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Heritage Neighborhood Marketplace Development
(Ladera Dr. / Unser Blvd.)

Traffic Impact Study

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City of Albuquerque
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&
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District 3

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**Heritage Neighborhood Marketplace Development
(Ladera Dr. / Market Rd.)
TRAFFIC IMPACT STUDY**

STUDY PURPOSE

This study is being conducted in conjunction with a request for approval of a retail commercial development plan such as the one shown in the Appendix (Page A-2) of this report. The purpose of this study is to identify the impact of the proposed Development on the adjacent transportation system, and to make recommendations to mitigate any significant adverse impact on the adjacent transportation system resulting from the implementation of the new development. This study is based on the assumption that the land uses and densities implemented in the development of the proposed site development plan will be similar to those defined in the table on Page A-5 in the Appendix of this report. Should the developer propose a combination of land uses and / or densities that would significantly increase the overall traffic generation for the Heritage Neighborhood Marketplace Development, an update to this study would be required reflecting the proposed new conditions.

STUDY PROCEDURES

A scoping meeting was held on Thursday, September 13, 2007 with City of Albuquerque Transportation Development staff (Tony Loyd and John Hartmann) prior to beginning the study to discuss scope and methodology to be utilized within the report. Specific items included format, intersections to be studied, intersection analysis procedures, existing traffic counts, trip distribution methodology, and implementation and horizon year definition. Additionally, the proposed scope of the study was reviewed with Tony Abbo, District 3 Traffic Engineer with the New Mexico Department of Transportation. He concurred with the scope of the study defined by the City of Albuquerque at the September 13 scoping meeting.

The basic procedure followed for this traffic impact study is outlined as follows:

- ◆ Calculate the generated trips for this proposed retail commercial development consisting of approximately 212,000 S.F. of floor space as specifically defined in the Trip Generation Table on Page A-9 in the Appendix of this report. The trips generated for the implementation year analysis (2010) will assume that 100% of the development has occurred.
- ◆ Calculate trip distribution for the newly generated trips by this development. The new commercial trips will be distributed based on year 2010 population data within a two (2) mile radius of the project as shown on Page A-22 in the Appendix of this report. The new office trips will be distribution based on the year 2010 population data city wide inversely proportional to the distance of the data subarea from the project location.
- ◆ Determine Trip Assignments for the newly generated trips based on the results of the Trip Distribution Analysis and logical routing to and from the new site.

- ◆ Include the generated trips for the proposed Watershed Residential and Retail Development, Storm Cloud Subdivision, I-40 / Unser Commercial Development, and 98th / Unser Commercial Development in the 2010 NO BUILD Volumes for this project.
- ◆ Obtain recent AM Peak Hour and PM Peak Hour Turning Movement Volumes Traffic Counts for the intersections of Tierra Pintada Dr. / Unser Blvd., Ladera Dr. / Unser Blvd., I-40 N. Ramp / Unser Blvd., Los Volcanes Rd. / Unser Blvd., Ladera Dr. / Unser Blvd., I-40 S. Ramp / Unser Blvd., Ladera Dr. / Market Rd., and Ladera Dr. / Laurelwood Parkway.
- ◆ Calculate Historic Growth Rates for background traffic volumes based on the Mid-Region Council of Governments' forecast AM and PM Peak Hour period link volumes extracted from the Mid-Region Council of Governments' regional transportation model (2030 data set).
- ◆ Calculate background traffic growth from the year of the most recent traffic counts to the implementation year for this analysis (2010).
- ◆ Add trips generated from the proposed Watershed Residential and Retail Development, Storm Cloud Subdivision, I-40 / Unser Commercial Development, and 98th / Unser Commercial Developments to the background traffic volumes. The trips from these previously approved developments will be included in the 2010 NO BUILD Volumes for this study.
- ◆ Add data from Trip Assignments Maps and Tables to the 2010 NO BUILD Volumes to obtain 2010 BUILD Volumes for this project.
- ◆ Provide signalized and / or unsignalized intersection analyses for the following intersections:

	INTERSECTION	TYPE CONTROL	NO BUILD ANALYSIS	BUILD ANALYSIS
1	Tierra Pintada Dr. / Unser Blvd.	Traffic Signal	2010	2010
2	Ladera Dr. / Unser Blvd.	Traffic Signal	2010	2010
3	I-40 N. Ramp / Unser Blvd.	Traffic Signal	2010	2010
4	Los Volcanes Rd. / Unser Blvd.	Traffic Signal	2010	2010
5	Ladera Dr. / Ouray Rd.	Traffic Signal	2010	2010
6	I-40 S. Ramp / Unser Blvd.	Stop Sign	2010	2010
7	Ladera Dr. / Market Rd.	Stop Sign	2010	2010
8	Ladera Dr. / Laurelwood Parkway	Stop Sign	2010	2010
9	Ladera Dr. / Driveway "A"	Stop Sign	N/A	2010
10	Driveway "B" / Market Rd.	Stop Sign	N/A	2010
11	Hanover Rd. / Driveway "C"	Stop Sign	N/A	2010
12	Driveway "D" / Unser Blvd.*	Stop Sign	N/A	2010

* - Implementation of Driveway "D" will require approval from the Transportation Coordinating Committee at the Mid-Region Council of Governments and the Access Control Committee at the New Mexico Department of Transportation.

GENERAL AREA CHARACTERISTICS

This project is located at the southeast corner of Ladera Dr. / Unser Blvd. behind the existing gasoline station at the hard corner of the intersection. The surrounding area to the south, east, and west is primarily zoned for commercial and industrial park type of development. The property is bound on the north by Interstate 40. The Vicinity Map on Page A-1 of the Appendix shows the zoning of the surrounding properties in the area surrounding this site. The project is located in a mild to moderately active development area.

AREA STREET NETWORK

Access to this new site will be primarily via Unser Blvd., Ladera Dr., Market Rd., and Hanover Rd. There is a proposed full access point into this development from Unser Blvd. The Unser Blvd. access will require approval from the New Mexico Department of Transportation's Access Control Board and the Mid-Region Council of Governments' Transportation Coordinating Committee. Secondary access points are proposed on Ladera Dr., Market St., and Hanover Rd.

Unser Blvd. is classified as a Limited Access Principal Arterial roadway from Arenal Rd. to Montano Rd. on the Long Range Roadway System Map for the Albuquerque Metropolitan Planning Area. It is a four lane divided paved urban roadway with raised medians from Central Ave. to Montano Rd. The posted speed limit on Unser Blvd. near Ladera Dr. is 45 M.P.H.

Ladera Dr. is classified as a Minor Arterial Street on the Long Range Roadway Plan for the Albuquerque Metropolitan Area. It is generally a four lane divided roadway section constructed to urban standards in the vicinity of Unser Blvd. The posted speed limit on Ladera Dr. near Unser Blvd. is 40 M.P.H.

Los Volcanes Rd. is classified as a Collector Street on the Long Range Roadway System Map for the Albuquerque Metropolitan Planning Area. It is a paved urban roadway with curbs and gutters on both sides and no medians from Unser Blvd. to Coors Blvd. West of Unser Blvd., it is not classified.

Ouray Rd. is classified as a Collector Street on the Long Range Roadway System Map for the Albuquerque Metropolitan Planning Area. It is a paved urban roadway with curbs and gutters on both sides and no medians from Unser Blvd. to Coors Blvd. West of Unser Blvd., it is not classified.

Interstate 25 is a major east-west freeway running through the center of the City of Albuquerque. There are currently ramps connecting Interstate 25 with Unser Blvd. and with Coors Blvd.

Market St., Hanover Rd., and Laurelwood Parkway are not classified on the Long Range Roadway Plan for the Albuquerque Metropolitan Area.

EXISTING TRAFFIC VOLUMES

2006 Average Weekday Traffic Volumes (AWDT) for major streets in the site plan area are shown on Page A-8 in the Appendix.

AM and PM Peak Hour turning movement counts were obtained by field traffic counts taken recently for the following intersections:

98th St. / Unser Blvd. (June 13, 2007)
Ladera Dr. / Unser Blvd. (June 12, 2007)
I-40 N. Ramp / Unser Blvd. (May 22, 2007)
Los Volcanes Rd. / Unser Blvd. (May 23, 2007)
Ladera Dr. / Ouray Rd. (September 20, 2007)
I-40 S. Ramp / Unser Blvd. (May 21, 2007)
Ladera Dr. / Market St. (September 21, 2007)
Ladera Dr. / Laurelwood Parkway (September 19, 2007)

The counts are included near the end of the Appendix.

EXISTING (2007) LEVELS OF SERVICE

The Highway Capacity Manual defines Level of Service (LOS) for signalized intersections in terms of average controlled delay per vehicle as follows:

LOS A	10.0" or less	Most Vehicles do not stop
LOS B	10.1 to 20.0"	Some Vehicles stop
LOS C	20.1 to 35.0"	Significant number of vehicles stop
LOS D	35.1 to 55.0"	Many vehicles stop.
LOS E	55.1 to 80.0"	Limit of acceptable delay.
LOS F	> 80.0"	Unacceptable delay.

Level of Service D is generally considered acceptable in urban areas and is the desirable base condition for analysis in a traffic study.

This study performs analysis for the 2010 NO BUILD and 2010 BUILD Conditions associated with the development of the Heritage Neighborhood Marketplace.

EXISTING TRANSIT SERVICE

This area currently is serviced by City Bus Route 162 (Ventana Ranch / Unser Route) which services this area at approximate one hour intervals from 5:30 am to 8:30 am and from 4:45 pm to 6:45 pm on weekdays. No other bus service is available at this time.

PROPOSED DEVELOPMENT

The subject area of land discussed in this report is comprised of approximately 20 acres. See the conceptual site development plan on Page A-2 in the Appendix of this report to acquire more detailed information about the proposed development. This site plan is conceptual at this point in time and is subject to some changes as progress takes place in

the design process. The plan should, however, provide a reliable basis upon which to analyze the impact of the development on the adjacent transportation system and provide guidelines for mitigating the impact and establishing access criteria.

There is one proposed primary access point (driveway) along Unser Blvd. for the new site (See Site Map on Page A-2 of Appendix). The driveway on Unser Blvd. (Driveway "D") is intended to be full access driveway. Secondary access is proposed on Ladera Dr., Market St., and Hanover Rd. All driveways are initially proposed as full access intersections. Implementation of the driveway on Unser Blvd. (Driveway "D") will be required to be approved by the Transportation Coordinating Committee (T.C.C.) at the Mid-Region Council of Governments (M.R.C.O.G.) and the State Access Control Committee of the New Mexico Department of Transportation.

TRIP GENERATION

Projected trips were calculated from data in the Institute of Transportation Engineers Trip Generation report (7th Edition, 2003). Trips for the development were determined based on land uses defined on the Preliminary Site Plan on Page A-2 in the Appendix of this report.

The resulting number of trips generated for the proposed development (100%) are summarized in the following table:

Heritage Neighborhood Marketplace (Ladera / Unser) **Trip Generation Data**

USE (ITE CODE)	DESCRIPTION	24 HR VOL	A. M. PEAK HR.		P. M. PEAK HR.	
		GROSS	ENTER	EXIT	ENTER	EXIT
Summary Sheet	Units					
Shopping Center (820)	103.10	6,927	97	62	308	333
Supermarket (850)	66.00	5,810	183	117	343	329
Fast Food Restaurant w/ Drive-Thru Window (934)	3.20	1,588	87	83	58	53
Fast Food Restaurant w/ Drive-Thru Window (934)	3.20	1,588	87	83	58	53
Drive-In Bank (912)	4	1,563	45	33	102	102
General Office Building (710)	33.00	568	68	9	20	96
Subtotal		18,044	567	387	889	966
Pass-by Trip Credit	30%				(267)	(290)
Net new Trips to System		18,044	567	387	622	676

* - All land uses are designated in Units of 1,000 S.F. of building area.

The Implementation Year Analysis (2010) for this study assumed a development of 100% of the project to be implemented. See Appendix Pages A-9 thru A-15 for more detailed information regarding trip generation rates (including Trip Generation Worksheets).

A 30% adjustment was made to the trip generation rates for PM Peak Hour Pass-by Trips for trips generated by this project.

TRIP DISTRIBUTION

Primary and Diverted Linked Trips:

Trips were distributed as follows:

Commercial Land Uses

Primary and diverted linked trips for the commercial land use development were distributed proportionally to the 2010 projected population of Data Analysis Subzones within a two mile radius of the proposed development. Population data for the years 2004 and 2030 were taken from the 2030 Socioeconomic Forecasts by Data Analysis Subzones for the MRCOG Region, S-07-01 (July, 2007), Appendix B and Appendix C, supplied by the Mid-Region Council of Governments (MRCOG). Population data from the years 2004 and 2030 was interpolated linearly to obtain 2010 population data to utilize for this analysis. Population Subzones were grouped based on the most likely major street(s) or route(s) to the subject development. The trip distribution worksheets and associated map of data analysis subzones is shown in the Appendix. The commercial Trip Distribution map can be found in the Appendix on Page A-23.

Office Land Uses

Primary and diverted linked trips for the office land use development were distributed proportionally to the 2010 projected population of Data Subareas citywide inversely proportional to the distance of the subarea from the project location. Population data for the years 2004 and 2030 were taken from the 2030 Socioeconomic Forecasts by Data Analysis Subzones for the MRCOG Region, S-07-01 (July, 2007), Appendix E and Appendix F, supplied by the Mid-Region Council of Governments (MRCOG). Population data from the years 2004 and 2030 was interpolated linearly to obtain 2010 population data to utilize for this analysis. Population Subareas were grouped based on the most likely major street(s) or route(s) to the subject development. The trip distribution worksheets and associated map of data analysis subzones is shown in the Appendix. The office Trip Distribution map can be found in the Appendix on Page A-33.

TRIP ASSIGNMENTS

Trip assignments for primary and diverted linked trips are first made on a percentage basis derived from data established in the trip distribution determination process and logical routing. Those percentages are then applied to the projected trips to determine individual traffic movements. Percentage trip assignments are shown on Pages A-24 thru A-25 (commercial uses) and on Pages 34 thru 35 (office uses) in the Appendix of this report.

BACKGROUND TRAFFIC GROWTH

Background traffic growth rates were considered for each individual approach to an intersection that was targeted for analysis based on data from the Mid-Region Council of Governments' Regional Transportation Model (2030 data set). Forecast AM and PM Peak Hour link volumes on major streets were extracted from the 2005 and 2030 volumes and utilized to establish a background traffic growth rate for projecting turning movements at the

intersections to be analyzed in this project. Since there are existing traffic count volumes at the intersections analyzed in this project, the link volumes based on the recent traffic counts were utilized instead of the MRCOG regional model link volumes. Utilizing the growth rates established in such a manner should result in forecast turning movement volumes that are consistent with the Mid-Region Council of Governments forecast link volumes.

PROJECTED PEAK HOUR TURNING MOVEMENTS FOR 2010 BUILDOUT

The calculated growth rates were applied to the most recent peak hour traffic counts (furnished by the City of Albuquerque and conducted for this study) and the trips from the approved *Southwest Mesa Subdivisions, Ladera Business Park, Vista Oriente Development, Storm Cloud Development, and I-40 / Unser Commercial Development* were added in to establish the 2010 background traffic volumes. To these volumes, the generated trips based on implementation of the proposed Heritage Neighborhood Marketplace Development Site Development Plan (100% development) were added to obtain 2010 BUILD volumes for the intersection analyses. See Appendix Pages A-41 thru A-71 for further information regarding 2010 turning movement counts.

INTERSECTION CAPACITY ANALYSIS

Intersection capacity analyses were performed in accordance with the procedures for signalized and unsignalized intersections utilized in the Synchro (Version 6, Build 614) Transportation System analysis software program. Synchro software deviates from the 2000 Highway Capacity Manual methods in several areas, but the results obtained using Synchro software are considered by the New Mexico Department of Transportation to be generally close to those based on the 2000 Highway Capacity Manual in most cases. For signalized intersections, the operational method of analysis was used for both the 2010 NO BUILD and BUILD conditions.

Capacity analyses were performed for the following traffic conditions.

- 2010 without development of the subject property (NO BUILD)
- 2010 with development as per the Conceptual Site Development Plan (BUILD)

Queuing analysis at signalized intersections is calculated based on Poisson's arrival method considering cycle length and peak hour volumes to achieve a 95% confidence level of maximum queue for the peak hour periods.

The results of the 2010 NO BUILD and 2010 BUILD capacity analyses are summarized in the following sections - *Results and Discussion of Intersection Capacity Analyses*.

RESULTS OF SIGNALIZED INTERSECTION CAPACITY ANALYSES

IMPLEMENTATION YEAR (2010)

1. Tierra Pintada Dr. / Unser Blvd. – A-72 thru A-80

The results of the implementation year analysis of the signalized intersection of Tierra Pintada Dr. / Unser Blvd. are summarized in the following tables:

Existing Geometry (Tierra Pintada Dr. / Unser Blvd.)

Approach	Left Turn Lanes	Thru/Lefts	Thru Lanes	Thru/Rights	Right Turn Lanes
EB Tierra Pintada Dr.	1	0	1	0	1
WB Tierra Pintada Dr.	1	0	1	0	1
NB Unser Blvd.	1	0	2	0	1
SB Unser Blvd.	1	0	2	0	1

Tierra Pintada Dr. / Unser Blvd.		AM Peak Hour		PM Peak Hour	
	2010	NO BUILD	BUILD	NO BUILD	BUILD
Existing Geometry		B – 15.2	B – 15.7	<i>C – 22.7</i>	<i>C – 30.4</i>

D – 39.7 - Bold Italicized Level-of-Service indicates that one or more individual turning movements is Level-of-Service E or worse.

The operation of the signalized intersection has been demonstrated to be acceptable for the projected 2010 BUILD conditions analyzed in this report. The westbound left turn movement during the PM Peak Hour period is "E" during the NO BUILD condition and "F" during the BUILD condition. The NO BUILD condition indicates that a permitted / protected left turn phase would be beneficial. However, analysis of the signalized intersection with the eastbound / westbound permitted / protected left turn yielded no significant improvement. Implementation of permitted / protected left turns in all four directions yields satisfactory results. (See analysis on Pages A-79 and A-80 in the Appendix of this report).

The following table summarizes the existing and calculated queuing at the signalized intersection:

Queueing Analysis Summary Sheet

Project:
Intersection:

Heritage Neighborhood Center
Tierra Pintada Dr / Unser Blvd

2010

Approach				Left Turns			Thru Movements			Right Turns		
Eastbound				# Lanes	Vol.	Length	# Lanes	Vol.	Length	# Lanes	Vol.	Length
<i>Existing Lane Length</i>				1	34	250	1	3	Cont	1	195	250
AM NO BUILD Queue				1	203	275	1	18	50	1	215	275
AM BUILD Queue				1	203	275	1	18	50	1	249	300
<i>Existing Lane Length</i>				1	31	250	1	2	Cont	1	68	250
PM NO BUILD Queue				1	151	225	1	30	75	1	114	175
PM BUILD Queue				1	151	225	1	30	75	1	154	225
Westbound				# Lanes	Vol.	Length	# Lanes	Vol.	Length	# Lanes	Vol.	Length
<i>Existing Lane Length</i>				1	14	250	1	0	Cont	1	4	250
AM NO BUILD Queue				1	106	150	1	12	50	1	28	75
AM BUILD Queue				1	112	175	1	12	50	1	28	75
<i>Existing Lane Length</i>				1	65	250	1	1	Cont	1	24	250
PM NO BUILD Queue				1	356	450	1	40	75	1	99	175
PM BUILD Queue				1	362	450	1	40	75	1	99	175
Northbound				# Lanes	Vol.	Length	# Lanes	Vol.	Length	# Lanes	Vol.	Length
<i>Existing Lane Length</i>				1	28	350	2	631	Cont	1	15	250
AM NO BUILD Queue				1	36	75	2	712	450	1	130	175
AM BUILD Queue				1	61	100	2	751	475	1	134	200
<i>Existing Lane Length</i>				1	179	350	2	840	Cont	1	22	250
PM NO BUILD Queue				1	213	300	2	962	625	1	226	300
PM BUILD Queue				1	253	325	2	1,037	650	1	232	325
Southbound				# Lanes	Vol.	Length	# Lanes	Vol.	Length	# Lanes	Vol.	Length
<i>Existing Lane Length</i>				1	10	230	2	783	Cont	1	21	250
AM NO BUILD Queue				1	49	100	2	954	575	1	50	100
AM BUILD Queue				1	49	100	2	1,016	600	1	50	100
<i>Existing Lane Length</i>				1	28	230	2	770	Cont	1	47	250
PM NO BUILD Queue				1	97	150	2	882	575	1	132	200
PM BUILD Queue				1	97	150	2	945	600	1	132	200

AM PM
Cycle Length: 110 120

NOTE: Queue lengths are in feet.

* - Queue Length of 1,001 indicates that the calculated queue > 1

The westbound left turn lane should be lengthened to a total length of 225 feet plus transition. However, at some time in the future, it appears that dual westbound left turn lanes may be implemented at this intersection, in which case the queue length for the westbound left turn lane will be reduced by almost 50%. Therefore, no recommendation is made.

2. Ladera Dr. / Unser Blvd. – A-81 thru A-87

The results of the implementation year analysis of the signalized intersection of Ladera Dr. / Unser Blvd. are summarized in the following tables:

Existing Geometry (Ladera Dr. / Unser Blvd.)

Approach	Left Turn Lanes	Thru/Lefts	Thru Lanes	Thru/Rights	Right Turn Lanes
EB Ladera Dr.	1	0	1	0	2
WB Ladera Dr.	2	0	0	1	0
NB Unser Blvd.	1	0	2	0	1
SB Unser Blvd.	1	0	1	1	0

* - Right Turn Lane by-passes the signal.

Ladera Dr. / Unser Blvd.		AM Peak Hour		PM Peak Hour	
	2010	NO BUILD	BUILD	NO BUILD	BUILD
Existing Geometry		<i>E – 65.0</i>	<i>E – 76.7</i>	<i>F – 185</i>	<i>F – 214</i>
Mitigated Geometry			<i>D – 43.3</i>		<i>D – 50.1</i>

D – 39.7 - Bold Italicized Level-of-Service indicates that one or more individual turning movements is Level-of-Service E or worse.

The operation of the signalized intersection has been demonstrated to be unacceptable for all projected conditions analyzed in this report. It is recommended that dual eastbound thru lanes, dual northbound left turn lanes, dual southbound left turn lanes, and a southbound right turn lane be constructed at this intersection to cause it to operate at LOS "D" for the projected 2010 Peak Hour volumes. The City of Albuquerque has plans to construct a westbound thru/right turn lane at the intersection in the near future.

The following table summarizes the existing and calculated queuing at the signalized intersection:

Queueing Analysis Summary Sheet

Project: Heritage Neighborhood Center
 Intersection: Ladera Dr / Unser Blvd

2010											
Approach			Left Turns			Thru Movements			Right Turns		
Eastbound			# Lanes	Vol.	Length	# Lanes	Vol.	Length	# Lanes	Vol.	Length
<i>Existing Lane Length</i>			1	175	250	1	251	Cont	2	365	250
AM NO BUILD Queue			1	199	250	1	446	500	2	557	375
AM BUILD Queue			1	199	250	1	498	550	2	557	375
<i>Existing Lane Length</i>			1	140	250	1	182	Cont	2	138	250
PM NO BUILD Queue			1	192	275	1	319	400	2	322	250
PM BUILD Queue			1	192	275	1	381	475	2	322	250
Westbound			# Lanes	Vol.	Length	# Lanes	Vol.	Length	# Lanes	Vol.	Length
<i>Existing Lane Length</i>			2	317	250	1	105	Cont	0	44	0
AM NO BUILD Queue			2	542	350	1	186	250	0	123	175
AM BUILD Queue			2	686	425	1	196	250	0	140	200
<i>Existing Lane Length</i>			2	281	250	1	264	Cont	0	107	0
PM NO BUILD Queue			2	594	425	1	480	575	0	249	325
PM BUILD Queue			2	853	550	1	496	575	0	279	375
Northbound			# Lanes	Vol.	Length	# Lanes	Vol.	Length	# Lanes	Vol.	Length
<i>Existing Lane Length</i>			1	48	250	2	429	Cont	1	224	250
AM NO BUILD Queue			1	134	200	2	653	425	1	376	425
AM BUILD Queue			1	163	225	2	704	450	1	376	425
<i>Existing Lane Length</i>			1	288	250	2	860	Cont	1	372	250
PM NO BUILD Queue			1	560	650	2	1,433	875	1	708	800
PM BUILD Queue			1	607	700	2	1,524	1,001 *	1	708	800
Southbound			# Lanes	Vol.	Length	# Lanes	Vol.	Length	# Lanes	Vol.	Length
<i>Existing Lane Length</i>			1	45	250	2	906	Cont	0	58	0
AM NO BUILD Queue			1	99	150	2	1,279	725	0	105	150
AM BUILD Queue			1	201	250	2	1,279	725	0	105	150
<i>Existing Lane Length</i>			1	94	250	2	547	Cont	0	184	0
PM NO BUILD Queue			1	263	350	2	1,087	700	0	333	425
PM BUILD Queue			1	372	450	2	1,087	700	0	333	425

AM **PM**
 Cycle Length: 110 120

NOTE: Queue lengths are in feet.

* - Queue Length of 1,001 indicates that the calculated queue > 1

The dual westbound left turn lane should be extended to a total length of 550 feet plus transition. Implementing this recommendation will require Driveway "A" to be restricted to a right-turn-in, right-turn-out only driveway. The northbound dual left turn lanes should be constructed to a total length of 425 feet plus transition (60% of 700 feet). The southbound dual left turn lanes should be constructed to a length of 275 feet (60% of 450) plus transition. The new southbound right turn lane should be constructed to a total length of 250 feet (50% of 425 feet) plus transition.

3. I-40 N. Ramp / Unser Blvd. – A-88 thru A-92

The results of the implementation year analysis of the signalized intersection of I-40 N. Ramp / Unser Blvd. are summarized in the following tables:

Existing Geometry (I-40 N. Ramp / Unser Blvd.)

Approach	Left Turn Lanes	Thru/Lefts	Thru Lanes	Thru/Rights	Right Turn Lanes
WB I-40 N. Ramp	1	1	0	0	1*
NB Unser Blvd.	1	0	2	0	0
SB Unser Blvd.	0	0	3	0	0

* - Right Turn Lane by-passes the signal.

I-40 N. Ramp / Unser Blvd.		AM Peak Hour		PM Peak Hour	
	2010	<u>NO BUILD</u>	<u>BUILD</u>	<u>NO BUILD</u>	<u>BUILD</u>
Existing Geometry		B – 11.4	B – 12.4	B - 13.3	B – 16.2

D - 39.7 - Bold Italicized Level-of-Service indicates that one or more individual turning movements is Level-of-Service E or worse.

The operation of the signalized intersection has been demonstrated to be acceptable for all projected conditions analyzed in this report. No recommendation for mitigation is made.

The following table summarizes the existing and calculated queuing at the signalized intersection:

Queueing Analysis Summary Sheet

Project: Heritage Neighborhood Center
 Intersection: I-40 N. ramp / Unser Blvd

2010

Approach	Left Turns			Thru Movements			Right Turns		
Eastbound	# Lanes	Vol.	Length	# Lanes	Vol.	Length	# Lanes	Vol.	Length
Existing Lane Length	0	0	0	0	0	Cont	0	0	0
AM NO BUILD Queue	0	0	0	0	0	0	0	0	0
AM BUILD Queue	0	0	0	0	0	0	0	0	0
Existing Lane Length	0	0	0	0	0	Cont	0	0	0
PM NO BUILD Queue	0	0	0	0	0	0	0	0	0
PM BUILD Queue	0	0	0	0	0	0	0	0	0
Westbound	# Lanes	Vol.	Length	# Lanes	Vol.	Length	# Lanes	Vol.	Length
Existing Lane Length	1	343	999	1	3	Cont	1	188	1,000
AM NO BUILD Queue	1	438	500	1	3	0	1	222	275
AM BUILD Queue	1	438	500	1	3	0	1	291	350
Existing Lane Length	1	626	999	1	0	Cont	1	771	1,000
PM NO BUILD Queue	1	770	850	1	0	0	1	840	1,001
PM BUILD Queue	1	770	850	1	0	0	1	905	1,001
Northbound	# Lanes	Vol.	Length	# Lanes	Vol.	Length	# Lanes	Vol.	Length
Existing Lane Length	1	24	350	2	687	Cont	0	0	0
AM NO BUILD Queue	1	46	100	2	992	600	0	0	0
AM BUILD Queue	1	46	100	2	1,138	675	0	0	0
Existing Lane Length	1	24	350	2	725	Cont	0	0	0
PM NO BUILD Queue	1	75	125	2	1,319	825	0	0	0
PM BUILD Queue	1	75	125	2	1,485	900	0	0	0
Southbound	# Lanes	Vol.	Length	# Lanes	Vol.	Length	# Lanes	Vol.	Length
Existing Lane Length	0	0	0	3	1,775	Cont	0	68	0
AM NO BUILD Queue	0	0	0	3	2,281	875	0	76	125
AM BUILD Queue	0	0	0	3	2,417	1,001 *	0	83	125
Existing Lane Length	0	0	0	3	903	Cont	0	70	0
PM NO BUILD Queue	0	0	0	3	1,559	700	0	108	175
PM BUILD Queue	0	0	0	3	1,806	775	0	120	200

AM PM
 Cycle Length: 110 120

NOTE: Queue lengths are in feet.

* - Queue Length of 1,001 indicates that the calculated queue > 1

The westbound right turn movement and the southbound right turn movement are free right turns that by-pass the signal. No queueing is anticipated for those right turns.

4. Los Volcanes Rd. / Unser Blvd. – A-93 thru A-97

The results of the implementation year analysis of the signalized intersection of Los Volcanes Rd. / Unser Blvd. are summarized in the following tables:

Existing Geometry (Los Volcanes Rd. / Unser Blvd.)

Approach	Left Turn Lanes	Thru/Lefts	Thru Lanes	Thru/Rights	Right Turn Lanes
EB Los Volcanes Rd.	2	0	0	1	0
WB Los Volcanes Rd.	1	0	1	0	1
NB Unser Blvd.	1	0	2	0	1
SB Unser Blvd.	1	0	2	0	1

* - Right Turn Lane by-passes the signal.

This analysis assumes that the I-40 / Unser Commercial Development traffic will be generated and that the mitigated geometry will be in place as required by the developer of that project as follows:

I-40/Unser Development Mitigated Geometry (Los Volcanes Rd. / Unser Blvd.)

Approach	Left Turn Lanes	Thru/Lefts	Thru Lanes	Thru/Rights	Right Turn Lanes
EB Los Volcanes Rd.	2	0	0	1	0
WB Los Volcanes Rd.	2	0	0	1	0
NB Unser Blvd.	2	0	3	0	1
SB Unser Blvd.	2	0	2	0	1

* - Right Turn Lane by-passes the signal.

