE SIDEWALK CULVERT AS SHOWN ON THE PLAN.

DETAILED RUNOFF CALCULATIONS FOR VARIOUS CONDITIONS ARE PROVIDED WITH THIS REPORT. FOLLOWING ARE SEVERAL KEY ITEMS FOR THE REGULATORY 100-YEAR STORM ON THE PROJECT SITE (DOES NOT INCLUDE KRISPY KREME SITE):

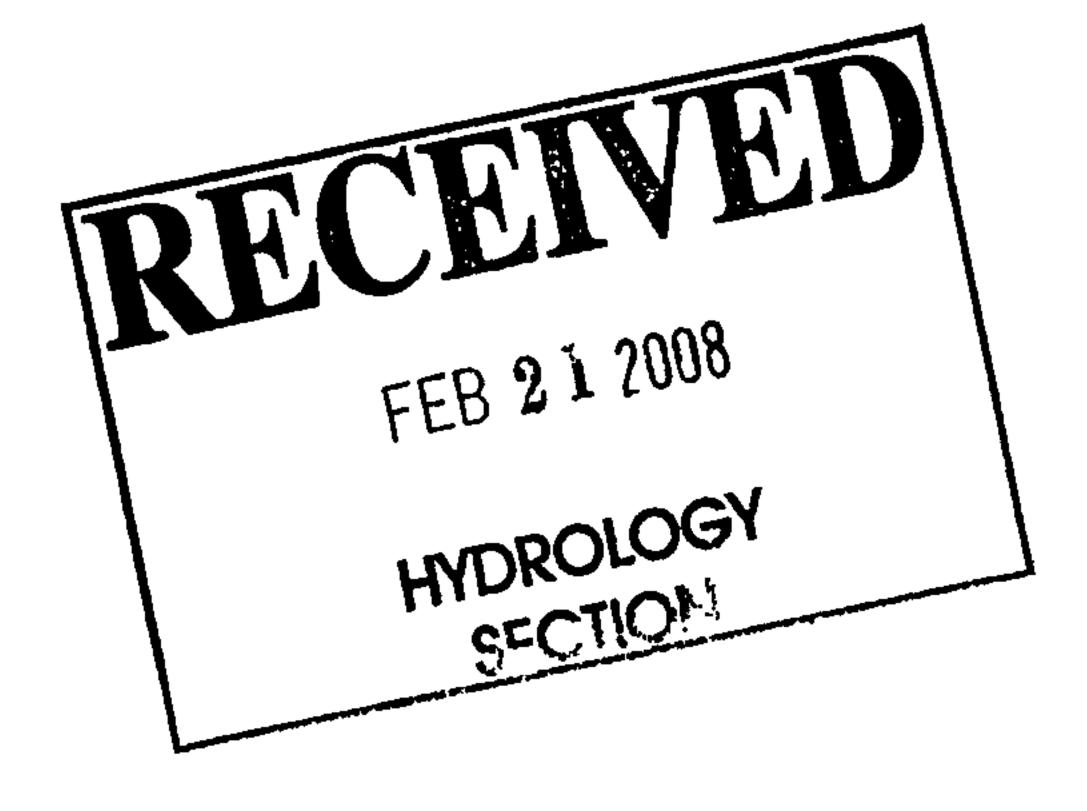
EXISTING (PRE-DEMOLITION) CONDITIONS Qpeak, cfs	37.85
PROPOSED CONDITONS, Qpeak,cfs	
TOTAL SITE	37.01
TO EXIST DRAINAGE OPENING NEAR NW CORNER	4.33
TO NO-OUTLET LANDSCAPE AT N SIDE OF BLDG.	0.77
10-DAY VOLUME, cf	1835

END OF REPORT TEXT



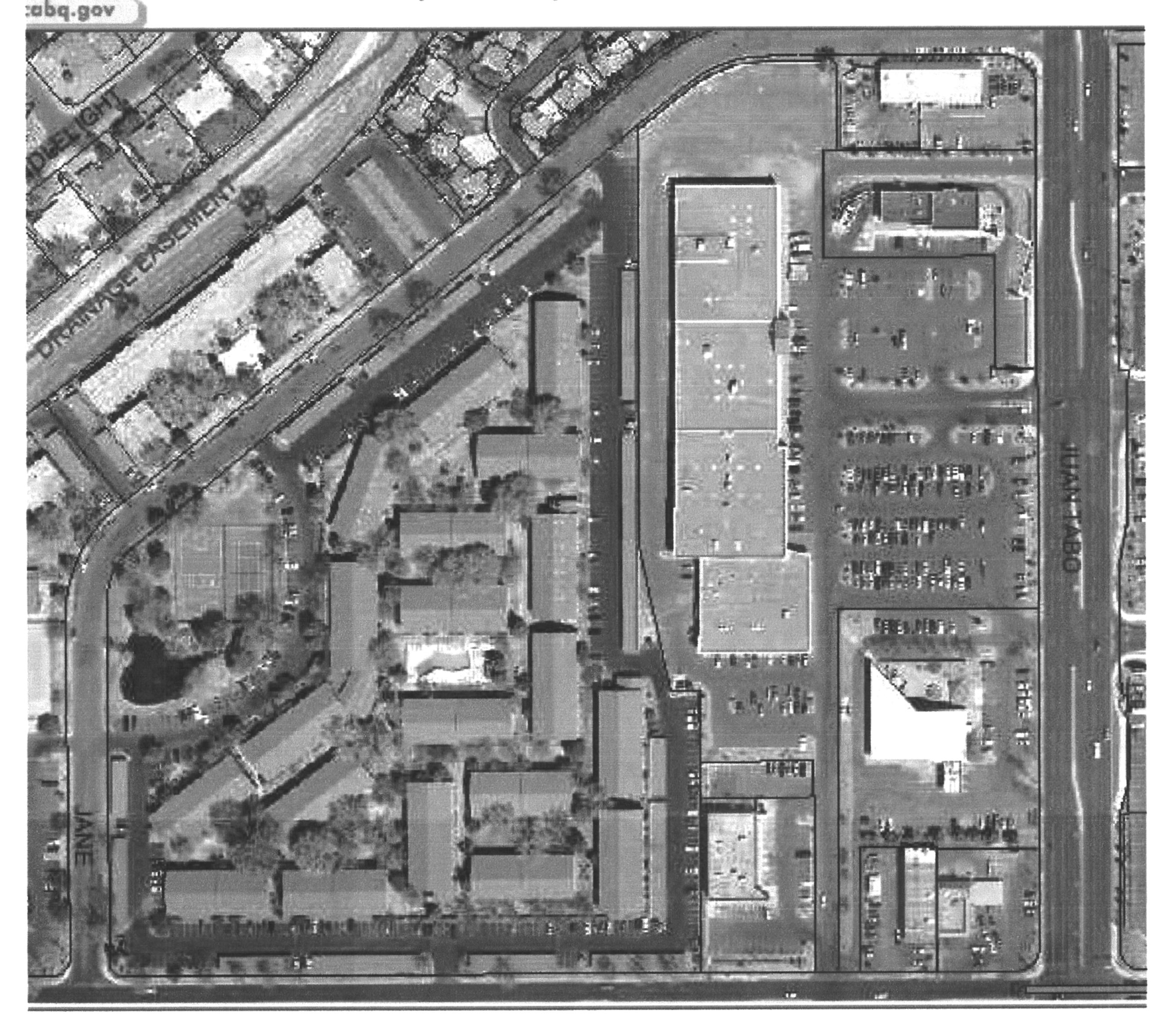


APPENDIX

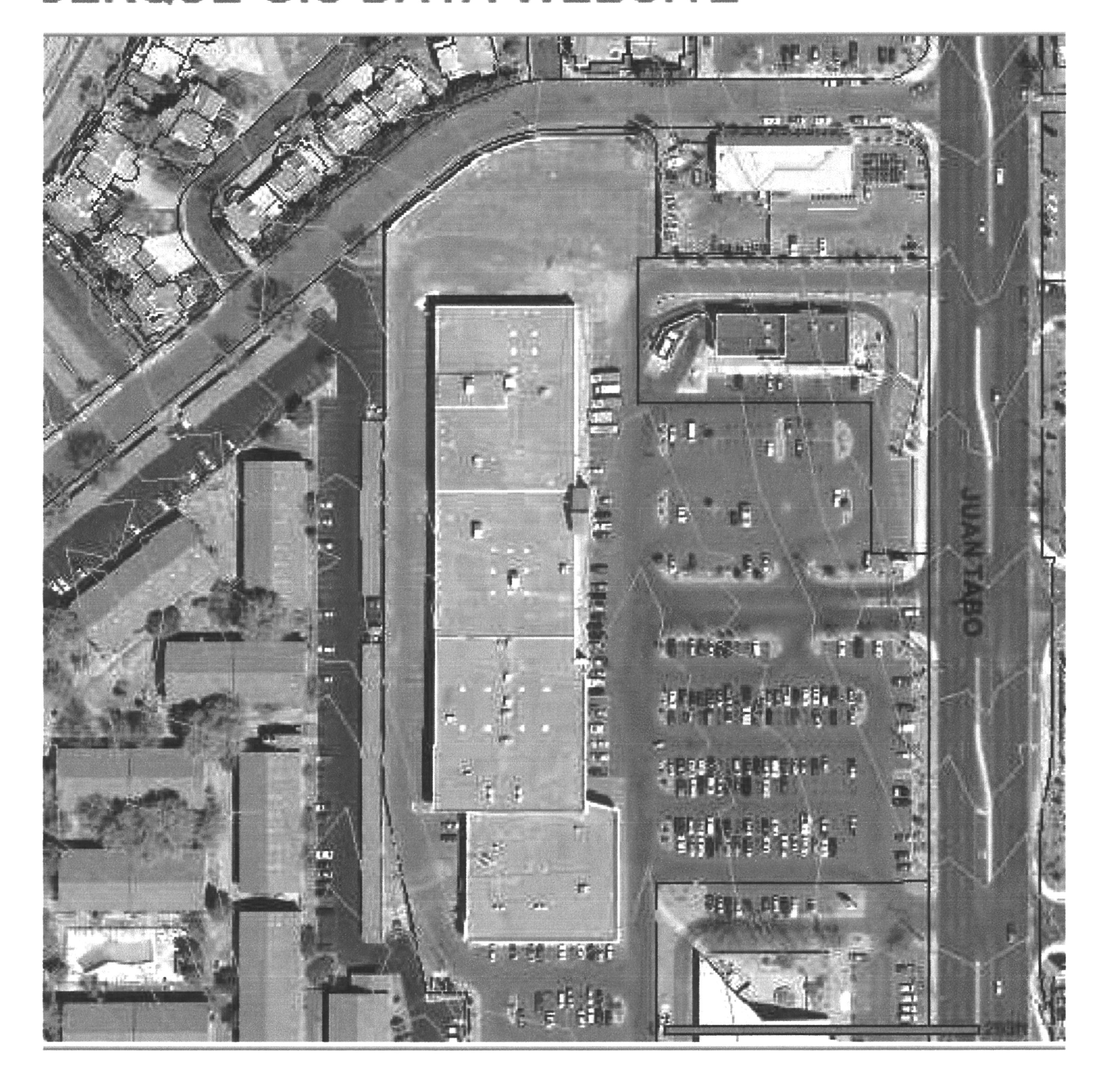


.BUQUERQUE

ALBUQUERQUE GIS DATA WEBSITE



JERQUE GIS DATA WEBSITE







PROJECT: DEFINED FITNESS @ 3301 JUAN TABO NE, ALBUQUERQUE, NM

ALBUQUERQUE, NM (1/93) CRITERIA - D.P.M. SIMPLE PROCEDURE FOR <= 40 ACRES (ASSUMES Tc=0.2hr = 12min)

BASIC TREATMENT CLASSES: A=UNDISTURBED, B=LAWNS, C=UNPAVED ROADS or STEEP B, D=ROOFS, PAVEMENT: SEE DPM 22.2 P A-5

PX100-6 = PRECIPITATION EXCESS FROM 100-YEAR 6-HOUR STORM; PX10-6 = SAME FOR 10-YEAR STORM

VOL10D = VOLUME OF RUNOFF FROM 10-DAY STORM; APPROXIMATE "6-HR" HYDRGRAPH PER DPM 22.2 SECTION A.8

PROJECT SITE, EXISTING CONDITIONS (BEFORE PART OF BUILDING DEMOLISHED)

NO LANDSCAPING NORTH OF BUILDING; LANDSCAPING AS TREATMENT "B"

Storm, yr	100	Zone	4		Qp6,cfs	37.85		VOL1D,cf	87,802		SF,total	320,077
	~+	TRTMT	AREA	AREA	PX100-6	QP100-6	QP100-6	VOL6HR	VOL1D	VOL4D	VOL10D	TRTMT
Rain,	inches	CLASS	SQ FT	ACRES	INCHES	CFS/AC	CFS	CU FT	CU FT	CU FT	AC-FT	PERCENT
0.2 hr	1.12	À	0.0	0.0000	0.80	2.20	0.000	0	0	0	0	0.00
1 hr	2.23	В	13,608.0	0.3124	1.08	2.92	0.912	1,225	1,225	1,225	1,225	4.25
6 hr	2.90	С	0.0	0.0000	1.46	3.73	0.000	0	0	0	0	0.00
1 day	3.65	D	306,469.0	7.0356	2.64	5.25	36.937	67,423	86,577	113,394	145,317	95.75
4 day	4.70	TOTAL	320,077.0	7.3480	2.574	5.151	37.849	68,648	87,802	114,618	146,542	100.00
10 day	5.95	_	SQ MI>	0.0114812	^Average	per acre ^	AC-FT>	1.5759	2.0157	2.6313	3.3641	<ac-ft< td=""></ac-ft<>
Apx. hydgro	ograph	Tconc	0.20	T Base	0.81	T Peak	0.19	² eak Lasts	0.24	Hydrograph	Qavg, cfs	19.92

PROJECT SITE, EXISTING CONDITIONS (BEFORE PART OF BUILDING DEMOLISHED) NO LANDSCAPING NORTH OF BUILDING: ALL LANDSCAPING AS TREATMENT "C"

110 PULL	UMI IIIU		I OI DOIEDI	ivu, nee er	HADOOM HA	a no sitem	I I WIE I V			_		
Storm,yr	100	Zone	4		Qp6,cfs	38.10		VOL1D,cf	88,233		SF,total	320,077
		TRTMT	AREA	AREA	PX100-6	QP100-6	QP100-6	VOL6HR	VOL1D	VOL4D	VOL10D	TRTMT
Rain,	inches	CLASS	SQ FT	ACRES	INCHES	CFS/AC	CFS	CU FT	CU FT	CU FT	AC-FT	PERCENT
0.2 hr	1.12	Α	0.0	0.0000	0.80	2.20	0.000	0	0	0	0	0.00
1 hr -	2.23	В	0.0	0.0000	1.08	2.92	0.000	0	0	0	0	0.00
6 hr	2.90	C	13,608.0	0.3124	1.46	3.73	1.165	1,656	1,656	1,656	1,656	4.25
1 day	3.65	D	306,469.0	7.0356	2.64	5.25	36.937	67,423	86,577	113,394	145,317	95.75
4 day	4.70	TOTAL	320,077.0	7.3480	2.590	5.185	38.102	69,079	88,233	115,049	146,973	100,00
10 day	5.95		SQ MI>	0.0114812	^ Average	per acre ^	AC-FT>	1.5858	2.0256	2.6412	3.3740	<ac-ft< td=""></ac-ft<>
Apx. hydgro	graph	Tconc	0.20	T Base	0.81	T Peak	0.19	² eak Lasts	0.24	Hydrograph	Qavg, cfs	20.05

ONLY STEEP LANDSCAPING AT N. SIDE OF BUILDING AS TREATMENT "C"

DIATE OFF	of tolling	7007 W II	101/11/14.0	inc or borr						_	أساح المستراب المسترار برياب بالرسوار	
Storm, yr	100	Zone	4		Qp6,cfs	37.01		VOL1D,cf	84,635		SF,total	320,077
		TRTMT	AREA	AREA	PX100-6	QP100-6	QP100-6	VOL6HR	VOL1D	VOL4D	VOL10D	TRTM
Rain,	inches	CLASS	SQ FT	ACRES	INCHES	CFS/AC	CFS	CU FT	CU FT	CU FT	AC-FT	PERCENT
0.2 hr	1.12	Α	0.0	0.0000	0.80	2.20	0.000	0	0	0	0	0.0
1 hr	2.23	В	26,323.0	0.6043	1.08	2.92	1.765	2,369	2,369	2,369	2,369	8.2
6 hr	2.90	С	4,476.0	0.1028	1.46	3.73	0.383	545	545	545	545	1.4
1 day	3.65	D	289,278.0	6.6409	2.64	5.25	34.865	63,641	81,721	107,033	137,166	90.3
4 day	4.70	TOTAL	320,077.0	7.3480	2.495	5.037	37.013	66,555	84,635	109,947	140,080	100.0
10 day	5.95		SQ MI>	0.0114812	^ Average	per acre ^	AC-FT>	1.5279	1.9429	2.5240	3.2158	<ac-ft< td=""></ac-ft<>
Apx. hydgro	ograph	Tconc	0.20	T Base	0.82	T Peak	0.20	² eak Lasts	0.23	Hydrograph	Qavg, cfs	19.32

PROJECT SITE, PROPOSED CONDITIONS

ALL LANDSCAPING AS TREATMENT "C"