Los Volcanes Rd. / Unser Blvd.	2010	AM Peak Hour		PM Peak Hour	
		<u>NO BUILD</u>	<u>BUILD</u>	<u>NO BUILD</u>	<u>BUILD</u>
I-40/Unser Mitigated Geometry		C – 30.5	C – 31.2	<i>D – 38.1</i>	<i>D – 41.7</i>

D – 39.7 - Bold Italicized Level-of-Service indicates that one or more individual turning movements is Level-of-Service E or worse.

The operation of the signalized intersection has been demonstrated to be acceptable for all projected conditions analyzed in this report. No recommendation for mitigation is made.

The following table summarizes the existing and calculated queuing at the signalized intersection:

Queueing Analysis Summary Sheet

Project:

Heritage Neighborhood Center (Ladera Dr / Unser Blvd)

Intersection:

Los Volcanes Rd / Unser Blvd

2010

Approach	Left Turns			Thru Movements			Right Turns		
Eastbound	# Lanes	Vol.	Length	# Lanes	Vol.	Length	# Lanes	Vol.	Length
Existing Lane Length	2	137	95	1	69	Cont	0	7	0
AM NO BUILD Queue	2	137	125	1	74	125	0	7	25
AM BUILD Queue	2	142	125	1	74	125	0	7	25
Existing Lane Length	2	54	95	1	15	Cont	0	4	0
PM NO BUILD Queue	2	82	100	1	33	75	0	6	25
PM BUILD Queue	2	87	100	1	33	75	0	6	25
Westbound	# Lanes	Vol.	Length	# Lanes	Vol.	Length	# Lanes	Vol.	Length
Existing Lane Length	2	95	Design	1	27	Cont	0	134	0
AM NO BUILD Queue	2	297	225	1	31	75	0	219	275
AM BUILD Queue	2	297	225	1	31	75	0	255	325
Existing Lane Length	2	73	Design	1	23	Cont	0	132	0
PM NO BUILD Queue	2	736	500	1	33	75	0	459	550
PM BUILD Queue	2	736	500	1	33	75	0	500	600
Northbound	# Lanes	Vol.	Length	# Lanes	Vol.	Length	# Lanes	Vol.	Length
Existing Lane Length	2	8	600	3	1,101	Cont	1	101	200
AM NO BUILD Queue	2	8	25	3	1,192	500	1	279	350
AM BUILD Queue	2	8	25	3	1,287	550	1	279	350
Existing Lane Length	2	10	600	3	793	Cont	1	120	200
PM NO BUILD Queue	2	12	25	3	1,083	500	1	463	550
PM BUILD Queue	2	12	25	3	1,190	550	1	463	550
Southbound	# Lanes	Vol.	Length	# Lanes	Vol.	Length	# Lanes	Vol.	Length
Existing Lane Length	2	260	Design	2	897	Cont	1	69	170
AM NO BUILD Queue	2	490	325	2	976	575	1	75	125
AM BUILD Queue	2	516	350	2	1,043	625	1	78	125
Existing Lane Length	2	108	Design	2	1,022	Cont	1	121	170
PM NO BUILD Queue	2	609	425	2	894	575	1	127	200
PM BUILD Queue	2	651	450	2	1,007	650	1	133	200

AM **PM**
 Cycle Length: 110 120

NOTE: Queue lengths are in feet.

* - Queue Length of 1,001 indicates that the calculated queue > 1

The dual westbound left turn lanes and the dual southbound left turn lanes will be constructed as per the design established by the developer of the I-40 / Unser Commercial Development and approved by the City of Albuquerque and the New Mexico Department of Transportation. The calculated length of the right turn lane can be reduced by 50% to account for right-turns-on-red and overlap phases.

Queueing Analysis Summary Sheet

Project:
Intersection:

Heritage Neighborhood Center (Ladera Dr / Unser Blvd)
Ladera Dr / Ouray Rd

2010

Approach	Left Turns			Thru Movements			Right Turns		
Eastbound	# Lanes	Vol.	Length	# Lanes	Vol.	Length	# Lanes	Vol.	Length
Existing Lane Length	1	20	140	2	365	Cont	0	247	0
AM NO BUILD Queue	1	20	50	2	365	250	0	247	300
AM BUILD Queue	1	24	50	2	414	300	0	287	350
Existing Lane Length	1	19	140	2	271	Cont	0	200	0
PM NO BUILD Queue	1	22	50	2	307	250	0	227	300
PM BUILD Queue	1	27	75	2	400	300	0	292	375
Westbound	# Lanes	Vol.	Length	# Lanes	Vol.	Length	# Lanes	Vol.	Length
Existing Lane Length	1	3	100	2	145	Cont	1	20	100
AM NO BUILD Queue	1	4	25	2	214	175	1	30	75
AM BUILD Queue	1	4	25	2	291	225	1	30	75
Existing Lane Length	1	20	100	2	379	Cont	1	107	100
PM NO BUILD Queue	1	23	50	2	427	325	1	121	200
PM BUILD Queue	1	23	50	2	506	375	1	121	200
Northbound	# Lanes	Vol.	Length	# Lanes	Vol.	Length	# Lanes	Vol.	Length
Existing Lane Length	1	98	120	1	77	Cont	0	5	0
AM NO BUILD Queue	1	101	150	1	79	125	0	5	25
AM BUILD Queue	1	156	225	1	79	125	0	5	25
Existing Lane Length	1	293	120	1	233	Cont	0	16	0
PM NO BUILD Queue	1	293	375	1	233	325	0	16	50
PM BUILD Queue	1	358	450	1	233	325	0	16	50
Southbound	# Lanes	Vol.	Length	# Lanes	Vol.	Length	# Lanes	Vol.	Length
Existing Lane Length	1	89	100	1	191	Cont	1	13	80
AM NO BUILD Queue	1	99	150	1	212	275	1	14	50
AM BUILD Queue	1	99	150	1	212	275	1	19	50
Existing Lane Length	1	34	100	1	123	Cont	1	13	80
PM NO BUILD Queue	1	36	75	1	131	200	1	14	50
PM BUILD Queue	1	36	75	1	131	200	1	20	50

Cycle Length: **AM** **PM**
 110 120

NOTE: Queue lengths are in feet.

* - Queue Length of 1,001 indicates that the calculated queue > 1

RESULTS OF UNSIGNALIZED INTERSECTION CAPACITY ANALYSES

IMPLEMENTATION YEAR (2010)

6. I-40 S. Ramp / Unser Blvd. – A-103 thru A-107

The results of the analysis of the unsignalized intersection of I-40 S. Ramp / Unser Blvd. are summarized in the following table:

	2010	AM Peak Hour		PM Peak Hour	
		NO	BUILD	NO	BUILD
		BUILD		BUILD	
I-40 S. Ramp / Unser Blvd.					
Minor Street (I-40 S. Ramp)					
EB Left		F - 93	F - 168	F - 637	F - 999
EB Through		N/A	N/A	N/A	N/A
EB Right		Free	Free	Free	Free
Minor Street (I-40 S. Ramp)					
WB Left		N/A	N/A	N/A	N/A
WB Through		N/A	N/A	N/A	N/A
WB Right		N/A	N/A	N/A	N/A
Major Street (Unser Blvd.)					
NB Left		N/A	N/A	N/A	N/A
SB Left		N/A	N/A	N/A	N/A

The analysis of the unsignalized intersection of the I-40 South Ramp / Unser Blvd. will operate at unacceptable levels-of-service for the forecast 2010 NO BUILD and BUILD Conditions as an unsignalized intersection. The mathematical analysis of the intersection indicated excessively long delays would be encountered by the eastbound left turn traffic off of the ramp. However, the calculations do not take into account the fact that there is an existing traffic signal approximately 1,650 feet to the north of this intersection and approximately 2,100 feet south of the intersection. The two adjacent intersections will create gaps in northbound and southbound traffic on Unser Blvd. at the I-40 South Ramp, thus permitting eastbound side street traffic to turn out onto Unser Blvd. with greater ease than what the calculations seem to indicate. There are no further measures that can be taken at this time to improve the operation of the intersection.

Rectification of the long delays at the intersection of the I-40 S. Ramp / Unser Blvd. may involve construction of a new traffic signal at the ramp. Current volumes indicate that a traffic signal is marginally warranted at the intersection. Following is the Peak Hour Warrant Graph for this intersection considering the projected 2010 AM and PM Peak Hour volumes:

7. Ladera Dr. / Market St. - Pages A-108 thru A-112

The results of the analysis of the unsignalized intersection of Ladera Dr. / Market St. are summarized in the following table:

	2010	AM Peak Hour		PM Peak Hour	
		NO BUILD	BUILD	NO BUILD	BUILD
Ladera Dr. / Market St.					
Minor Street (Market St.)					
NB Left		C - 19	F - 82	D - 26	F - 292
NB Thru		B - 11	B - 11	B - 12	B - 15
NB Right		B - 11	B - 11	B - 12	B - 15
Minor Street (Market St.)					
SB Left		C - 16	D - 31	C - 18	F - 121
SB Thru		C - 16	D - 31	C - 18	F - 121
SB Right		C - 16	D - 31	C - 18	F - 121
Major Street (Ladera Dr.)					
EB Left		A - 1	A - 1	A - 1	A - 9
WB Left		A - 9	B - 10	A - 10	B - 14

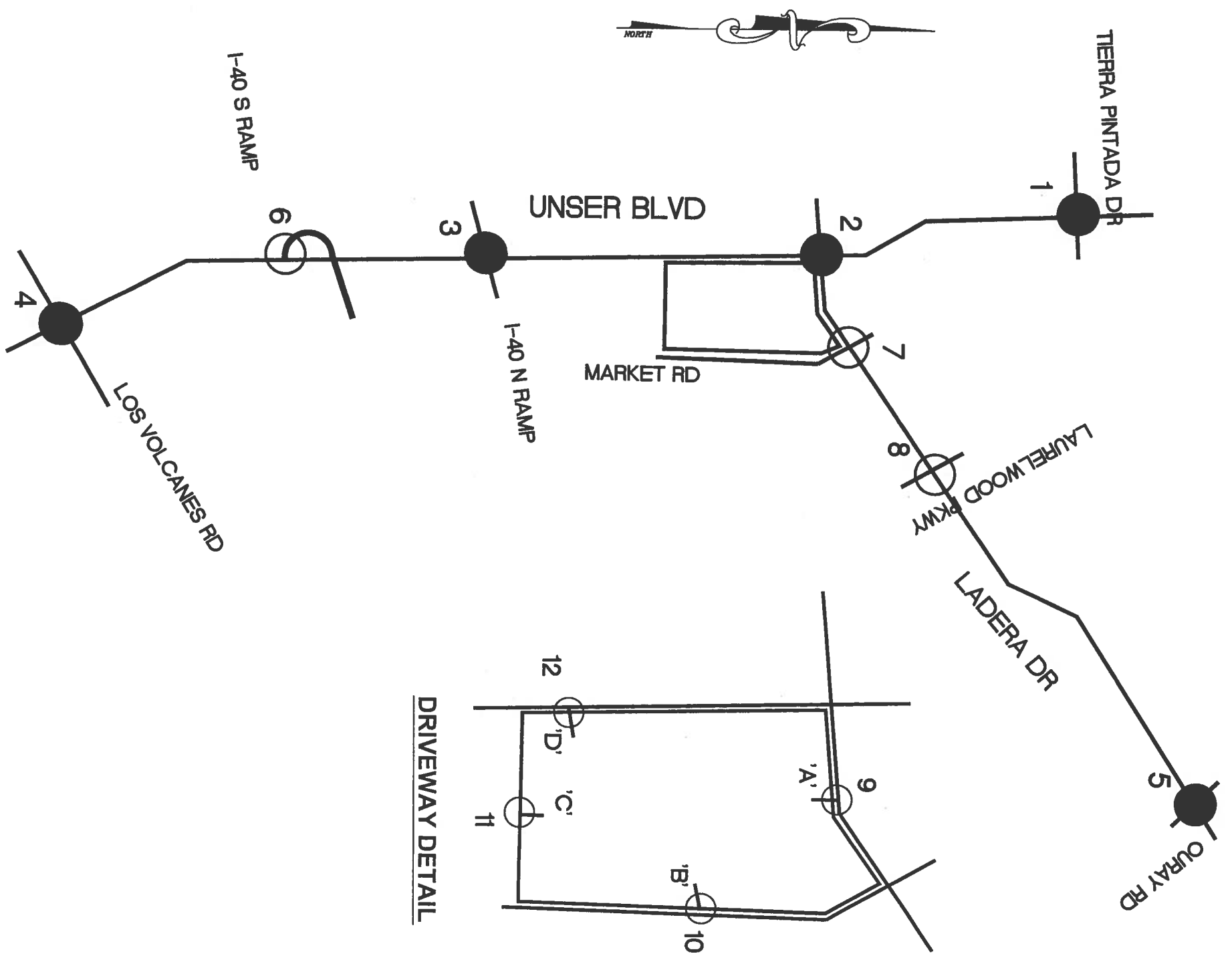
This analysis forecasts that the unsignalized intersection of Ladera Dr. / Market St. will experience long delays on the side street upon implementation of the proposed Heritage Neighborhood Marketplace. The intersection of Ladera Dr. / Market St. is probably too close to Unser Blvd. for consideration for a traffic signal. Market St. is approximately 750 feet east of Unser Blvd. (centerline to centerline).

The fact that there is an existing traffic signals to the west of the intersection of Market St. / Ladera Dr. will aid in creating gaps in the eastbound traffic on Ladera Dr. so as to facilitate the turning movements from the side streets onto the major street to some degree, but this report still forecasts long delays for the Market St. traffic at Ladera Dr. No recommendation for mitigation is made.

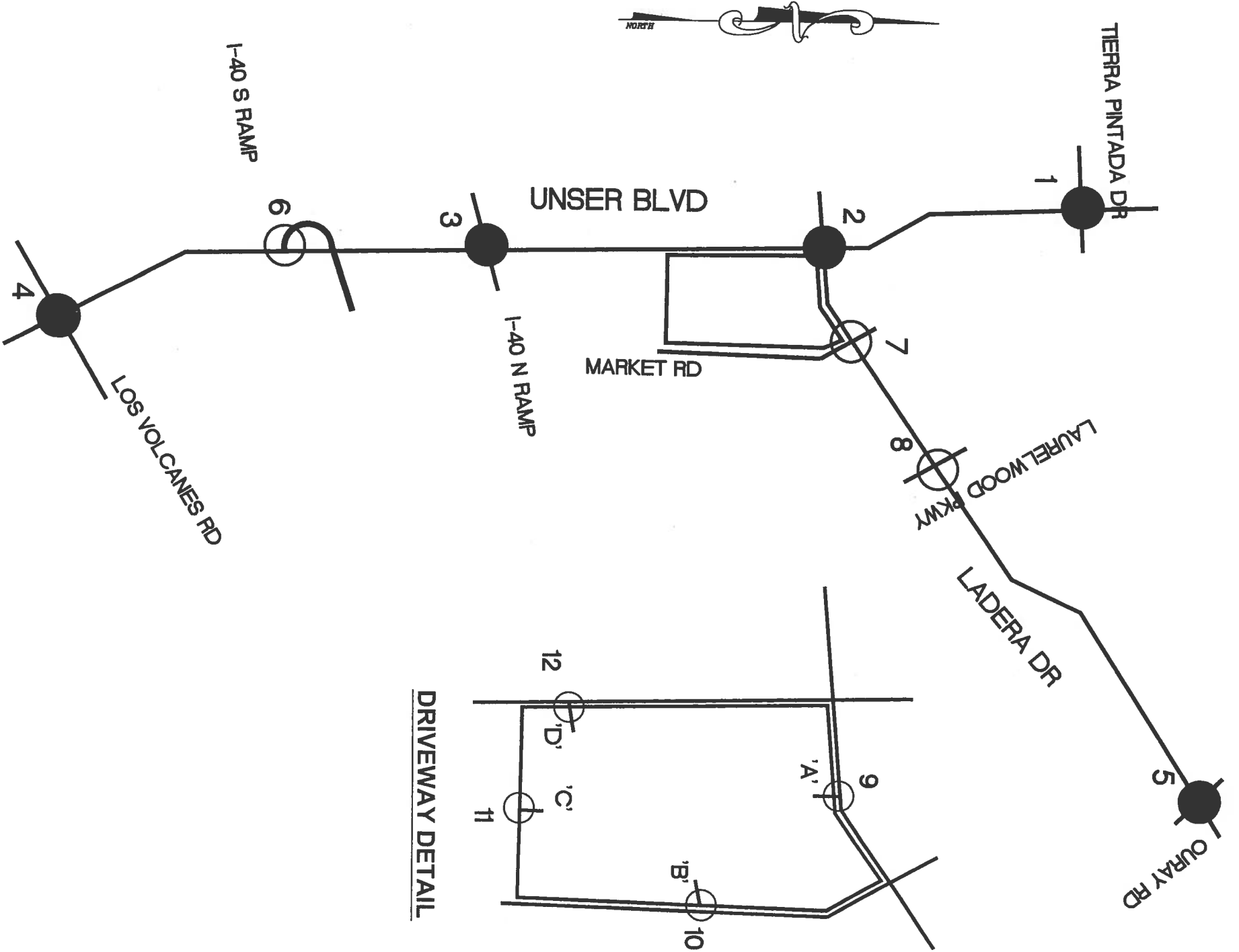
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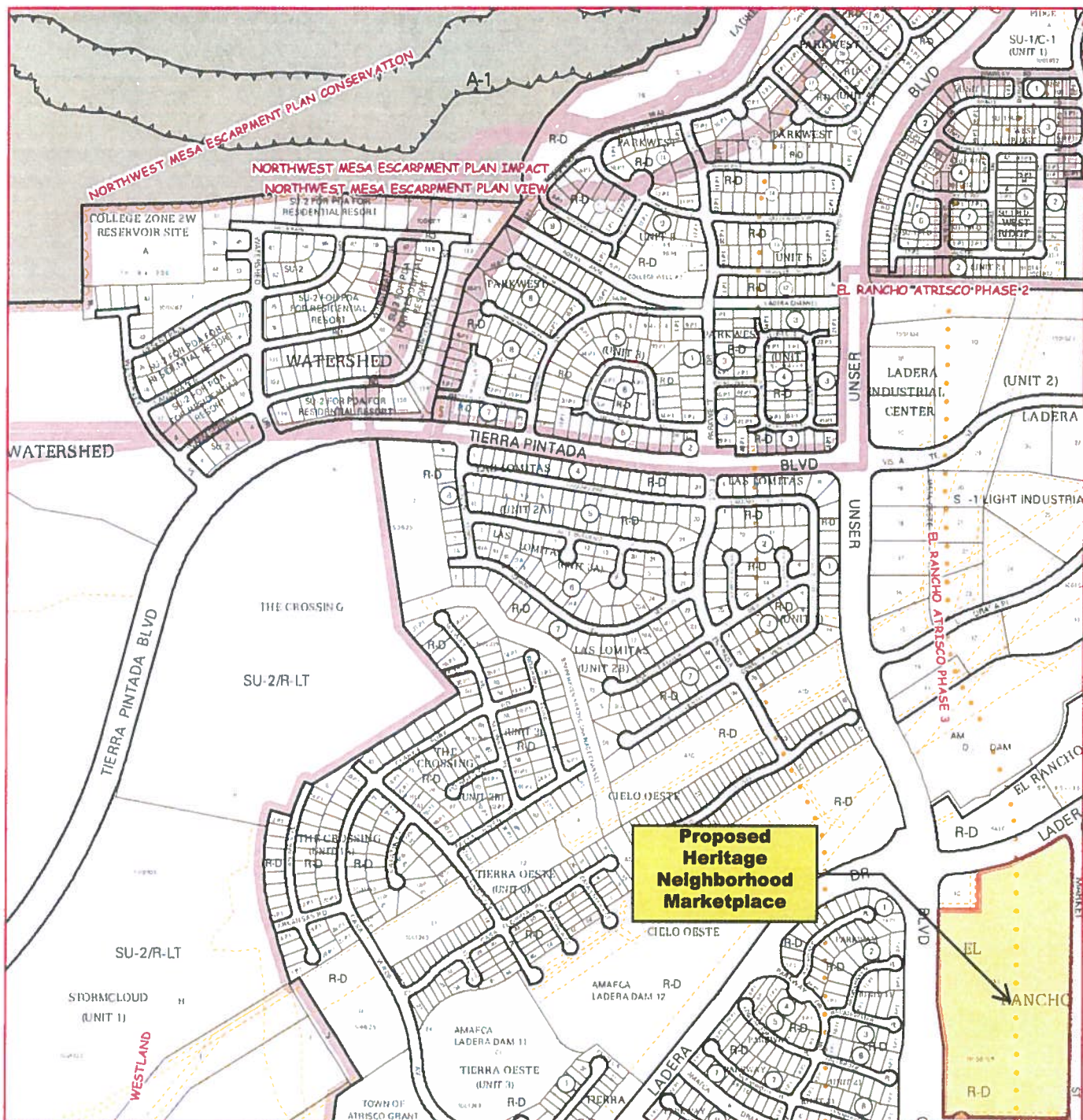
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DRIVEWAY DETAIL



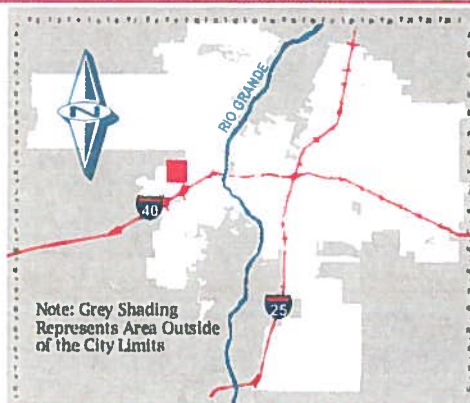
APPENDIX



For more current information and more details visit: <http://www.cabq.gov/gis>



Map amended through: 9/5/2006



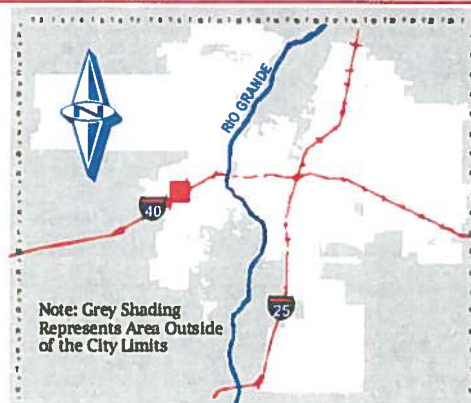
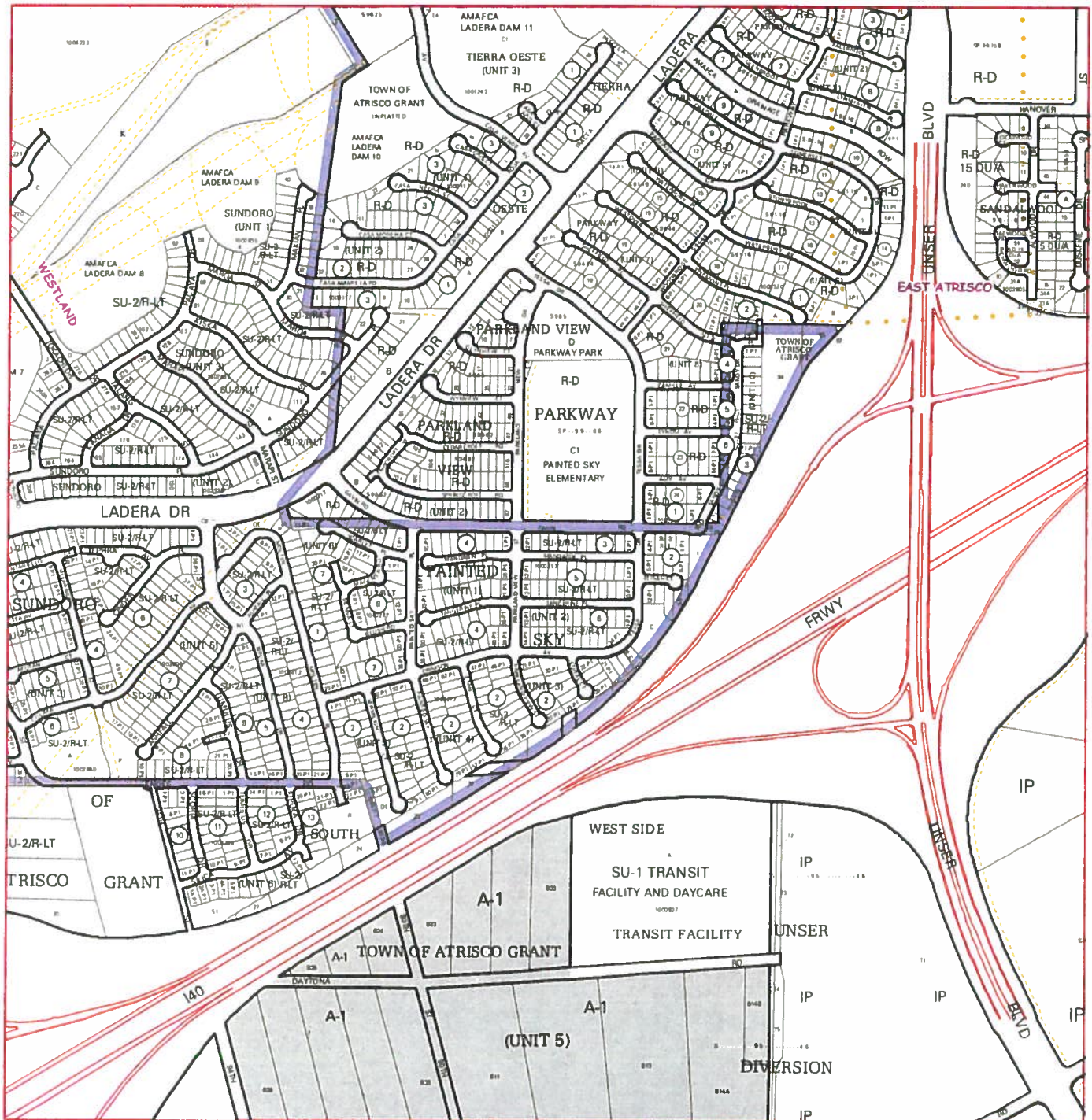
Zone Atlas Page:

H-09-Z

Selected Symbols

- SECTOR PLANS
- Escarpment
- Design Overlay Zones
- 2 Mile Airport Zone
- City Historic Zones
- Airport Noise Contours
- H-1 Buffer Zone
- Wall Overlay Zone
- Petroglyph Mon.





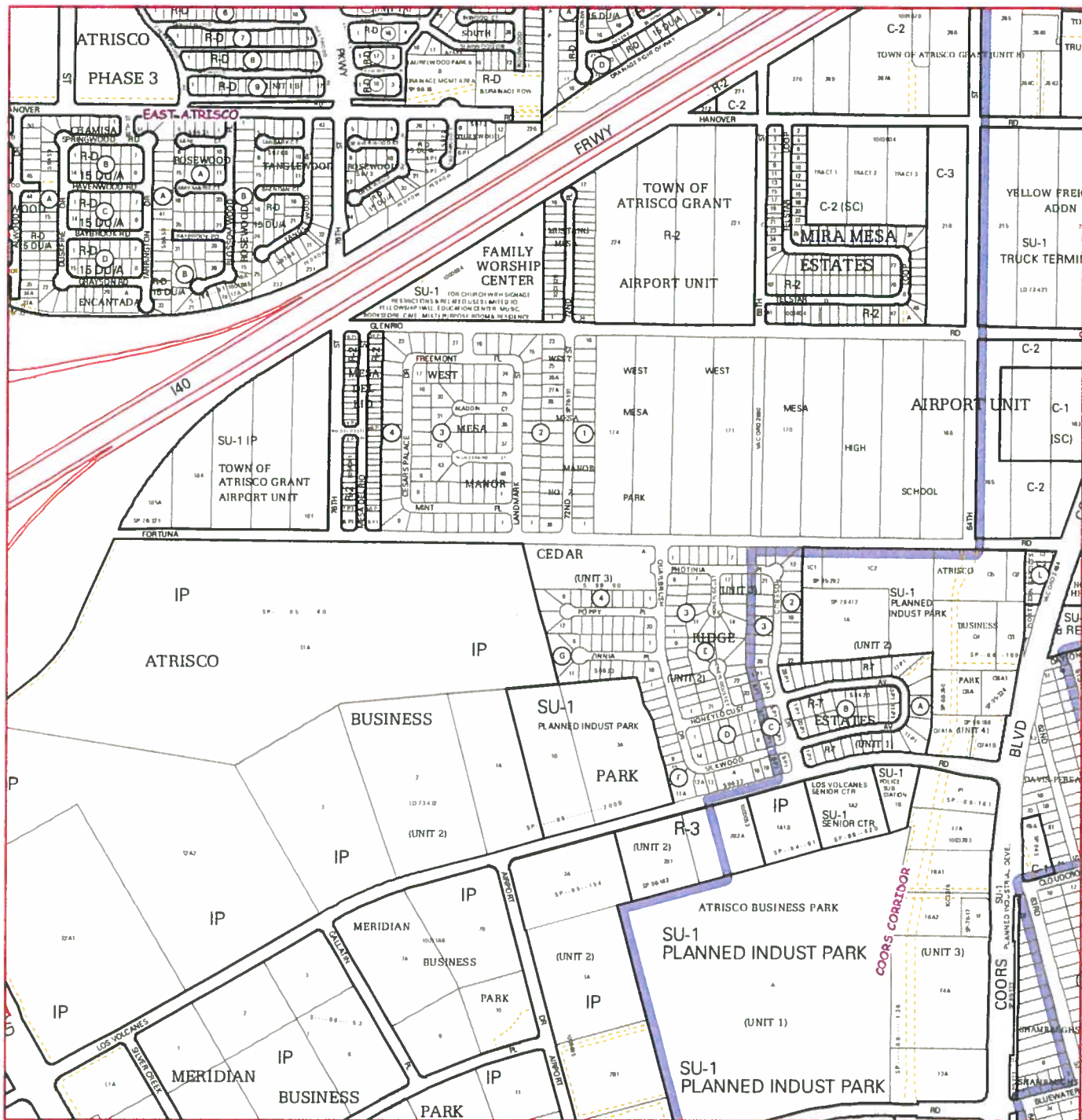
Zone Atlas Page:

J-09-Z

Selected Symbols

- SECTOR PLANS**
- Design Overlay Zones
 - City Historic Zones
 - H-1 Buffer Zone
 - Petroglyph Mon.
 - Escarpment
 - 2 Mile Airport Zone
 - Airport Noise Contours
 - Wall Overlay Zone





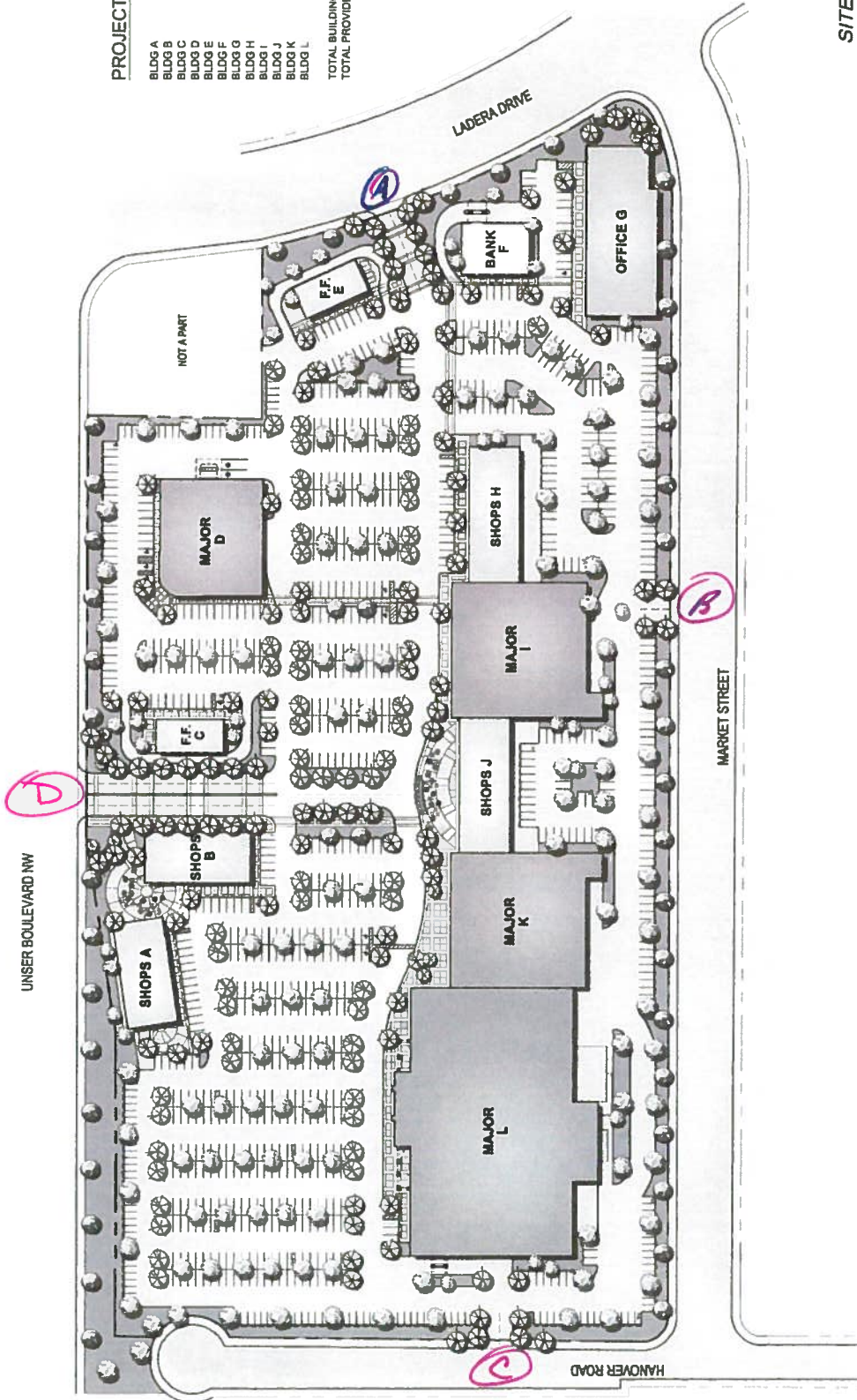
For more current information and more details visit: <http://www.cabq.gov/gis>



PROJECT SUMMARY

BLDG A	7,500 SF
BLDG B	7,500 SF
BLDG C	3,200 SF
BLDG D	16,500 SF
BLDG E	3,200 SF
BLDG F	4,500 SF
BLDG G	33,000 SF
BLDG H	9,600 SF
BLDG I	28,200 SF
BLDG J	9,600 SF
BLDG K	28,200 SF
BLDG L	96,000 SF
TOTAL BUILDING AREA	213,000 SF
TOTAL PROVIDED PARKING	1,068 STALLS
	5 STALLS/1,000 SF

To Be Revised



SITE PLAN SCHEME 4



HERITAGE NEIGHBORHOOD MARKETPLACE ALBUQUERQUE, NM

HERITAGE NEIGHBORHOOD LLC

ARCHITECTS ORANGE



- Facility with HCV Potential
- Limited Access Roadway
- Principal Arterial
- Minor Arterial
- Collector

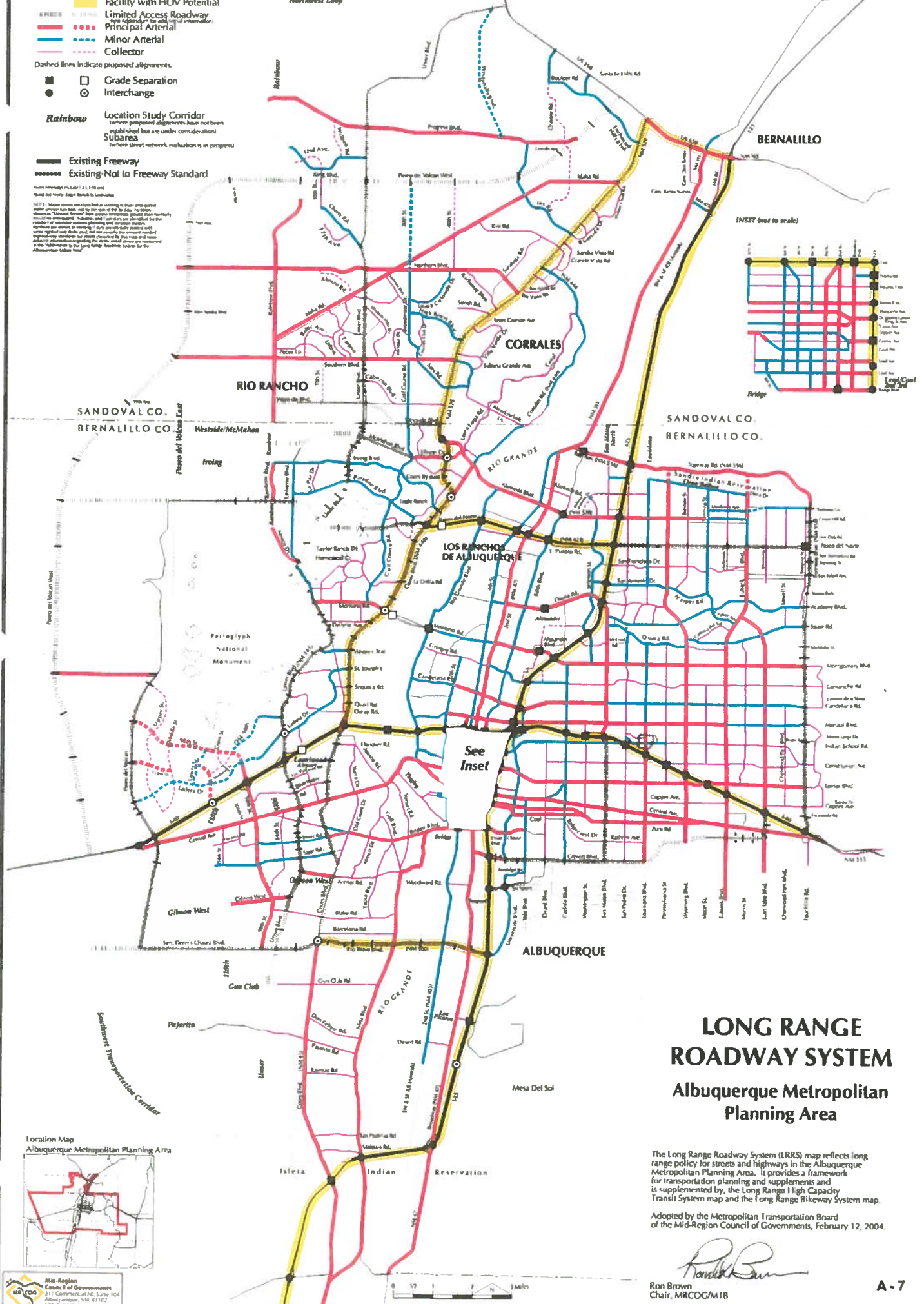
Dashed lines indicate proposed alignments

- Grade Separation
- Interchange

Rainbow Location Study Corridor
where proposed alignments have not been
established but are under consideration
Subarea
where street network evaluation is in progress

- Existing Freeway
- Existing-Not to Freeway Standard

Notes: 1. Major streets are shown as existing to their present
widths unless otherwise noted. 2. The color of the line designates
the facility type. 3. Limited Access Roadways are shown with
dashed lines. 4. Principal Arterials are shown with solid lines.
5. Minor Arterials are shown with dashed lines. 6. Collectors are
shown with dotted lines. 7. Grade Separations are shown with
small squares. 8. Interchanges are shown with circles. 9. The
Rainbow Location Study Corridor is shown with a thick black line.
10. Subareas are shown with thick black outlines. 11. The
Long Range Roadway System is shown with a thick black line.
12. The Long Range High Capacity Transit System is shown
with a thick black line.



LONG RANGE ROADWAY SYSTEM Albuquerque Metropolitan Planning Area

The Long Range Roadway System (LRRS) map reflects long range policy for streets and highways in the Albuquerque Metropolitan Planning Area. It provides a framework for transportation planning and supplements and is supplemented by the Long Range High Capacity Transit System map and the Long Range Bikeway System map.

Adopted by the Metropolitan Transportation Board of the Mid-Region Council of Governments, February 12, 2004.

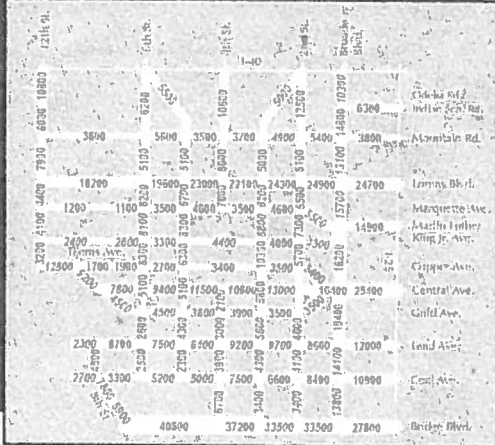
Ron Brown
Ron Brown
Chair, MRCOG/MTB

Location Map
Albuquerque Metropolitan Planning Area



Mid-Region Council of Governments
211 Commercial NE, Suite 104
Albuquerque, NM 87102
505 241 1700

Inset for Downtown



Inset for Uptown



Average Weekday Traffic Flows

- 0 - 900
- 1000 - 4900
- 5000 - 14900
- 15000 - 24900
- 25000 - 34900
- 35000 - 44900
- 45000 - 54900
- 55000 - 194900

Standard Data Link Volume is based on traffic count data accepted by the NM Department of Transportation Traffic Monitoring System (TMS) as standard in accordance with the New Mexico State Traffic Monitoring Standards (NMSTMS).

Non-Standard Data Link Volume is based either on traffic count data not in compliance with the NMSTMS or on professional judgement; NMDOT recommends that nonstandard data be used with caution.



Map prepared by the Mid-Region Council of Governments in cooperation with the New Mexico Department of Transportation, the local governments in the Albuquerque Metropolitan Planning Area, and the U.S. Department of Transportation, Federal Highway Administration.

Inset for Los Lunas, Valencia County



2006 Traffic Flows for the Greater Albuquerque Area

8/29/2007

Heritage Neighborhood Marketplace (Ladera / Unser) **Trip Generation Data**

COMMENT	USE (ITE CODE)							
	DESCRIPTION							
	24 HR VOL		A. M. PEAK HR.		P. M. PEAK HR.			
		GROSS	ENTER	EXIT	ENTER	EXIT		
Summary Sheet								
	Units							
BLDGSA, B, D, H, I, J, K	Shopping Center (820)	103.10	6,927	97	62	308	333	
Building L	Supermarket (850)	66.00	5,810	183	117	343	329	
Building C	Fast Food Restaurant w/ Drive-Thru Window (934)	3.20	1,588	87	83	58	53	
Building E	Fast Food Restaurant w/ Drive-Thru Window (934)	3.20	1,588	87	83	58	53	
Building F	Drive-In Bank (912)	4	1,563	45	33	102	102	
Building G	General Office Building (710)	33.00	568	68	9	20	96	
	Subtotal		18,044	567	387	889	966	
	Pass-by Trip Credit							
	30%					(267)	(290)	
	Net new Trips to System		18,044	567	387	622	676	

*Heritage Neighborhood Marketplace (Ladera / Unser)
Trip Generation Data*

USE (ITE CODE)	24 HOUR TWO-WAY VOLUME	A.M. PEAK HOUR		P.M. PEAK HOUR		
		ENTER	EXIT	ENTER	EXIT	
Units						
		6,927	97	62	308	333
		103.10				
Shopping Center (820)						

ITE Trip Generation Equations:

Average Vehicle Trip Ends on a Weekday (24 HOUR TWO-WAY VOLUME)

$$\text{Ln}(T) = 0.65 \text{ Ln}(X) + 5.83$$

50% Enter, 50% Exit

Average Vehicle Trip Ends on a Weekday, Peak Hour of Adjacent Street Traffic, One Hour Between 7am and 9am (A.M. PEAK HOUR)

$$\text{Ln}(T) = 0.6 \text{ Ln}(X) + 2.29$$

61% Enter, 39% Exit

Average Vehicle Trip Ends on a Weekday, Peak Hour of Adjacent Street Traffic, One Hour Between 4pm and 6pm (P.M. PEAK HOUR)

$$\text{Ln}(T) = 0.66 \text{ Ln}(X) + 3.403$$

48% Enter, 52% Exit

Comments:

BLDGS A, B, D, H, I, J, K

Based on ITE Trip Generation Manual - 7th Edition

Heritage Neighborhood Marketplace (Ladera / Unser)
Trip Generation Data

USE (ITE CODE)	24 HOUR TWO-WAY VOLUME		A.M. PEAK HOUR		P.M. PEAK HOUR
	GROSS	ENTER	EXIT	ENTER	EXIT
Supermarket (850)	Units				
	66.00	5,810	183	117	343
	1,000 S.F.				
					329

ITE Trip Generation Equations:

Average Vehicle Trip Ends on a Weekday (24 HOUR TWO-WAY VOLUME)

$$T = 50\% \text{ Enter, } 66.95 (X) + 1391.56 \text{ } 50\% \text{ Exit}$$

Average Vehicle Trip Ends on a Weekday, Peak Hour of Adjacent Street Traffic, One Hour Between 7am and 9am (A.M. PEAK HOUR)

$$\text{Ln}(T) = 61\% \text{ Enter, } 1.7 \text{ Ln}(X) + -1.42 \text{ } 39\% \text{ Exit}$$

Average Vehicle Trip Ends on a Weekday, Peak Hour of Adjacent Street Traffic, One Hour Between 4pm and 6pm (P.M. PEAK HOUR)

$$\text{Ln}(T) = 51\% \text{ Enter, } 0.79 \text{ Ln}(X) + 3.2 \text{ } 49\% \text{ Exit}$$

Comments:
Building L

Based on ITE Trip Generation Manual - 7th Edition

Heritage Neighborhood Marketplace (Ladera / Unser)

Trip Generation Data

USE (ITE CODE)	24 HOUR TWO-WAY VOLUME	A.M. PEAK HOUR		P.M. PEAK HOUR		
		ENTER	EXIT	ENTER	EXIT	
Units						
		1,588	87	83	58	53
		3.20				
		1,000 S.F.				
Fast Food Restaurant w/ Drive-Thru Window (934)						

ITE Trip Generation Equations:

Average Vehicle Trip Ends on a Weekday (24 HOUR TWO-WAY VOLUME)

$$T = \frac{496.12 (X) + 0}{50\% \text{ Enter, } 50\% \text{ Exit}}$$

Average Vehicle Trip Ends on a Weekday, Peak Hour of Adjacent Street Traffic, One Hour Between 7am and 9am (A.M. PEAK HOUR)

$$T = \frac{53.11 (X) + 0}{51\% \text{ Enter, } 49\% \text{ Exit}}$$

Average Vehicle Trip Ends on a Weekday, Peak Hour of Adjacent Street Traffic, One Hour Between 4pm and 6pm (P.M. PEAK HOUR)

$$T = \frac{34.64 (X) + 0}{52\% \text{ Enter, } 48\% \text{ Exit}}$$

Comments:
Building C

Based on ITE Trip Generation Manual - 7th Edition

Heritage Neighborhood Marketplace (Ladera / Unser)

Trip Generation Data

USE (ITE CODE)	24 HOUR TWO-WAY VOLUME	A.M. PEAK HOUR		P.M. PEAK HOUR	
		ENTER	EXIT	ENTER	EXIT
Units					
		3.20			
		1,000 S.F.			
		1,588	87	83	58
					53

Fast Food Restaurant w/ Drive-Thru Window (934)

ITE Trip Generation Equations:

Average Vehicle Trip Ends on a Weekday (24 HOUR TWO-WAY VOLUME)

$$T = \frac{496.12 (X) + 0}{50\% \text{ Enter, } 50\% \text{ Exit}}$$

Average Vehicle Trip Ends on a Weekday, Peak Hour of Adjacent Street Traffic, One Hour Between 7am and 9am (A.M. PEAK HOUR)

$$T = \frac{53.11 (X) + 0}{51\% \text{ Enter, } 49\% \text{ Exit}}$$

Average Vehicle Trip Ends on a Weekday, Peak Hour of Adjacent Street Traffic, One Hour Between 4pm and 6pm (P.M. PEAK HOUR)

$$T = \frac{34.64 (X) + 0}{52\% \text{ Enter, } 48\% \text{ Exit}}$$

Comments:
Building E

Based on ITE Trip Generation Manual - 7th Edition

Heritage Neighborhood Marketplace (Ladera / Unser)

Trip Generation Data

USE (ITE CODE)	24 HOUR TWO-WAY VOLUME	A.M. PEAK HOUR		P.M. PEAK HOUR	
		ENTER	EXIT	ENTER	EXIT
Units					
Drive-In Bank (912)					
	GROSS	ENTER	EXIT	ENTER	EXIT
		4			
		1,563	45	33	102
		Drive-In Windows			

ITE Trip Generation Equations:

Average Vehicle Trip Ends on a Weekday (24 HOUR TWO-WAY VOLUME)

$$\ln(T) = \begin{matrix} 1.326 \\ 50\% \text{ Enter,} \end{matrix} \ln(X) + \begin{matrix} 5.516 \\ 50\% \text{ Exit} \end{matrix}$$

Average Vehicle Trip Ends on a Weekday, Peak Hour of Adjacent Street Traffic, One Hour Between 7am and 9am (A.M. PEAK HOUR)

$$T = \begin{matrix} 19.38 \\ 58\% \text{ Enter,} \end{matrix} (X) + \begin{matrix} 0 \\ 42\% \text{ Exit} \end{matrix}$$

Average Vehicle Trip Ends on a Weekday, Peak Hour of Adjacent Street Traffic, One Hour Between 4pm and 6pm (P.M. PEAK HOUR)

$$T = \begin{matrix} 51.08 \\ 50\% \text{ Enter,} \end{matrix} (X) + \begin{matrix} 0 \\ 50\% \text{ Exit} \end{matrix}$$

Comments:
Building F

Based on ITE Trip Generation Manual - 7th Edition

Heritage Neighborhood Marketplace (Ladera / Unser)

Trip Generation Data

USE (ITE CODE)	24 HOUR TWO-WAY VOLUME		A.M. PEAK HOUR		P.M. PEAK HOUR	
	GROSS	ENTER	EXIT	ENTER	EXIT	EXIT
Units						
33.00						
1,000 S.F.						
General Office Building (710)						
	568	68	9	20	96	

ITE Trip Generation Equations:

Average Vehicle Trip Ends on a Weekday (24 HOUR TWO-WAY VOLUME)

$$\ln(T) = \begin{matrix} 0.77 & \ln(X) + & 3.65 \\ 50\% & \text{Enter,} & 50\% \text{ Exit} \end{matrix}$$

Average Vehicle Trip Ends on a Weekday, Peak Hour of Adjacent Street Traffic, One Hour Between 7am and 9am (A.M. PEAK HOUR)

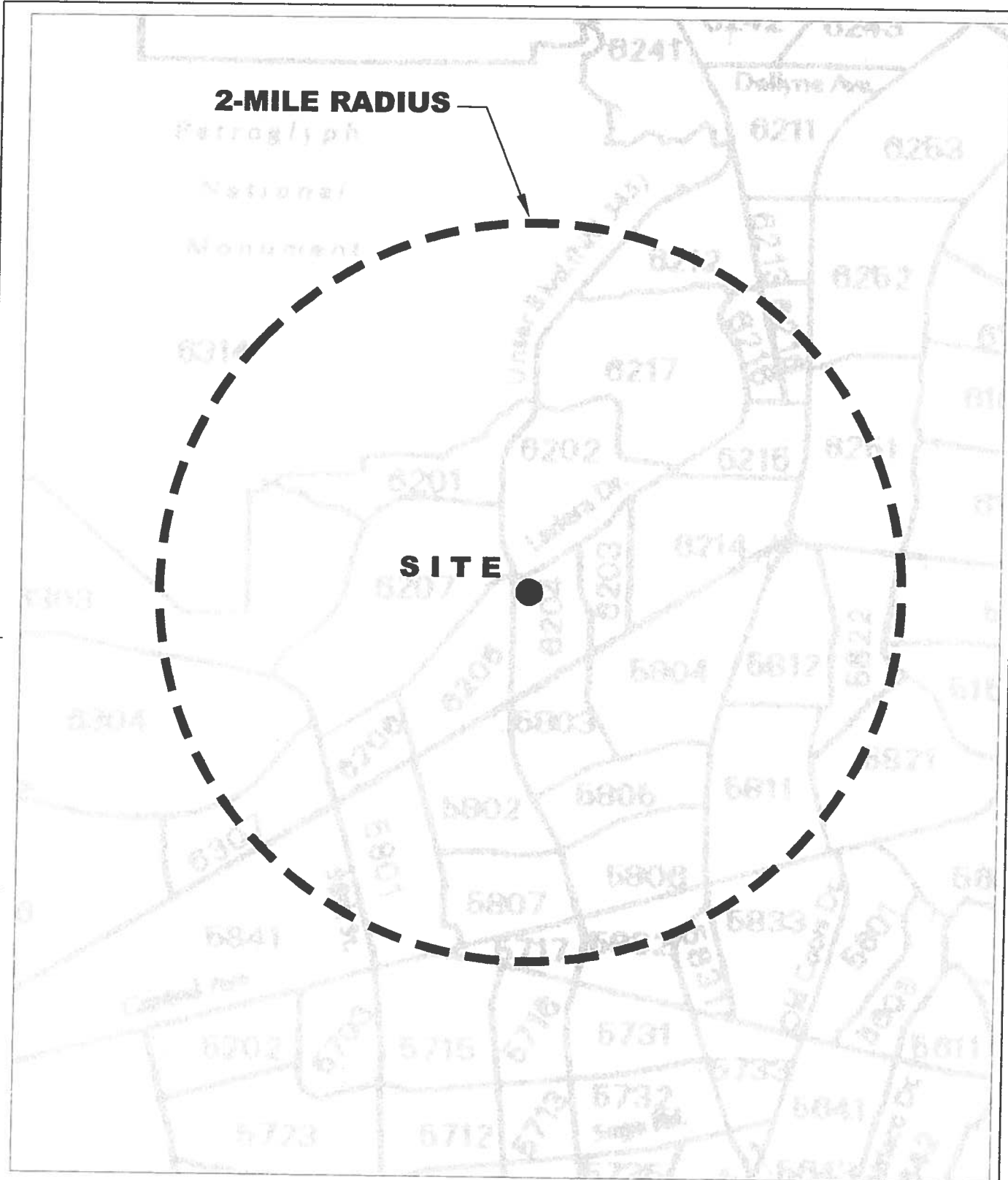
$$\ln(T) = \begin{matrix} 0.8 & \ln(X) + & 1.55 \\ 88\% & \text{Enter,} & 12\% \text{ Exit} \end{matrix}$$

Average Vehicle Trip Ends on a Weekday, Peak Hour of Adjacent Street Traffic, One Hour Between 4pm and 6pm (P.M. PEAK HOUR)

$$T = \begin{matrix} 1.12 & (X) + & 78.81 \\ 17\% & \text{Enter,} & 83\% \text{ Exit} \end{matrix}$$

Comments:
Building G

Based on ITE Trip Generation Manual - 7th Edition



DATA ANALYSIS SUBZONE (DASZ) MAP
Heritage Neighborhood Center (Ladera Dr / Unser Blvd)

Trip Distribution Table

Heritage Neighborhood Center (Ladera Dr / Unser Blvd)

Data Analysis Subzone Population Data for determination of Local Trip Distribution for Proposed Retail Commercial Trips

2004 and 2030 Data Taken from Mid-Region Council of Governments' 2030 Socioeconomic
2030 Socioeconomic Forecasts by Data Analysis Subzones for the Mid-Region of New Mexico

DASZ #	% Sub Area in Study	2004 Population		2030 Population	Interpolated Population for the Year 2010	Population in Study	Percent Population	(UN)			(TE)			(MN)		
		2004	2030					% Utilizing	Population	% Utilizing	% Population Utilizing	Population	% Utilizing	% Population Utilizing	Population	% Utilizing
Boundary Specified on DASZ Map																
5717	90%	3	350		83	75	0.22%	0%								
5801	95%	542	926	631	599	599	1.78%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	
5802	100%	467	432	459	459	459	1.35%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	
5803	100%	0	0	0	0	0	0.00%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	
5804	100%	1983	2412	2,082	2,082	2,082	8.12%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	
5805	100%	79	97		83	83	0.24%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	
5806	100%	609	635	615	615	615	1.81%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	
5807	100%	737	1424	898	898	898	2.63%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	
5811	95%	3959	3816	3,928	3,730	3,730	10.86%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	
5812	100%	2322	2177	2,289	2,289	2,289	6.73%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	
5821	20%	1901	1916	1,904	381	381	1.12%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	
5822	100%	998	1006	1,000	1,000	1,000	2.84%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	
5832	50%	1214	1283	1,230	615	615	1.81%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	
5841	10%	479	438	470	47	47	0.14%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	
6201	100%	1158	1691	1,281	1,281	1,281	3.77%	25%	0.94%	320	0%	0.00%	0	0%	0.00%	
6202	100%	910	1520	1,051	1,051	1,051	3.09%	30%	0.83%	315	30%	0.83%	315	5%	0.15%	
6203	100%	870	835	882	882	882	2.53%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	
6204	100%	1209	1357	1,243	1,243	1,243	3.65%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	
6205	100%	1331	1312	1,327	1,327	1,327	3.90%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	
6206	100%	0	854	197	197	197	0.58%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	
6207	100%	1998	4709	2,624	2,624	2,624	7.71%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	
6212	40%	1889	2225	1,967	787	787	2.31%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	
6214	100%	3411	3331	3,393	3,393	3,393	9.97%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	
6215	100%	1769	1673	1,747	1,747	1,747	5.14%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	
6216	55%	333	400	348	191	191	0.56%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	
6217	100%	2742	2653	2,721	2,721	2,721	8.00%	100%	8.00%	2,721	0%	0.00%	0	0%	0.00%	
6218	100%	1758	1989	1,811	1,811	1,811	5.32%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	
6251	60%	1893	1788	1,869	1,121	1,121	3.30%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	
6303	50%	0	2684	618	310	310	0.91%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	
6304	30%	0	4261	983	295	295	0.87%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	
6307	55%	0	1460	337	185	185	0.54%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	
6314	25%	2	0	2	1	1	0.00%	100%	0.00%	1	0%	0.00%	0	0%	0.00%	
							100.00%				3,358				315	0.83%
											9.87%				0.83%	0.15%

Trip Distribution Table

Heritage Neighborhood Center (Ladera Dr / Unser Blvd)

Data Analysis Subzone Population Data for determination of Local Trip Distribution for Proposed Retail Commercial Trips

2004 and 2030 Data Taken from Mid-Region Council of Governments' 2030 Socioeconomic

2030 Socioeconomic Forecasts by Data Analysis Subzones for the Mid-Region of New Mexico

DASZ #	% Sub Area in Study	2004 Population		2030 Population	Interpolated Population for the Year 2010	Population in Study	Percent Population	(LN)			(ON)			(LE)		
		2004	2030	% Utilizing				% Population Utilizing	Population	% Utilizing	% Population Utilizing	Population	% Utilizing	% Population Utilizing		
Boundary Specified on DASZ Map																
5717	90%	3	350	83	75	0.22%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0
5801	95%	542	926	631	598	1.76%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0
5802	100%	467	432	459	459	1.35%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0
5803	100%	0	0	0	0	0.00%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0
5804	100%	1983	2412	2,082	2,082	6.12%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0
5805	100%	79	97	83	83	0.24%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0
5806	100%	609	635	615	615	1.81%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0
5807	100%	737	1424	866	898	2.63%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0
5811	95%	3959	3816	3,928	3,730	10.96%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0
5812	100%	2322	2177	2,289	2,289	6.73%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0
5821	20%	1901	1916	1,804	381	1.12%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0
5822	100%	998	1006	1,000	1,000	2.94%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0
5832	50%	1214	1283	1,230	615	1.81%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0
5841	10%	479	438	470	47	0.14%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0
6201	100%	1158	1691	1,281	1,281	3.77%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0
6202	100%	910	1520	1,051	1,051	3.09%	5%	0.15%	53	30%	0.93%	315	0%	0.00%	0	0
6203	100%	870	835	862	862	2.53%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0
6204	100%	1209	1357	1,243	1,243	3.85%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0
6205	100%	1331	1312	1,327	1,327	3.90%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0
6206	100%	0	854	197	197	0.58%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0
6207	100%	1998	4709	2,624	2,624	7.71%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0
6212	40%	1889	2225	1,967	787	2.31%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0
6214	100%	3411	3331	3,393	3,393	9.97%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0
6215	100%	1769	1673	1,747	1,747	5.14%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0
6216	55%	333	400	348	191	0.56%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	787
6217	100%	2742	2653	2,721	2,721	8.00%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	874
6218	100%	1758	1989	1,811	1,811	5.32%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	191
6251	60%	1893	1788	1,869	1,121	3.30%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	1,811
6303	50%	0	2684	619	310	0.91%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	561
6304	30%	0	4261	983	295	0.87%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0
6307	55%	0	1460	337	185	0.54%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0
6314	25%	2	0	2	1	0.00%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0
						40,050	34,018	100.00%	53	0.15%	315	0.83%	4,223	12.41%		