Storm,yr	100	Zone	4		Qp6,cfs	37.50		VOL1D,cf	85,468		SF,total	320,077
	_	TRTMT	AREA	AREA	PX100-6	QP100-6	QP100-6	VOL6HR	VOL1D	VOL4D	VOL10D	TRTMT
Rain,	inches	CLASS	SQ FT	ACRES	INCHES	CFS/AC	CFS	CU FT	CUFT	CUFT	AC-FT	PERCENT
0.2 hr	1.12	Α	0.0	0.0000	0.80	2.20	0.000	0	0	0	0	0.00
1 hr	2.23	В	0.0	0.0000	1.08	2.92	0.000	0	0	0	0	0.00
6 hr	2.90	С	30,799.0	0.7070	1.46	3.73	2.637	3,747	3,747	3,747	3,747	9.62
1 day	3.65	D	289,278.0	6.6409	2.64	5.25	34.865	63,641	81,721	107,033	137,166	90.38
4 day	4.70	TOTAL	320,077.0	7.3480	2.526	5.104	37.502	67,388	85,468	110,780	140,913	100.00
10 day	5.95		SQ MI>	0.0114812	^ Average	per acre ^	AC-FT>	1.5470	1.9621	2.5432	3.2349	<ac-ft< td=""></ac-ft<>
px. hydgrograph Tconc 0.20		T Base	0.82	T Peak	0.20	² eak Lasts	0.23	Hydrograph	Qavg, cfs	19.56		

LANDSC APE AREA JUST NORTH OF BUILDING - "STEEP" LS AS TREATMENT "C":

100-YR RUNOFF DOES NOT LEAVE THIS AREA

Ctorm to		7000	4		One ofo	N 77	1	VOL1D,cf	1,363	ſ	SF,total	7,949
Storm,yr	100	Zone	4		Qp6,cfs	0.77						
		TRTMT	AREA	AREA	PX100-6	QP100-6	QP100-6	VOL6HR	VOL1D	VOL4D	VOL10D	TRTMT
Rain,	inches	CLASS	SQ FT	ACRES	INCHES	CFS/AC	CFS	CU FT	CU FT	CUFT	AC-FT	PERCENT
0.2 hr	1.12	Α	0.0	0.0000	0.80	2.20	0.000	0	0	0	0	0.0
1 hr	2.23	В	0	0.0000	1.08	2.92	0.000	0	0	0	0	0.0
6 hr	2.90	С	5,488.0	0.1260	1.46	3.73	0.470	668	668	668	668	69.0
1 day	3.65	D	2,461.0	0.0565	2.64	5.25	0.297	541	695	911	1,167	30.9
4 day	4.70	TOTAL	7,949.0	0.1825	1.825	4.201	0.767	1,209	1,363	1,578	1,835	100.0
10 day	5.95		SQ MI>	0.0002851	^ Average	per acre ^	AC-FT>	0.0278	0.0313	0.0362	0.0421	<ac-ft< td=""></ac-ft<>
Apx. hydgro				T Base	0.84	T Peak	0.25	⁵ eak Lasts	0.08	Hydrograph	Qavg, cfs	0.35

TO EXISTING DRAINAGE OPENING NEAR NW CORNER

Storm, yr	100	Zone	4		Qp6,cfs	4.33		VOL1D,cf	9,532	<u></u>	SF,total	38,257
	,	TRTMT	AREA	AREA	PX100-6	QP100-6	QP100-6	VOL6HR	VOL1D	VOL4D	VOL10D	TRTM
Rain,	inches	CLASS	SQ FT	ACRES	INCHES	CFS/AC	CFS	CU FT	CU FT	CU FT	AC-FT	PERCEN
0.2 hr	1.12	Α	0.0	0.0000	0.80	2.20	0.000	0	0	0	0	0.0
1 hr	2.23	В	0	0.0000	1.08	2.92	0.000	0	0	0	0	0.0
6 hr	2.90	C	7,934.0	0.1821	1.46	3.73	0.679	965	965	965	965	20.7
1 day	3.65	D	30,323.0	0.6961	2.64	5.25	3.655	6,671	8,566	11,220	14,378	79.2
4 day	4.70	TOTAL	38,257.0	0.8783	2.395	4.935	4.334	7,636	9,532	12,185	15,343	100.0
10 day	5.95		SQ MI>	0.0013723	^ Average	per acre ^	AC-FT>	0.1753	0.2188	0.2797	0.3522	<ac-ft< td=""></ac-ft<>
ox. hydarc	x. hydgrograph Tconc 0.20 TB			T Base	0.82	T Peak	0.21	⁵ eak Lasts	0.20	Hydrograph	Qavg, cfs	2.22



PROJECT: DEFINED FITNESS @ 3301 JUAN TABO NE, ALBUQUERQUE, NM

ALBUQUERQUE, NM (1/93) CRITERIA - D.P.M. SIMPLE PROCEDURE FOR <= 40 ACRES (ASSUMES Tc=0.2hr=12min)

BASIC TREATMENT CLASSES: A=UNDISTURBED, B=LAWNS, C=UNPAVED ROADS or STEEP B, D=ROOFS, PAVEMENT: SEE DPM 22.2 P A-5

PX100-6 = PRECIPITATION EXCESS FROM 100-YEAR 6-HOUR STORM; PX10-6 = SAME FOR 10-YEAR STORM

VOL10D = VOLUME OF RUNOFF FROM 10-DAY STORM; APPROXIMATE "6-HR" HYDRGRAPH PER DPM 22.2 SECTION A.8

PROJECT SITE, EXISTING CONDITIONS (BEFORE PART OF BUILDING DEMOLISHED)

NO LANDSCAPING NORTH OF BUILDING; LANDSCAPING AS TREATMENT "B"

	••••••							سندب جبري و خلافت	ومسبوب والمساور الماسات			بسنسجج والمار
Storm,yr	10	Zone	4		Qp6,cfs	25.57		VOL1D,cf	56,452		SF,total	320,077
		TRTMT	AREA	AREA	PX10-6	QP10-6	QP10-6	VOL6HR	VOL1D	VOL4D	VOL10D	TRTMT
Rain,	inches	CLASS	SQ FT	ACRES	INCHES	CFS/AC	CFS	CU FT	CU FT	CU FT	AC-FT	PERCENT
0.2 hr	0.75	Α	0.0	0.0000	0.28	0.87	0.000	0	0	0	0	0.00
1 hr	1.49	В	13,608.0	0.3124	0.46	1.45	0.453	522	522	522	522	4.25
6 hr	1.93	C	0.0	0.0000	0.73	2.26	0.000	0	0	0	0	0.00
1 day	2.43	D	306,469.0	7.0356	1.69	3.57	25.117	43,161	55,931	73,808	95,261	95.75
4 day	3.13	TOTAL	320,077.0	7.3480	1.638	3.480	25.570	43,683	56,452	74,330	95,782	100.00
10 day	3.97		SQ MI>	0.0114812	^ Average	per acre^	AC-FT>	1.0028	1.2960	1.7064	2.1989	<ac-ft< td=""></ac-ft<>
Apx. hydgro	ox. hydgrograph Tconc 0.20		T Base	0.75	T Peak	0.19	Peak Lasts 0.24		Hydrograph	12.68		

PROJECT SITE, EXISTING CONDITIONS (BEFORE PART OF BUILDING DEMOLISHED)

NO LANDSCAPING NORTH OF BUILDING; ALL LANDSCAPING AS TREATMENT "C"

Storm,yr	10	Zone	4		Qp6,cfs	25.82		VOL1D,cf	56,758		SF,total	320,077
		TRTMT	AREA	AREA	PX10-6	QP10-6	QP10-6	VOL6HR	VOL1D	VOL4D	VOL10D	TRTMT
Rain,	inches	CLASS	SQ FT	ACRES	INCHES	CFS/AC	CFS	CU FT	CU FT	CU FT	AC-FT	PERCENT
0.2 hr	0.75	Α	0.0	0.0000	0.28	0.87	0.000	0	0	0	0	0.00
1 hr	1.49	В	0.0	0.0000	0.46	1.45	0.000	0	0	0	0	0.00
6 hr	1.93	C	13,608.0	0.3124	0.73	2.26	0.706	828	828	828	828	4.25
1 day	2.43	D	306,469.0	7.0356	1.69	3.57	25.117	43,161	55,931	73,808	95,261	95.75
4 day	3.13	TOTAL	320,077.0	7.3480	1.649	3.514	25.823	43,989	56,758	74,636	96,089	100.00
10 day	3.97		SQ MI>	0.0114812	^ Average	per acre ^	AC-FT>	1.0098	1.3030	1.7134	2.2059	<ac-ft< td=""></ac-ft<>
Apx. hydgro	graph	Tconc	0.20	T Base	0.75	T Peak	0.19	Peak Lasts	0.24	Hydrograph	Qavg, cfs	12.77

PROJECT SITE, PROPOSED CONDITIONS

ONLY STEEP LANDSCAPING AT N. SIDE OF BUILDING AS TREATMENT "C"

	<u> </u>					· · · · · · · · · · · · · · · · · · ·	.	انتناك والمستقل المستوية	والمستحد المستحد والمراجع والمراجع	•		البرقابطنسبب وبرواكم
Storm,yr	10	Zone	4		Qp6,cfs	24.82		VOL1D,cf	54,075		SF,total	320,077
		TRTMT	AREA	AREA	PX10-6	QP10-6	QP10-6	VOL6HR	VOL1D	VOL4D	VOL10D	TRTMT
Rain,	inches	CLASS	SQ FT	ACRES	INCHES	CFS/AC	CFS	CU FT	CU FT	CU FT	AC-FT	PERCENT
0.2 hr	0.75	Α	0.0	0.0000	0.28	0.87	0.000	0	0	0	0	0.00
1 hr	1.49	В	26,323.0	0.6043	0.46	1.45	0.876	1,009	1,009	1,009	1,009	8.22
6 hr	1.93	С	4,476.0	0.1028	0.73	2.26	0.232	272	272	272	272	1.40
1 day	2.43	D	289,278.0	6.6409	1.69	3.57	23.708	40,740	52,793	69,668	89,917	90.38
4 day	3.13	TOTAL.	320,077.0	7.3480	1.575	3.377	24.816	42,021	54,075	70,949	91,199	100.00
10 day	3.97		SQ MI>	0.0114812	^ Average	per acre ^	AC-FT>	0.9647	1.2414	1.6288	2.0936	<ac-ft< td=""></ac-ft<>
Apx. hydgro	graph	Tconc	0.20	T Base	0.76	T Peak	0.20	Peak Lasts	0.23	Hydrograph	Qavg, cfs	12.20

PROJECT SITE, PROPOSED CONDITIONS

ALL LANDSCAPING AS TREATMENT "C"

Storm,yr	10	Zone	4		Qp6,cfs	25.31		VOL1D,cf	54,667		SF,total	320,077
_		TRTMT	AREA	AREA	PX10-6	QP10-6	QP10-6	VOL6HR	VOL1D	VOL4D	VOL10D	TRTMT
Rain,	inches	CLASS	SQ FT	ACRES	INCHES	CFS/AC	CFS	CU FT	CU FT	CU FT	AC-FT	PERCENT
0.2 hr	0.75	Α	0.0	0.0000	0.28	0.87	0.000	0	0	0	0	0.00
1 hr	1.49	В	0.0	0.0000	0.46	1.45	0.000	0	0	0	Đ	0.00
6 hr	1.93	С	30,799.0	0.7070	0.73	2.26	1.598	1,874	1,874	1,874	1,874	9.62
1 day	2.43	D	289,278.0	6.6409	1.69	3.57	23.708	40,740	52,793	69,668	89,917	90.38
4 day	3.13	TOTAL	320,077.0	7.3480	1.598	3.444	25.306	42,614	54,667	71,541	91,791	100.00
10 day	3.97		SQ MI>	0.0114812	^ Average	per acre ^	AC-FT>	0.9783	1.2550	1.6424	2.1072	<ac-ft< td=""></ac-ft<>
Apx. hydgro	graph	Tconc	0.20	T Base	0.75	T Peak	0.20	Peak Lasts	0.23	Hydrograph	Qavg, cfs	12.37

LANDSC APE AREA JUST NORTH OF BUILDING - "STEEP" LS AS TREATMENT "C":

100-YR RUNOFF DOES NOT LEAVE THIS AREA

100 111110							_	والمستخدمات والمستخدمات				
Storm,yr	10	Zone	4		Qp6,cfs	0.49		VOL1D,cf	783		SF,total	7,949
		TRTMT	AREA	AREA	PX10-6	QP10-6	QP10-6	VOL6HR	VOL1D	VOL4D	VOL10D	TRTMT
Rain,	inches	CLASS	SQ FT	ACRES	INCHES	CFS/AC	CFS	CU FT	CU FT	CU FT	AC-FT	PERCENT
0.2 hr	0.75	Α	0.0	0.0000	0.28	0.87	0.000	0	0	0	0	0.00
1 hr	1.49	В	0	0.0000	0.46	1.45	0.000	0	0	0	0	0.00
6 hr	1.93	C	5,488.0	0.1260	0.73	2.26	0.285	334	334	334	334	69.04
1 day	2.43	D	2,461.0	0.0565	1.69	3.57	0.202	347	449	593	765	30.96
4 day	3.13	TOTAL	7,949.0	0.1825	1.027	2.666	0.486	680	783	927	1,099	100.00
10 day	3.97		SQ MI>	0.0002851	^ Average	per acre^	AC-FT>	0.0156	0.0180	0.0213	0.0252	<ac-ft< td=""></ac-ft<>
Apx. hydgro	graph	Тсопс ().20	T Base	0.73	T Peak	0.25	Peak Lasts	0.08	Hydrograph	Qavg, cfs	0.20

TO EXISTING DRAINAGE OPENING NEAR NW CORNER

Storm,yr	10	Zone	4		Qp6,cfs	2.90		VOL1D,cf	6,017		SF,total	38,257
		TRTMT	AREA	AREA	PX10-6	QP10-6	QP10-6	VOL6HR	VOL1D	VOL4D	VOL10D	TRTMT
Rain,	inches	CLASS	SQ FT	ACRES	INCHES	CFS/AC	CFS	CU FT	CU FT	CU FT	AC-FT	PERCENT
0.2 hr	0.75	Α	0.0	0.0000	0.28	0.87	0.000	0	0	0	0	0.00
1 hr	1.49	В	0	0.0000	0.46	1.45	0.000	0	0	0	0	0.00
6 hr	1.93	С	7,934.0	0.1821	0.73	2.26	0.412	483	483	483	483	20.74
1 day	2.43	D	30,323.0	0.6961	1.69	3.57	2.485	4,270	5,534	7,303	9,425	79.26
4 day	3.13	TOTAL	38,257.0	0.8783	1.491	3.298	2.897	4,753	6,017	7,785	9,908	100.00
10 day	3.97	···	SQ MI>	0.0013723	^ Average	per acre^	AC-FT>	0.1091	0.1381	0.1787	0.2275	<ac-ft< td=""></ac-ft<>
Apx. hydgro		Toonc	0.20	T Base	0.75	T Peak	0.21	² eak Lasts	0.20	Hydrograph	Qavg, cfs	1.38

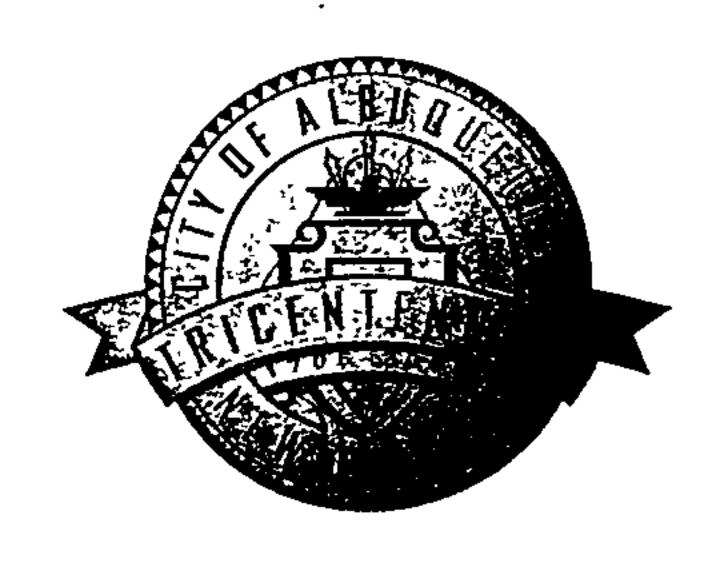
10

Defined Fitness, 3301 Juan Tabo NE Albuquerque NM Landscape area north of new construction Average end area method is most appropriate this long, narrow location

"CONE" FF	RUSTUM / GEO	METRIC MEAN	AREA		A	V END ARE	AVERAGE		
ELEV FT	AREA	DVOL CONE	VOLFT^3	VOL AC-FT	DVOL AV AREV	OLFT^3	VOL AC-FT	VOLFT^3	VOL AC-FT
24.2	2 2,596	466	2154	0.049444	467	2257	0.051816	2205	0.050630
2	4 2,072	1391	1688	0.038750	1440	1790	0.041100	1739	0.039925
2	3 807	297	297	0.006824	351	351	0.008053	324	0.007439
22.2	2 70)	0	0.000000		0	0.00000	0	0.000000
	0-day vol = 18	835cf				V ENID ADE			

"CONE" FRUST	UM/GEO	METRIC MEAN		AV END AREA				
ELEV FT ARE	EA	DVOL CONE	VOLFT^3	VOL AC-FT	DVOL AV AREVOL	.FT^3	VOL AC-FT	
24.019	2,596	44	1732	0.039766	44	1835	0.042118	
24	2,072	1391	1688	0.038750	1440	1790	0.041100	
23	807	297	297	0.006824	351	351	0.008053	
22.2	70		0	0.000000		0	0.000000	

CITY OF ALBUQUERQUE



December 24, 2007

Tucker Green, PE
Per Se Engineering
2116 Lead Ave SE
Albuquerque, NM 87106

Re: Sandia Plaza – Defined Fitness Grading and Drainage Plan

Engineer's Stamp dated 12-23-07 (G21/D38)

Dear Mr. Green,

P.O. Box 1293 Pased upon the information provided in your submittal dated 12-24-07, the above referenced plan is approved for Building Permit. Please attach a copy of this approved plan to the construction sets prior to sign-off by Hydrology.