Trip Distribution Table

Heritage Neighborhood Center (Ladera Dr / Unser Blvd)

Data Analysis Subzone Population Data for determination of Local Trip Distribution for Proposed Retail Commercial Trips

2004 and 2030 Data Taken from Mid-Region Council of Governments' 2030 Socioeconomic
2030 Socioeconomic Forecasts by Delta Analysis Subzones for the Mid-Region of New Mexico

DASZ #	% Sub Area in Study	2004 Population		2030 Population	Interpolated Population for the Year 2010	Population In Study	Percent Population	(OS)			(LS)			(MS)			
		2004	2030	% Utilizing				% Population Utilizing	Population	% Utilizing	% Population Utilizing	Population	% Utilizing	% Population Utilizing			
Boundary Specified on DASZ Map																	
5717	90%	3	350	83	75	0.22%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0	
5801	95%	542	926	631	599	1.76%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0	
5802	100%	467	432	459	459	1.35%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0	
5803	100%	0	0	0	0	0.00%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0	
5804	100%	1983	2412	2082	2082	6.12%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0	
5805	100%	79	97	83	83	0.24%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0	
5806	100%	609	635	615	615	1.81%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0	
5807	100%	737	1424	886	896	2.83%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0	
5811	95%	3859	3816	3928	3730	10.98%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0	
5812	100%	2322	2177	2289	2289	6.73%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0	
5821	20%	1901	1916	1904	381	1.12%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0	
5822	100%	998	1006	1000	1000	2.94%	50%	1.47%	500	0%	0.00%	0	0%	0.00%	0	0	
5832	50%	1214	1283	1230	615	1.81%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0	
5841	10%	479	438	470	47	0.14%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0	
6201	100%	1158	1691	1281	1281	3.77%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0	
6202	100%	910	1520	1051	1051	3.09%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0	
6203	100%	870	835	862	862	2.53%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0	
6204	100%	1209	1357	1243	1243	3.65%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0	
6205	100%	1331	1312	1327	1327	3.90%	0%	0.00%	0	100%	2.53%	882	50%	1.83%	882	0	
6206	100%	0	854	197	197	0.58%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	822	
6207	100%	1998	4708	2624	2624	7.71%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0	
6212	40%	1889	2225	1967	787	2.31%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0	
6214	100%	3411	3331	3393	3393	9.87%	50%	4.98%	1697	0%	0.00%	0	0%	0.00%	0	0	
6215	100%	1769	1673	1747	1747	5.14%	50%	2.57%	874	0%	0.00%	0	0%	0.00%	0	0	
6216	55%	333	400	348	191	0.56%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0	
6217	100%	2742	2653	2721	2721	8.00%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0	
6218	100%	1758	1989	1811	1811	5.32%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0	
6251	60%	1893	1788	1899	1121	3.30%	50%	1.65%	561	0%	0.00%	0	0%	0.00%	0	0	
6303	50%	0	2684	619	310	0.81%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0	
6304	30%	0	4261	983	265	0.87%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0	
6307	55%	0	1480	337	185	0.54%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0	
6314	25%	2	0	2	1	0.00%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0	
				40,050	34,018	100.00%				3,631				1,484			822
																1.83%	

Heritage Neighborhood Center (Ladera Dr / Unser Blvd)

2004 and 2030 Data Taken from Mid-Region Council of Governments' 2030 Socioeconomic
2030 Socioeconomic Forecasts by Data Analysis Subzones for the Mid-Region of New Mexico

DASZ #	% Sub Area in Study	2004 Population		2030 Population	Interpolated Population for the Year 2010	Population in Study	Percent Population	(IE)			(LVE)			(US)		
		2004						% Utilizing	Population	% Utilizing	Population	% Utilizing	Population	% Utilizing	Population	
Boundary Specified on DASZ Map																
5717	90%	3	350	83	75	0%	0.22%	0	0.00%	0	0%	0.00%	0	100%	0.22%	
5801	95%	542	926	631	599	0%	1.76%	0	0.00%	0	0%	0.00%	0	0%	0.00%	
5802	100%	467	459	459	459	0%	1.35%	0	0.00%	0	0%	0.00%	0	35%	0.47%	
5803	100%	0	0	0	0	0%	0.00%	0	0.00%	0	100%	0.00%	0	0%	0.00%	
5804	100%	1983	2412	2,082	2,082	35%	6.12%	729	2.14%	0	0%	0.00%	0	65%	3.89%	
5805	100%	79	97	83	83	0%	0.24%	0	0.00%	0	0%	0.00%	0	100%	0.24%	
5806	100%	609	635	615	615	0%	1.81%	0	0.00%	0	0%	0.00%	0	100%	1.81%	
5807	100%	737	1424	898	898	0%	2.83%	0	0.00%	0	0%	0.00%	0	100%	2.83%	
5811	95%	3959	3816	3,926	3,730	0%	10.96%	0	0.00%	0	50%	5.48%	1,885	50%	5.48%	
5812	100%	2322	2177	2,289	2,289	67%	6.73%	1,831	5.38%	114	20%	1.35%	458	0	0	
5821	20%	1901	1916	1,904	381	30%	1.12%	0	0.34%	0	0%	0.00%	0	70%	0.78%	
5822	100%	998	1006	1,000	1,000	50%	2.94%	500	1.47%	0	0%	0.00%	0	0%	0.00%	
5832	50%	1214	1283	1,230	615	0%	1.81%	0	0.00%	0	0%	0.00%	0	100%	1.81%	
5841	10%	479	438	470	47	0%	0.14%	0	0.00%	0	0%	0.00%	0	0%	0.00%	
6201	100%	1158	1091	1,281	1,281	0%	3.77%	0	0.00%	0	0%	0.00%	0	0%	0.00%	
6202	100%	910	1520	1,051	1,051	0%	3.99%	0	0.00%	0	0%	0.00%	0	0%	0.00%	
6203	100%	870	835	882	882	0%	2.53%	0	0.00%	0	0%	0.00%	0	0%	0.00%	
6204	100%	1209	1357	1,243	1,243	0%	3.65%	0	0.00%	0	0%	0.00%	0	0%	0.00%	
6205	100%	1331	1312	1,327	1,327	0%	3.90%	0	0.00%	0	0%	0.00%	0	0%	0.00%	
6206	100%	0	854	197	197	0%	0.55%	0	0.00%	0	0%	0.00%	0	0%	0.00%	
6207	100%	1998	4709	2,624	2,624	0%	7.71%	0	0.00%	0	0%	0.00%	0	0%	0.00%	
6212	40%	1889	2225	1,967	787	0%	2.31%	0	0.00%	0	0%	0.00%	0	0%	0.00%	
6214	100%	3411	3331	3,383	3,383	0%	9.97%	0	0.00%	0	0%	0.00%	0	0%	0.00%	
6215	100%	1769	1673	1,747	1,747	0%	5.14%	0	0.00%	0	0%	0.00%	0	0%	0.00%	
6216	55%	333	400	348	191	0%	0.56%	0	0.00%	0	0%	0.00%	0	0%	0.00%	
6217	100%	3429	2653	2,721	2,721	0%	8.00%	0	0.00%	0	0%	0.00%	0	0%	0.00%	
6218	100%	1758	1989	1,811	1,811	0%	5.32%	0	0.00%	0	0%	0.00%	0	0%	0.00%	
6251	60%	1893	1788	1,868	1,121	0%	3.30%	0	0.00%	0	0%	0.00%	0	0%	0.00%	
6303	50%	0	2684	619	310	0%	0.81%	0	0.00%	0	0%	0.00%	0	0%	0.00%	
6304	30%	0	4261	983	295	0%	0.87%	0	0.00%	0	0%	0.00%	0	0%	0.00%	
6307	55%	0	1460	337	185	100%	0.54%	185	0.54%	0	0%	0.00%	0	0%	0.00%	
6314	25%	2	0	2	1	0%	0.00%	0	0.00%	0	0%	0.00%	0	0%	0.00%	
						34,018	100.00%	3,359		2,323				5,930		
								9.87%		6.83%				17.43%		

Trip Distribution Table

Heritage Neighborhood Center (Ladera Dr / Unser Blvd)

Data Analysis Subzone Population Data for determination of Local Trip Distribution for Proposed Retail Commercial Trips

2004 and 2030 Data Taken from Mid-Region Council of Governments' 2030 Socioeconomic
2030 Socioeconomic Forecasts by Data Analysis Subzones for the Mid-Region of New Mexico

DASZ #	% Sub Area in Study	2004 Population		2030 Population	Interpolated Population for the Year 2010	Population in Study	Percent Population	Los Volcanes RU West			Interstate 40 West			Ladera Dr West		
		2004						% Utilizing	% Population Utilizing	Population	% Utilizing	% Population Utilizing	Population	% Utilizing	% Population Utilizing	Population
Boundary Specified on DASZ Map																
5717	90%	3	350	83	75	0.22%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	
5801	95%	542	926	631	599	1.76%	0%	0.00%	0	100%	1.76%	599	0%	0.00%	0	0
5802	100%	467	432	459	459	1.35%	65%	0.88%	298	0%	0.00%	0	0%	0.00%	0	0
5803	100%	0	0	0	0	0.00%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0
5804	100%	1983	2412	2,082	2,082	6.12%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0
5805	100%	79	97	83	83	0.24%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0
5806	100%	609	635	615	615	1.81%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0
5807	100%	737	1424	896	896	2.63%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0
5811	95%	3859	3816	3,926	3,730	10.96%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0
5812	100%	2322	2177	2,289	2,289	6.73%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0
5821	20%	1901	1916	1,904	381	1.12%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0
5822	100%	998	1006	1,000	1,000	2.84%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0
5832	50%	1214	1283	1,230	615	1.81%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0
5841	10%	479	438	470	47	0.14%	0%	0.00%	0	100%	0.14%	47	0%	0.00%	0	0
6201	100%	1158	1691	1,281	1,281	3.77%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0
6202	100%	910	1520	1,051	1,051	3.09%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0
6203	100%	870	835	862	862	2.53%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0
6204	100%	1209	1357	1,243	1,243	3.65%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0
6205	100%	1331	1312	1,327	1,327	3.90%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0
6206	100%	0	854	197	197	0.58%	0%	0.00%	0	0%	0.00%	0	100%	3.90%	1,327	1,327
6207	100%	1998	4709	2,824	2,824	7.71%	0%	0.00%	0	0%	0.00%	0	100%	0.58%	197	197
6212	40%	1899	2225	1,967	787	2.31%	0%	0.00%	0	0%	0.00%	0	50%	3.88%	1,312	1,312
6214	100%	3411	3331	3,393	3,393	9.97%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0
6215	100%	1769	1673	1,747	1,747	5.14%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0
6216	55%	333	400	348	191	0.56%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0
6217	100%	2742	2653	2,721	2,721	8.00%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0
6218	100%	1758	1989	1,811	1,811	5.32%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0
6251	60%	1893	1788	1,869	1,121	3.30%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0
6303	50%	0	2884	819	310	0.91%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0
6304	30%	0	4261	983	285	0.87%	0%	0.00%	0	0%	0.00%	0	100%	0.91%	310	310
6307	55%	0	1460	337	185	0.54%	0%	0.00%	0	0%	0.00%	0	100%	0.87%	285	285
6314	25%	2	0	0	1	0.00%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0	0
							100.00%	298			646			3,441		
								0.88%			1.90%			10.12%		

Trip Distribution Table

Heritage Neighborhood Center (Ladera Dr / Unscr Blvd)

Data Analysis Subzone Population Data for determination of Local Trip Distribution for Proposed Retail Commercial Trips

2004 and 2030 Data Taken from Mid-Region Council of Governments' 2030 Socioeconomic

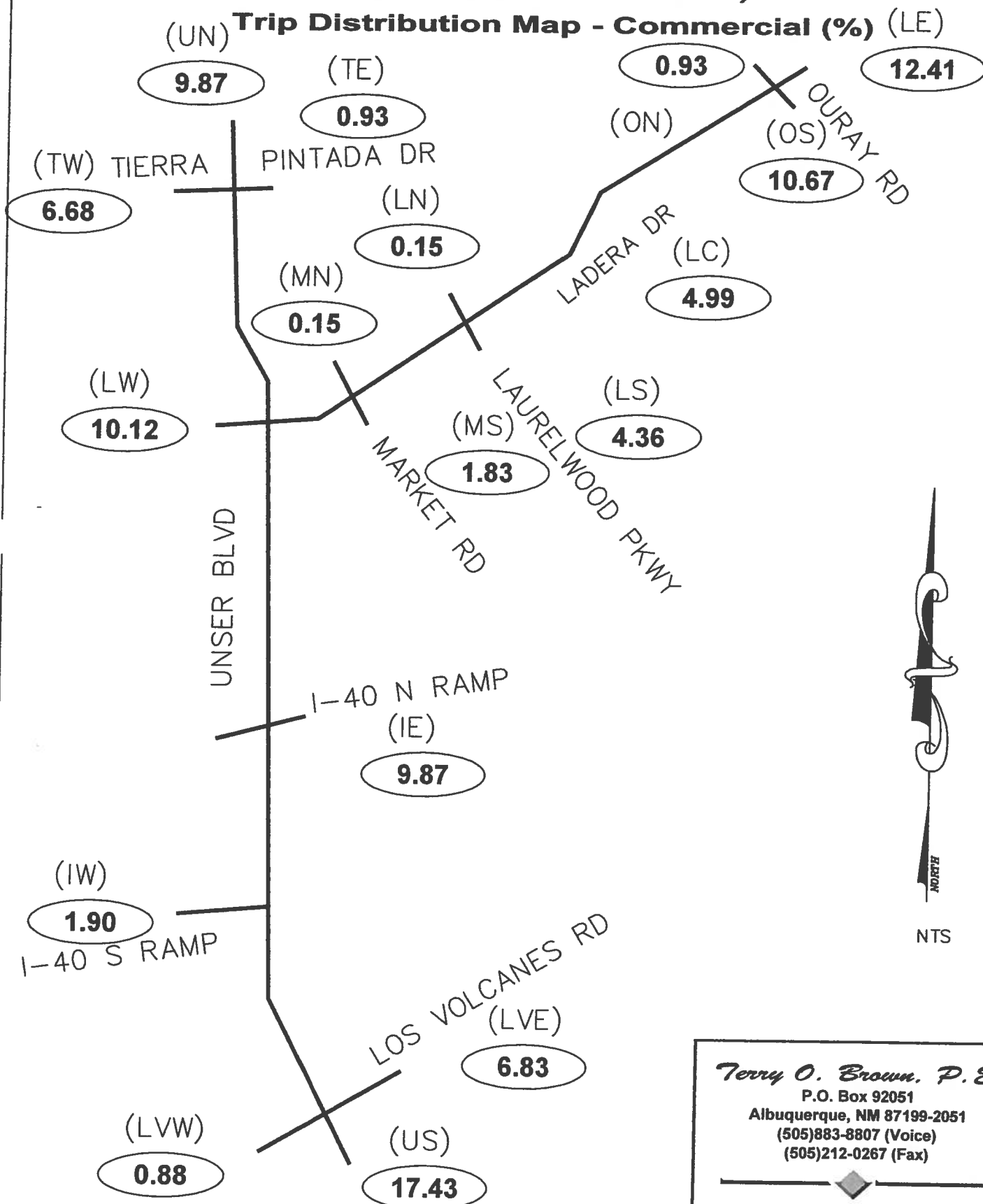
2030 Socioeconomic Forecasts by Data Analysis Subzones for the Mid-Region of New Mexico

DASZ #	% Sub Area in Study	2004 Population		Interpolated Population for the Year 2010	Population In Study	Percent Population	(TW)		(LC)			
		2004	2030				% Utilizing	% Population Utilizing	Population Utilizing	% Utilizing	% Population Utilizing	Population
Boundary Specified on DASZ Map												
5717	90%	3	350	83	75	0.22%	0%	0.00%	0	0%	0.00%	0
5801	95%	542	926	631	599	1.76%	0%	0.00%	0	0%	0.00%	0
5802	100%	467	432	459	459	1.35%	0%	0.00%	0	0%	0.00%	0
5803	100%	0	0	0	0	0.00%	0%	0.00%	0	0%	0.00%	0
5804	100%	1983	2412	2,082	2,082	6.12%	0%	0.00%	0	0%	0.00%	0
5805	100%	79	97	83	83	0.24%	0%	0.00%	0	0%	0.00%	0
5806	100%	609	635	615	615	1.81%	0%	0.00%	0	0%	0.00%	0
5807	100%	737	1424	866	866	2.63%	0%	0.00%	0	0%	0.00%	0
5811	95%	3959	3816	3,928	3,730	10.96%	0%	0.00%	0	0%	0.00%	0
5812	100%	2322	2177	2,289	2,289	6.73%	0%	0.00%	0	0%	0.00%	0
5821	20%	1901	1916	1,904	381	1.12%	0%	0.00%	0	0%	0.00%	0
5822	100%	998	1006	1,000	1,000	2.94%	0%	0.00%	0	0%	0.00%	0
5832	50%	1214	1283	1,230	615	1.81%	0%	0.00%	0	0%	0.00%	0
5841	10%	479	438	470	47	0.14%	0%	0.00%	0	0%	0.00%	0
6201	100%	1158	1691	1,281	1,281	3.77%	75%	2.82%	961	0%	0.00%	0
6202	100%	910	1520	1,051	1,051	3.09%	0%	0.00%	0	0%	0.00%	0
6203	100%	870	835	862	862	2.53%	0%	0.00%	0	0%	0.00%	0
6204	100%	1209	1357	1,243	1,243	3.65%	0%	0.00%	0	0%	0.00%	0
6205	100%	1331	1312	1,327	1,327	3.90%	0%	0.00%	0	0%	0.00%	0
6206	100%	0	854	197	197	0.58%	0%	0.00%	0	0%	0.00%	0
6207	100%	1998	4709	2,624	2,624	7.71%	50%	3.86%	1,312	0%	0.00%	0
6212	40%	1889	2225	1,987	787	2.31%	0%	0.00%	0	0%	0.00%	0
6214	100%	3411	3331	3,393	3,393	9.97%	0%	0.00%	0	0%	0.00%	0
6215	100%	1769	1673	1,747	1,747	5.14%	0%	0.00%	0	50%	4.99%	1,697
6216	55%	333	400	348	191	0.96%	0%	0.00%	0	0%	0.00%	0
6217	100%	2742	2853	2,721	2,721	8.00%	0%	0.00%	0	0%	0.00%	0
6218	100%	1758	1989	1,811	1,811	5.32%	0%	0.00%	0	0%	0.00%	0
6251	60%	1893	1788	1,869	1,121	3.30%	0%	0.00%	0	0%	0.00%	0
6303	50%	0	2684	619	310	0.91%	0%	0.00%	0	0%	0.00%	0
6304	30%	0	4261	983	285	0.87%	0%	0.00%	0	0%	0.00%	0
6307	55%	0	1460	337	185	0.54%	0%	0.00%	0	0%	0.00%	0
6314	25%	2	0	2	1	0.00%	0%	0.00%	0	0%	0.00%	0
						100.00%			2,273			1,697
									6.68%			4.99%

Heritage Neighborhood Center

(Ladera Dr / Unser Blvd)

Trip Distribution Map - Commercial (%)



NTS

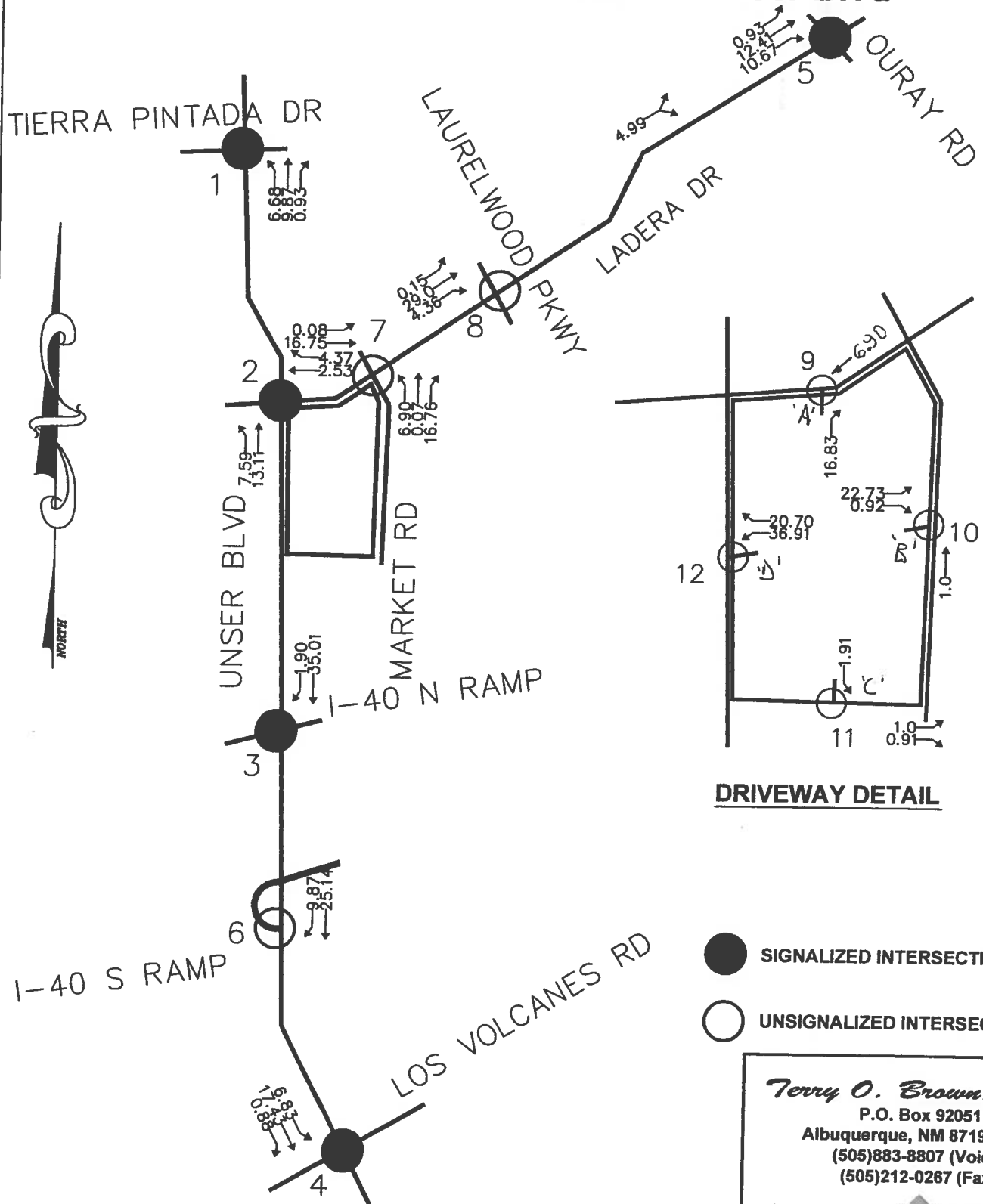
Terry O. Brown, P.E.

P.O. Box 92051
Albuquerque, NM 87199-2051
(505)883-8807 (Voice)
(505)212-0267 (Fax)

Heritage Neighborhood Center

(Ladera Dr / Unser Blvd)

Trip Assignments - Commercial (% Exiting)
Case F - Full access on Unser Blvd



DRIVEWAY DETAIL

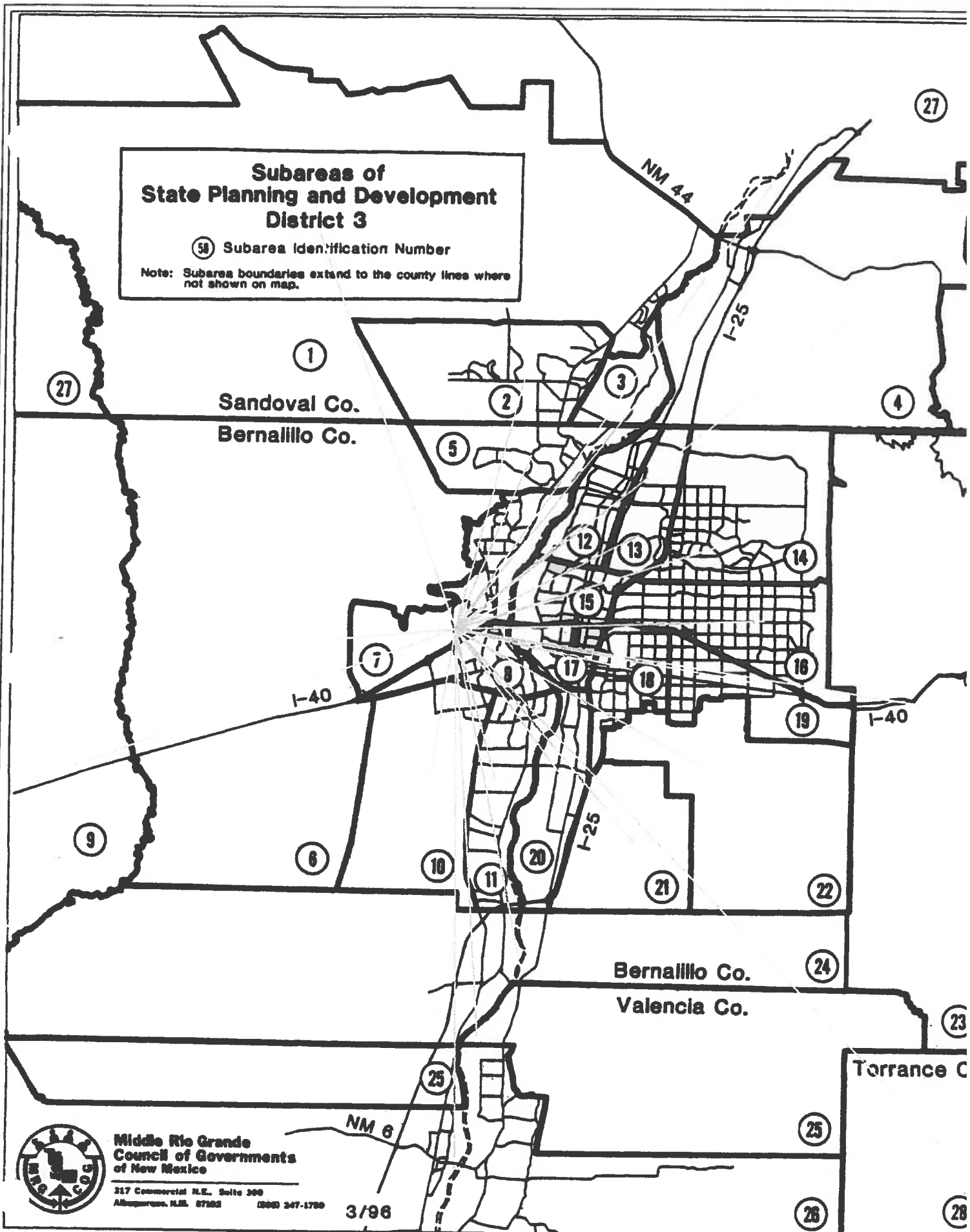
Terry O. Brown, P.E.

P.O. Box 92051
 Albuquerque, NM 87199-2051
 (505)883-8807 (Voice)
 (505)212-0267 (Fax)

Subareas of State Planning and Development District 3

⑤① Subarea Identification Number

Note: Subarea boundaries extend to the county lines where not shown on map.



Middle Rio Grande
Council of Governments
of New Mexico

317 Commercial N.E., Suite 300
Albuquerque, N.M. 87102 (505) 247-1799

3/96

Heritage Neighborhood Center (Ladera Dr / Unser Blvd)
Trip Distribution - Subarea Map

Figure 4

Trip Distribution Table

Heritage Neighborhood Center

Sub Area Population Data:

For determination of Trip Distribution for Proposed Office Development Trips

2004 and 2030 Data Taken from Multi-Region Council of Governments 2030 Socioeconomic
 Forecasts by Data Analysis Solutions for the Mid-Region of New Mexico

Sub Area I.D.#	% Sub Area In Study	2004 Population	2030 Population	Interpolated Population for the Year 2010	Population in Study	Dist. (Mi.)	Population / Distance	% Population / Distance	(UN)			(TE)			(MN)		
									% Utilizing	% Population / Dist. Utilizing	Population	% Utilizing	% Population / Dist. Utilizing	Population	% Utilizing	% Population / Dist. Utilizing	Population
1	100%	28,972	38,738	28,918	2,010	20.77	97	0.08%	20%	0.02%	19	0%	0.00%	0	0%	0.00%	0
2	100%	39,348	40,810	39,639	29,918	12.80	2,338	2.00%	50%	1.00%	1,169	0%	0.00%	0	0%	0.00%	0
3	100%	7,985	8,728	8,084	8,084	14.94	540	0.46%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
4	100%	13,387	14,936	13,744	13,744	25.46	540	0.46%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
5	100%	35,968	44,203	37,988	37,988	9.38	4,037	3.45%	50%	1.73%	2,019	0%	0.00%	0	0%	0.00%	0
6	100%	2,784	3,950	3,053	3,053	8.78	348	0.30%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
7*	100%	48,565	59,815	51,115	51,115	1.01	50,861	43.52%	32%	13.88%	16,230	3%	1.29%	1,511	0%	0.04%	51
8	100%	27,546	28,553	27,778	27,778	2.88	9,642	8.25%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
9	100%	1,678	1,888	1,728	1,728	19.97	86	0.07%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
10	100%	39,532	4,822	31,522	31,522	7.44	4,238	3.63%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
11	100%	32,051	33,202	32,317	32,317	7.91	4,088	3.50%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
12	100%	18,144	18,148	18,144	18,144	8.98	1,798	1.64%	50%	0.77%	899	0%	0.00%	0	0%	0.00%	0
13	100%	8,715	10,146	9,045	9,045	10.18	888	0.76%	50%	0.38%	444	0%	0.00%	0	0%	0.00%	0
14	100%	93,104	94,278	93,375	93,375	14.00	6,668	6.71%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
15	100%	24,891	25,262	24,823	24,823	5.83	4,258	3.64%	33%	1.20%	1,405	0%	0.00%	0	0%	0.00%	0
16	100%	108,882	108,353	108,780	108,780	15.28	7,120	6.09%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
17	100%	20,920	21,198	20,984	20,984	5.70	3,885	3.16%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
18	100%	42,078	41,870	41,984	41,984	8.81	4,711	4.03%	6%	0.00%	0	0%	0.00%	0	0%	0.00%	0
19	100%	59,027	58,888	58,995	58,995	15.54	3,765	3.25%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
20	100%	9,482	9,899	9,532	9,532	9.11	1,048	0.90%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
21	100%	6	6	6	6	13.87	0	0.00%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
22	100%	4,231	3,829	4,092	4,092	17.69	231	0.20%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
23	100%	18,140	20,390	18,659	18,659	23.18	805	0.69%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
24	100%	2,393	2,554	2,430	2,430	16.48	147	0.13%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
25	100%	1,009	1,062	1,021	1,021	20.23	50	0.04%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
26	100%	75,508	85,654	77,848	77,848	26.20	2,872	2.54%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
27	100%	20,955	22,278	21,280	21,280	18.97	1,065	0.91%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
28	100%	18,524	21,890	20,024	20,024	35.18	569	0.49%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
29	100%	11,360	13,771	11,916	11,916	46.90	254	0.22%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
		811,863	836,918	817,844	780,015		116,879	100.00%		18.98%	22,185			1,511		0.04%	51
											18,988			1.29%			0.04%

* - Subarea in which the site is located.

Trip Distribution Table

Heritage Neighborhood Center

Sub Area Population Data:

For determination of Trip Distribution for Proposed Office Development Trips

2004 and 2030 Data Taken from Mid-Region Council of Governments 2030 Socioeconomic

Forecasts by Data Analysis Services for the Mid-Region of New Mexico

Sub Area I.D.#	% Sub Area In Study	2004 Population	2030 Population	Interpolated Population for the Year	Population in Study	Dist. (Mi.)	Population / Distance	(LN)			(ON)			(LE)		
								% Utilizing	% Population / Dist Utilizing	Population	% Utilizing	% Population / Dist Utilizing	Population	% Utilizing	% Population / Dist Utilizing	Population
1	100%	26,972	39,738	29,918	2,010	20.77	97	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
2	100%	39,348	40,810	39,839	29,918	12.80	2,338	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
3	100%	7,885	8,728	8,084	8,084	14.94	540	0%	0.00%	0	0%	0.00%	0	0%	0.00%	1,189
4	100%	13,387	14,938	13,744	13,744	25.46	540	0%	0.00%	0	0%	0.00%	0	0%	0.00%	540
5	100%	35,988	44,203	37,868	37,868	8.38	4,037	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
6	100%	2,784	3,950	3,053	3,053	8.78	348	0%	0.00%	0	0%	0.00%	0	0%	0.00%	2,018
7*	100%	48,585	58,615	51,115	51,115	1.01	50,861	0%	0.04%	51	1%	0.26%	300	0%	0.00%	19,083
8	100%	27,548	28,553	27,778	27,778	2.88	9,842	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
9	100%	1,678	1,888	1,728	1,728	19.97	86	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
10	100%	38,532	4,822	31,522	31,522	7.44	4,238	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
11	100%	32,051	33,202	32,317	32,317	7.91	4,088	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
12	100%	18,144	18,148	18,144	18,144	8.98	1,798	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
13	100%	8,715	10,148	9,045	9,045	10.18	888	0%	0.00%	0	0%	0.00%	0	0%	0.00%	899
14	100%	83,104	84,279	83,375	83,375	14.00	8,868	0%	0.00%	0	0%	0.00%	0	0%	0.00%	444
15	100%	24,691	25,262	24,823	24,823	5.83	4,258	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
16	100%	108,882	108,353	108,780	108,780	15.28	7,120	0%	0.00%	0	0%	0.00%	0	0%	0.00%	1,405
17	100%	20,920	21,198	20,984	20,984	5.70	3,885	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
18	100%	42,078	41,870	41,984	41,984	8.91	4,711	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
19	100%	59,027	58,888	58,995	58,995	15.54	3,785	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
20	100%	9,482	9,999	9,532	9,532	9.11	1,048	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
21	100%	6	6	6	6	13.87	0	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
22	100%	4,231	3,629	4,092	4,092	17.69	231	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
23	100%	18,140	20,390	18,656	18,656	23.18	805	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
24	100%	2,383	2,554	2,430	2,430	16.48	147	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
25	100%	1,009	1,062	1,021	1,021	20.23	50	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
26	100%	75,506	85,654	77,848	77,848	26.20	2,972	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
27	100%	20,855	22,278	21,260	21,260	19.97	1,065	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
28	100%	19,524	21,990	20,024	20,024	35.18	589	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
29	100%	11,380	13,771	11,916	11,916	46.90	254	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
		811,883	838,916	817,844	780,015	116,879	51	0.04%	0.26%	300	0.26%	0.26%	300	0.26%	0.26%	25,538
							0.04%			0.04%			0.04%			21,85%

* - Subarea in which the site is located.

Trip Distribution Table

Heritage Neighborhood Center

Sub Area Population Data:

For delamination of Trip Distribution for Proposed Office Development Trips

2004 and 2030 Data Taken from Mid Region Council of Government's 2030 Scenario Mapping

Forecast by Data Analysis Software for the Mid Region of New Mexico

Sub Area I.D.#	% Sub Area In Study	2004 Population		2030 Population		Interpolated Population for the Year 2010	Population in Study	Dist. (Mi.)	Population / Distance	(OS) Quarry Rd South			(LS) Marchwood Dr. South			(MS) Market Pl. South		
		2004	2030	2004	2030	2010				% Utilizing	% Population / Dist. Utilizing	Population	% Utilizing	% Population / Dist. Utilizing	Population	% Utilizing	% Population / Dist. Utilizing	Population
1	100%	26,972	39,738	29,918	2,010	2,010	20.77	97	0.00%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
2	100%	39,348	40,610	39,839	29,918	29,918	12.80	2,338	0.00%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
3	100%	7,865	8,728	8,064	8,064	8,064	14.94	540	0%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
4	100%	13,387	14,938	13,744	13,744	13,744	25.46	540	0%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
5	100%	35,968	44,203	37,868	37,868	37,868	9.38	4,037	0%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
6	100%	2,764	3,950	3,053	3,053	3,053	8.78	348	0%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
7*	100%	48,565	59,615	51,115	51,115	51,115	1.01	60,861	2.84%	7%	2.84%	3,316	0%	0.00%	1,404	1%	0.50%	590
8	100%	27,540	28,553	27,778	27,778	27,778	2.88	9,842	0.00%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
9	100%	1,878	1,888	1,728	1,728	1,728	19.97	86	0%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
10	100%	39,532	4,822	31,522	31,522	31,522	7.44	4,239	0%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
11	100%	32,051	33,202	32,317	32,317	32,317	7.91	4,088	0%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
12	100%	16,144	16,146	16,144	16,144	16,144	8.86	1,798	0%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
13	100%	8,715	10,146	9,045	9,045	9,045	10.18	888	0%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
14	100%	83,104	94,279	83,375	83,375	83,375	14.00	6,968	0%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
15	100%	24,891	25,262	24,823	24,823	24,823	5.83	4,268	0%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
16	100%	108,882	108,353	108,760	108,760	108,760	15.28	7,120	0%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
17	100%	20,920	21,196	20,984	20,984	20,984	5.70	3,686	0%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
18	100%	42,078	41,870	41,984	41,984	41,984	8.91	4,711	0%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
19	100%	59,027	56,888	58,965	58,965	58,965	15.54	3,786	0%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
20	100%	9,482	9,699	9,532	9,532	9,532	9.11	1,048	0%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
21	100%	6	6	6	6	6	13.87	0	0%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
22	100%	4,231	3,629	4,092	4,092	4,092	17.89	231	0%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
23	100%	18,140	20,390	18,659	18,659	18,659	23.18	806	0%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
24	100%	2,363	2,554	2,430	2,430	2,430	16.48	147	0%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
25	100%	1,008	1,062	1,021	1,021	1,021	20.23	50	0%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
26	100%	75,508	85,654	77,848	77,848	77,848	28.20	2,972	0%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
27	100%	20,955	22,276	21,260	21,260	21,260	19.97	1,065	0%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
28	100%	19,524	21,690	20,024	20,024	20,024	35.18	569	0%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
29	100%	11,360	13,771	11,916	11,916	11,916	46.80	254	0%	0%	0.00%	0	0%	0.00%	0	0%	0.00%	0
		811,863	836,916	817,644	817,644	817,644	116,879	3,316	2.84%	0%	2.84%	3,316	0%	0.00%	1,404	0%	0.50%	590
																		0.50%

* - Subarea in which the site is located.

Trip Distribution Table

Heritage Neighborhood Center

Sub Area Population Data:

For determination of Trip Distribution for Proposed Office Development Trips

2004 and 2030 Data Taken from Mid Region Council of Governments' 2030 Socioeconomic Forecasts by Data Analysis Subzones for the Mid-Region of New Mexico

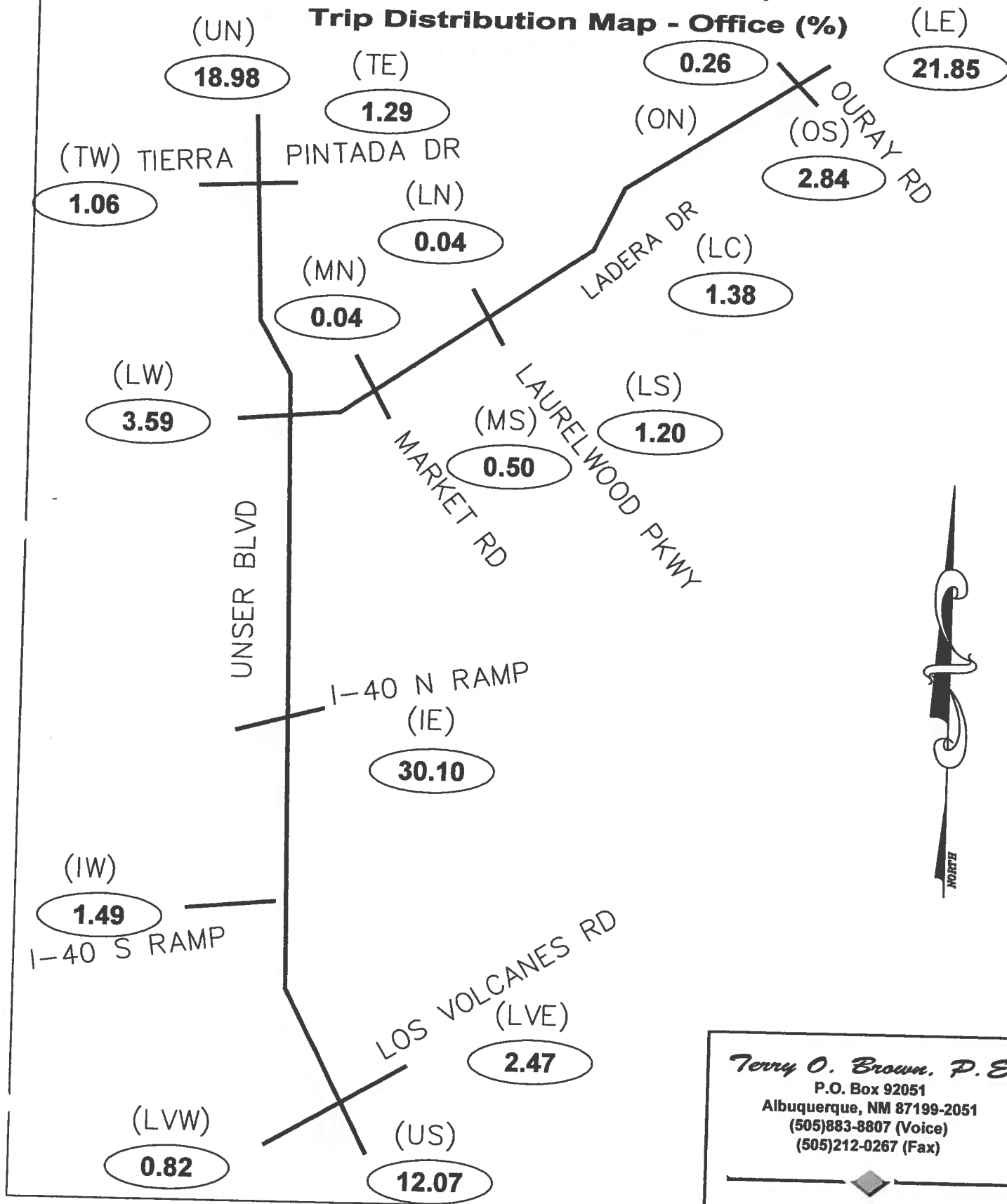
Sub Area I.D.#	% Sub Area in Study	2004' Population	2030 Population	Interpolated Population for the Year	Population in Study	Dist. (Mi.)	Population / Distance	(IE)			(LVE)			(US)		
								% Utilizing	Population	% Population / Dist Utilizing	% Utilizing	Population	% Population / Dist Utilizing	% Utilizing	Population	% Population / Dist Utilizing
1	100%	26,972	39,738	29,918	2,010	20.77	97	30%	77	0.07%	0%	0	0.00%	0%	0	0.00%
2	100%	39,348	40,610	39,839	29,918	12.80	2,338	0%	0	0.00%	0%	0	0.00%	0%	0	0.00%
3	100%	7,865	8,728	8,064	8,064	14.94	540	0%	0	0.00%	0%	0	0.00%	0%	0	0.00%
4	100%	13,387	14,936	13,744	13,744	25.48	540	100%	540	0.48%	0%	0	0.00%	0%	0	0.00%
5	100%	35,988	44,203	37,868	37,868	9.38	4,037	0%	0	0.00%	0%	0	0.00%	0%	0	0.00%
6	100%	2,764	3,950	3,053	3,053	8.78	348	0%	0	0.00%	0%	0	0.00%	0%	0	0.00%
7*	100%	48,565	59,615	51,115	51,115	1.01	50,861	0%	0	0.00%	0%	0	0.00%	0%	0	0.00%
8	100%	27,548	28,553	27,778	27,778	2.88	9,642	0%	0	0.00%	0%	0	0.00%	0%	0	0.00%
9	100%	1,878	1,888	1,728	1,728	19.97	86	0%	0	0.00%	0%	0	0.00%	0%	0	0.00%
10	100%	39,532	4,822	31,522	31,522	7.44	4,239	0%	0	0.00%	0%	0	0.00%	0%	0	0.00%
11	100%	32,051	33,202	32,317	32,317	7.91	4,088	0%	0	0.00%	0%	0	0.00%	0%	0	0.00%
12	100%	16,144	16,146	16,144	16,144	8.98	1,798	0%	0	0.00%	0%	0	0.00%	0%	0	0.00%
13	100%	8,715	10,146	9,045	9,045	10.18	888	0%	0	0.00%	0%	0	0.00%	0%	0	0.00%
14	100%	93,104	94,279	93,375	93,375	14.00	6,668	100%	6,668	5.71%	0%	0	0.00%	0%	0	0.00%
15	100%	24,891	25,282	24,823	24,823	5.83	4,258	34%	1,448	1.24%	0%	0	0.00%	0%	0	0.00%
16	100%	108,882	108,353	108,760	108,760	15.28	7,120	100%	7,120	6.09%	0%	0	0.00%	0%	0	0.00%
17	100%	20,920	21,196	20,984	20,984	5.70	3,685	100%	3,685	3.15%	0%	0	0.00%	0%	0	0.00%
18	100%	42,078	41,670	41,884	41,884	8.91	4,711	100%	4,711	4.03%	0%	0	0.00%	0%	0	0.00%
19	100%	59,027	58,888	58,955	58,955	15.54	3,795	100%	3,795	3.25%	0%	0	0.00%	0%	0	0.00%
20	100%	9,482	9,699	9,532	9,532	9.11	1,046	100%	1,046	0.90%	0%	0	0.00%	0%	0	0.00%
21	100%	6	6	6	6	13.87	0	100%	0	0.00%	0%	0	0.00%	0%	0	0.00%
22	100%	4,231	3,629	4,092	4,092	17.98	231	100%	231	0.20%	0%	0	0.00%	0%	0	0.00%
23	100%	18,140	20,390	18,659	18,659	23.18	805	100%	805	0.69%	0%	0	0.00%	0%	0	0.00%
24	100%	2,393	2,554	2,430	2,430	16.48	147	100%	147	0.13%	0%	0	0.00%	0%	0	0.00%
25	100%	1,009	1,062	1,021	1,021	20.23	50	100%	50	0.04%	0%	0	0.00%	0%	0	0.00%
26	100%	75,508	85,654	77,648	77,648	26.20	2,972	100%	2,972	2.54%	0%	0	0.00%	0%	0	0.00%
27	100%	20,955	22,276	21,260	21,260	19.97	1,065	100%	1,065	0.91%	0%	0	0.00%	0%	0	0.00%
28	100%	19,524	21,680	20,024	20,024	35.18	569	100%	569	0.49%	0%	0	0.00%	0%	0	0.00%
29	100%	11,360	13,771	11,916	11,916	46.90	254	100%	254	0.22%	0%	0	0.00%	0%	0	0.00%
		811,863	836,916	817,644	780,015		116,879		35,185			2,893			14,111	
								30.10%	30,10%			2,47%			12.07%	

* - Subarea in which the site is located.

Heritage Neighborhood Center

(Ladera Dr / Unser Blvd)

Trip Distribution Map - Office (%)



Terry O. Brown, P.E.

P.O. Box 92051

Albuquerque, NM 87199-2051

(505)883-8807 (Voice)

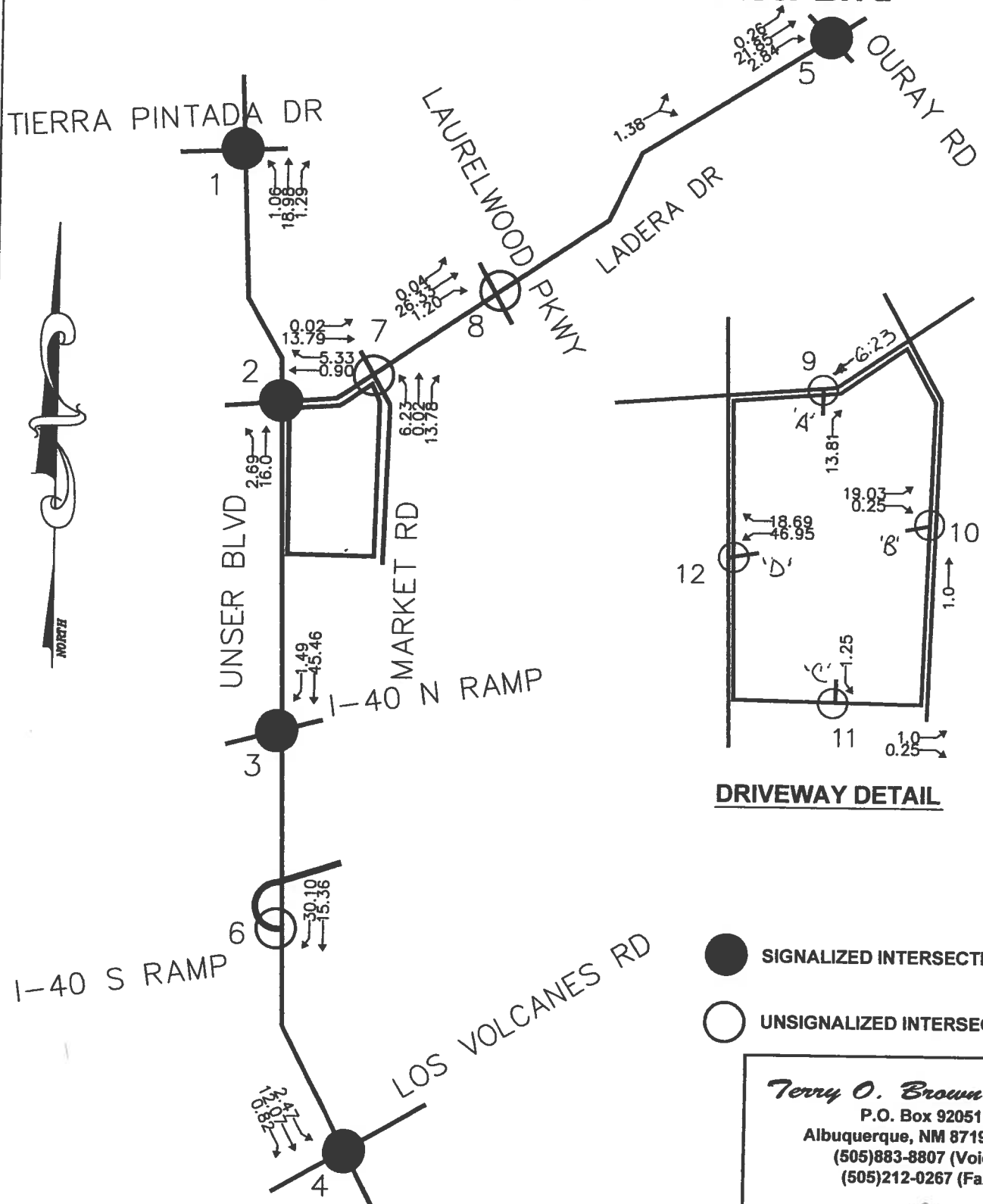
(505)212-0267 (Fax)

Heritage Neighborhood Center

(Ladera Dr / Unser Blvd)

Trip Assignments - Office (% Exiting)

Case F - Full access on Unser Blvd

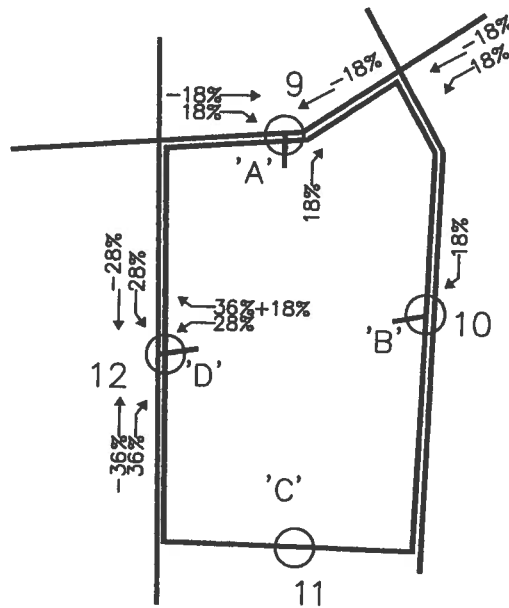


Heritage Neighborhood Center

(Ladera Dr / Unser Blvd)

Passby Trips

Case F - Full access on Unser Blvd



DRIVEWAY DETAIL

-  **SIGNALIZED INTERSECTIONS**
-  **UNSIGNALIZED INTERSECTION**

Terry O. Brown, P.E.
P.O. Box 92051
Albuquerque, NM 87199-2051
(505)883-8807 (Voice)
(505)212-0267 (Fax)

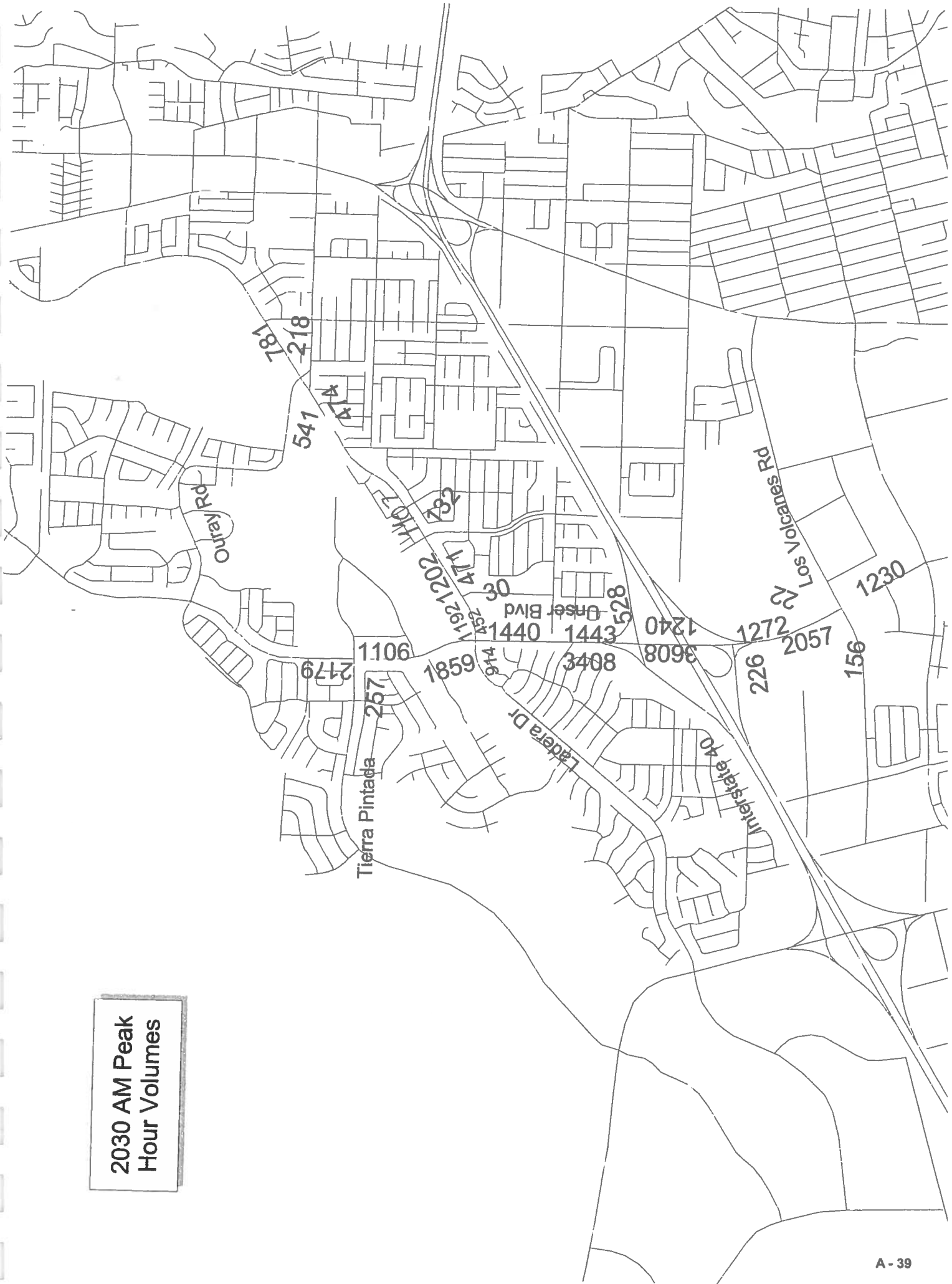
2005 AM Peak
Hour Volumes



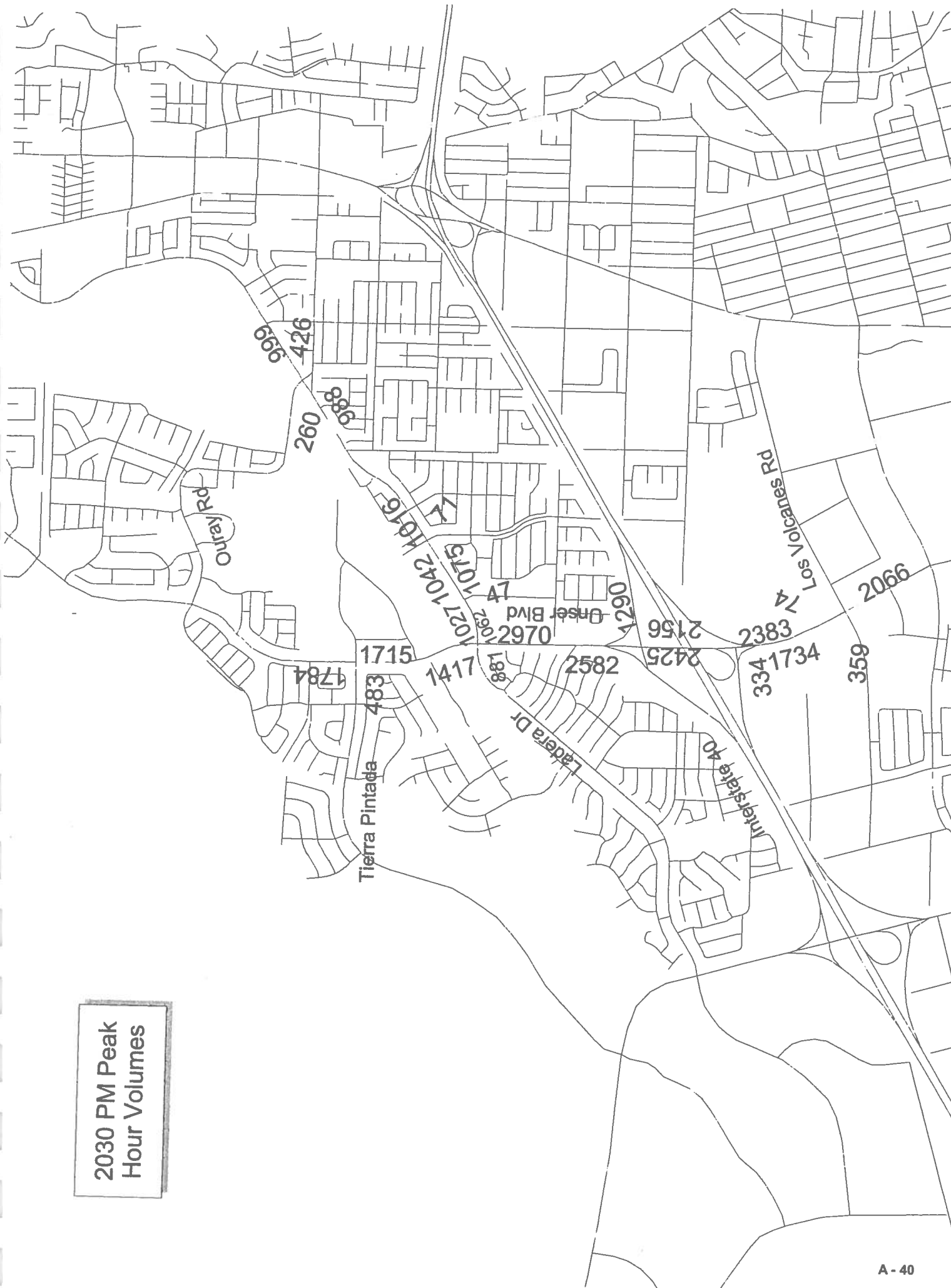
2005 PM Peak
Hour Volumes



2030 AM Peak
Hour Volumes



2030 PM Peak
Hour Volumes



Heritage Neighborhood Center (Ladera Dr / Unser Blvd)

Projected Turning Movements SUMMARY PROPOSED DEVELOPMENT (2010) - 100% Development

Case F - full access at Intersection 12

INTERSECTION: Summary

Tierra Pintada Dr / Unser Blvd

(1)	0.97			0.94			PHF						
3.0% Truck	Eastbound (Tierra Pintada Dr)			Westbound (Tierra Pintada Dr)			Northbound (Unser Blvd)			Southbound (Unser Blvd)			
Existing (2007)	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
2010 (NO BUILD - A.M.)	34	3	195	14	0	4	28	631	15	10	783	21	
2010 (BUILD - A.M.)	203	18	215	106	12	28	36	712	130	49	954	50	
	203	18	249	112	12	28	61	751	134	49	1,016	50	
	0.90			0.75			0.94			0.88			PHF
	Eastbound (Tierra Pintada Dr)			Westbound (Tierra Pintada Dr)			Northbound (Unser Blvd)			Southbound (Unser Blvd)			
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
Existing (2007)	31	2	68	65	1	24	179	840	22	28	770	47	
2010 (NO BUILD - P.M.)	151	30	114	356	40	99	213	962	226	97	882	132	
2010 (BUILD - P.M.)	151	30	154	362	40	99	253	1,037	232	97	945	132	

Ladera Dr / Unser Blvd

(2)

3.0% Truck

Existing (2007)

2010 (NO BUILD - A.M.)