Also, prior to Certificate of Occupancy release, Engineer Certification of the grading plan per the DPM checklist will be required.

If you have any questions, you can contact me at 924-3986

New Mexico 87103

www.cabq.gov

Albuquerque

Kin 11/2

Sincerely,

Bradley L. Bingham, PE

Principal Engineer, Planning Dept Development and Building Services

C: file

DRAINAGE AND TRANSPORTATION INFORMATION SHEET (Rev. 12/05)

PROJECT TITLE: SANDIA PLAZA IMPRODE DRB#: EPC#: 1000074 (PREME	WORK ORDER#:
LEGAL DESCRIPTION: THACH H-3-A	LA RANA DE LOS ALFOS, UNITZ N.E.
ENGINEERING FIRM: PER SE GUS INEC ADDRESS: 216 LEAD AUC. THE CFTY, STATE: 445UD. NA	ence contact tucker Green
OWNER: SANDIA PLAZA PARTNES LLC ADDRESS: SESO ENBANK N.E SUI CITY, STATE: ALBUQ NM. P7///	
ARCHITECT: GARLAN BRUM ARCHITECT: ADDRESS: 2403 SAN MALOO E CITY, STATE: SUITE W-1 AUBUD.	
SURVEYOR: HARRIS CWYLLING INC ADDRESS: 24/2 MONROE & A CITY, STATE: 41500. NM.	PHONE:
CONTRACTOR: BLANCHARD COUSE. CO. ADDRESS: 5859 EURA-14 BLYO CITY, STATE: MUDICA. NAA	SE SL. B42HONE: 803-7169 ZIP CODE: 877111
TYPE OF SUBMITTAL: DRAINAGE REPORT DRAINAGE PLAN 12 SUBMITTAL DRAINAGE PLAN RESUBMITTAL CONCEPTUAL G & D PLAN GRADING PLAN EROSION CONTROL PLAN ENGINEER'S CERT (HYDROLOGY) CLOMR/LOMR TRAFFIC CIRCULATION LAYOUT ENGINEER/ARCHITECT CERT (TCL) ENGINEER/ARCHITECT CERT (DRB S.P.)	CHECK TYPE OF APPROVAL SOUGHT: SIA/FINANCIAL GUARANTEE RELEASE PRELIMINARY PLAT APPROVAL S. DEV. PLAN FOR SUB'D APPROVAL S. DEV. FOR BLDG. PERMIT APPROVAL SECTOR PLAN APPROVAL FINAL PLAT APPROVAL FOUNDATION PERMIT APPROVAL BUILDING PERMIT APPROVAL CERTIFICATE OF OCCUPANCY (PERM) GRADING PERMIT APPROVAL
ENGINEER/ARCHITECT CERT (AA) OTHER (SPECIFY) WAS A PRE-DESIGN CONFERENCE ATTENDED: YES	WORK ORDER APPROVAL OTHER (SPECIFY) DEC 2 4 2007
NOCOPY PROVIDED	HYDROLOGY SECTION
Requests for approvals of Site Development Plans and/or Sub- particular nature, location and scope to the proposed developm levels of submittal may be required based on the following:	DATE: division Plats shall be accompanied by a drainage submittal. The cent define the degree of drainage detail. One or more of the following
•	for approval of Site Development Plans greater than five (5) acres and
	ng permits, paving permits and site plans less than five (5) acres.
3. Drainage Report: Required for subdivision containin	g more than ten (10) lots or constituting five (5) acres or more.

PROJECT: Defined Fitness, 33301 Juan Tabo

ALBUQUERQUE, NM (1/93) CRITERIA - D.P.M. SIMPLE PROCEDURE FOR <= 40 ACRES (ASSUMES Tc=0.2hr =12min)

BASIC TREATMENT CLASSES: A=UNDISTURBED, B=LAWNS, C=UNPAVED ROADS or STEEP B, D=ROOFS, PAVEMENT: SEE DPM 22.2 P A-5

PX100-6 = PRECIPITATION EXCESS FROM 100-YEAR 6-HOUR STORM; PX10-6 = SAME FOR 10-YEAR STORM

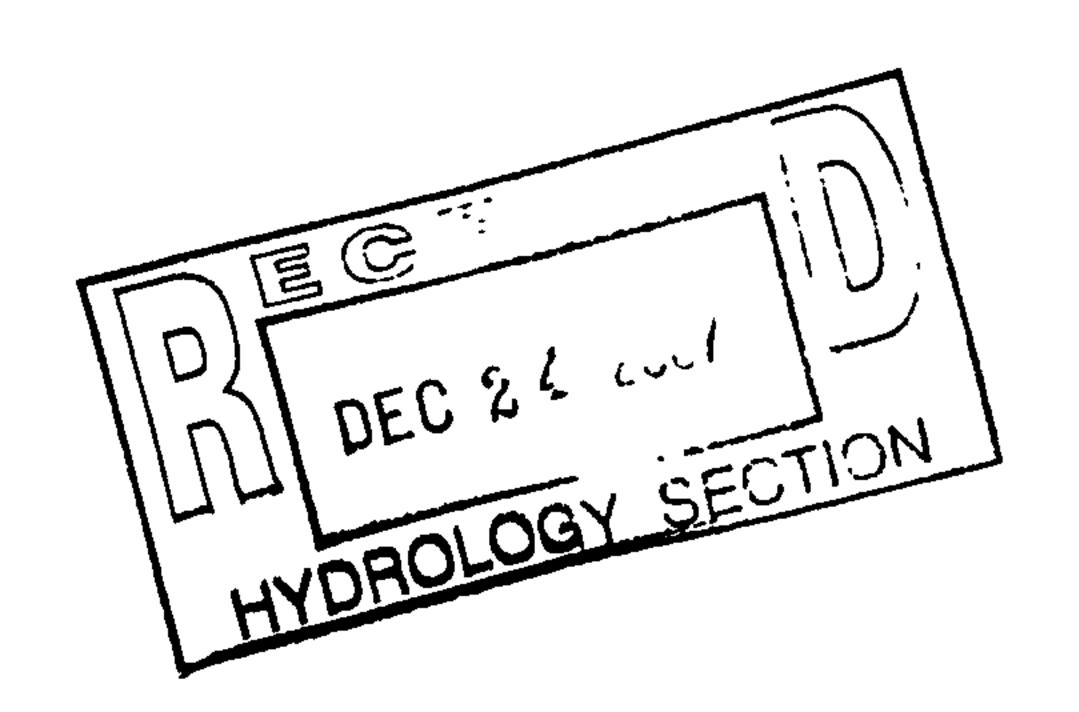
VOL10D = VOLUME OF RUNOFF FROM 10-DAY STORM; APPROXIMATE "6-HR" HYDRGRAPH PER DPM 22.2 SECTION A.8

City of Albuquerque approximation for Santa Fe

Landscape area north of inside swimming pool

Basic Area: Steep xeric LS on filter fabric as "B"

			TIREOT IGNITE			<u> </u>					·	
Storm,yr	100	Zone	4		Qp6,cfs	0.72		VOL1D,cf	1,312		SF,total	7,271
	·	TRTMT	AREA	AREA	PX100-6	QP100-6	QP100-6	VOL6HR	VOL1D	VOL4D	VOL10D	TRTMT
Rain,	inches	CLASS	SQ FT	ACRES	INCHES	CFS/AC	CFS	CU FT	CU FT	CU FT	AC-FT	PERCENT
0.2 hr	1.12	Α	0.0	0.0000	0.80	2.20	0.000	0	0	0	0	0.00
1 hr	2.23	В	0.0	0.0000	1.08	2.92	0.000	0	0	0	0	0.00
6 hr	2.90	С	4,615.0	0.1059	1.46	3.73	0.395	561	561	561	561	63.47
1 day	3.65	D	2,656.0	0.0610	2.64	5.25	0.320	584	750	983	1,259	36.53
4 day	4.70	TOTAL	7,271.0	0.1669	1.891	4.285	0.715	1,146	1,312	1,544	1,821	100.00
10 day	5.95		SQ MI>	0.0002608	^ Average	per acre ^	AC-FT>	0.0263	0.0301	0.0355	0.0418	<ac-ft< td=""></ac-ft<>
Apx. hydgro	graph	Tconc	0.20	T Base	0.84	T Peak	0.24	^{>} eak Lasts	0.09	Hydrograph	Qavg, cfs	0.33





CITY OF ALBUQUERQUE



January 4, 2008

Garlan Bryan, R.A. 2403 San Mateo Blvd. NE W-1 Albuquerque, NM 87110

Re: Sandia Plaza Improvements, 3301 Juan Tabo Blvd NE, Traffic Circulation Layout Architect's Stamp dated 12-10-07 (G21-D038)

Dear Mr. Bryan,

File

The above referenced plan is not approved through the TCL process. Due to the existing zoning, this project is site plan controlled and will need to be submitted through the DRB. If this plan has already been approved through the DRB process, be sure to include it in the plan set when applying for building permit.

If you have any questions, you can contact me at 924-3991.

Albuquerque

P.O. Box 1293

New Mexico 87103

www.cabq.gov

Kristal D. Metro, P.E.

Sincerely,

Traffic Engineer, Planning Dept.

Development and Building Services

DRAINAGE AND TRANS	(Rev. 12/05) (Rev. 12/05)
PROJECT TITLE: SANDIA PLAZA IMP	PROPERTY GENERAL SZONE MAP/DRG. FILB # 6-21-2
DRB#: PPC#:	WORK ORDER#:
LEGAL DESCRIPTION: TRACK H-3-4	LA RANA DE LOS ACKOS UNITEZ
CITY ADDRESS: 330/ UAN LABO	·NE
	ecente, contact: tucker Gree-
ADDRESS: 2116 Lead Ave. N	56 PHONE: 232-9394
CFTY, STATE: A-4518 NM	ZIP CODE: 87/06
CA. 10 N. A A 1.10 C. 11.1	· · · · · · · · · · · · · · · · · · ·
OWNER: SAUDIA PLAZA PHATNES LLC ADDRESS: 5850 EMBANK M.E. SI	CONTACT: PAUL BLANKHARD ULL B-62 PHONE: 225-0000
CITY, STATE: ALBUQ NM. P7//	
CITAL STATE: WASHINGTON	
ARCHITECT: GARLAN BOUND AGE	helect. CONTACT: ROB RAYNE
ADDRESS: 2403 SAN MALOS	BLUD ME PHONE: 184-9694
CITY, STATE: SuitE W-/ ALBUZ	ZIP CODE: 1/2//D
URVEYOR: HARRIS SOLVEHILLA 14	C. CONTACT: TOMY HARRES.
ADDRESS: 24/2 MONROE &.	CONTACT: TOMY MARRIS PHONE: 889-8056
ADDRESS: 24/2 MONROE &. CITY, STATE: 41600. NM.	ZIP CODE: 87/10
CONTRACTOR: BLANCHARD CONST. CO ADDRESS: \$859 EURA-14 BLYC	. CONTACT: WAYDCLL HALL
ADDRESS: 8859 Eug A-14 B410	203-7169
CITY, STATE: MLOVA. NA	ZIP CODE: \\ \frac{2}{2}\land{1}\land{1}
TYPE OF SUBMITTAL:	CHECK TYPE OF APPROVAL SOUGHT:
DRAINAGE REPORT	SIA/FINANCIAL GUARANTEE RELEASE
DRAINAGE PLAN IN SUBMITTAL	- PRELIMINARY PLAT APPROVAL
DRAINAGE PLAN RESUBMITTAL	S. DEV. PLAN FOR SUB'D APPROVAL
CONCEPTUAL G & D PLAN GRADING PLAN	S. DEV. FOR BLDG. PERMIT APPROVAL
EROSION CONTROL PLAN	SECTOR PLAN APPROVAL
ENGINEER'S CERT (HYDROLOGY)	FINAL PLAT APPROVAL FOUNDATION PERMIT APPROVAL
CLOMR/LOMR	BUILDING PERMIT APPROVAL
TRAFFIC CIRCULATION LAYOUT	CERTIFICATE OF OCCUPANCY (PERM)
ENGINEER/ARCHITECT CERT (TCL)	CERTIFICATE OF OCCUPANCY (TEMP)
ENGINEER/ARCHITECT CERT (DRB S.P.)	GRADING DROMT ADDROVAT
ENGINEER/ARCHITECT CERT (AA)	
OTHER (SPECIFY)	WORK ORDER APPROVAL
	OTHER (SPECIFY) D 2007
•	II DEC XO LOUI
VAS A PRE-DESIGN CONFERENCE ATTENDED:	11:11
YES	1 HYDROLOGY SECTIC
XNO	1 HYDRULL.
COPY PROVIDED	
UBMITTED BY: POGERT RAMIER	
COMMITTED BI: KOREKE KAMPENDE	DATE: 12/10/27-
A Lemment Fint emperovale of Cita Themanous Ween and In-G	abdivision Plats shall be accompanied by a drainage submittal. The
raticular nature. Josephen any evene to the accesses general. Administration the military of the transferrest tempolities of	noment define the decree of decine a descit the comment of a terminate supplied in the comment.
excis of submittal may be required based on the following:	pment define the degree of drainage detail. One or more of the following
AT SENTIMENT WEND OF THE HIS OF THE OWNER OF THE TOTAL SENTENCES.	
1. Conceptual Gradina and Brainson Plan. Recuire	d for appioval of Site Development Plans greater than five (5) acres and
Sector Plans.	—
	• • • • • • • • • • • • • • • • • • •

2. Drainage Plane: Required for building permits, grading permits, paving permits and site plans less than five (5) acres.

3. Drainage Report: Required for subdivision containing more than ten (10) lots or constituting five (5) acres or more,



City of Albuquerque

October 6, 1999

Ronald R. Bohannan, P.E. Tierra West, LLC 4421 McLeod NE Suite D Albuquerque, NM 87109

RE: SANDIA PLAZA SHOPPING CENTER (G21-D38). TIERRA WEST LETTER
TO CITY ENGINEER DATED SEPTEMBER 29, 1999 REGARDING JUNE 24, 1999
SUBMITTAL FOR BUILDING AND PAVING PERMITS.

Dear Mr. Bohannan:

Thank you for clarifying whether or not there are any existing drainage easements to discharge stormwater offsite. Evidently, there is no record of any complaints received by and/or problems reported to the City over the past 25 years..

This renovation does not appreciably affect either the pattern or the quantity of runoff. The main buildings were gutted for remodling and two new buildings added in former parking areas.

***The G&D Plan stamped May 14,1999 is approved for subject permits. No additional off-site property rights are required.

If I can be of further assistance, please feel free to contact me at 924-3984.

Sincerely,

John P. Murray,

Hydrology

c: WR

Ties

Tierra West, LLC

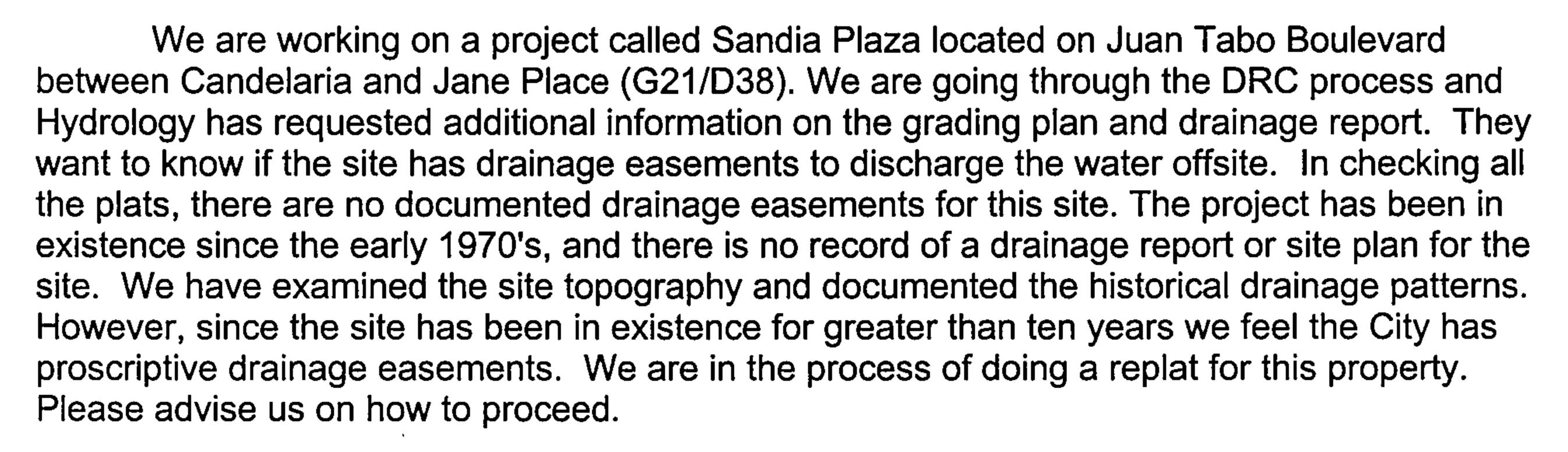
September 29, 1999

Mr. Fred Aguirre City Engineer City of Albuquerque P.O. Box 1293 Albuquerque, NM 87103

Re:

Sandia Plaza

Dear Mr. Aguirre:



If you have any questions or need additional information, please contact me or Sara Lavy.