2010 (BUILD - A.M.)

Eastbound (Ladera Dr)			Westbound (Ladera Dr)			Northbound (Unser Blvd)			Southbound (Unser Blvd)		
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
175	251	365	317	105	44	48	429	224	45	906	58
199	446	557	542	186	123	134	653	376	99	1,279	105
199	460	597	542	196	140	163	704	376	125	1,355	105

0.93

0.93

0.95

0.96

PHF

Existing (2007)

2010 (NO BUILD - P.M.)

2010 (BUILD - P.M.)

Eastbound (Ladera Dr)			Westbound (Ladera Dr)			Northbound (Unser Blvd)			Southbound (Unser Blvd)		
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
140	182	138	281	264	107	288	860	372	94	547	184
192	319	322	594	480	249	560	1,433	708	263	1,087	333
192	334	369	594	496	279	607	1,524	708	290	1,169	333

I-40 N. ramp / Unser Blvd

(3)

3.0% Truck

Existing (2007)

2010 (NO BUILD - A.M.)

2010 (BUILD - A.M.)

0.85			0.91			0.97			0.89			PHF
Eastbound (I-40 N. ramp)			Westbound (I-40 N. ramp)			Northbound (Unser Blvd)			Southbound (Unser Blvd)			
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
0	0	0	343	3	188	24	687	0	0	1,775		68
0	0	0	438	3	222	46	992	0	0	2,281		76
0	0	0	438	3	291	46	1,138	0	0	2,417		83

0.85

0.94

0.87

0.92

PHF

Existing (2007)

2010 (NO BUILD - P.M.)

2010 (BUILD - P.M.)

0.85			0.94			0.87			0.92			PHF
Eastbound (I-40 N. ramp)			Westbound (I-40 N. ramp)			Northbound (Unser Blvd)			Southbound (Unser Blvd)			
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
0	0	0	626	0	771	24	725	0	0	903		70
0	0	0	770	0	840	75	1,319	0	0	1,559		108
0	0	0	770	0	905	75	1,485	0	0	1,806		120

Los Volcanes Rd / Unser Blvd

(4)	0.75												0.91			0.82			PHF			
	Eastbound (Los Volcanes Rd)			Westbound (Los Volcanes Rd)			Northbound (Unser Blvd)			Southbound (Unser Blvd)												
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right										
	137	69	7	95	27	134	8	1,101	101	260	897	69										
	137	74	7	297	31	219	8	1,192	279	490	976	75										
Existing (2007)	0.83												0.92			0.85			0.89			PHF
2010 (NO BUILD - A.M.)	Eastbound (Los Volcanes Rd)			Westbound (Los Volcanes Rd)			Northbound (Unser Blvd)			Southbound (Unser Blvd)												
2010 (BUILD - A.M.)	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right										
	142	74	7	297	31	255	8	1,287	279	516	1,043	78										
	0.83												0.92			0.85			0.89			PHF
	Eastbound (Los Volcanes Rd)			Westbound (Los Volcanes Rd)			Northbound (Unser Blvd)			Southbound (Unser Blvd)												
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right										
	54	15	4	73	23	132	10	793	120	108	1,022	121										
	82	33	6	736	33	459	12	1,083	463	609	894	127										
Existing (2007)	0.83												0.92			0.85			0.89			PHF
2010 (NO BUILD - P.M.)	Eastbound (Los Volcanes Rd)			Westbound (Los Volcanes Rd)			Northbound (Unser Blvd)			Southbound (Unser Blvd)												
2010 (BUILD - P.M.)	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right										
	87	33	6	736	33	500	12	1,190	463	651	1,007	133										

Heritage Neighborhood Center (Ladera Dr / Unser Blvd)

Projected Turning Movements SUMMARY PROPOSED DEVELOPMENT (2010) - 100% Development

Case F - full access at Intersection 12

INTERSECTION:

Summary

Ladera Dr / Ouray Rd

(5)

3.0% Truck

Existing (2007)

2010 (NO BUILD - A.M.)

2010 (BUILD - A.M.)

0.91			0.89			0.79			0.89			PHF
Eastbound (Ladera Dr)			Westbound (Ladera Dr)			Northbound (Ouray Rd)			Southbound (Ouray Rd)			
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
20	365	247	3	145	20	98	77	5	89	191	13	
20	365	247	4	214	30	101	79	5	99	212	14	
24	414	287	4	291	30	156	79	5	99	212	19	

Existing (2007)

2010 (NO BUILD - P.M.)

2010 (BUILD - P.M.)

0.90			0.87			0.94			0.89			PHF
Eastbound (Ladera Dr)			Westbound (Ladera Dr)			Northbound (Ouray Rd)			Southbound (Ouray Rd)			
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
19	271	200	20	379	107	293	233	16	34	123	13	
22	307	227	23	427	121	293	233	16	36	131	14	
27	400	292	23	506	121	358	233	16	36	131	20	

I-40 S. ramp / Unser Blvd

(6)

3.0% Truck

Existing (2007)

2010 (NO BUILD - A.M.)

2010 (BUILD - A.M.)

0.75			0.85			0.80			0.88			PHF
Eastbound (I-40 S. ramp)			Westbound (I-40 S. ramp)			Northbound (Unser Blvd)			Southbound (Unser Blvd)			
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
31	0	9	0	0	0	3	535	580	4	821	740	
50	0	41	0	0	0	3	804	660	5	1,329	883	
60	0	41	0	0	0	3	939	660	45	1,425	883	

Existing (2007)

2010 (NO BUILD - P.M.)

2010 (BUILD - P.M.)

0.90			0.85			0.92			0.97			PHF
Eastbound (I-40 S. ramp)			Westbound (I-40 S. ramp)			Northbound (Unser Blvd)			Southbound (Unser Blvd)			
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
69	0	21	0	0	0	0	641	364	0	1,162	308	
94	0	77	0	0	0	0	1,335	576	0	1,908	344	
105	0	77	0	0	0	0	1,489	576	86	2,069	344	

Ladera Dr / Market Rd

(7)

3.0% Truck

Existing (2007)

2010 (NO BUILD - A.M.)

2010 (BUILD - A.M.)

0.88			0.79			0.86			0.85			PHF
Eastbound (Ladera Dr)			Westbound (Ladera Dr)			Northbound (Market Rd)			Southbound (Market Rd)			
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
0	425	28	14	365	0	113	0	72	0	0	0	
0	425	28	15	398	0	113	0	72	0	0	0	
0	489	34	201	398	0	140	0	136	0	1	0	

Existing (2007)

2010 (NO BUILD - P.M.)

2010 (BUILD - P.M.)

0.93			0.80			0.88			0.85			PHF
Eastbound (Ladera Dr)			Westbound (Ladera Dr)			Northbound (Market Rd)			Southbound (Market Rd)			
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
0	513	156	48	463	0	83	0	36	0	0	0	
0	559	170	52	505	0	83	0	36	0	0	0	
0	669	176	260	505	0	129	0	146	0	1	0	

Heritage Neighborhood Center (Ladera Dr / Unser Blvd)

Projected Turning Movements SUMMARY PROPOSED DEVELOPMENT (2010) - 100% Development

Case F - full access at Intersection 12

INTERSECTION: Summary

Ladera Dr / Laurelwood Pkwy

0.90			0.75			0.89			0.75			PHF
Eastbound (Ladera Dr)			Westbound (Ladera Dr)			Northbound (Laurelwood Pkwy)			Southbound (Laurelwood Pkwy)			
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
1	490	28	5	255	0	78	0	65	5	0	8	
1	490	28	7	363	0	78	0	65	5	0	8	
2	602	44	7	526	0	101	0	65	5	0	9	

0.89			0.91			0.85			0.75			PHF
Eastbound (Ladera Dr)			Westbound (Ladera Dr)			Northbound (Laurelwood Pkwy)			Southbound (Laurelwood Pkwy)			
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
9	422	86	76	546	9	37	0	34	1	0	3	
10	481	98	82	589	10	37	0	34	1	0	3	
11	674	124	82	769	10	63	0	34	1	0	4	

- Ladera Dr / Driveway 'A'

0.79			0.79			0.85			0.85			PHF
Eastbound (Ladera Dr)			Westbound (Ladera Dr)			Northbound (Driveway 'A')			Southbound (Driveway 'A')			
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
0	520	0	0	466	0	0	0	0	0	0	0	
0	520	0	0	466	0	0	0	0	0	0	0	
0	526	33	0	493	0	0	0	65	0	0	0	

0.93			0.93			0.85			0.85			PHF
Eastbound (Ladera Dr)			Westbound (Ladera Dr)			Northbound (Driveway 'A')			Southbound (Driveway 'A')			
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
0	648	0	0	652	0	0	0	0	0	0	0	
0	706	0	0	711	0	0	0	0	0	0	0	
0	664	85	0	709	0	0	0	163	0	0	0	

Driveway 'B' / Market Rd

0.85			0.85			0.86			0.86			PHF
Eastbound (Driveway 'B')			Westbound (Driveway 'B')			Northbound (Market Rd)			Southbound (Market Rd)			
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
0	0	0	0	0	0	0	185	0	0	42	0	
0	0	0	0	0	0	0	185	0	0	42	0	
88	0	3	0	0	0	5	189	0	0	48	187	

0.85			0.85			0.88			0.88			PHF
Eastbound (Driveway 'B')			Westbound (Driveway 'B')			Northbound (Market Rd)			Southbound (Market Rd)			
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
0	0	0	0	0	0	0	119	0	0	204	0	
0	0	0	0	0	0	0	119	0	0	204	0	
150	0	5	0	0	0	6	126	0	0	210	257	

Heritage Neighborhood Center (Ladera Dr / Unser Blvd)

Projected Turning Movements SUMMARY

PROPOSED DEVELOPMENT (2010) - 100% Development

Case F - full access at Intersection 12

INTERSECTION: Summary

Hanover Rd / Driveway 'C' (11)

3.0% Truck

Existing (2007)

2010 (NO BUILD - A.M.)

2010 (BUILD - A.M.)

0.85			0.85			0.85			0.85			PHF
Eastbound (Hanover Rd)			Westbound (Hanover Rd)			Northbound (Driveway 'C')			Southbound (Driveway 'C')			
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
0	0	0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	11	0	0	0	7	0	0	0

Existing (2007)

2010 (NO BUILD - P.M.)

2010 (BUILD - P.M.)

0.85			0.85			0.85			0.85			PHF
Eastbound (Hanover Rd)			Westbound (Hanover Rd)			Northbound (Driveway 'C')			Southbound (Driveway 'C')			
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
0	0	0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	11	0	0	0	12	0	0	0

Driveway 'D' / Unser Blvd (12)

3.0% Truck

Existing (2007)

2010 (NO BUILD - A.M.)

2010 (BUILD - A.M.)

0.85			0.85			0.85			0.85			PHF
Eastbound (Driveway 'D')			Westbound (Driveway 'D')			Northbound (Unser Blvd)			Southbound (Unser Blvd)			
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
0	0	0	0	0	0	0	701	0	0	1,588	0	0
0	0	0	0	0	0	0	797	0	0	1,806	0	0
0	0	0	144	0	80	0	797	216	116	1,806	0	0

Existing (2007)

2010 (NO BUILD - P.M.)

2010 (BUILD - P.M.)

0.85			0.85			0.95			0.95			PHF
Eastbound (Driveway 'D')			Westbound (Driveway 'D')			Northbound (Unser Blvd)			Southbound (Unser Blvd)			
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
0	0	0	0	0	0	0	1,520	0	0	966	0	0
0	0	0	0	0	0	0	1,709	0	0	1,086	0	0
0	0	0	340	0	295	0	1,613	327	204	1,011	0	0

Heritage Neighborhood Center (Ladera Dr / Unser Blvd)

Projected Turning Movements SUMMARY

PROPOSED DEVELOPMENT (2010) - 100% Development

Case F - full access at Intersection 12

INTERSECTION: Summary

Hanover Rd / Driveway 'C'

(11)

3.0% Truck

Existing (2007)

2010 (NO BUILD - A.M.)

2010 (BUILD - A.M.)

0.85			0.85			0.85			0.85			PHF
Eastbound (Hanover Rd)			Westbound (Hanover Rd)			Northbound (Driveway 'C')			Southbound (Driveway 'C')			
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
0	0	0	0	0	0	0	0	0	0	0	0	
0	0	0	0	0	0	0	0	0	0	0	0	
0	0	0	0	0	11	0	0	0	7	0	0	

Existing (2007)

2010 (NO BUILD - P.M.)

2010 (BUILD - P.M.)

0.85			0.85			0.85			0.85			PHF
Eastbound (Hanover Rd)			Westbound (Hanover Rd)			Northbound (Driveway 'C')			Southbound (Driveway 'C')			
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
0	0	0	0	0	0	0	0	0	0	0	0	
0	0	0	0	0	0	0	0	0	0	0	0	
0	0	0	0	0	11	0	0	0	12	0	0	

Driveway 'D' / Unser Blvd

(12)

3.0% Truck

Existing (2007)

2010 (NO BUILD - A.M.)

2010 (BUILD - A.M.)

0.85			0.85			0.85			0.85			PHF
Eastbound (Driveway 'D')			Westbound (Driveway 'D')			Northbound (Unser Blvd)			Southbound (Unser Blvd)			
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
0	0	0	0	0	0	0	701	0	0	1,588	0	
0	0	0	0	0	0	0	797	0	0	1,806	0	
0	0	0	144	0	80	0	797	216	116	1,806	0	

Existing (2007)

2010 (NO BUILD - P.M.)

2010 (BUILD - P.M.)

0.85			0.85			0.95			0.95			PHF
Eastbound (Driveway 'D')			Westbound (Driveway 'D')			Northbound (Unser Blvd)			Southbound (Unser Blvd)			
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
0	0	0	0	0	0	0	1,520	0	0	966	0	
0	0	0	0	0	0	0	1,709	0	0	1,086	0	
0	0	0	340	0	295	0	1,613	327	204	1,011	0	

Heritage Neighborhood Center (Ladera Dr / Unser Blvd)

Projected Turning Movements Worksheet

Tierra Pintada Dr / Unser Blvd

INTERSECTION :

E-W Street: Tierra Pintada Dr (1)

N-S Street: Unser Blvd

Year of Existing Counts 2007

Implementation Year 2010

Growth Rates

	0.47%			0.00%			2.79%			7.29%		
	Eastbound (Tierra Pintada Dr)			Westbound (Tierra Pintada Dr)			Northbound (Unser Blvd)			Southbound (Unser Blvd)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Existing Volumes	34	3	195	14	0	4	28	631	15	10	783	21
Background Traffic Growth	0	0	3	0	0	0	2	53	1	2	171	5
Subtotal	34	3	198	14	0	4	30	684	16	12	954	26
Watershed Residential & Retail	78	0	17	0	0	0	6	0	0	0	0	24
Storm Cloud	87	0	0	0	0	0	0	0	0	0	0	0
98th / Unser Development	4	15	0	92	12	24	0	28	114	37	0	0
Subtotal (NO BUILD - A.M.)	203	18	215	106	12	28	36	712	130	49	954	50
Percent Commercial Trips Generated(Entering)	0.00%	0.00%	6.68%	0.93%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	9.87%	0.00%
Percent Commercial Trips Generated(Exiting)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	6.68%	9.87%	0.93%	0.00%	0.00%	0.00%
Percent Office Trips Generated(Entering)	0.00%	0.00%	1.06%	1.29%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	18.98%	0.00%
Percent Office Trips Generated(Exiting)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	1.06%	18.98%	1.29%	0.00%	0.00%	0.00%
Total Trips Generated	0	0	34	6	0	0	25	39	4	0	62	0
Total AM Peak Hour BUILD Volumes	203	18	249	112	12	28	61	751	134	49	1,016	50

	16.44%			0.00%			2.82%			4.83%		
	Eastbound (Tierra Pintada Dr)			Westbound (Tierra Pintada Dr)			Northbound (Unser Blvd)			Southbound (Unser Blvd)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Existing Volumes	31	2	68	65	1	24	179	840	22	28	770	47
Background Traffic Growth	15	1	34	0	0	0	15	71	2	4	112	7
Subtotal	46	3	102	65	1	24	194	911	24	32	882	54
Watershed Residential & Retail	44	0	12	0	0	0	19	0	0	0	0	78
Storm Cloud	54	0	0	0	0	0	0	0	0	0	0	0
98th / Unser Development	7	27	0	291	39	75	0	51	202	65	0	0
Subtotal (NO BUILD - P.M.)	151	30	114	356	40	99	213	962	226	97	882	132
Percent Commercial Trips Generated(Entering)	0.00%	0.00%	6.68%	0.93%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	9.87%	0.00%
Percent Commercial Trips Generated(Exiting)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	6.68%	9.87%	0.93%	0.00%	0.00%	0.00%
Percent Office Trips Generated(Entering)	0.00%	0.00%	1.06%	1.29%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	18.98%	0.00%
Percent Office Trips Generated(Exiting)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	1.06%	18.98%	1.29%	0.00%	0.00%	0.00%
Total Trips Generated	0	0	40	6	0	0	40	75	6	0	63	0
Total PM Peak Hour BUILD Volumes	151	30	154	362	40	99	253	1,037	232	97	945	132

	Entering	Exiting		
Number of Commercial Trips Generated	499	378	A.M.	100% Commercial Development
	602	580	P.M.	
Number of Office Trips Generated	68	9	A.M.	100% Office Development
	20	96	P.M.	

	Eastbound (Tierra Pintada Dr)			Westbound (Tierra Pintada Dr)			Northbound (Unser Blvd)			Southbound (Unser Blvd)		
2007 AM Peak Hr. Volumes	34	3	195	14	0	4	28	631	15	10	783	21
2007 PM Peak Hr. Volumes	31	2	68	65	1	24	179	840	22	28	770	47

MRCOG Forecast Volumes Worksheet

Based on 2007 Traffic Count

2007 AM Link Volume 232 18 674 814

2007 PM Link Volume 101 90 1,041 845

Based on MRCOG Model (2030 Data Set)

2005 AM Link Volume 46 0 368 1545

2005 PM Link Volume 41 0 1447 1218

2030 AM Link Volume 257 0 1106 2179

2030 PM Link Volume 483 0 1715 1784

Growth Rate to Apply to Existing Counts to Match 2030 Forecasts

2007-2030 AM Growth Rates 0.47% -4.35% 2.79% 7.29%

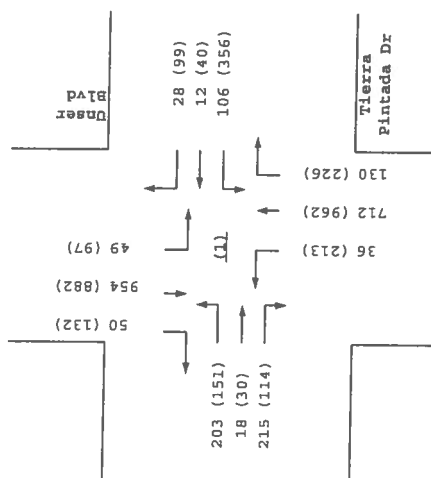
2007-2030 PM Growth Rates 16.44% -4.35% 2.82% 4.83%

Growth Rate to Apply to 2005 Model Volumes to Match 2030 Forecasts

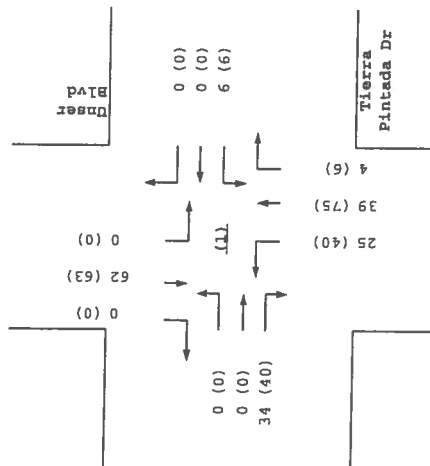
2005-2030 AM Growth Rates 18.35% #DIV/0! 8.02% 1.64%

2005-2030 PM Growth Rates 43.12% #DIV/0! 0.74% 1.86%

2010
NO BUILD

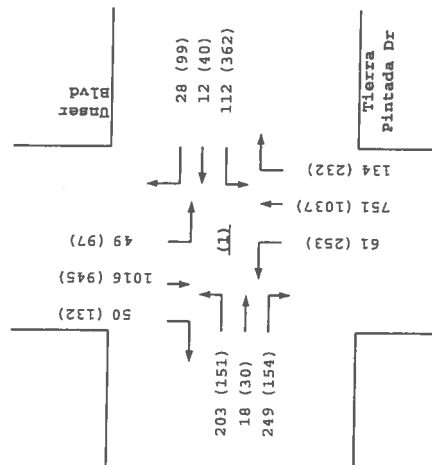


Trips



Tierra Pintada Dr / Unser Blvd

2010
BUILD



Heritage Neighborhood Center (Ladera Dr / Unser Blvd)
Projected Turning Movements Worksheet
Ladera Dr / Unser Blvd

INTERSECTION: E-W Street Ladera Dr (2)
N-S Street Unser Blvd
Year of Existing Counts 2007
Implementation Year 2010

	0.68%			6.77%			4.58%			3.66%		
	Eastbound (Ladera Dr)			Westbound (Ladera Dr)			Northbound (Unser Blvd)			Southbound (Unser Blvd)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Existing Volumes	175	251	365	317	105	44	48	429	224	45	906	58
Background Traffic Growth	4	5	7	64	21	9	7	59	31	5	100	6
Subtotal	179	256	372	381	126	53	55	488	255	50	1,006	64
I-40 / Unser Development	0	0	43	161	0	0	32	32	121	0	42	0
Ladera Business Park	0	0	0	0	0	3	0	28	0	5	47	0
Previous Development from below	20	190	142	0	60	67	47	105	0	44	184	41
Subtotal (NO BUILD - A.M.)	199	446	557	542	186	123	134	653	376	99	1,279	105
Percent Commercial Trips Generated(Entering)	0.00%	2.53%	7.59%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	4.37%	13.11%	0.00%
Percent Commercial Trips Generated(Exiting)	0.00%	0.00%	0.00%	0.00%	2.53%	4.37%	7.59%	13.11%	0.00%	0.00%	0.00%	0.00%
Percent Office Trips Generated(Entering)	0.00%	0.90%	2.69%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	5.33%	16.00%	0.00%
Percent Office Trips Generated(Exiting)	0.00%	0.00%	0.00%	0.00%	0.90%	5.33%	2.69%	16.00%	0.00%	0.00%	0.00%	0.00%
Total Trips Generated	0	14	40	0	10	17	29	51	0	26	76	0
Total AM Peak Hour BUILD Volumes	199	460	597	542	196	140	163	704	376	125	1,355	105

	3.98%			2.50%			4.15%			3.12%		
	Eastbound (Ladera Dr)			Westbound (Ladera Dr)			Northbound (Unser Blvd)			Southbound (Unser Blvd)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Existing Volumes	140	182	138	281	264	107	288	860	372	94	547	184
Background Traffic Growth	17	22	16	21	20	8	36	107	46	9	51	17
Subtotal	157	204	154	302	284	115	324	967	418	103	598	201
I-40 / Unser Development	0	0	78	292	0	0	77	77	290	0	77	0
Ladera Business Park	0	0	0	0	0	14	0	123	0	23	204	0
Previous Development from below	35	115	90	0	196	120	159	266	0	137	208	132
Subtotal (NO BUILD - P.M.)	192	319	322	594	480	249	560	1,433	708	263	1,087	333
Percent Commercial Trips Generated(Entering)	0.00%	2.53%	7.59%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	4.37%	13.11%	0.00%
Percent Commercial Trips Generated(Exiting)	0.00%	0.00%	0.00%	0.00%	2.53%	4.37%	7.59%	13.11%	0.00%	0.00%	0.00%	0.00%
Percent Office Trips Generated(Entering)	0.00%	0.90%	2.69%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	5.33%	16.00%	0.00%
Percent Office Trips Generated(Exiting)	0.00%	0.00%	0.00%	0.00%	0.90%	5.33%	2.69%	16.00%	0.00%	0.00%	0.00%	0.00%
Total Trips Generated	0	15	47	0	16	30	47	91	0	27	82	0
Total PM Peak Hour BUILD Volumes	192	334	369	594	496	279	607	1,524	708	290	1,169	333

Number of Commercial Trips Generated
499 378 A.M. 100% Commercial Development
602 580 P.M.
Number of Office Trips Generated
68 9 A.M. 100% Office Development
20 96 P.M.

	Eastbound (Ladera Dr)			Westbound (Ladera Dr)			Northbound (Unser Blvd)			Southbound (Unser Blvd)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
2007 AM Peak Hr. Volumes	175	251	365	317	105	44	48	429	224	45	906	58
2007 PM Peak Hr. Volumes	140	182	138	281	264	107	288	860	372	94	547	184

Previous Developments - AM Peak Hour Volumes

	Eastbound (Ladera Dr)			Westbound (Ladera Dr)			Northbound (Unser Blvd)			Southbound (Unser Blvd)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Watershed Residential & Retail	0	78	0	0	24	0	0	6	0	0	17	0
Storm Cloud Dev. w/ others	0	112	142	0	36	0	47	44	0	0	131	28
98th / Unser Development	20	0	0	0	0	67	0	55	0	44	36	13
Subtotal	20	190	142	0	60	67	47	105	0	44	184	41

Previous Developments - PM Peak Hour Volumes

	Eastbound (Ladera Dr)			Westbound (Ladera Dr)			Northbound (Unser Blvd)			Southbound (Unser Blvd)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Watershed Residential & Retail	0	44	0	0	78	0	0	19	0	0	12	0
Storm Cloud Dev. w/ others	0	71	90	0	118	0	159	149	0	0	83	91
98th / Unser Development	35	0	0	0	0	120	0	98	0	137	113	41
Subtotal	35	115	90	0	196	120	159	266	0	137	208	132

MRCOG Forecast Volumes Worksheet

Based on 2007 Traffic Count

2007 AM Link Volume 791 466 701 1,009

2007 PM Link Volume 460 652 1,520 825

Based on MRCOG Model (2030 Data Set)

2005 AM Link Volume 299 355 530 1526

2005 PM Link Volume 270 261 2016 1163

2030 AM Link Volume 914 1192 1440 1859

2030 PM Link Volume 881 1027 2970 1417

Growth Rate to Apply to Existing Counts to Match 2030 Forecasts

2007-2030 AM Growth Rates 0.68% 6.77% 4.58% 3.66%

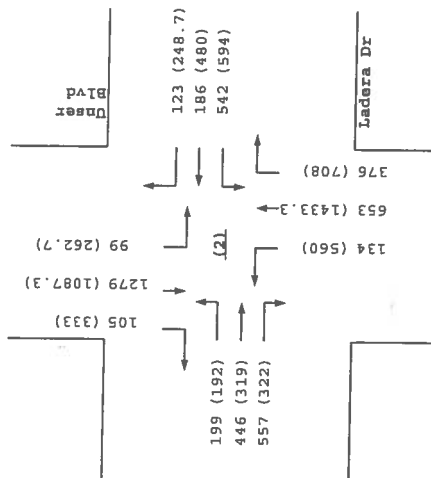
2007-2030 PM Growth Rates 3.98% 2.50% 4.15% 3.12%

Growth Rate to Apply to 2005 Model Volumes to Match 2025 Forecasts

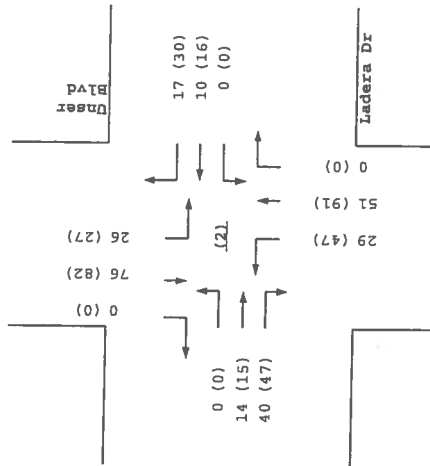
2005-2030 AM Growth Rates 8.23% 9.43% 6.87% 0.87%

2005-2030 PM Growth Rates 9.05% 11.74% 1.89% 0.87%

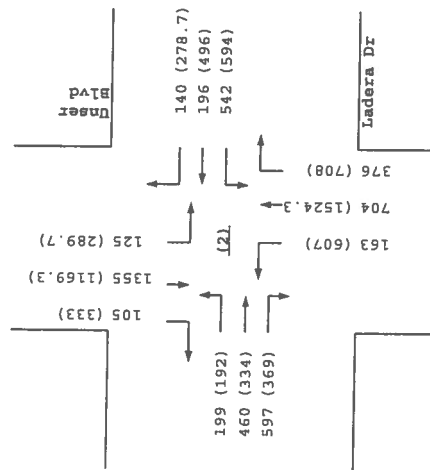
2010
NO BUILD



Trips



2010
BUILD



Ladera Dr / Unser Blvd

Heritage Neighborhood Center (Ladera Dr / Unser Blvd)
Projected Turning Movements Worksheet
I-40 N. ramp / Unser Blvd

INTERSECTION: E-W Street I-40 N. ramp (3)
 N-S Street Unser Blvd

Year of Existing Counts 2007
 Implementation Year 2010

	0.00%			0.00%			3.23%			3.69%		
	Eastbound (I-40 N. ramp)			Westbound (I-40 N. ramp)			Northbound (Unser Blvd)			Southbound (Unser Blvd)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Existing Volumes	0	0	0	343	3	188	24	687	0	0	1,775	68
Background Traffic Growth	0	0	0	0	0	0	2	67	0	0	197	8
Subtotal	0	0	0	343	3	188	26	754	0	0	1,972	76
I-40 / Unser Development	0	0	0	28	0	0	20	186	0	0	246	0
Southwest Mesa Subdivisions	0	0	0	67	0	0	0	26	0	0	10	0
Previous Development from below	0	0	0	0	0	34	0	26	0	0	53	0
Subtotal (NO BUILD - A.M.)	0	0	0	438	3	222	46	992	0	0	2,281	76
Percent Commercial Trips Generated(Entering)	0.00%	0.00%	0.00%	0.00%	0.00%	9.87%	0.00%	27.04%	0.00%	0.00%	0.00%	0.00%
Percent Commercial Trips Generated(Exiting)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	35.01%	1.90%
Percent Office Trips Generated(Entering)	0.00%	0.00%	0.00%	0.00%	0.00%	30.10%	0.00%	16.85%	0.00%	0.00%	0.00%	0.00%
Percent Office Trips Generated(Exiting)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	45.46%	1.49%
Total Trips Generated	0	0	0	0	0	69	0	146	0	0	136	7
Total AM Peak Hour BUILD Volumes	0	0	0	438	3	291	46	1,138	0	0	2,417	83

	0.00%			0.00%			3.00%			3.00%		
	Eastbound (I-40 N. ramp)			Westbound (I-40 N. ramp)			Northbound (Unser Blvd)			Southbound (Unser Blvd)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Existing Volumes	0	0	0	626	0	771	24	725	0	0	903	70
Background Traffic Growth	0	0	0	0	0	0	2	65	0	0	81	5
Subtotal	0	0	0	626	0	771	26	790	0	0	984	76
I-40 / Unser Development	0	0	0	51	0	0	49	445	0	0	447	0
Southwest Mesa Subdivisions	0	0	0	93	0	0	0	38	0	0	6	31
Previous Development from below	0	0	0	0	0	69	0	46	0	0	122	1
Subtotal (NO BUILD - P.M.)	0	0	0	770	0	840	75	1,319	0	0	1,559	108
Percent Commercial Trips Generated(Entering)	0.00%	0.00%	0.00%	0.00%	0.00%	8.87%	0.00%	27.04%	0.00%	0.00%	0.00%	0.00%
Percent Commercial Trips Generated(Exiting)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	35.01%	1.90%
Percent Office Trips Generated(Entering)	0.00%	0.00%	0.00%	0.00%	0.00%	30.10%	0.00%	16.85%	0.00%	0.00%	0.00%	0.00%
Percent Office Trips Generated(Exiting)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	45.46%	1.49%
Total Trips Generated	0	0	0	0	0	65	0	166	0	0	247	12
Total PM Peak Hour BUILD Volumes	0	0	0	770	0	905	75	1,485	0	0	1,806	120

Number of Commercial Trips Generated
 499 378 A.M. 100% Commercial Development
 602 580 P.M.
 Number of Office Trips Generated
 68 9 A.M. 100% Office Development
 20 96 P.M.