Sincerely,

Ronald R. Bohannan, P.E.

CC:

Paul Blanchard

JN: 980004

SCI

980004: 9804-aguirre-easement.ltr

CITY ENGINEER



City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

April 20, 2000

Ronald R. Bohannan, P.E. Tierra West, LLC 4421 McLeod NE Suite D Albuquerque, NM 87109

RE: SANDIA PLAZA SHOPPING CENTER (G21-D38). GRADING AND DRAINAGE PLAN FOR SITE DEVELOPMENT PLAN FOR SUBDIVISION AND BUILDING PERMIT APPROVALS, AND FOR BUILDING AND PAVING PERMIT APPROVALS. ENGINEER'S STAMP DATED MARCH 3, 2000.

Dear Mr. Bohannan:

Based on the infornation provided on your April 6, 2000 submittal, the above referenced project is approved for Site Development Plan for Subdivision, Site Development Plan for Building Permit, Building Permit, and Paving Permit.

Please attach a copy of this approved plan to the construction sets prior to sign-off by Hydrology.

Prior to Certificate of Occupancy approval, an Engineer's Certification per the DPM will be required.

If I can be of further assistance, please feel free to contact me at 924-3984.

Sincerely,

John P. Murray, P.E.

Hydrology

c:

Whitney Reierson

DRAINAGE INFORMATION SHEET

PROJECT TITLE:	Sandia Plaza Shopping Center	ZONE ATLAS/DRNG	3. FILE #: <u>G-21/D38</u>
DRB #:	EPC #:	WORK ORDER #:	
LEGAL DESCRIPTI	ON: Tract H3A of La Reina	a de los Altos Unit 2	
CITY ADDRESS:	North of Candelaria and wes	t of Juan Tabo Boulevard	
ENGINEERING FIR	M: TIERRA WEST, LLC	CONTACT:	RONALD R. BOHANNAN
ADDRESS:	8509 Jefferson NE, Alb, NM 8711	PHONE:	(505) 858-3100
OWNER:		CONTACT:	
ADDRESS:		PHONE:	
ARCHITECT:		CONTACT:	,
ADDRESS:		PHONE:	
SURVEYOR:	Precision Surveys	CONTACT:	Larry Medrano
ADDRESS:	2929 Coors Blvd NW Suite 309	PHONE:	(505)839-0569
CONTRACTOR:		CONTACT:	
ADDRESS:		PHONE:	
DRAIN CONCI	AL: AGE REPORT AGE PLAN EPTUAL GRADING & DRAINAC	PRELIMI SE PLAN X S. DEV. F	PROVAL SOUGHT: PLAN APPROVAL NARY PLAT APPROVAL PLAN FOR SUB'D. APPROVAL PLAN FOR BLDG. PERMIT APPROVAL
EROSI	ON CONTROL PLAN	SECTOR	PLAN APPROVAL
ENGIN OTHER	EER'S CERTIFICATION	FOUNDA	LAT APPROVAL ATION PERMIT APPROVAL G PERMIT APPROVAL
PRE-DESIGN MEET	ring:		CATE OF OCCUPANCY APPROVAL G PERMIT APPROVAL
X NO			PERMIT APPROVAL DRAINAGE REPORT
COPY	PROVIDED	OTHER D	GE REQUIREMENTS 国
DA'	TE SUBMITTED:	04/05/00	DROLOGY SECTION

BY: Sara Lavy

8509 Jefferson NE Albuquerque, NM 87113 (505) 858-3100 fax (505) 858-1118 e-mail: twdms@aol.com 1-800-245-3102

April 5, 2000

Mr. John P. Murray Hydrology Department City of Albuquerque P.O. Box 1293 Albuquerque, NM 87103

Re: Sandia Plaza Shopping Center (G21-D38)

Dear Mr. Murray:

We are resubmitting the Grading and Drainage Plan because of minor changes to the site plan. The Grading Plan was previously approved on May 14, 1999. EPC requested the changes and they do not affect any of the grades or the drainage patterns. The changes consist of added pedestrian connections and landscaping.

If you have any questions regarding this matter, please do not hesitate to call me.

Sincerely,

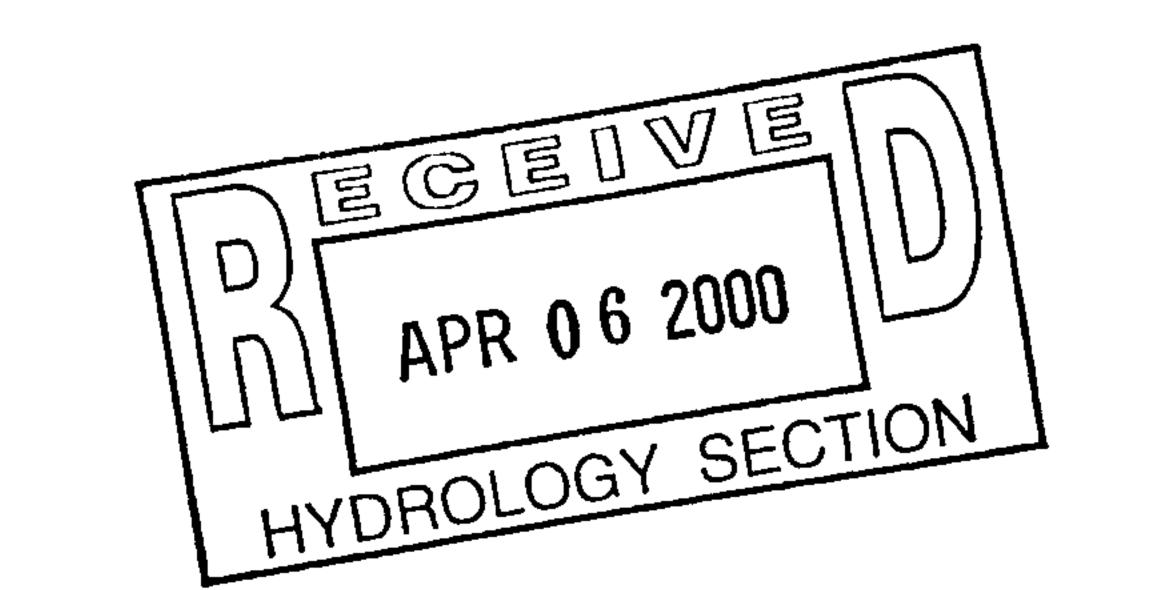
Sara Lavy

cc: Paul Blanchard

JN: 980004

scl

980004: 9804-hyd-resubmittal 4-5-00.kr





March 3, 1998

Ronald R. Bohannan, P.E. Tierra West, LLC 4421 McLeod NE Suite D Albuquerque, NM 87109

RE: SANDIA PLAZA SHOPPING CENTER (G21-D38). DRAINAGE REPORT FOR SITE DEVELOPMENT PLAN FOR BUILDING PERMIT, BUILDING PERMIT, AND GRADING PERMIT APPROVALS. ENGINEER'S STAMP DATED JANUARY 29, 1998.

Dear Mr. Bohannan:

Based on the information provided on your February 9, 1998 submittal, the above referenced project is approved for Site Development for Building Permit and Grading Permit.

Prior to Building Permit approval, please address the following comments:

- 1. Is there a retention pond in Basin 2? Your report seems to indicate that retention is taking place. Per the City Drainage Ordinance, retention is not allowed.
- 2. Provide finish floor elevations for the existing buildings in the shopping center.

If I can be of further assistance, please feel free to contact me at 924-3984.

Sincerely,

Asa Ann Manwill, P.E.

Hydrology

c: Andrew Garcia

File

DRAINAGE INFORMATION SHEET

PROJECT TITLE	: Sandia Plaza Shopping Center	ZONE ATLAS/DRN	G. FILE #: <u>G-21</u>
DRB #:	EPC #:	WORK ORDER #:	
LEGAL DESCRIP	Tion: Tract H3A of La Reina de los Alto	s Unit 2	
CITY ADDRESS:	North of Candelaria and west of Juan T	abo Boulevard	
ENGINEERING FI	RM: TIERRA WEST, LLC	_ CONTACT:	RONALD R. BOHANNAN
ADDRESS:	4421 McCleod Rd. NE Suite D, 87109	_ PHONE:	(505) 883-7592
OWNER:		_ CONTACT:	
ADDRESS:		PHONE:	
ARCHITECT:	de la Torre Architects	_ CONTACT:	Jorge de la Torre
ADDRESS:	7801 Academy NE Building 2 Suite 200	_ PHONE:	(505)828-9611
SURVEYOR:	Precision Surveys	_ CONTACT:	Larry Medrano
ADDRESS:	2929 Coors Blvd NW Suite 309	PHONE:	(505)839-0569
CONTRACTOR:	ONTRACTOR:		
ADDRESS:		_ PHONE:	
DRAII CONC X GRAI EROS ENGII OTHE YES X NO	NAGE REPORT NAGE PLAN CEPTUAL GRADING & DRAINAGE PLAN DING PLAN SION CONTROL PLAN NEER'S CERTIFICATION ER	PRELIMI S. DEV. X S. DEV. SECTOR FINAL P FOUNDA X BUILDIN CERTIFIC X PAVING S. A. D. I	PPROVAL SOUGHT: I PLAN APPROVAL INARY PLAT APPROVAL PLAN FOR SUB'D. APPROVAL PLAN FOR BLDG. PERMIT APPROVAL R PLAN APPROVAL LAT APPROVAL ATION PERMIT APPROVAL G PERMIT APPROVAL CATE OF OCCUPANCY APPROVAL G PERMIT APPROVAL PERMIT APPROVAL PERMIT APPROVAL DRAINAGE REPORT GE REQUIREMENTS
D	ATE SUBMITTED: 02/09/98 BY: Sara McCollam	OTHER	国の同じ図画 FEB 0 9 1998 HYDROLOGY SECTION

DRAINAGE REPORT

for

Sandia Plaza Shopping Center

Prepared by

Tierra West, LLC 4421 McLeod Road NE, Suite D Albuquerque, New Mexico 87109

Prepared for

de la Torre Architects
7801 Academy Rd NE Building 2 Suite 200
Albuquerque, New Mexico 87109

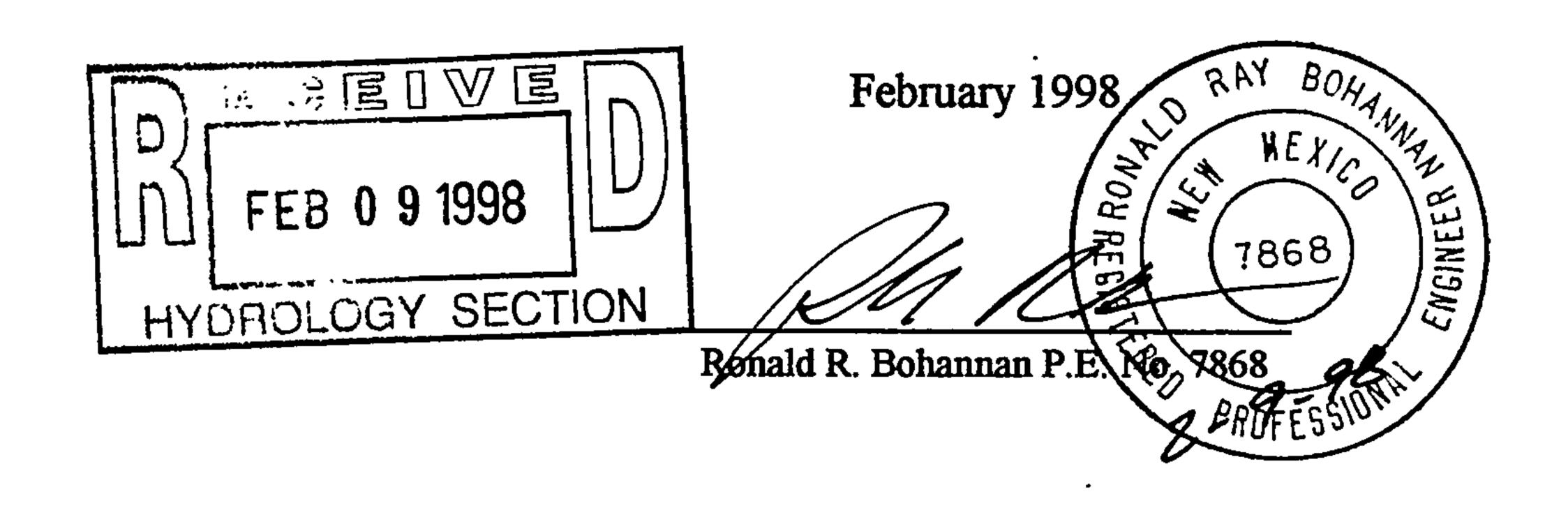
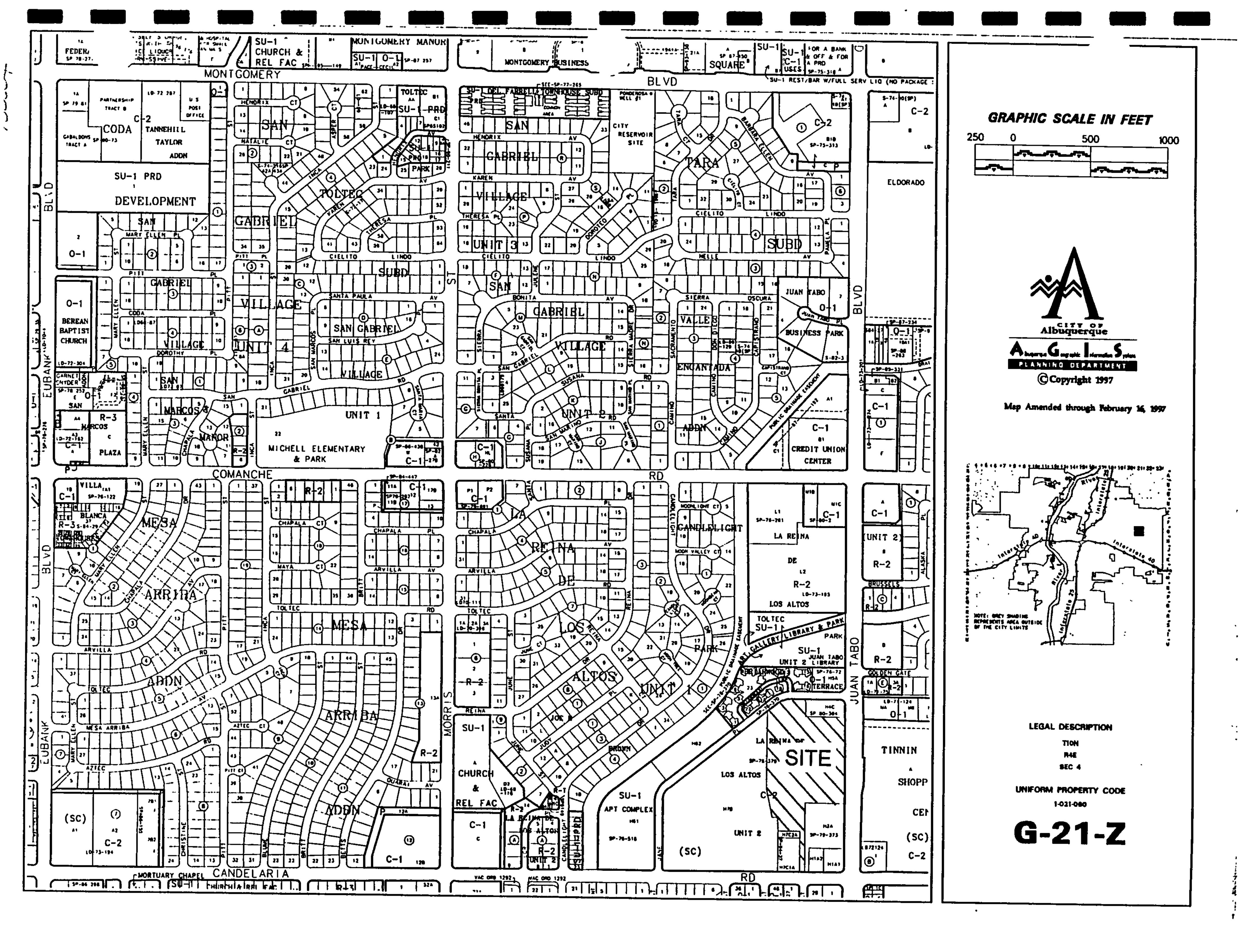


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Zone Atlas Map G-21
Location
Existing Drainage Conditions
Basin Layout
FIRM Map and Soil Conditions 4
Proposed On-Site Drainage Management Plan
FIRM Map 35001C0357 D 5
Runoff Calculations
Drainage Facilities Analysis
AHYMO Runoff Input and Summary Output for Developed Drainage Basins



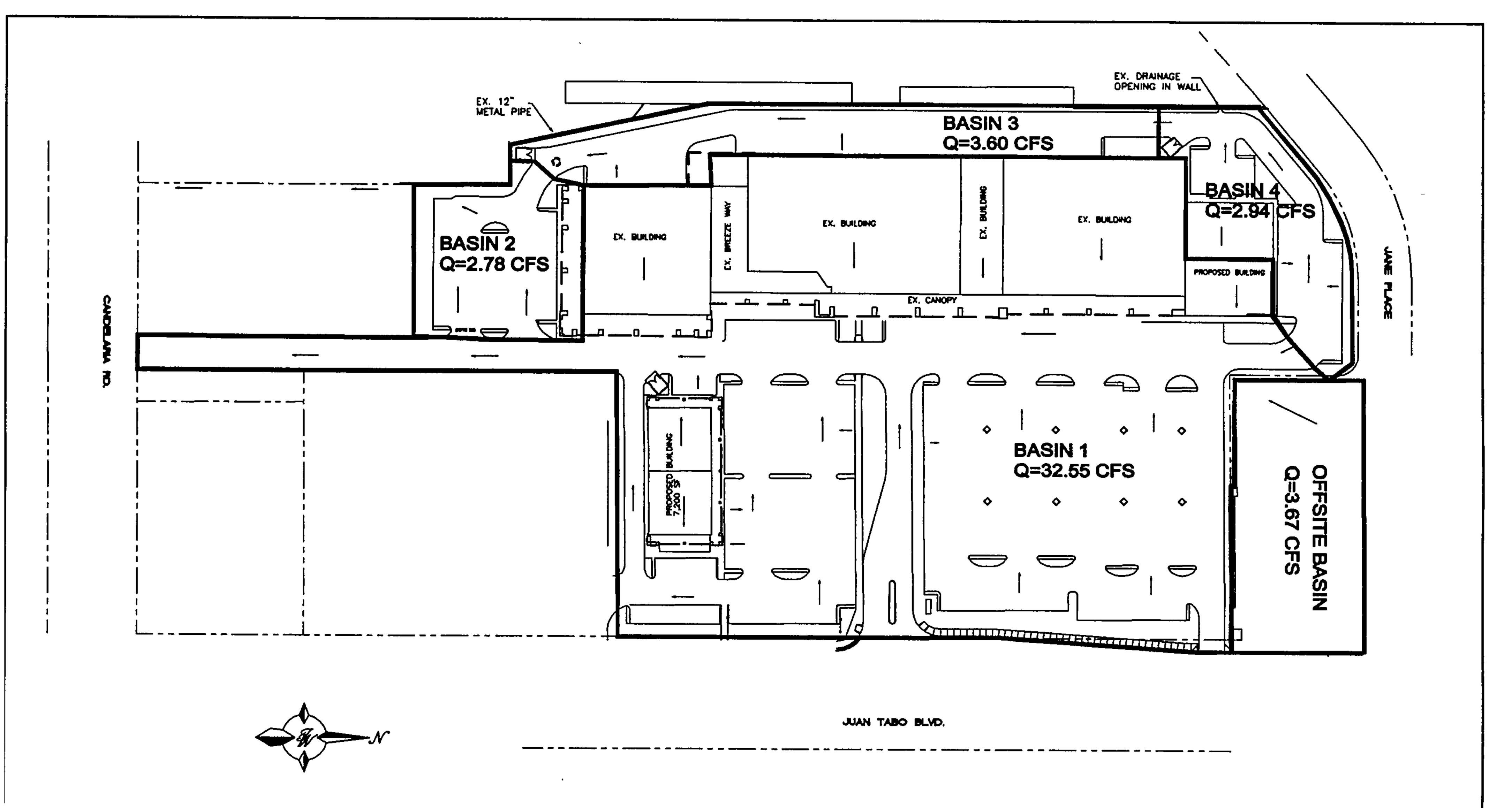
Location

Sandia Plaza Shopping Center is an existing site that is located on Juan Tabo
Boulevard near Candelaria Road. The site is shown on the attached Zone Atlas Map G-21
and contains approximately 8.36 acres. The site legal description is Tract H3A of La Reina de los Altos Unit 2.

The site has been developed for a number of years, and currently there is a pawn shop, a hairdresser, a restaurant and a vacant building on the site. The current owner is preparing to update the shopping center by adding additional parking spaces and landscaping improvements in the parking lot. Other proposed improvements include added a 8,700 square foot proposed building on the north side of the shopping center and a new 7,200 square foot pad site on the southeast side of the site. Parking lot islands, tree planters, and an entrance driveway are also proposed improvements for the site. The purpose of this report is to provide the drainage analysis and management plan for the shopping center.

Existing Drainage Conditions

The site has been developed continuously prior to 1972 and the existing conditions closely represent the developed conditions. Currently the site has four on-site basins. Basin 1 has a developed discharge rate of 32.55 cfs and free discharges to Candelaria Road. This is the largest basin and consists of the main portion of the site. A driveway to Candelaria acts as a drainage channel and has a capacity of 187.30 cfs. This is more than adequate for Basin 1 and the offsite basin. Basin 2, on the south side of the site, has a developed discharge rate of 2.78 cfs. This basin free discharges to a pond located on the south side of the site. The pond overflows the south curb and enters the adjacent lot. The adjoining lot provides a corridor for the flows behind the building, filled with large gravel for the flows to reach Candelaria Road. The flows enter a concrete rundown and are conveyed by existing sidewalk



BASIN LAYOUT

culverts to Candelaria Road. Basin 3 has a developed discharge rate of 3.60 cfs. This basin is located in the back of the existing buildings and free discharges to a 12" exposed metal pipe which runs south towards Candelaria Road. The developed runoff rate of 3.60 cfs is less than the pipe capacity of 4.57 cfs. The pipe daylights into an existing concrete rundown and sidewalk culverts convey the flows to Candelaria Road. Basin 4 is on the north side of the site with a developed discharge rate 2.94 cfs. This basin free discharges to a 2' curb cut in the back of the buildings and then into the public street of Jane Place.

There is an offsite drainage basin located to the north of the site, consisting of a Yamaha Dealership which free discharges 3.67 cfs to the site. This flow joins the flows from Basin 1 and discharges to Candelaria Road. There is an existing retaining wall on the west side of the site that prevents off-site drainage from entering the site. There is a water block on Juan Tabo to the east and existing developed sites on the south that keep any offsite drainage from entering the site from those directions. There is no offsite drainage entering the site from the any other direction.

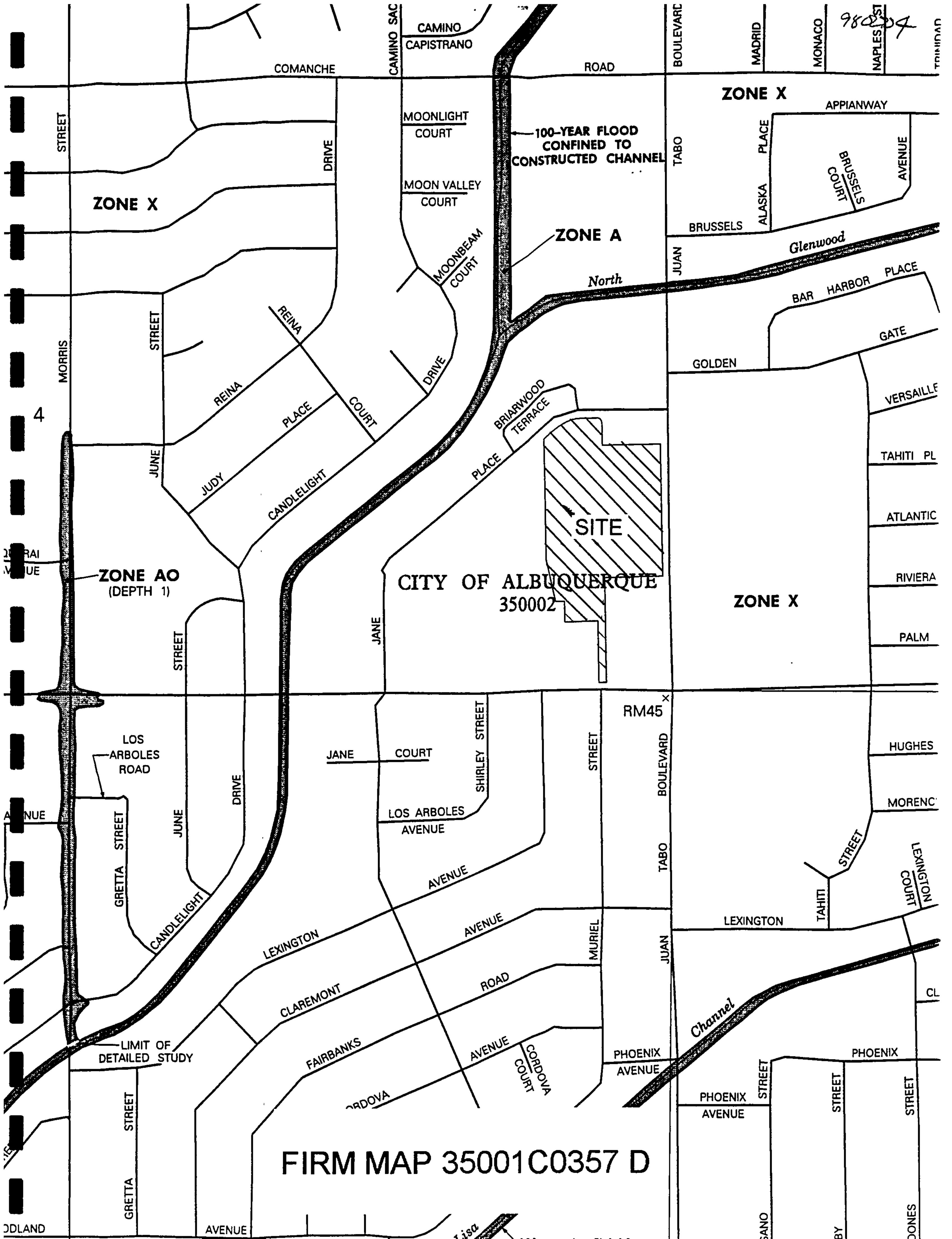
The driveway to Candelaria Road will act as an emergency overflow in the event of a storm larger than a 100 year storm.

FIRM Map and Soil Conditions

The site is located on FIRM Map 35001C0357 D as shown on the attached excerpt.

The map shows that the site does not lie within any 100 year flood plains.

According to the Soil Conservation Service Soil Survey of Bernalillo County, the site contains one soil type. The site consists of an Embudo-Tijeras complex which has a medium runoff and a moderate hazard of water erosion.



Proposed On-Site Drainage Management Plan

The proposed drainage management plan is to continue the existing drainage patterns. The site was developed prior to 1972 before the City of Albuquerque kept complete drainage records. Consequently, there is no drainage report on file for this site although it is already developed. The proposed improvements to the site will not change the existing drainage conditions. The site was previously impervious and the new improvements will not change the land condition. The current drainage solution performs adequately and there have been no complaints from the tenants on the site or the adjacent property owners.

Runoff
Calculations

RUNOFF CALCULATIONS

The site is @ Zone 4

LAND TREATMENT

Existing

B = 10%

D = 90%

DEPTH (INCHES) @ 100-YEAR STORM

 $P_{60} = 2.23$ inches

 $P_{360} = 2.90 \text{ inches}$

 $P_{1440} = 3.65 \text{ inches}$

DEPTH (INCHES) @ 10-YEAR STORM

 $P_{60} = 2.23 \times 0.667$ = 1.49 inches

 $P_{360} = 1.93$

 $P_{1440} = 2.43$

See the summary output from AHYMO calculations.

Drainage Basins

BASIN	AREA (SF)	AREA (AC)	AREA (MI ²)
1	281239.30	6.4564	0.010088
2	23863.75	0.5478	0.000856
3	30960.45	0.7108	0.001111
4	25224.83	0.5791	0.000905
5	31553.95	0.7244	0.001132
Total	392842.28	9.0184	0.014091

Runoff Calculation Results

BASIN	Q-100	Q-10	V-100	V-10
	CFS	CFS	AC-FT	AC-FT
1	32.55	21.32	1.337	0.837
2	2.78	1.82	0.133	0.071
3	3.60	2.36	0.147	0.092
4	2.94	1.92	0.120	0.075
5	3.67	2.40	0.150	0.094
Total	45.54	29.82	1.887	1.169

Drainage Facilities
Analysis

Curb Cut

Weir Equation:

$$Q=CLH^{3/2}$$

Basin 4:

Q=Flow (cfs) C=2.95

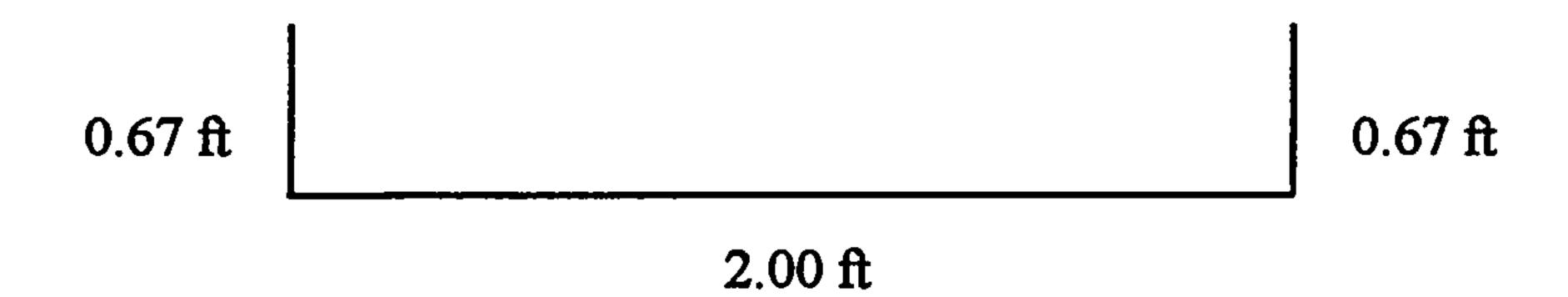
H = 0.67 ft

L = 2.0 ft

$$Q=2.95*2.0*0.67^{3/2}$$

$$Q = 3.24 \text{ cfs}$$

3.24 cfs > 3.13 cfs



$$Q = \frac{1.49}{n} A R^{2/3} S^{1/2}$$

$$Q = Flow (cfs)$$

$$n = 0.017$$

$$A = Area$$

$$S = Slope$$

$$A = 0.37 * 32 * 0.5 + 32 * 0.5 = 21.92$$

$$WP = 2*0.5+2*\sqrt{0.37^2+16^2} = 33.0$$

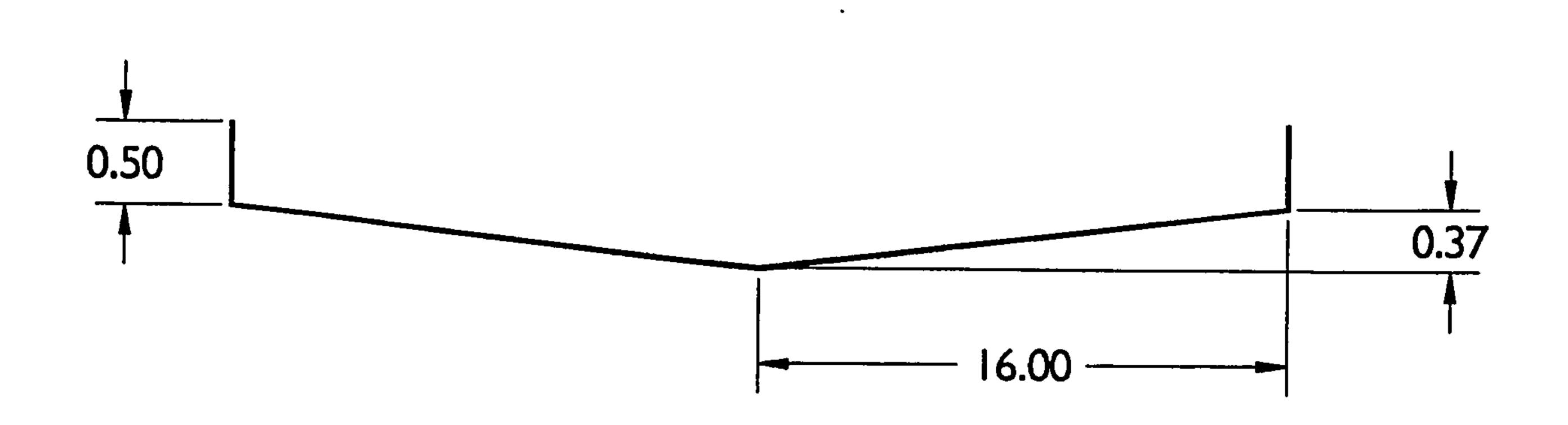
$$R = \frac{A}{WP} = \frac{21.92}{33.00} = 0.6642$$

Manning's Equation:

$$Q = \frac{1.49}{0.017} * 21.92 * 0.6642^{2/3} * 0.0164^{1/2}$$

$$Q = 187.30 \ cfs$$

Capacity of street = 187.30 cfs
Required capacity of street = 32.33 cfs
187.30 cfs > 32.33 cfs