	Eastbound (I-40 N. ramp)			Westbound (I-40 N. ramp)			Northbound (Unser Blvd)			Southbound (Unser Blvd)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
2007 AM Peak Hr. Volumes	0	0	0	343	3	188	24	687	0	0	1,775	68
2007 PM Peak Hr. Volumes	0	0	0	626	0	771	24	725	0	0	903	70

Previous Developments - AM Peak Hour Volumes

	Eastbound (I-40 N. ramp)			Westbound (I-40 N. ramp)			Northbound (Unser Blvd)			Southbound (Unser Blvd)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Watershed Residential	0	0	0	0	0	5	0	0	0	0	17	0
98th / Unser Development	0	0	0	0	0	29	0	26	0	0	36	0
Subtotal	0	0	0	0	0	34	0	26	0	0	53	0

Previous Developments - PM Peak Hour Volumes

	Eastbound (I-40 N. ramp)			Westbound (I-40 N. ramp)			Northbound (Unser Blvd)			Southbound (Unser Blvd)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Watershed Residential	0	0	0	0	0	17	0	0	0	0	10	0
98th / Unser Development	0	0	0	0	0	52	0	46	0	0	112	1
Subtotal	0	0	0	0	0	69	0	46	0	0	122	1

MRCOG Forecast Volumes Worksheet

Based on 2007 Traffic Count

2007 AM Link Volume	0	534	711	1,843
2007 PM Link Volume	0	1,397	749	973

Based on MRCOG Model (2030 Data Set)

2005 AM Link Volume	0	494	392	2,230
2005 PM Link Volume	0	1,405	1,026	1,599
2030 AM Link Volume	0	528	1,240	3,408
2030 PM Link Volume	0	1,290	2,156	2,582

Growth Rate to Apply to Existing Counts to Match 2030 Forecasts

2007-2030 AM Growth Rates	#DIV/0!	-0.05%	3.23%	3.69%
2007-2030 PM Growth Rates	#DIV/0!	-0.33%	8.17%	7.19%

Growth Rate to Apply to 2005 Model Volumes to Match 2025 Forecasts

2005-2030 AM Growth Rates	#DIV/0!	0.28%	8.65%	2.11%
2005-2030 PM Growth Rates	#DIV/0!	-0.33%	4.41%	2.46%

Heritage Neighborhood Center (Ladera Dr / Unser Blvd)
Projected Turning Movements Worksheet
Los Volcanes Rd / Unser Blvd

INTERSECTION : E-W Street: Los Volcanes Rd (4)
N-S Street: Unser Blvd
Year of Existing Counts 2007
Implementation Year 2010

	0.00%			0.00%			0.07%			2.95%		
	Eastbound (Los Volcanes Rd)			Westbound (Los Volcanes Rd)			Northbound (Unser Blvd)			Southbound (Unser Blvd)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Existing Volumes	137	69	7	95	27	134	8	1,101	101	260	897	69
Background Traffic Growth	0	0	0	0	0	0	0	2	0	23	79	6
Subtotal	137	69	7	95	27	134	8	1,103	101	283	976	75
I-40 / Unser Commercial Trips	0	5	0	202	4	85	0	89	178	207	0	0
Subtotal (NO BUILD - A.M.)	137	74	7	297	31	219	8	1,192	279	490	976	75
Percent Commercial Trips Generated(Entering)	0.88%	0.00%	0.00%	0.00%	0.00%	6.83%	0.00%	17.43%	0.00%	0.00%	0.00%	0.00%
Percent Commercial Trips Generated(Exiting)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	6.83%	17.43%	0.88%
Percent Office Trips Generated(Entering)	0.82%	0.00%	0.00%	0.00%	0.00%	2.47%	0.00%	12.07%	0.00%	0.00%	0.00%	0.00%
Percent Office Trips Generated(Exiting)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	2.47%	12.07%	0.82%
Total Trips Generated	5	0	0	0	0	36	0	95	0	26	67	3
Total AM Peak Hour BUILD Volumes	142	74	7	297	31	255	8	1,287	279	516	1,043	78

	17.03%			0.00%			5.38%			1.68%		
	Eastbound (Los Volcanes Rd)			Westbound (Los Volcanes Rd)			Northbound (Unser Blvd)			Southbound (Unser Blvd)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Existing Volumes	54	15	4	73	23	132	10	793	120	108	1,022	121
Background Traffic Growth	28	8	2	0	0	0	2	128	19	5	51	6
Subtotal	82	23	6	73	23	132	12	921	139	113	1,073	127
I-40 / Unser Commercial Trips	0	10	0	663	10	327	0	162	324	496	-179	0
Subtotal (NO BUILD - P.M.)	82	33	6	736	33	459	12	1,083	463	609	894	127
Percent Commercial Trips Generated(Entering)	0.88%	0.00%	0.00%	0.00%	0.00%	6.83%	0.00%	17.43%	0.00%	0.00%	0.00%	0.00%
Percent Commercial Trips Generated(Exiting)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	6.83%	17.43%	0.88%
Percent Office Trips Generated(Entering)	0.82%	0.00%	0.00%	0.00%	0.00%	2.47%	0.00%	12.07%	0.00%	0.00%	0.00%	0.00%
Percent Office Trips Generated(Exiting)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	2.47%	12.07%	0.82%
Total Trips Generated	5	0	0	0	0	41	0	107	0	42	113	6
Total PM Peak Hour BUILD Volumes	87	33	6	736	33	500	12	1,190	463	651	1,007	133

	Entering	Exiting		
Number of Commercial Trips Generated	499	378	A.M.	100% Commercial Development
	602	580	P.M.	
Number of Office Trips Generated	68	9	A.M.	100% Office Development
	20	96	P.M.	

	Eastbound (Los Volcanes Rd)			Westbound (Los Volcanes Rd)			Northbound (Unser Blvd)			Southbound (Unser Blvd)		
2007 AM Peak Hr. Volumes	137	69	7	95	27	134	8	1,101	101	260	897	69
2007 PM Peak Hr. Volumes	54	15	4	73	23	132	10	793	120	108	1,022	121

MRCOG Forecast Volumes Worksheet

Based on 2007 Traffic Count

2007 AM Link Volume	213	256	1,210	1,226
2007 PM Link Volume	73	228	923	1,251

Based on MRCOG Model (2030 Data Set)

2005 AM Link Volume	79	35	1,232	1,112
2005 PM Link Volume	104	107	1,323	1,404
2030 AM Link Volume	156	22	1,230	2,057
2030 PM Link Volume	359	74	2,066	1,734

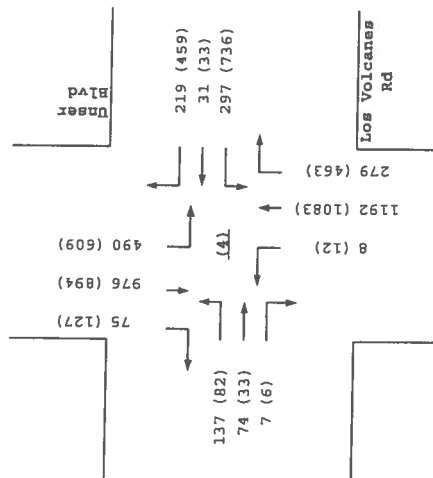
Growth Rate to Apply to Existing Counts to Match 2030 Forecasts

2007-2030 AM Growth Rates	-1.16%	-3.97%	0.07%	2.95%
2007-2030 PM Growth Rates	17.03%	-2.94%	5.38%	1.68%

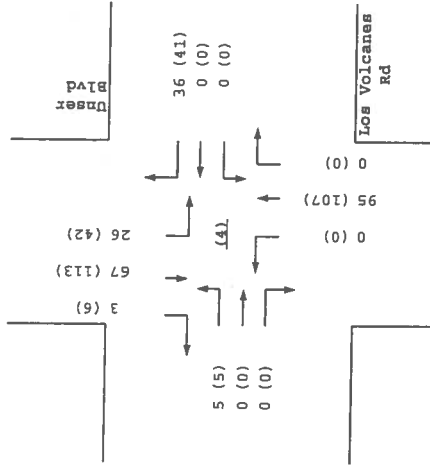
Growth Rate to Apply to 2005 Model Volumes to Match 2030 Forecasts

2005-2030 AM Growth Rates	3.90%	-1.49%	-0.01%	3.40%
2005-2030 PM Growth Rates	9.81%	-1.23%	2.25%	0.94%

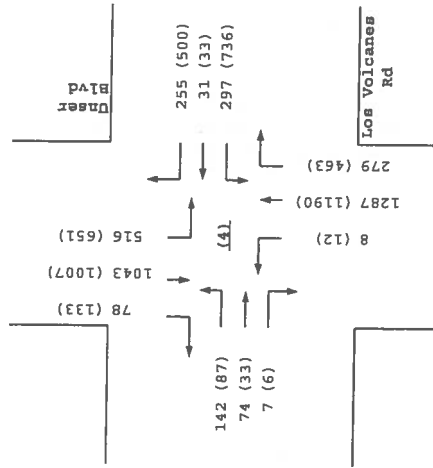
2010
NO BUILD



Trips



2010
BUILD



Los Volcanes Rd / Unser Blvd

Heritage Neighborhood Center (Ladera Dr / Unser Blvd)
Projected Turning Movements Worksheet
I-40 S. ramp / Unser Blvd

INTERSECTION: E-W Street I-40 S. ramp (6)
 N-S Street Unser Blvd
 Year of Existing Counts 2007
 Implementation Year 2010

	20.22%			0.00%			0.60%			5.68%		
	Eastbound (I-40 S. ramp)			Westbound (I-40 S. ramp)			Northbound (Unser Blvd)			Southbound (Unser Blvd)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Existing Volumes	31	0	9	0	0	0	3	535	580	4	821	740
Background Traffic Growth	19	0	5	0	0	0	0	10	10	1	140	126
Subtotal	50	0	14	0	0	0	3	545	590	5	961	866
I-40 / Unser Development	0	0	27	0	0	0	0	207	21	0	274	0
Southwest Mesa Subdivisions	0	0	0	0	0	0	0	26	49	0	77	0
Previous Development from below	0	0	0	0	0	0	0	26	0	0	17	17
Subtotal (NO BUILD - A.M.)	50	0	41	0	0	0	3	804	660	5	1,329	883
Percent Commercial Trips Generated(Entering)	1.90%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	25.14%	0.00%	0.00%	0.00%	0.00%
Percent Commercial Trips Generated(Exiting)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	8.87%	25.14%	0.00%
Percent Office Trips Generated(Entering)	1.49%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	15.36%	0.00%	0.00%	0.00%	0.00%
Percent Office Trips Generated(Exiting)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	30.10%	15.36%	0.00%
Total Trips Generated	10	0	0	0	0	0	0	135	0	40	96	0
Total AM Peak Hour BUILD Volumes	60	0	41	0	0	0	3	939	660	45	1,425	883

	11.79%			0.00%			5.96%			2.82%		
	Eastbound (I-40 S. ramp)			Westbound (I-40 S. ramp)			Northbound (Unser Blvd)			Southbound (Unser Blvd)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Existing Volumes	69	0	21	0	0	0	0	641	364	0	1,162	308
Background Traffic Growth	24	0	7	0	0	0	0	115	65	0	98	26
Subtotal	93	0	28	0	0	0	0	756	429	0	1,260	334
I-40 / Unser Development	0	0	49	0	0	0	0	496	51	0	498	0
Southwest Mesa Subdivisions	0	0	0	0	0	0	0	38	96	0	98	0
Previous Development from below	1	0	0	0	0	0	0	45	0	0	52	10
Subtotal (NO BUILD - P.M.)	94	0	77	0	0	0	0	1,335	576	0	1,908	344
Percent Commercial Trips Generated(Entering)	1.90%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	25.14%	0.00%	0.00%	0.00%	0.00%
Percent Commercial Trips Generated(Exiting)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	8.87%	25.14%	0.00%
Percent Office Trips Generated(Entering)	1.49%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	15.36%	0.00%	0.00%	0.00%	0.00%
Percent Office Trips Generated(Exiting)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	30.10%	15.36%	0.00%
Total Trips Generated	11	0	0	0	0	0	0	154	0	86	161	0
Total PM Peak Hour BUILD Volumes	105	0	77	0	0	0	0	1,489	576	86	2,069	344

Number of Commercial Trips Generated
 499 378 A.M. 100% Commercial Development
 602 580 P.M.
 Number of Office Trips Generated
 68 9 A.M. 100% Office Development
 20 96 P.M.

	Eastbound (I-40 S. ramp)			Westbound (I-40 S. ramp)			Northbound (Unser Blvd)			Southbound (Unser Blvd)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
2007 AM Peak Hr. Volumes	31	0	9	0	0	0	3	535	580	4	821	740
2007 PM Peak Hr. Volumes	69	0	21	0	0	0	0	641	364	0	1,162	308

Previous Developments - AM Peak Hour Volumes

	Eastbound (I-40 S. ramp)			Westbound (I-40 S. ramp)			Northbound (Unser Blvd)			Southbound (Unser Blvd)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Watershed Residential	0	0	0	0	0	0	0	0	0	0	0	17
98th / Unser Development	0	0	0	0	0	0	0	26	0	0	17	0
Subtotal	0	0	0	0	0	0	0	26	0	0	17	17

Previous Developments - PM Peak Hour Volumes

	Eastbound (I-40 S. ramp)			Westbound (I-40 S. ramp)			Northbound (Unser Blvd)			Southbound (Unser Blvd)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Watershed Residential	0	0	0	0	0	0	0	0	0	0	0	10
98th / Unser Development	1	0	0	0	0	0	0	45	0	0	52	0
Subtotal	1	0	0	0	0	0	0	45	0	0	52	10

MRCOG Forecast Volumes Worksheet

Based on 2007 Traffic Count												
2007 AM Link Volume		40		0			1,118			1,565		
2007 PM Link Volume		90		0			1,005			1,470		
Based on MRCOG Model (2030 Data Set)												
2005 AM Link Volume		26		0			1226			1108		
2005 PM Link Volume		201		0			1404			1973		
2030 AM Link Volume		226		0			1272			3608		
2030 PM Link Volume		334		0			2383			2425		
Growth Rate to Apply to Existing Counts to Match 2030 Forecasts												
2007-2030 AM Growth Rates		20.22%		#DIV/0!			0.60%			5.68%		
2007-2030 PM Growth Rates		11.79%		#DIV/0!			5.96%			2.82%		
Growth Rate to Apply to 2005 Model Volumes to Match 2025 Forecasts												
2005-2030 AM Growth Rates		30.77%		#DIV/0!			0.15%			9.03%		
2005-2030 PM Growth Rates		2.65%		#DIV/0!			2.79%			0.92%		

Heritage Neighborhood Center (Ladera Dr / Unser Blvd)

Projected Turning Movements Worksheet

Ladera Dr / Ouray Rd

INTERSECTION :

E-W Street: Ladera Dr

(5)

N-S Street: Ouray Rd

Year of Existing Counts

2007

Implementation Year

2010

Growth Rates

Existing Volumes

Background Traffic Growth

Subtotal (NO BUILD - A.M.)

Percent Commercial Trips Generated(Entering)

Percent Commercial Trips Generated(Exiting)

Percent Office Trips Generated(Entering)

Percent Office Trips Generated(Exiting)

Total Trips Generated

Total AM Peak Hour BUILD Volumes

0.00%			15.86%			0.92%			3.68%		
Eastbound (Ladera Dr)			Westbound (Ladera Dr)			Northbound (Ouray Rd)			Southbound (Ouray Rd)		
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
20	365	247	3	145	20	98	77	5	89	191	13
0	0	0	1	69	10	3	2	0	10	21	1
20	365	247	4	214	30	101	79	5	99	212	14
0.00%	0.00%	0.00%	0.00%	12.41%	0.00%	10.67%	0.00%	0.00%	0.00%	0.00%	0.93%
0.93%	12.41%	10.67%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%
0.00%	0.00%	0.00%	0.00%	21.85%	0.00%	2.84%	0.00%	0.00%	0.00%	0.00%	0.26%
0.26%	21.85%	2.84%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%
4	49	40	0	77	0	55	0	0	0	0	5
24	414	287	4	291	30	156	79	5	99	212	19

Existing Volumes

Background Traffic Growth

Subtotal (NO BUILD - P.M.)

Percent Commercial Trips Generated(Entering)

Percent Commercial Trips Generated(Exiting)

Percent Office Trips Generated(Entering)

Percent Office Trips Generated(Exiting)

Total Trips Generated

Total PM Peak Hour BUILD Volumes

4.42%			4.24%			0.00%			2.30%		
Eastbound (Ladera Dr)			Westbound (Ladera Dr)			Northbound (Ouray Rd)			Southbound (Ouray Rd)		
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
19	271	200	20	379	107	293	233	16	34	123	13
3	36	27	3	48	14	0	0	0	2	8	1
22	307	227	23	427	121	293	233	16	36	131	14
0.00%	0.00%	0.00%	0.00%	12.41%	0.00%	10.67%	0.00%	0.00%	0.00%	0.00%	0.93%
0.93%	12.41%	10.67%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%
0.00%	0.00%	0.00%	0.00%	21.85%	0.00%	2.84%	0.00%	0.00%	0.00%	0.00%	0.26%
0.26%	21.85%	2.84%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%
5	93	65	0	79	0	65	0	0	0	0	6
27	400	292	23	506	121	358	233	16	36	131	20

Number of Commercial Trips Generated

Entering Exiting

499 378

A.M.

100% Commercial Development

602 580

P.M.

Number of Office Trips Generated

68 9

A.M.

100% Office Development

20 96

P.M.

2007 AM Peak Hr. Volumes

2007 PM Peak Hr. Volumes

Eastbound (Ladera Dr)			Westbound (Ladera Dr)			Northbound (Ouray Rd)			Southbound (Ouray Rd)		
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
20	365	247	3	145	20	98	77	5	89	191	13
19	271	200	20	379	107	293	233	16	34	123	13

MRCOG Forecast Volumes Worksheet

Based on 2007 Traffic Count

2007 AM Link Volume

632

168

180

293

2007 PM Link Volume

490

506

542

170

Based on MRCOG Model (2030 Data Set)

2005 AM Link Volume

94

130

151

218

2005 PM Link Volume

191

187

211

102

2030 AM Link Volume

474

781

218

541

2030 PM Link Volume

988

999

426

260

Growth Rate to Apply to Existing Counts to Match 2030 Forecasts

2007-2030 AM Growth Rates

-1.09%

15.86%

0.92%

3.68%

2007-2030 PM Growth Rates

4.42%

4.24%

-0.93%

2.30%

Growth Rate to Apply to 2005 Model Volumes to Match 2030 Forecasts

2005-2030 AM Growth Rates

16.17%

20.03%

1.77%

5.93%

2005-2030 PM Growth Rates

16.69%

17.37%

4.08%

6.20%

Heritage Neighborhood Center (Ladera Dr / Unser Blvd)
 Projected Turning Movements Worksheet
I-40 S. ramp / Unser Blvd

INTERSECTION: E-W Street I-40 S. ramp (5)
 N-S Street Unser Blvd

Year of Existing Counts 2007
 Implementation Year 2010

	20.22%			0.00%			0.60%			5.68%		
	Eastbound (I-40 S. ramp)			Westbound (I-40 S. ramp)			Northbound (Unser Blvd)			Southbound (Unser Blvd)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Existing Volumes	31	0	9	0	0	0	3	535	580	4	821	740
Background Traffic Growth	19	0	5	0	0	0	0	10	10	1	140	128
Subtotal	50	0	14	0	0	0	3	545	590	5	961	868
I-40 / Unser Development	0	0	27	0	0	0	0	207	21	0	274	0
Southwest Mesa Subdivisions	0	0	0	0	0	0	0	26	49	0	77	0
Previous Development from below	0	0	0	0	0	0	0	26	0	0	17	17
Subtotal (NO BUILD - A.M.)	50	0	41	0	0	0	3	804	660	5	1,329	883
Percent Commercial Trips Generated(Entering)	1.90%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	25.14%	0.00%	0.00%	0.00%	0.00%
Percent Commercial Trips Generated(Exiting)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	9.87%	25.14%	0.00%
Percent Office Trips Generated(Entering)	1.49%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	15.36%	0.00%	0.00%	0.00%	0.00%
Percent Office Trips Generated(Exiting)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	30.10%	15.36%	0.00%
Total Trips Generated	10	0	0	0	0	0	0	135	0	40	96	0
Total AM Peak Hour BUILD Volumes	60	0	41	0	0	0	3	939	660	45	1,425	883

	11.79%			0.00%			5.96%			2.82%		
	Eastbound (I-40 S. ramp)			Westbound (I-40 S. ramp)			Northbound (Unser Blvd)			Southbound (Unser Blvd)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Existing Volumes	69	0	21	0	0	0	0	641	364	0	1,162	308
Background Traffic Growth	24	0	7	0	0	0	0	115	65	0	98	26
Subtotal	93	0	28	0	0	0	0	756	429	0	1,260	334
I-40 / Unser Development	0	0	49	0	0	0	0	498	51	0	498	0
Southwest Mesa Subdivisions	0	0	0	0	0	0	0	38	96	0	98	0
Previous Development from below	1	0	0	0	0	0	0	45	0	0	52	10
Subtotal (NO BUILD - P.M.)	94	0	77	0	0	0	0	1,335	576	0	1,908	344
Percent Commercial Trips Generated(Entering)	1.90%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	25.14%	0.00%	0.00%	0.00%	0.00%
Percent Commercial Trips Generated(Exiting)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	9.87%	25.14%	0.00%
Percent Office Trips Generated(Entering)	1.49%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	15.36%	0.00%	0.00%	0.00%	0.00%
Percent Office Trips Generated(Exiting)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	30.10%	15.36%	0.00%
Total Trips Generated	11	0	0	0	0	0	0	154	0	86	161	0
Total PM Peak Hour BUILD Volumes	105	0	77	0	0	0	0	1,489	576	86	2,069	344

Number of Commercial Trips Generated	Entering	Exiting	A.M.	100% Commercial Development
	498	378		
Number of Office Trips Generated	602	580	P.M.	100% Office Development
	68	9		
	20	96	P.M.	

	Eastbound (I-40 S. ramp)			Westbound (I-40 S. ramp)			Northbound (Unser Blvd)			Southbound (Unser Blvd)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
2007 AM Peak Hr. Volumes	31	0	9	0	0	0	3	535	580	4	821	740
2007 PM Peak Hr. Volumes	69	0	21	0	0	0	0	641	364	0	1,162	308

Previous Developments - AM Peak Hour Volumes

	Eastbound (I-40 S. ramp)			Westbound (I-40 S. ramp)			Northbound (Unser Blvd)			Southbound (Unser Blvd)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Watershed Residential	0	0	0	0	0	0	0	0	0	0	0	17
98th / Unser Development	0	0	0	0	0	0	0	26	0	0	17	0
Subtotal	0	0	0	0	0	0	0	26	0	0	17	17

Previous Developments - PM Peak Hour Volumes

	Eastbound (I-40 S. ramp)			Westbound (I-40 S. ramp)			Northbound (Unser Blvd)			Southbound (Unser Blvd)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Watershed Residential	0	0	0	0	0	0	0	0	0	0	0	10
98th / Unser Development	1	0	0	0	0	0	0	45	0	0	52	0
Subtotal	1	0	0	0	0	0	0	45	0	0	52	10

MRCOG Forecast Volumes Worksheet

Based on 2007 Traffic Count

2007 AM Link Volume	40	0	1,118	1,565
2007 PM Link Volume	90	0	1,005	1,470

Based on MRCOG Model (2030 Data Set)

2005 AM Link Volume	26	0	1,226	1,108
2005 PM Link Volume	201	0	1,404	1,973
2030 AM Link Volume	226	0	1,272	3,608
2030 PM Link Volume	334	0	2,383	2,425

Growth Rate to Apply to Existing Counts to Match 2030 Forecasts

2007-2030 AM Growth Rates	20.22%	#DIV/0!	0.60%	5.68%
2007-2030 PM Growth Rates	11.79%	#DIV/0!	5.96%	2.82%

Growth Rate to Apply to 2005 Model Volumes to Match 2025 Forecasts

2005-2030 AM Growth Rates	30.77%	#DIV/0!	0.15%	9.03%
2005-2030 PM Growth Rates	2.65%	#DIV/0!	2.79%	0.92%

Heritage Neighborhood Center (Ladera Dr / Unser Blvd)

Projected Turning Movements Worksheet

Ladera Dr / Ouray Rd

INTERSECTION : E-W Street: Ladera Dr (5)
 N-S Street: Ouray Rd
 Year of Existing Counts 2007
 Implementation Year 2010

	0.00%			15.86%			0.92%			3.68%		
	Eastbound (Ladera Dr)			Westbound (Ladera Dr)			Northbound (Ouray Rd)			Southbound (Ouray Rd)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Existing Volumes	20	365	247	3	145	20	98	77	5	89	191	13
Background Traffic Growth	0	0	0	1	69	10	3	2	0	10	21	1
Subtotal (NO BUILD - A.M.)	20	365	247	4	214	30	101	79	5	99	212	14
Percent Commercial Trips Generated(Entering)	0.00%	0.00%	0.00%	0.00%	12.41%	0.00%	10.67%	0.00%	0.00%	0.00%	0.00%	0.93%
Percent Commercial Trips Generated(Exiting)	0.93%	12.41%	10.67%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%
Percent Office Trips Generated(Entering)	0.00%	0.00%	0.00%	0.00%	21.85%	0.00%	2.84%	0.00%	0.00%	0.00%	0.00%	0.26%
Percent Office Trips Generated(Exiting)	0.26%	21.85%	2.84%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%
Total Trips Generated	4	49	40	0	77	0	55	0	0	0	0	5
Total AM Peak Hour BUILD Volumes	24	414	287	4	291	30	156	79	5	99	212	19

	4.42%			4.24%			0.00%			2.30%		
	Eastbound (Ladera Dr)			Westbound (Ladera Dr)			Northbound (Ouray Rd)			Southbound (Ouray Rd)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Existing Volumes	19	271	200	20	379	107	293	233	16	34	123	13
Background Traffic Growth	3	36	27	3	48	14	0	0	0	2	8	1
Subtotal (NO BUILD - P.M.)	22	307	227	23	427	121	293	233	16	36	131	14
Percent Commercial Trips Generated(Entering)	0.00%	0.00%	0.00%	0.00%	12.41%	0.00%	10.67%	0.00%	0.00%	0.00%	0.00%	0.93%
Percent Commercial Trips Generated(Exiting)	0.93%	12.41%	10.67%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%
Percent Office Trips Generated(Entering)	0.00%	0.00%	0.00%	0.00%	21.85%	0.00%	2.84%	0.00%	0.00%	0.00%	0.00%	0.26%
Percent Office Trips Generated(Exiting)	0.26%	21.85%	2.84%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%
Total Trips Generated	5	93	65	0	79	0	65	0	0	0	0	6
Total PM Peak Hour BUILD Volumes	27	400	292	23	506	121	358	233	16	36	131	20

	Entering	Exiting	
Number of Commercial Trips Generated	499	378	A.M. 100% Commercial Development
	602	580	P.M.
Number of Office Trips Generated	68	9	A.M. 100% Office Development
	20	96	P.M.