Pipe Capacity

Manning's Equation: Q = 1.49/n * A * R^(2/3) * S^(1/2) A = Area

R = D/4

S = Slope

n = 0.013

Pipe	Đ	Slope	Area	R	Q Provided	Q Required	Velocity
	(in)	(%)	(ft^2)		(cfs)	(cfs)	(ft/s)
Metal Pipe	12	1.64	0.79	0.25	4.57	7.65	9.74

AHYMO
Runoff Input
and
Summary Output
for
Existing Drainage Basins

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阿斯斯斯斯
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SANDIA PLAZA SHOPPING CENTER
          100-YEAR, 6-HR STORM (UNDER DEVELOPED CONDITIONS)
  START
                     TIME=0.0
  * BASIN 1
                     TYPE=1 RAIN QUARTER=0.0 IN
  RAINFALL
                     RAIN ONE=2.23 IN RAIN SIX=2.90 IN
                     RAIN DAY=3.65 IN DT=0.02253 HR
                    ID=1 HYD NO=100.1 AREA=0.010088 SQ MI
  COMPUTE NM HYD
                     PER A=0.00 PER B=10.00 PER C=0.00 PER D=90.00
                     TP=-0.1333 HR MASS RAINFALL=-1
 PRINT HYD
                     ID=1 CODE=1
* BASIN 2
  COMPUTE NM HYD
                     ID=1 HYD NO=100.2 AREA=0.000856 SQ MI
                     PER A=0.00 PER B=10.00 PER C=0.00 PER D=90.00
                     TP=-0.1333 HR MASS RAINFALL=-1
 PRINT HYD
                     ID=1 CODE=1
 * BASIN 3
                    ID=1 HYD NO=100.3 AREA=0.001111 SQ MI
  COMPUTE NM HYD
                     PER A=0.00 PER B=10.00 PER C=0.00 PER D=90.00
                     TP=-0.1333 HR MASS RAINFALL=-1
  PRINT HYD
                     ID=1 CODE=1
  * BASIN 4
  COMPUTE NM HYD
                     ID=1 HYD NO=100.4 AREA=0.000905 SQ MI
                     PER A=0.00 PER B=10.00 PER C=0.00 PER D=90.00
                     TP=-0.1333 HR MASS RAINFALL=-1
  PRINT HYD
                     ID=1 CODE=1
  * BASIN 5
  COMPUTE NM HYD
                    ID=1 HYD NO=100.5 AREA=0.001132 SQ MI
                     PER A=0.00 PER B=10.00 PER C=0.00 PER D=90.00
                     TP=-0.1333 HR MASS RAINFALL=-1
  PRINT HYD
                     ID=1 CODE=1
      **********************
         10-YEAR, 6-HR STORM (UNDER DEVELOPED CONDITIONS)
  *******************
  START
                     TIME=0.0
   BASIN 1
```

RAINFALL

TYPE=1 RAIN QUARTER=0.0 IN

RAIN ONE=1.49 IN RAIN SIX=1.93 IN RAIN DAY=2.43 IN DT=0.02253 HR

COMPUTE NM HYD ID=1 HYD NO=110.1 AREA=0.010088 SQ MI

PER A=0.00 PER B=10.00 PER C=0.00 PER D=90.00

TP=-0.1333 HR MASS RAINFALL=-1

PRINT HYD ID=1 CODE=1

* BASIN 2

COMPUTE NM HYD ID=1 HYD NO=110.2 AREA=0.000856 SQ MI

PER A=0.00 PER B=10.00 PER C=0.00 PER D=90.00

TP=-0.1333 HR MASS RAINFALL=-1

PRINT HYD ID=1 CODE=1

10-1 000L-

*

* BASIN 3

COMPUTE NM HYD ID=1 HYD NO=110.3 AREA=0.001111 SQ MI

PER A=0.00 PER B=10.00 PER C=0.00 PER D=90.00

TP=-0.1333 HR MASS RAINFALL=-1

PRINT HYD ID=1 CODE=1

* BASIN 4

COMPUTE NM HYD ID=1 HYD NO=110.4 AREA=0.000905 SQ MI

PER A=0.00 PER B=10.00 PER C=0.00 PER D=90.00

TP=-0.1333 HR MASS RAINFALL=-1

PRINT HYD ID=1 CODE=1

!

* BASIN 5

COMPUTE NM HYD ID=1 HYD NO=110.5 AREA=0.001132 SQ MI

PER A=0.00 PER B=10.00 PER C=0.00 PER D=90.00

TP=-0.1333 HR MASS RAINFALL=-1

PRINT HYD ID=1 CODE=1

FINISH

110.50 -

COMPUTE NM HYD

FINISH

3.319 PER IMP= 90.00

1.55636

.094

1.510

	•								_		∳ □[
		FROM	TO		PEAK	RUNOFF		TIME TO	CFS	PAGE =	: 1
	HYDROGRAPH	ID	ID	AREA	DISCHARGE	VOLUME	RUNOFF	PEAK	PER		
COMMAND	IDENTIFICATION	NO.	NO.	(SQ MI)	(CFS)	(AC-FT)	(INCHES)	(HOURS)	ACRE	NOTATI	ON
START										TIME=	.00
RAINFALL 7	TYPE= 1									RAIN6=	2.900
COMPUTE NM	HYD 100.10	-	1	-01009	32.55	1.337	2.48449	1.510	5.042	PER IMP=	90.00
COMPUTE NM	HYD 100.20	-	1	.00086	2.78	.113	2.48449	1.510	5.080	PER IMP=	90.00
COMPUTE NM	HYD 100.30	-	1	.00111	3.60	.147	2.48449	1.510	5.069	PER IMP=	90.00
COMPUTE NM	HYD 100.40	-	1	.00091	2.94	.120	2.48449	1.510	5.077	PER IMP=	90.00
COMPUTE NM	HYD 100.50	-	1	.00113	3.67	.150	2.48449	1.510	5.069	PER IMP=	90.00
START										TIME=	.00
RAINFALL 1	TYPE= 1				•					RAIN6=	1.930
COMPUTE NM	HYD 110.10	-	1	.01009	21.32	.837	1.55636	1.510	3.303	PER IMP=	90.00
COMPUTE NM	HYD 110.20	-	1	.00086	1.82	.071	1.55636	1.510	3.325	PER IMP=	90.00
COMPUTE NM	HYD 110.30	-	1	.00111	2.36	.092	1.55636	1.510	3.319	PER IMP=	90.00
COMPUTE NM	HYD 110.40	•	1	.00091	1.92	.075	1.55636	1.510	3.323	PER IMP=	90.00

2.40

.00113



May 18,1998

Ronald R. Bohannan
Tierra West ,LCC
4421 McLeod NE Suite D
Albuquerque, New Mexico 87109

RE: REVISED DRAINAGE PLAN FOR SANDIA PLAZA SHOPPING CENTER (G21-D38)
REVISION DATED 4/20/98

Dear Mr. Bohannan:

Based on the information provided on your April 21,1998 resubmittal, the above referenced site is approved for Building Permit.

Please attach a copy of this approved plan to the construction sets prior to sign-off by Hydrology.

Also, prior to Certificate of Occupancy release, Engineer Certification per the DPM will be required.

If I can be of further assistance, please feel free to contact me at 924-3986.

C: Andrew Garcia

Sincerely

Bernie J. Montoya CE Associate Engineer



DRAINAGE INFORMATION SHEET

PROJECT TITLE:	Sandia Plaza Shopping Center	ZONE ATLAS/DRN	G. FILE #: <u>G-21/D38</u>
DRB #:	EPC #:	_ WORK ORDER #:	
LEGAL DESCRIPTI	ON: Tract H3A of La Reina de los Alto	s Unit 2	
CITY ADDRESS:	North of Candelaria and west of Juan T	abo Boulevard	
ENGINEERING FIRE	M: TIERRA WEST, LLC	_ CONTACT:	RONALD R. BOHANNAN
ADDRESS:	4421 McCleod Rd. NE Suite D, 87109	PHONE:	(505) 883-7592
OWNER:		_ CONTACT:	
ADDRESS:		PHONE:	
ARCHITECT:	de la Torre Architects	_ CONTACT:	Jorge de la Torre
ADDRESS:	7801 Academy Ne Building 2 Suite 200	_ PHONE:	(505) 828-9611
SURVEYOR:	Precision Surveys_	CONTACT:	Larry Medrano
ADDRESS:	2929 Coors Blvd NW Suite 309	_ PHONE:	(505)839-0569
CONTRACTOR:		CONTACT:	
ADDRESS:		_ PHONE:	
DRAINA CONCE	DRAINAGE PLAN CONCEPTUAL GRADING & DRAINAGE PLAN		PLAN APPROVAL NARY PLAT APPROVAL PLAN FOR SUB'D. APPROVAL PLAN FOR BLDG. PERMIT APPROVAL
	ON CONTROL PLAN		R PLAN APPROVAL
OTHER	EER'S CERTIFICATION	<u> </u>	LAT APPROVAL TION PERMIT APPROVAL
PRE-DESIGN MEET		X BUILDIN	G PERMIT APPROVAL CATE OF OCCUPANCY APPROVAL
YES		GRADIN	G PERMIT APPROVAL
X NO		PAVING	PERMIT APPROVAL
COPY F	PROVIDED	S. A. D. I	DRAINAGE REPORT
			GE REQUIREMENTS
		OTHER	
DAT	TE SUBMITTED: 04/21/98		DECEUVE APR 2 1 1998
	RY: Sara McCollam		HYDROLOGY SECTION

BY:

Sara McCollam



April 21, 1998

Mr. Fred Aguirre City of Albuquerque P.O. Box 1293 Albuquerque, NM 87103

Re:

Sandia Plaza Shopping Center (G21-D38)

Dear Mr. Aguirre:

We are resubmitting the Grading and Drainage Plan for the Sandia Plaza Shopping Center in response to a comment by Lisa Manwill. A 3' curb opening has been added to the southwest corner of Basin 2 to convey the 100 year flow of 2.78 cfs into the landscaped drainage channel. The channel then carries the flow to Candelaria Road. The calculation for the length of weir has been included for your review. No changes have been made to the drainage report.

If you have any questions regarding this matter, please do not hesitate to call me.

Sincerely,

Sara McCollam

Enclosures

CC:

Jorge de la Torre

JN: 980004

scm

980004: 9804resubmittal2.ltr

Curb Cut

Weir Equation:

$$Q=CLH^{3/2}$$

Basin 2:

Q = Flow = 2.78 cfs

C = 2.95

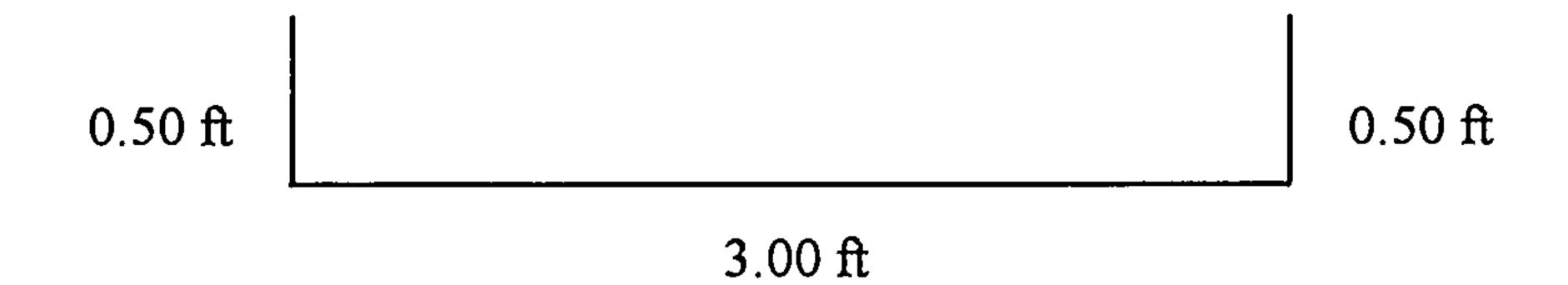
H = Height of Weir = 0.50 ft

L = Length of Weir

$$L = \frac{2.78}{2.95 * 0.50^{3/2}}$$

L = 2.66 cfs

Use length of 3.0 ft for weir





June 23, 1999

Mr. Fred Aguirre City of Albuquerque P.O. Box 1293 Albuquerque, NM 87103

Re: Sandia Plaza Shopping Center, G-21/D38

Dear Mr. Aguirre:

Enclosed is a revision to the above referenced shopping center. This revision has modified the parking field. The project has an approved grading plan dated April 20, 1998 under G-21/D38 and is located on Juan Tabo Boulevard between Candelaria and Jane Place. The site is the location of an existing building and parking lot that are currently being remodeled. We are resubmitting the grading plan to accommodate changes to the layout. A pad has been added to the north side of the site and grades have been provided for it.

We are not sure if the site will need to be approved by EPC or not. Currently, we are only requesting paving and building permit approval. Should we need to go to EPC, we will submit for the appropriate approval.

If you have any questions regarding this matter, please do not hesitate to call me.

Sincerely,

Sara McCollam

CC:

Garlan Bryan
Paul Blanchard

JN: 980004

scm

980004: 9804-drainage.ltr

DRAINAGE INFORMATION SHEET

PROJECT TITLE:	Sandia Plaza Shopping Center	ZONE ATLAS/DRNG. FILE #: <u>G-21/D38</u>					
DRB #:	EPC #:	WORK ORDER #:					
LEGAL DESCRIPT	ION: Tract H3A of La Reina de los Alt	os Unit 2					
CITY ADDRESS:	North of Candelaria and west of Juan	Tabo Boulevard					
ENGINEERING FIR	RM: TIERRA WEST, LLC	CONTACT: RONALD R. BOHANNAN					
ADDRESS:	4421 McCleod Rd. NE Suite D, 87109	PHONE: (505) 883-7592					
OWNER:		CONTACT:					
ADDRESS:		PHONE:					
ARCHITECT:		CONTACT:					
ADDRESS:		PHONE:					
SURVEYOR:	Precision Surveys	CONTACT: Larry Medrano					
ADDRESS:	2929 Coors Blvd NW Suite 309	PHONE: (505)839-0569					
CONTRACTOR:		CONTACT:					
ADDRESS:		PHONE:					
DRAIN CONCE X GRADI EROSI	AGE REPORT AGE PLAN EPTUAL GRADING & DRAINAGE PLAN ING PLAN ON CONTROL PLAN EER'S CERTIFICATION	SKETCH PLAN APPROVAL PRELIMINARY PLAT APPROVAL S. DEV. PLAN FOR SUB'D. APPROVAL S. DEV. PLAN FOR BLDG. PERMIT APPROVAL SECTOR PLAN APPROVAL FINAL PLAT APPROVAL FOUNDATION PERMIT APPROVAL					
PRE-DESIGN MEET YES X NO COPY	TING: PROVIDED	X BUILDING PERMIT APPROVAL CERTIFICATE OF OCCUPANCY APPROVAL GRADING PERMIT APPROVAL X PAVING PERMIT APPROVAL S. A. D. DRAINAGE REPORT DRAINAGE REQUIREMENTS OTHER					
DAT	TE SUBMITTED: 06/22/99 BY: Sara McCollam	D					



City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

July 16, 1999

Ronald R. Bohanan, P.E. Tierra West, LLC 4421 McLeod NE Suite D Albuquerque, NM 87109

Attn: Sara McCollam

RE: SANDIA PLAZA SHOPPING CENTER (G21-D38). GRADING AND GRADING PLAN FOR BUILDING PERMIT AND PAVING PERMIT APPROVALS. ENGINEER'S STAMP DATED MAY 14, 1999.

Dear Mr. Bohanan:

Based on the information provided in your June 24, 1999 submittal and the DRC held on July 15, 1999, City Hydrology has the following comments:

On July 13th, the Fire Marshall's Office had informed us of a (minor) fuel spill at this site. Upon a field investigation, Mr. Meinz, PWD Engineering (Hydrology), found some existing conditions which need to be addressed and corrected. He discussed these at length at the DRC. Foremost among these was the confirming of drainage easements with the neighboring apartment complex and the condition and capacity of the on-site facilities.

Mr. Meinz also provided comments on red lined markups of the plans.

If I can be of further assistance, please feel free to call me at 924-3984.

Sincerely,

John P. Murray, P.E.

Hydrology

c: L. Meinz VFile

DRAINAGE INFORMATION SHEET

PROJECT TITLE:	Sandia Plaza Shopping Center	ZONE ATLAS/DRN	NG. FILE #: <u>G-21/D38</u>
DRB #:	EPC #:	WORK ORDER #:	 -
LEGAL DESCRIPT	ION: Tract H3A of La Reina de los Altos	Unit 2	
CITY ADDRESS:	North of Candelaria and west of Juan Tal	o Boulevard	
ENGINEERING FIR	M: <u>TIERRA WEST, LLC</u>	CONTACT:	RONALD R. BOHANNAN
ADDRESS:	4421 McCleod Rd. NE Suite D, 87109	PHONE:	(505) 883-7592
OWNER:		CONTACT:	Don Morgan
ADDRESS:		PHONE:	
ARCHITECT:	de la Torre Architects	CONTACT:	Jorge de la Torre
ADDRESS:	7801 Academy Ne Building 2 Suite 200	PHONE:	(505) 828-9611
SURVEYOR:	Precision Surveys	CONTACT:	Larry Medrano
ADDRESS:	2929 Coors Bivd NW Suite 309	PHONE:	(505)839-0569
CONTRACTOR:		CONTACT:	
ADDRESS:		PHONE:	<u></u>
	AL: AGE REPORT AGE PLAN	SKETC	PPROVAL SOUGHT: H PLAN APPROVAL INARY PLAT APPROVAL
CONCE	EPTUAL GRADING & DRAINAGE PLAN	- · · · · · · · · · · · · · · · · · · ·	PLAN FOR SUB'D. APPROVAL
X GRADI	NG PLAN	S. DEV.	PLAN FOR BLDG. PERMIT APPROVAL
EROSI	ON CONTROL PLAN	SECTO	R PLAN APPROVAL
ENGIN	EER'S CERTIFICATION	FINAL F	PLAT APPROVAL
OTHER		 _	ATION PERMIT APPROVAL
	This is a "verbal mo"		NG PERMIT APPROVAL
PRE-DESIGN MEET	ING: Mycomments from.	CERTIF	ICATE OF OCCUPANCY APPROVAL
YES	last time was mor-	GRADIN	IG PERMIT APPROVAL
XNO	Cleared up - No -		PERMIT APPROVAL
COPY F	provided retention panding	S DEALNIA	DRAINAGE REPORT GE REQUIREMENTS
	will be allower	OTHER	
		910 sec. (10)	Exercide pondo
	BY:Sara McCollam	LAX	DAOLOGY GECTION RE-DELLOMULT

DRAINAGE REPORT

for

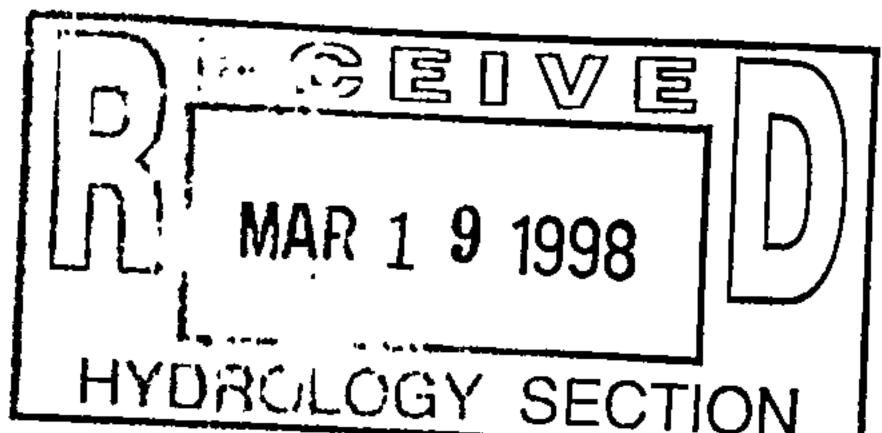
Sandia Plaza Shopping Center

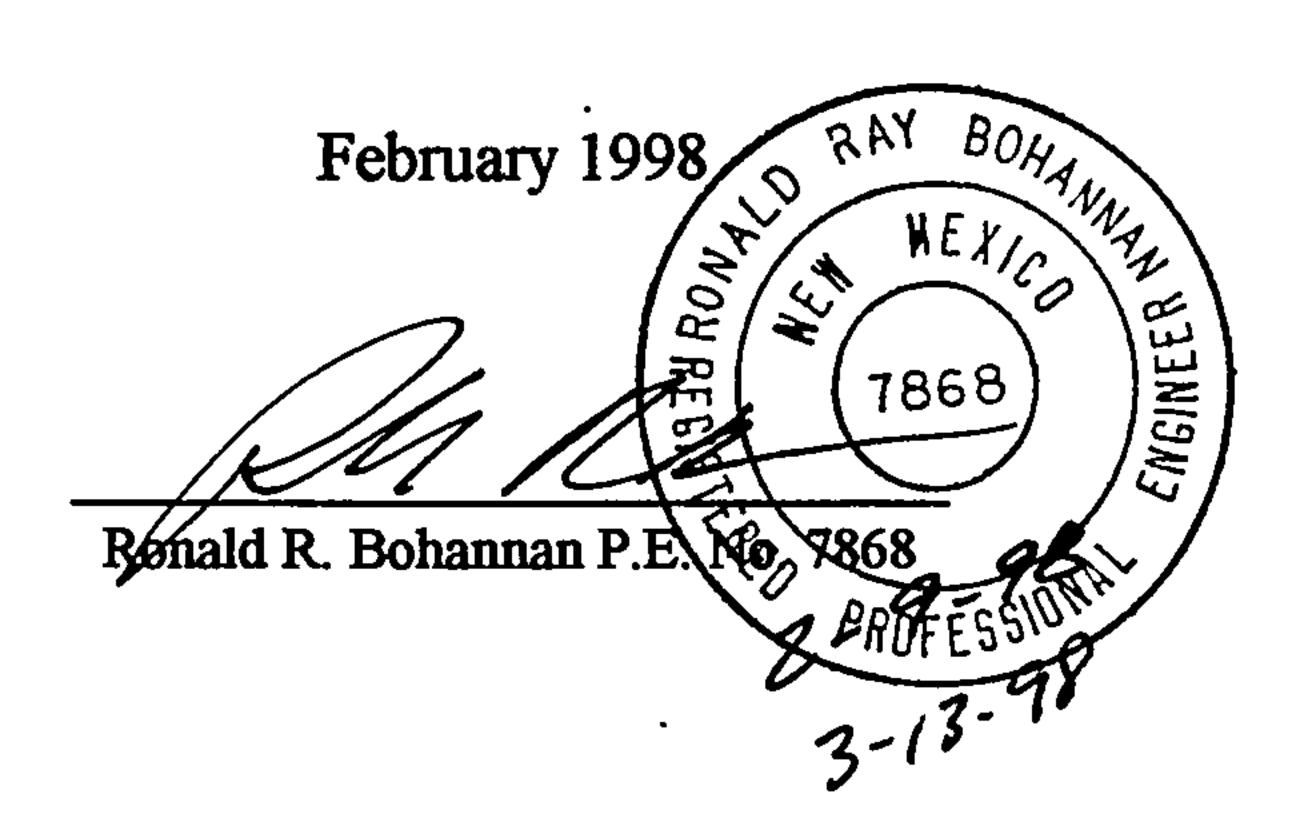
Prepared by

Tierra West, LLC 4421 McLeod Road NE, Suite D Albuquerque, New Mexico 87109

Prepared for

de la Torre Architects
7801 Academy Rd NE Building 2 Suite 200
Albuquerque, New Mexico 87109







March 13, 1998

Mrs. Lisa Manwill
City of Albuquerque
P.O. Box 1293
Albuquerque, NM 87103

Re: Sandia Plaza Shopping Center (G21-D38)

Dear Mr. Manwill:

This is a letter in response to your comments regarding the drainage report and the Grading and Drainage Plan for the Sandia Plaza Shopping Center.

- 1. The flows from Basin 2 drain to the southwest corner and pond until it exceeds the 6" curb to drain to a landscaped area that channels the flows to a sidewalk culvert and then to Candelaria Street. If necessary, it would be possible to make a 3' curb cut and regrade the landscape area to allow the water to drain to the channel.
- Finished floor elevations have been added to the existing buildings in the shopping center.

If you have any questions regarding this matter, please do not hesitate to call me.

Sincerely,

Sara McCollam

Enclosures

cc: Jorge de la Torre

JN: 980004

scm

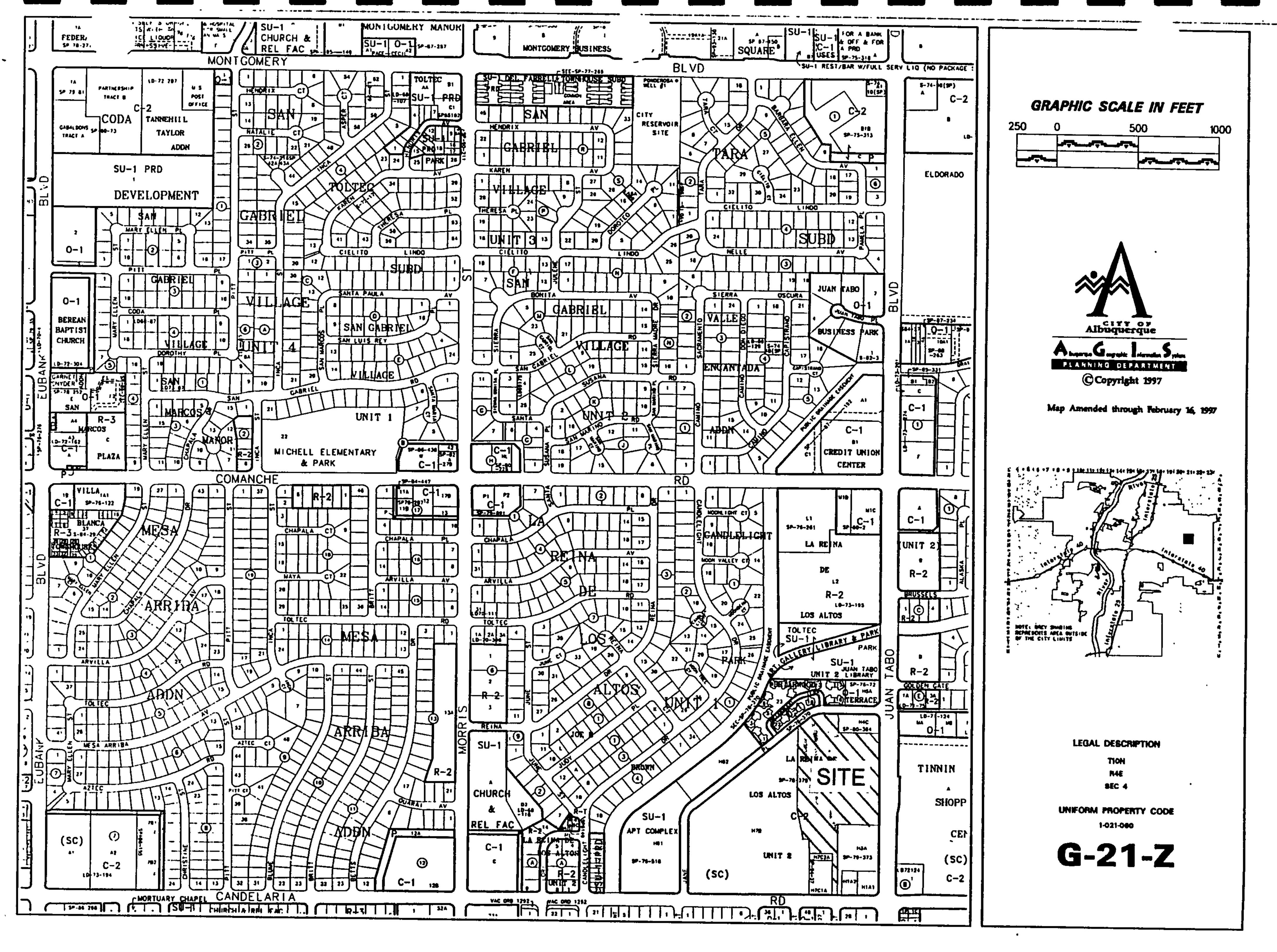
MAR 1 9 1998

HYDRULOGY SECTION

980004: 9804resubmittal.itr

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Existing Drainage Conditions
Basin Layout
FIRM Map and Soil Conditions
Proposed On-Site Drainage Management Plan
FIRM Map 35001C0357 D 5
Runoff Calculations
Drainage Facilities Analysis
AHYMO Runoff Input and Summary Output for Developed Drainage Basins



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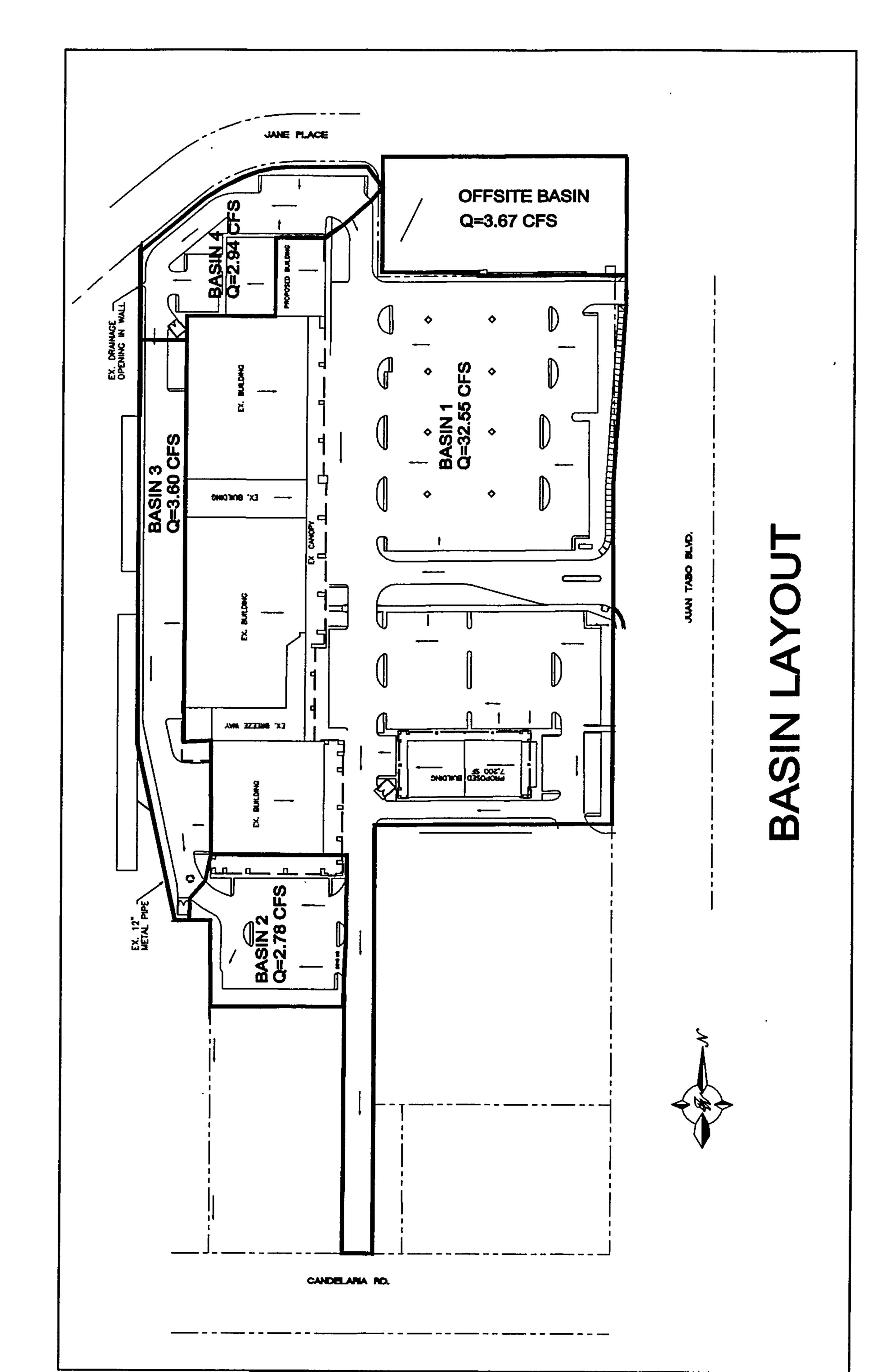
Location

Sandia Plaza Shopping Center is an existing site that is located on Juan Tabo
Boulevard near Candelaria Road. The site is shown on the attached Zone Atlas Map G-21
and contains approximately 8.36 acres. The site legal description is Tract H3A of La Reina de los Altos Unit 2.

The site has been developed for a number of years, and currently there is a pawn shop, a hairdresser, a restaurant and a vacant building on the site. The current owner is preparing to update the shopping center by adding additional parking spaces and landscaping improvements in the parking lot. Other proposed improvements include added a 8,700 square foot proposed building on the north side of the shopping center and a new 7,200 square foot pad site on the southeast side of the site. Parking lot islands, tree planters, and an entrance driveway are also proposed improvements for the site. The purpose of this report is to provide the drainage analysis and management plan for the shopping center.

Existing Drainage Conditions

The site has been developed continuously prior to 1972 and the existing conditions closely represent the developed conditions. Currently the site has four on-site basins. Basin 1 has a developed discharge rate of 32.55 cfs and free discharges to Candelaria Road. This is the largest basin and consists of the main portion of the site. A driveway to Candelaria acts as a drainage channel and has a capacity of 187.30 cfs. This is more than adequate for Basin 1 and the offsite basin. Basin 2, on the south side of the site, has a developed discharge rate of 2.78 cfs. This basin free discharges to the south side of the site. The basin drains to the southwest corner and enters the adjacent lot. The adjoining lot provides a corridor for the flows behind the building, filled with large gravel for the flows to reach Candelaria Road. The flows enter a concrete rundown and are conveyed by existing sidewalk culverts to Candelaria



Road. Basin 3 has a developed discharge rate of 3.60 cfs. This basin is located in the back of the existing buildings and free discharges to a 12" exposed metal pipe which runs south towards Candelaria Road. The developed runoff rate of 3.60 cfs is less than the pipe capacity of 4.57 cfs. The pipe daylights into an existing concrete rundown and sidewalk culverts convey the flows to Candelaria Road. Basin 4 is on the north side of the site with a developed discharge rate 2.94 cfs. This basin free discharges to a 2' curb cut in the back of the buildings and then into the public street of Jane Place.

There is an offsite drainage basin located to the north of the site, consisting of a Yamaha Dealership which free discharges 3.67 cfs to the site. This flow joins the flows from Basin 1 and discharges to Candelaria Road. There is an existing retaining wall on the west side of the site that prevents off-site drainage from entering the site. There is a water block on Juan Tabo to the east and existing developed sites on the south that keep any offsite drainage from entering the site from those directions. There is no offsite drainage entering the site from the any other direction.

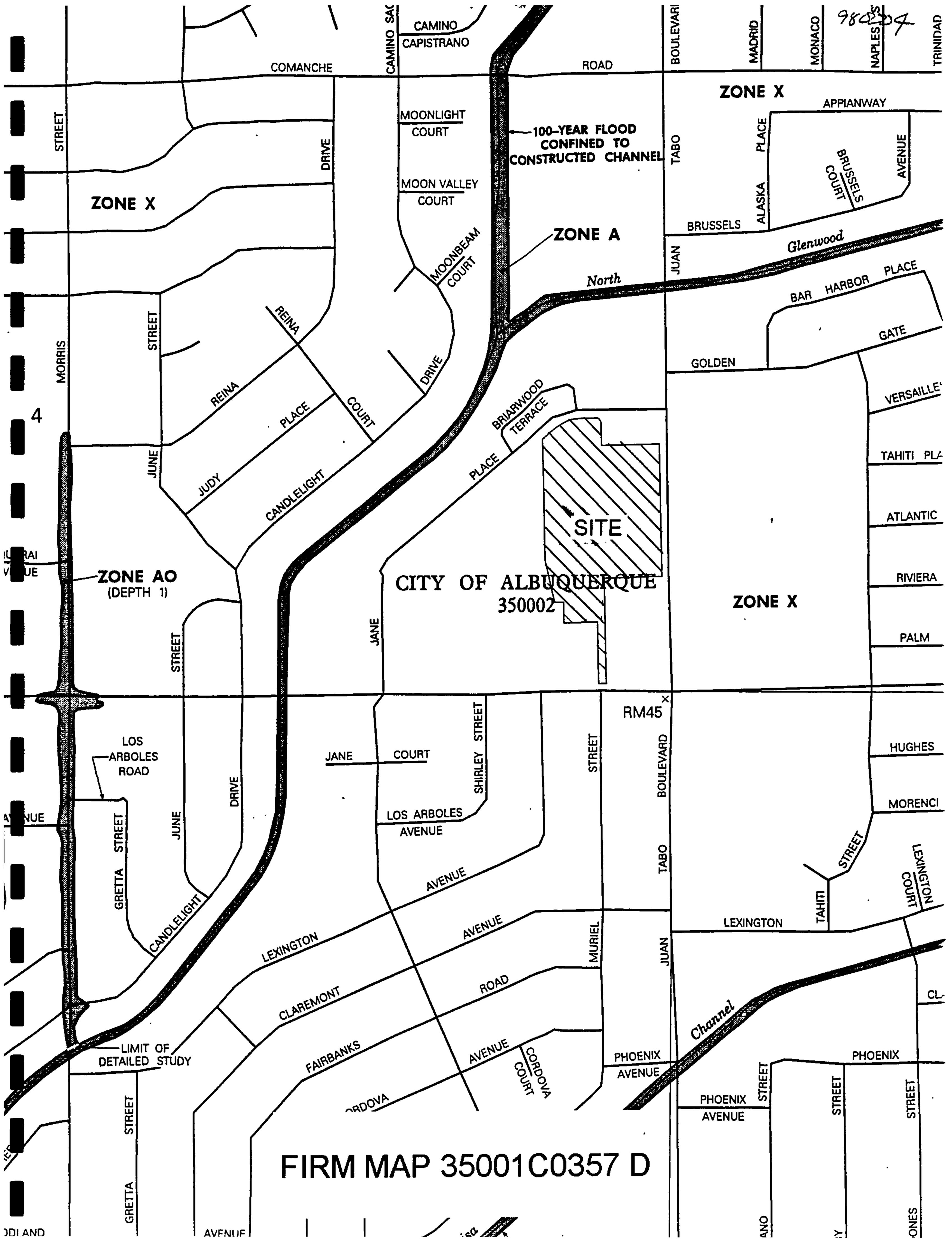
The driveway to Candelaria Road will act as an emergency overflow in the event of a storm larger than a 100 year storm.

FIRM Map and Soil Conditions

The site is located on FIRM Map 35001C0357 D as shown on the attached excerpt.

The map shows that the site does not lie within any 100 year flood plains.

According to the Soil Conservation Service Soil Survey of Bernalillo County, the site contains one soil type. The site consists of an Embudo-Tijeras complex which has a medium runoff and a moderate hazard of water erosion.



Proposed On-Site Drainage Management Plan

The proposed drainage management plan is to continue the existing drainage patterns. The site was developed prior to 1972 before the City of Albuquerque kept complete drainage records. Consequently, there is no drainage report on file for this site although it is already developed. The proposed improvements to the site will not change the existing drainage conditions. The site was previously impervious and the new improvements will not change the land condition. The current drainage solution performs adequately and there have been no complaints from the tenants on the site or the adjacent property owners.

Runoff
Calculations

RUNOFF CALCULATIONS

The site is @ Zone 4

LAND TREATMENT

Existing

B = 10%

D = 90%

DEPTH (INCHES) @ 100-YEAR STORM

 $P_{60} = 2.23$ inches

 $P_{360} = 2.90 \text{ inches}$

 $P_{1440} = 3.65 \text{ inches}$

DEPTH (INCHES) @ 10-YEAR STORM

 $P_{60} = 2.23 \times 0.667$

= 1.49 inches

 $P_{360} = 1.93$

 $P_{1440} = 2.43$

See the summary output from AHYMO calculations.

Drainage Basins

BASIN	AREA (SF)	AREA (AC)	AREA (MI ²)
1	281239.30	6.4564	0.010088
2	23863.75	0.5478	0.000856
3	30960.45	0.7108	0.001111
4	25224.83	0.5791	0.000905
5	31553.95	0.7244	0.001132
Total	392842.28	9.0184	0.014091

Runoff Calculation Results

BASIN	Q-100	Q-10	V-100	V-10
	CFS	CFS	AC-FT	AC-FT
1	32.55	21.32	1.337	0.837
2	2.78	1.82	0.133	0.071
3	3.60	2.36	0.147	0.092
4	2.94	1.92	0.120	0.075
5	3.67	2.40	0.150	0.094
Total	45.54	29.82	1.887	1.169

Drainage Facilities
Analysis

Curb Cut

Weir Equation:

$$Q = CLH^{3/2}$$

Basin 4:

Q=Flow (cfs) C = 2.95

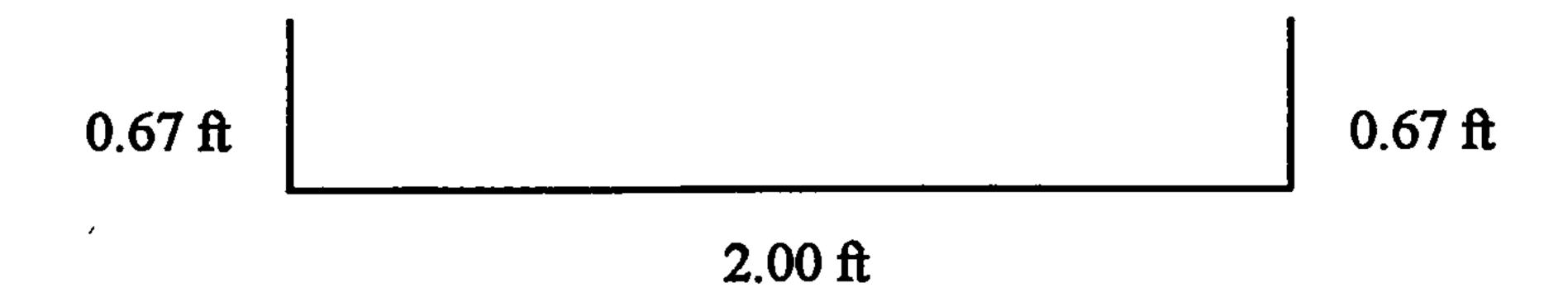
H = 0.67 ft

L=2.0 ft

$$Q=2.95*2.0*0.67^{3/2}$$

Q = 3.24 cfs

3.24 cfs > 3.13 cfs



Street Channel

Manning's Equation:

$$Q = \frac{1.49}{n} AR^{2/3} S^{1/2}$$

$$Q = Flow (cfs)$$

$$n = 0.017$$

$$A = Area$$

R = Hydraulic Radius

$$S = Slope$$

$$A=0.37*32*0.5+32*0.5=21.92$$

$$WP = 2*0.5+2*\sqrt{0.37^2+16^2}=33.0$$

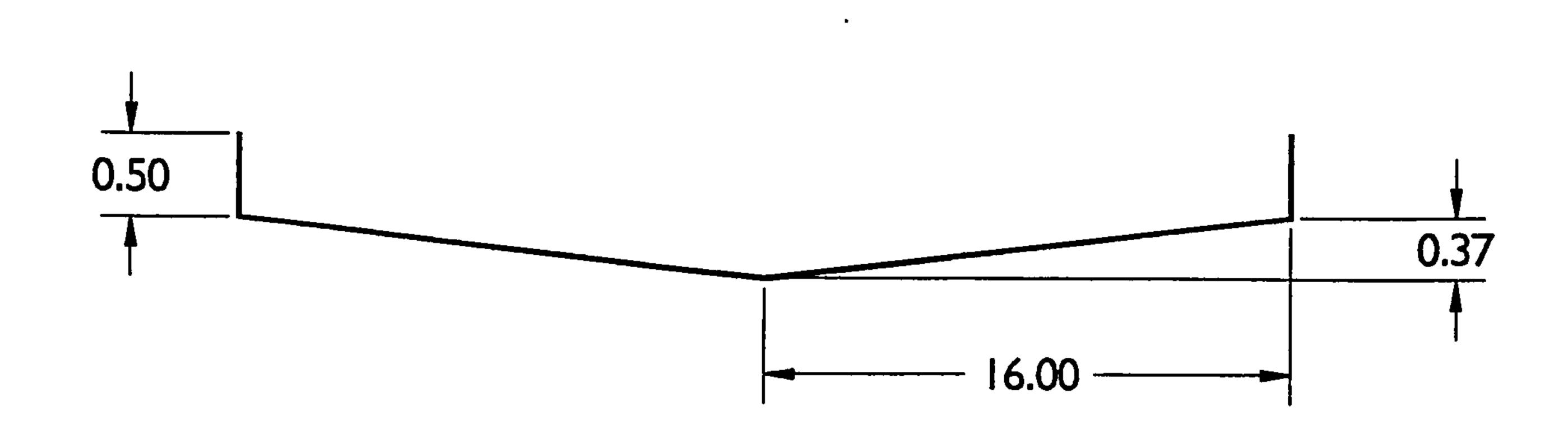
$$R = \frac{A}{WP} = \frac{21.92}{33.00} = 0.6642$$

Manning's Equation:

$$Q = \frac{1.49}{0.017} * 21.92 * 0.6642^{2/3} * 0.0164^{1/2}$$

$$Q = 187.30 \ cfs$$

. Capacity of street = 187.30 cfs
Required capacity of street = 32.33 cfs
187.30 cfs > 32.33 cfs



Pipe Capacity

Manning's Equation: $Q = 1.49/n * A * R^{2/3} * S^{1/2}$

A = Area

R = D/4

S = Slope

n = 0.013

Pipe	D	Slope Area		R	Q Provided	Q Required	Velocity
	(in)	(%)	(ft^2)		(cfs)	(cfs)	(ft/s)
Metal Pipe	12	1.64	0.79	0.25	4.57	7.65	9.74

AHYMO
Runoff Input
and
Summary Output
for
Existing Drainage Basins

```
SANDIA PLAZA SHOPPING CENTER
         100-YEAR, 6-HR STORM (UNDER DEVELOPED CONDITIONS)
                     TIME=0.0
 * BASIN 1
                     TYPE=1 RAIN QUARTER=0.0 IN
RAINFALL
                     RAIN ONE=2.23 IN RAIN SIX=2.90 IN
                     RAIN DAY=3.65 IN DT=0.02253 HR
                     ID=1 HYD NO=100.1 AREA=0.010088 SQ MI
                     PER A=0.00 PER B=10.00 PER C=0.00 PER D=90.00
                     TP=-0.1333 HR MASS RAINFALL=-1
PRINT HYD
                     ID=1 CODE=1
* BASIN 2
COMPUTE NM HYD
                     ID=1 HYD NO=100.2 AREA=0.000856 SQ MI
                     PER A=0.00 PER B=10.00 PER C=0.00 PER D=90.00
                     TP=-0.1333 HR MASS RAINFALL=-1
PRINT HYD
                     ID=1 CODE=1
  BASIN 3
COMPUTE NM HYD
                     ID=1 HYD NO=100.3 AREA=0.001111 SQ MI
                     PER A=0.00 PER B=10.00 PER C=0.00 PER D=90.00
                     TP=-0.1333 HR MASS RAINFALL=-1
PRINT HYD
                     ID=1 CODE=1
  BASIN 4
COMPUTE NM HYD
                     ID=1 HYD NO=100.4 AREA=0.000905 SQ MI
                     PER A=0.00 PER B=10.00 PER C=0.00 PER D=90.00
                     TP=-0.1333 HR MASS RAINFALL=-1
                     ID=1 CODE=1
PRINT HYD
  BASIN 5
                     ID=1 HYD NO=100.5 AREA=0.001132 SQ MI
_COMPUTE NM HYD
                     PER A=0.00 PER B=10.00 PER C=0.00 PER D=90.00
                     TP=-0.1333 HR MASS RAINFALL=-1
                     ID=1 CODE=1
PRINT HYD
         10-YEAR, 6-HR STORM (UNDER DEVELOPED CONDITIONS)
                     TIME=0.0
  BASIN 1
RAINFALL
                     TYPE=1 RAIN QUARTER=0.0 IN
                     RAIN ONE=1.49 IN RAIN SIX=1.93 IN
```

RAIN DAY=2.43 IN DT=0.02253 HR

COMPUTE NM HYD

ID=1 HYD NO=110.1 AREA=0.010088 SQ MI

PER A=0.00 PER B=10.00 PER C=0.00 PER D=90.00

TP=-0.1333 HR MASS RAINFALL=-1

PRINT HYD

ID=1 CODE=1

* BASIN 2

COMPUTE NM HYD

ID=1 HYD NO=110.2 AREA=0.000856 SQ MI

PER A=0.00 PER B=10.00 PER C=0.00 PER D=90.00

TP=-0.1333 HR MASS RAINFALL=-1

PRINT HYD

ID=1 CODE=1

* BASIN 3

COMPUTE NM HYD

ID=1 HYD NO=110.3 AREA=0.001111 SQ MI

PER A=0.00 PER B=10.00 PER C=0.00 PER D=90.00

TP=-0.1333 HR MASS RAINFALL=-1

RINT HYD

ID=1 CODE=1

* BASIN 4

COMPUTE NM HYD

ID=1 HYD NO=110.4 AREA=0.000905 SQ MI

PER A=0.00 PER B=10.00 PER C=0.00 PER D=90.00

TP=-0.1333 HR MASS RAINFALL=-1

RINT HYD

ID=1 CODE=1

* BASIN 5

COMPUTE NM HYD

ID=1 HYD NO=110.5 AREA=0.001132 SQ MI

PER A=0.00 PER B=10.00 PER C=0.00 PER D=90.00

TP=-0.1333 HR MASS RAINFALL=-1

RINT HYD

ID=1 CODE=1

INISH

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994
INPUT FILE = a:p.dat

RUN DATE (MON/DAY/YR) =02/06/1998
USER NO.= R_BOHANN.IO1

COMMAND	HYDROGRAPH IDENTIFICATION		TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE =	
START										TIME=	.00
	YPE= 1									RAIN6=	2.900
COMPUTE NM		-	1	.01009	32.55	1.337	2.48449	1.510		PER IMP=	90.00
COMPUTE NM			1	.00086	2.78	.113	2.48449	1.510		PER IMP=	90.00
COMPUTE NM			1	.00111	3.60	.147	2.48449	1.510	5.069	PER IMP=	90.00
COMPUTE NM			1	_00091	2.94	.120	2.48449	1.510	5.077	PER IMP=	90.00
COMPUTE NM		-	1	.00113	3.67	-150	2.48449	1.510	5.069	PER IMP=	90.00
START										TIME=	.00
RAINFALL T	YPE= 1	1								RAIN6=	1.930
COMPUTE NM	HYD 110.10	-	1	.01009	21.32	.837	1.55636	1.510	3.303	PER IMP=	90.00
COMPUTE NM	HYD 110.20	-	· 1	.00086	1.82	.071	1.55636	1.510	3.325	PER IMP=	90.00
COMPUTE NM	HYD 110.30	-	1	.00111	2.36	.092	1.55636	1.510	3.319	PER IMP=	90.00
COMPUTE NM	HYD 110.40	-	1	-00091	1.92	.075	1.55636	1.510	3.323	PER IMP=	90.00
COMPUTE NM	HYD 110.50	-	1	.00113	2.40	.094	1.55636	1.510	3.319	PER IMP=	90.00
FINISH											