	Eastbound (Ladera Dr)			Westbound (Ladera Dr)			Northbound (Ouray Rd)			Southbound (Ouray Rd)		
2007 AM Peak Hr. Volumes	20	365	247	3	145	20	98	77	5	89	191	13
2007 PM Peak Hr. Volumes	19	271	200	20	379	107	293	233	16	34	123	13

MRCOG Forecast Volumes Worksheet

Based on 2007 Traffic Count

2007 AM Link Volume	632	168	180	293
2007 PM Link Volume	490	506	542	170

Based on MRCOG Model (2030 Data Set)

2005 AM Link Volume	94	130	151	218
2005 PM Link Volume	191	187	211	102
2030 AM Link Volume	474	781	218	541
2030 PM Link Volume	988	999	426	260

Growth Rate to Apply to Existing Counts to Match 2030 Forecasts

2007-2030 AM Growth Rates	-1.09%	15.86%	0.92%	3.68%
2007-2030 PM Growth Rates	4.42%	4.24%	-0.93%	2.30%

Growth Rate to Apply to 2005 Model Volumes to Match 2030 Forecasts

2005-2030 AM Growth Rates	16.17%	20.03%	1.77%	5.93%
2005-2030 PM Growth Rates	16.69%	17.37%	4.08%	6.20%

Heritage Neighborhood Center (Ladera Dr / Unser Blvd)
Projected Turning Movements Worksheet
I-40 S. ramp / Unser Blvd

INTERSECTION: E-W Street I-40 S. ramp (6)
N-S Street Unser Blvd
Year of Existing Counts 2007
Implementation Year 2010

	20.22%			0.00%			0.60%			5.68%		
	Eastbound (I-40 S. ramp)			Westbound (I-40 S. ramp)			Northbound (Unser Blvd)			Southbound (Unser Blvd)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Existing Volumes	31	0	9	0	0	0	3	535	580	4	821	740
Background Traffic Growth	19	0	5	0	0	0	0	10	10	1	140	126
Subtotal	50	0	14	0	0	0	3	545	590	5	961	866
I-40 / Unser Development	0	0	27	0	0	0	0	207	21	0	274	0
Southwest Mesa Subdivisions	0	0	0	0	0	0	0	26	49	0	77	0
Previous Development from below	0	0	0	0	0	0	0	26	0	0	17	17
Subtotal (NO BUILD - A.M.)	50	0	41	0	0	0	3	804	660	5	1,329	883
Percent Commercial Trips Generated(Entering)	1.90%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	25.14%	0.00%	0.00%	0.00%	0.00%
Percent Commercial Trips Generated(Exiting)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	9.87%	25.14%	0.00%
Percent Office Trips Generated(Entering)	1.49%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	15.36%	0.00%	0.00%	0.00%	0.00%
Percent Office Trips Generated(Exiting)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	30.10%	15.36%	0.00%
Total Trips Generated	10	0	0	0	0	0	0	135	0	40	96	0
Total AM Peak Hour BUILD Volumes	60	0	41	0	0	0	3	939	660	45	1,425	883

	11.79%			0.00%			5.96%			2.82%		
	Eastbound (I-40 S. ramp)			Westbound (I-40 S. ramp)			Northbound (Unser Blvd)			Southbound (Unser Blvd)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Existing Volumes	69	0	21	0	0	0	0	641	364	0	1,162	308
Background Traffic Growth	24	0	7	0	0	0	0	115	65	0	98	26
Subtotal	93	0	28	0	0	0	0	756	429	0	1,260	334
I-40 / Unser Development	0	0	49	0	0	0	0	496	51	0	498	0
Southwest Mesa Subdivisions	0	0	0	0	0	0	0	38	96	0	98	0
Previous Development from below	1	0	0	0	0	0	0	45	0	0	52	10
Subtotal (NO BUILD - P.M.)	94	0	77	0	0	0	0	1,335	576	0	1,908	344
Percent Commercial Trips Generated(Entering)	1.90%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	25.14%	0.00%	0.00%	0.00%	0.00%
Percent Commercial Trips Generated(Exiting)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	9.87%	25.14%	0.00%
Percent Office Trips Generated(Entering)	1.49%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	15.36%	0.00%	0.00%	0.00%	0.00%
Percent Office Trips Generated(Exiting)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	30.10%	15.36%	0.00%
Total Trips Generated	11	0	0	0	0	0	0	154	0	86	161	0
Total PM Peak Hour BUILD Volumes	105	0	77	0	0	0	0	1,489	576	86	2,069	344

Number of Commercial Trips Generated
499 378 A.M. 100% Commercial Development
602 580 P.M.
Number of Office Trips Generated
68 9 A.M. 100% Office Development
20 96 P.M.

	Eastbound (I-40 S. ramp)			Westbound (I-40 S. ramp)			Northbound (Unser Blvd)			Southbound (Unser Blvd)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
2007 AM Peak Hr. Volumes	31	0	9	0	0	0	3	535	580	4	821	740
2007 PM Peak Hr. Volumes	69	0	21	0	0	0	0	641	364	0	1,162	308

Previous Developments - AM Peak Hour Volumes

	Eastbound (I-40 S. ramp)			Westbound (I-40 S. ramp)			Northbound (Unser Blvd)			Southbound (Unser Blvd)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Watershed Residential	0	0	0	0	0	0	0	0	0	0	0	17
98th / Unser Development	0	0	0	0	0	0	0	26	0	0	17	0
Subtotal	0	0	0	0	0	0	0	26	0	0	17	17

Previous Developments - PM Peak Hour Volumes

	Eastbound (I-40 S. ramp)			Westbound (I-40 S. ramp)			Northbound (Unser Blvd)			Southbound (Unser Blvd)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Watershed Residential	0	0	0	0	0	0	0	0	0	0	0	10
98th / Unser Development	1	0	0	0	0	0	0	45	0	0	52	0
Subtotal	1	0	0	0	0	0	0	45	0	0	52	10

MRCOG Forecast Volumes Worksheet

Based on 2007 Traffic Count

2007 AM Link Volume	40	0	1,118	1,565
2007 PM Link Volume	90	0	1,005	1,470

Based on MRCOG Model (2030 Data Set)

2005 AM Link Volume	26	0	1,226	1,108
2005 PM Link Volume	201	0	1,404	1,973
2030 AM Link Volume	226	0	1,272	3,608
2030 PM Link Volume	334	0	2,383	2,425

Growth Rate to Apply to Existing Counts to Match 2030 Forecasts

2007-2030 AM Growth Rates	20.22%	#DIV/0!	0.60%	5.68%
2007-2030 PM Growth Rates	11.79%	#DIV/0!	5.96%	2.82%

Growth Rate to Apply to 2005 Model Volumes to Match 2025 Forecasts

2005-2030 AM Growth Rates	30.77%	#DIV/0!	0.15%	9.03%
2005-2030 PM Growth Rates	2.65%	#DIV/0!	2.79%	0.92%

Heritage Neighborhood Center (Ladera Dr / Unser Blvd)
Projected Turning Movements Worksheet
I-40 S. ramp / Unser Blvd

INTERSECTION: E-W Street: I-40 S. ramp (6)
N-S Street: Unser Blvd

Year of Existing Counts: 2007
Implementation Year: 2010

	20.22%			0.00%			0.60%			5.68%		
	Eastbound (I-40 S. ramp)			Westbound (I-40 S. ramp)			Northbound (Unser Blvd)			Southbound (Unser Blvd)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Existing Volumes	31	0	9	0	0	0	3	535	580	4	821	740
Background Traffic Growth	19	0	5	0	0	0	0	10	10	1	140	126
Subtotal	50	0	14	0	0	0	3	545	590	5	961	866
I-40 / Unser Development	0	0	27	0	0	0	0	207	21	0	274	0
Southwest Mesa Subdivisions	0	0	0	0	0	0	0	26	49	0	77	0
Previous Development from below	0	0	0	0	0	0	0	26	0	0	17	17
Subtotal (NO BUILD - A.M.)	50	0	41	0	0	0	3	804	660	5	1,320	883
Percent Commercial Trips Generated(Entering)	1.90%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	25.14%	0.00%	0.00%	0.00%	0.00%
Percent Commercial Trips Generated(Exiting)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	8.87%	25.14%	0.00%
Percent Office Trips Generated(Entering)	1.49%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	15.36%	0.00%	0.00%	0.00%	0.00%
Percent Office Trips Generated(Exiting)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	30.10%	15.36%	0.00%
Total Trips Generated	10	0	0	0	0	0	0	135	0	40	96	0
Total AM Peak Hour BUILD Volumes	60	0	41	0	0	0	3	939	660	45	1,425	883

	11.79%			0.00%			5.96%			2.82%		
	Eastbound (I-40 S. ramp)			Westbound (I-40 S. ramp)			Northbound (Unser Blvd)			Southbound (Unser Blvd)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Existing Volumes	69	0	21	0	0	0	0	641	364	0	1,162	308
Background Traffic Growth	24	0	7	0	0	0	0	115	65	0	98	26
Subtotal	93	0	28	0	0	0	0	756	429	0	1,260	334
I-40 / Unser Development	0	0	49	0	0	0	0	496	51	0	498	0
Southwest Mesa Subdivisions	0	0	0	0	0	0	0	38	96	0	98	0
Previous Development from below	1	0	0	0	0	0	0	45	0	0	52	10
Subtotal (NO BUILD - P.M.)	94	0	77	0	0	0	0	1,335	576	0	1,908	344
Percent Commercial Trips Generated(Entering)	1.90%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	25.14%	0.00%	0.00%	0.00%	0.00%
Percent Commercial Trips Generated(Exiting)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	8.87%	25.14%	0.00%
Percent Office Trips Generated(Entering)	1.49%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	15.36%	0.00%	0.00%	0.00%	0.00%
Percent Office Trips Generated(Exiting)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	30.10%	15.36%	0.00%
Total Trips Generated	11	0	0	0	0	0	0	154	0	86	161	0
Total PM Peak Hour BUILD Volumes	105	0	77	0	0	0	0	1,489	576	86	2,069	344

	Entering	Exiting	
Number of Commercial Trips Generated	499	378	A.M. 100% Commercial Development
	602	580	P.M.
Number of Office Trips Generated	68	9	A.M. 100% Office Development
	20	96	P.M.

	Eastbound (I-40 S. ramp)			Westbound (I-40 S. ramp)			Northbound (Unser Blvd)			Southbound (Unser Blvd)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
2007 AM Peak Hr. Volumes	31	0	9	0	0	0	3	535	580	4	821	740
2007 PM Peak Hr. Volumes	69	0	21	0	0	0	0	641	364	0	1,162	308

Previous Developments - AM Peak Hour Volumes

	Eastbound (I-40 S. ramp)			Westbound (I-40 S. ramp)			Northbound (Unser Blvd)			Southbound (Unser Blvd)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Watershed Residential	0	0	0	0	0	0	0	0	0	0	0	17
98th / Unser Development	0	0	0	0	0	0	0	26	0	0	17	0
Subtotal	0	0	0	0	0	0	0	26	0	0	17	17

Previous Developments - PM Peak Hour Volumes

	Eastbound (I-40 S. ramp)			Westbound (I-40 S. ramp)			Northbound (Unser Blvd)			Southbound (Unser Blvd)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Watershed Residential	0	0	0	0	0	0	0	0	0	0	0	10
98th / Unser Development	1	0	0	0	0	0	0	45	0	0	52	0
Subtotal	1	0	0	0	0	0	0	45	0	0	52	10

MRCOG Forecast Volumes Worksheet

Based on 2007 Traffic Count

2007 AM Link Volume	40	0	1,118	1,565
2007 PM Link Volume	90	0	1,005	1,470

Based on MRCOG Model (2030 Data Set)

2005 AM Link Volume	26	0	1226	1108
2005 PM Link Volume	201	0	1404	1973
2030 AM Link Volume	226	0	1272	3608
2030 PM Link Volume	334	0	2383	2425

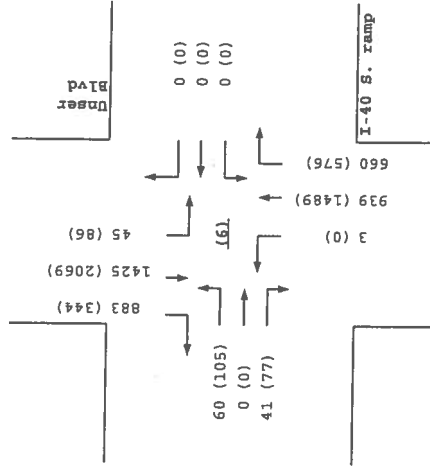
Growth Rate to Apply to Existing Counts to Match 2030 Forecasts

2007-2030 AM Growth Rates	20.22%	#DIV/0!	0.60%	5.68%
2007-2030 PM Growth Rates	11.79%	#DIV/0!	5.96%	2.82%

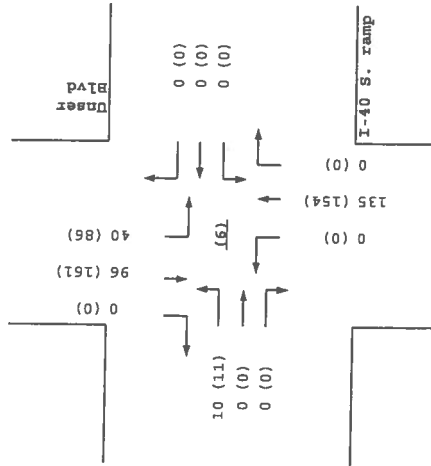
Growth Rate to Apply to 2005 Model Volumes to Match 2025 Forecasts

2005-2030 AM Growth Rates	30.77%	#DIV/0!	0.15%	9.03%
2005-2030 PM Growth Rates	2.65%	#DIV/0!	2.79%	0.92%

2010
BUILD

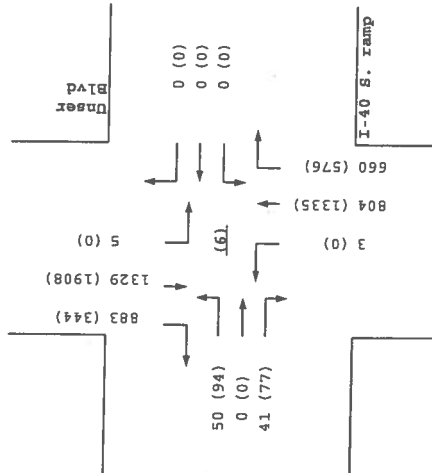


Trips



I-40 S. ramp / Unser Blvd

2010
NO BUILD



Heritage Neighborhood Center (Ladera Dr / Unser Blvd)
Projected Turning Movements Worksheet
Ladera Dr / Market Rd

INTERSECTION: E-W Street: **Ladera Dr** (7)
N-S Street: **Market Rd**
Year of Existing Counts: **2007**
Implementation Year: **2010**

	0.00%			3.00%			0.00%			0.00%		
	Eastbound (Ladera Dr)			Westbound (Ladera Dr)			Northbound (Market Rd)			Southbound (Market Rd)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Existing Volumes	0	425	28	14	365	0	113	0	72	0	0	0
Background Traffic Growth	0	0	0	1	33	0	0	0	0	0	0	0
Subtotal (NO BUILD - A.M.)	0	425	28	15	398	0	113	0	72	0	0	0
Percent Commercial Trips Generated(Entering)	0.00%	0.00%	1.00%	33.51%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.15%	0.00%
Percent Commercial Trips Generated(Exiting)	0.08%	16.75%	0.00%	0.00%	0.00%	0.00%	6.90%	0.07%	16.76%	0.00%	0.00%	0.00%
Percent Office Trips Generated(Entering)	0.00%	0.00%	1.00%	27.57%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.04%	0.00%
Percent Office Trips Generated(Exiting)	0.02%	13.79%	0.00%	0.00%	0.00%	0.00%	6.23%	0.02%	13.78%	0.00%	0.00%	0.00%
Total Trips Generated	0	64	6	186	0	0	27	0	64	0	1	0
Total AM Peak Hour BUILD Volumes	0	489	34	201	398	0	140	0	136	0	1	0

	3.00%			3.00%			0.00%			0.00%		
	Eastbound (Ladera Dr)			Westbound (Ladera Dr)			Northbound (Market Rd)			Southbound (Market Rd)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Existing Volumes	0	513	156	48	463	0	83	0	36	0	0	0
Background Traffic Growth	0	46	14	4	42	0	0	0	0	0	0	0
Subtotal (NO BUILD - P.M.)	0	559	170	52	505	0	83	0	36	0	0	0
Percent Commercial Trips Generated(Entering)	0.00%	0.00%	1.00%	33.51%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.15%	0.00%
Percent Commercial Trips Generated(Exiting)	0.08%	16.75%	0.00%	0.00%	0.00%	0.00%	6.90%	0.07%	16.76%	0.00%	0.00%	0.00%
Percent Office Trips Generated(Entering)	0.00%	0.00%	1.00%	27.57%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.04%	0.00%
Percent Office Trips Generated(Exiting)	0.02%	13.79%	0.00%	0.00%	0.00%	0.00%	6.23%	0.02%	13.78%	0.00%	0.00%	0.00%
Total Trips Generated	0	110	6	208	0	0	46	0	110	0	1	0
Total PM Peak Hour BUILD Volumes	0	669	176	260	505	0	129	0	146	0	1	0

	Entering	Exiting		
Number of Commercial Trips Generated	499	378	A.M.	100% Commercial Development
	602	580	P.M.	
Number of Office Trips Generated	68	9	A.M.	100% Office Development
	20	96	P.M.	

	Eastbound (Ladera Dr)			Westbound (Ladera Dr)			Northbound (Market Rd)			Southbound (Market Rd)		
2007 AM Peak Hr. Volumes	0	425	28	14	365	0	113	0	72	0	0	0
2007 PM Peak Hr. Volumes	0	513	156	48	463	0	83	0	36	0	0	0

MRCOG Forecast Volumes Worksheet

Based on 2007 Traffic Count

2007 AM Link Volume	453	379	185	0
2007 PM Link Volume	669	511	119	0

Based on MRCOG Model (2030 Data Set)

2005 AM Link Volume	355	355	0	0
2005 PM Link Volume	261	261	0	0
2030 AM Link Volume	452	1202	30	0
2030 PM Link Volume	1062	1042	47	0

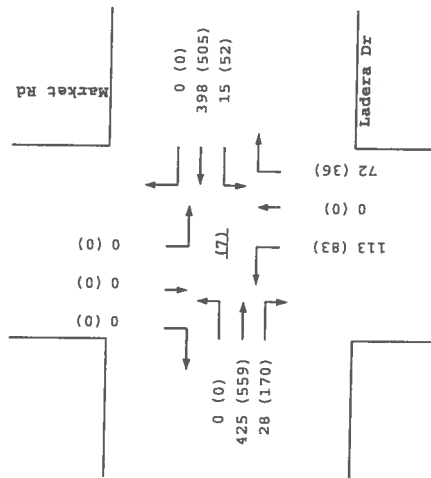
Growth Rate to Apply to Existing Counts to Match 2030 Forecasts

2007-2030 AM Growth Rates	-0.01%	9.44%	-3.64%	#DIV/0!
2007-2030 PM Growth Rates	2.55%	4.52%	-2.63%	#DIV/0!

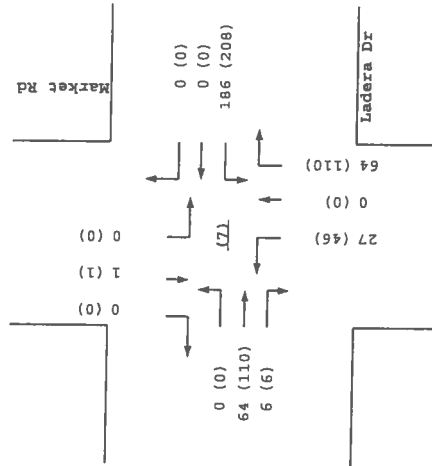
Growth Rate to Apply to 2005 Model Volumes to Match 2030 Forecasts

2005-2030 AM Growth Rates	1.09%	9.54%	#DIV/0!	#DIV/0!
2005-2030 PM Growth Rates	12.28%	11.97%	#DIV/0!	#DIV/0!

2010
NO BUILD

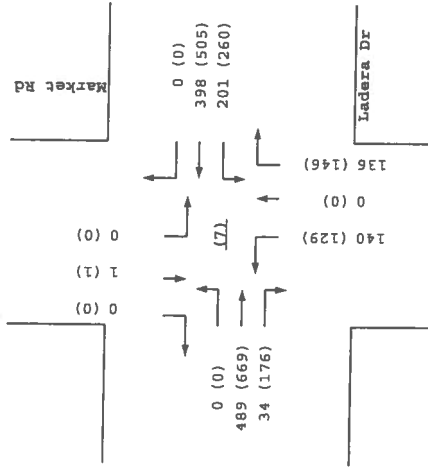


Trips



Ladera Dr / Market Rd

2010
BUILD



Heritage Neighborhood Center (Ladera Dr / Unser Blvd)
Projected Turning Movements Worksheet
Ladera Dr / Laurelwood Pkwy

INTERSECTION: E-W Street: Ladera Dr (8)
N-S Street: Laurelwood Pkwy
Year of Existing Counts 2007
Implementation Year 2010
Growth Rates

	0.00%			14.16%			0.00%			0.00%		
	Eastbound (Ladera Dr)			Westbound (Ladera Dr)			Northbound (Laurelwood Pkwy)			Southbound (Laurelwood Pkwy)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Existing Volumes	1	490	28	5	255	0	78	0	65	5	0	8
Background Traffic Growth	0	0	0	2	108	0	0	0	0	0	0	0
Subtotal (NO BUILD - A.M.)	1	490	28	7	363	0	78	0	65	5	0	8
Percent Commercial Trips Generated(Entering)	0.00%	0.00%	0.00%	0.00%	29.00%	0.00%	4.36%	0.00%	0.00%	0.00%	0.00%	0.15%
Percent Commercial Trips Generated(Exiting)	0.15%	29.00%	4.36%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%
Percent Office Trips Generated(Entering)	0.00%	0.00%	0.00%	0.00%	26.33%	0.00%	1.20%	0.00%	0.00%	0.00%	0.00%	0.04%
Percent Office Trips Generated(Exiting)	0.04%	26.33%	1.20%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%
Total Trips Generated	1	112	16	0	163	0	23	0	0	0	0	1
Total AM Peak Hour BUILD Volumes	2	602	44	7	526	0	101	0	65	5	0	9

	4.69%			2.65%			0.00%			0.00%		
	Eastbound (Ladera Dr)			Westbound (Ladera Dr)			Northbound (Laurelwood Pkwy)			Southbound (Laurelwood Pkwy)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Existing Volumes	9	422	86	76	546	9	37	0	34	1	0	3
Background Traffic Growth	1	59	12	6	43	1	0	0	0	0	0	0
Subtotal (NO BUILD - P.M.)	10	481	98	82	589	10	37	0	34	1	0	3
Percent Commercial Trips Generated(Entering)	0.00%	0.00%	0.00%	0.00%	29.00%	0.00%	4.36%	0.00%	0.00%	0.00%	0.00%	0.15%
Percent Commercial Trips Generated(Exiting)	0.15%	29.00%	4.36%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%
Percent Office Trips Generated(Entering)	0.00%	0.00%	0.00%	0.00%	26.33%	0.00%	1.20%	0.00%	0.00%	0.00%	0.00%	0.04%
Percent Office Trips Generated(Exiting)	0.04%	26.33%	1.20%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%
Total Trips Generated	1	193	26	0	180	0	26	0	0	0	0	1
Total PM Peak Hour BUILD Volumes	11	674	124	82	769	10	63	0	34	1	0	4

Number of Commercial Trips Generated
 Entering 499 Exiting 378 A.M. 100% Commercial Development
 602 580 P.M.
 Number of Office Trips Generated
 68 9 A.M. 100% Office Development
 20 96 P.M.

	Eastbound (Ladera Dr)			Westbound (Ladera Dr)			Northbound (Laurelwood Pkwy)			Southbound (Laurelwood Pkwy)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
2007 AM Peak Hr. Volumes	1	490	28	5	255	0	78	0	65	5	0	8
2007 PM Peak Hr. Volumes	9	422	86	76	546	9	37	0	34	1	0	3

MRCOG Forecast Volumes Worksheet

Based on 2007 Traffic Count

2007 AM Link Volume	519	260	143	13
2007 PM Link Volume	517	631	71	4

Based on MRCOG Model (2030 Data Set)

2005 AM Link Volume	355	355	154	0
2005 PM Link Volume	261	261	80	0
2030 AM Link Volume	471	1107	132	0
2030 PM Link Volume	1075	1016	71	0

Growth Rate to Apply to Existing Counts to Match 2030 Forecasts

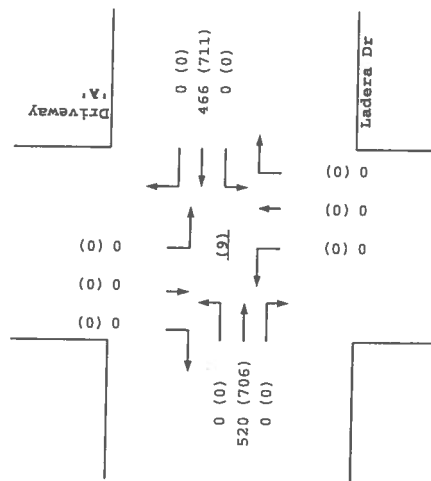
2007-2030 AM Growth Rates	-0.40%	14.16%	-0.33%	-4.35%
2007-2030 PM Growth Rates	4.69%	2.65%	0.00%	-4.35%

Growth Rate to Apply to 2005 Model Volumes to Match 2030 Forecasts

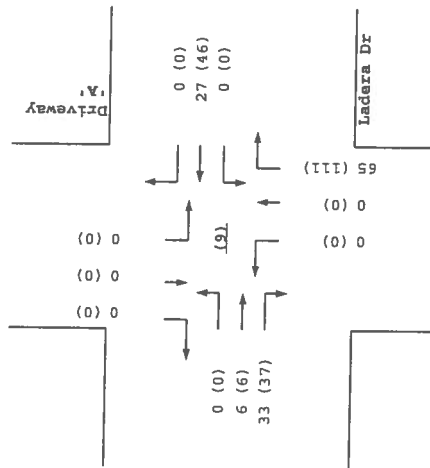
2005-2030 AM Growth Rates	1.31%	8.47%	-0.57%	#DIV/0!
2005-2030 PM Growth Rates	12.48%	11.57%	-0.45%	#DIV/0!

10/29/2007

2010
NO BUILD

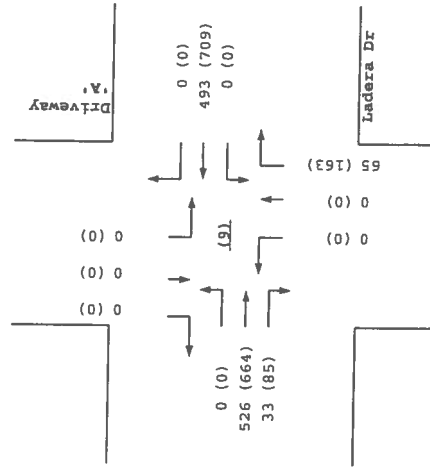


Trips



Ladera Dr / Driveway 'A'

2010
BUILD



Heritage Neighborhood Center (Ladera Dr / Unser Blvd)
Projected Turning Movements Worksheet
Driveway 'B' / Market Rd

INTERSECTION : E-W Street: **Driveway 'B'** (10)
N-S Street: **Market Rd**
Year of Existing Counts: 2007
Implementation Year: 2010
Growth Rates: 0.00%

	0.00%			0.00%			0.00%			0.00%		
	Eastbound (Driveway 'B')			Westbound (Driveway 'B')			Northbound (Market Rd)			Southbound (Market Rd)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Existing Volumes	0	0	0	0	0	0	0	185	0	0	42	0
Background Traffic Growth	0	0	0	0	0	0	0	0	0	0	0	0
Subtotal (NO BUILD - A.M.)	0	0	0	0	0	0	0	185	0	0	42	0
Percent Commercial Trips Generated(Entering)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.92%	0.00%	0.00%	0.00%	1.00%	33.66%
Percent Commercial Trips Generated(Exiting)	22.73%	0.00%	0.92%	0.00%	0.00%	0.00%	0.00%	1.00%	0.00%	0.00%	0.00%	0.00%
Percent Office Trips Generated(Entering)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.25%	0.00%	0.00%	0.00%	1.00%	27.61%
Percent Office Trips Generated(Exiting)	19.03%	0.00%	0.25%	0.00%	0.00%	0.00%	0.00%	1.00%	0.00%	0.00%	0.00%	0.00%
Total Trips Generated	88	0	3	0	0	0	5	4	0	0	6	187
Total AM Peak Hour BUILD Volumes	88	0	3	0	0	0	5	189	0	0	48	187

	Eastbound (Driveway 'B')			Westbound (Driveway 'B')			Northbound (Market Rd)			Southbound (Market Rd)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Existing Volumes	0	0	0	0	0	0	0	119	0	0	204	0
Background Traffic Growth	0	0	0	0	0	0	0	0	0	0	0	0
Subtotal (NO BUILD - P.M.)	0	0	0	0	0	0	0	119	0	0	204	0
Percent Commercial Trips Generated(Entering)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.92%	0.00%	0.00%	0.00%	1.00%	33.66%
Percent Commercial Trips Generated(Exiting)	22.73%	0.00%	0.92%	0.00%	0.00%	0.00%	0.00%	1.00%	0.00%	0.00%	0.00%	0.00%
Percent Office Trips Generated(Entering)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.25%	0.00%	0.00%	0.00%	1.00%	27.61%
Percent Office Trips Generated(Exiting)	19.03%	0.00%	0.25%	0.00%	0.00%	0.00%	0.00%	1.00%	0.00%	0.00%	0.00%	0.00%
Total Trips Generated	150	0	5	0	0	0	6	7	0	0	6	209
Total PM Peak Hour BUILD Volumes	150	0	5	0	0	0	6	126	0	0	210	257

Number of Commercial Trips Generated	Entering 499	Exiting 378	A.M.	100% Commercial Development
Number of Office Trips Generated	602	580	P.M.	
	68	9	A.M.	100% Office Development
	20	96	P.M.	

	Eastbound (Driveway 'B')			Westbound (Driveway 'B')			Northbound (Market Rd)			Southbound (Market Rd)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
2007 AM Peak Hr. Volumes	0	0	0	0	0	0	0	185	0	0	42	0
2007 PM Peak Hr. Volumes	0	0	0	0	0	0	0	119	0	0	204	0

Heritage Neighborhood Center (Ladera Dr / Unser Blvd)
Projected Turning Movements Worksheet
Driveway 'D' / Unser Blvd

INTERSECTION: E-W Street: Driveway 'D' (12)
N-S Street: Unser Blvd
Year of Existing Counts 2007
Implementation Year 2010

	0.00%			0.00%			4.58%			4.58%		
	Eastbound (Driveway 'D')			Westbound (Driveway 'D')			Northbound (Unser Blvd)			Southbound (Unser Blvd)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Existing Volumes	0	0	0	0	0	0	0	701	0	0	1,588	0
Background Traffic Growth	0	0	0	0	0	0	0	96	0	0	218	0
Subtotal (NO BUILD - A.M.)	0	0	0	0	0	0	0	797	0	0	1,806	0
Percent Commercial Trips Generated(Entering)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	36.91%	20.70%	0.00%	0.00%
Percent Commercial Trips Generated(Exiting)	0.00%	0.00%	0.00%	36.91%	0.00%	20.70%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%
Percent Office Trips Generated(Entering)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	46.95%	18.69%	0.00%	0.00%
Percent Office Trips Generated(Exiting)	0.00%	0.00%	0.00%	46.95%	0.00%	18.69%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%
Total Trips Generated	0	0	0	144	0	80	0	0	216	116	0	0
Total AM Peak Hour BUILD Volumes	0	0	0	144	0	80	0	797	216	116	1,806	0

	0.00%			0.00%			4.15%			4.15%		
	Eastbound (Driveway 'D')			Westbound (Driveway 'D')			Northbound (Unser Blvd)			Southbound (Unser Blvd)		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Existing Volumes	0	0	0	0	0	0	0	1,520	0	0	966	0
Background Traffic Growth	0	0	0	0	0	0	0	189	0	0	120	0
Subtotal (NO BUILD - P.M.)	0	0	0	0	0	0	0	1,709	0	0	1,086	0
Percent Commercial Trips Generated(Entering)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	36.91%	20.70%	0.00%	0.00%
Percent Commercial Trips Generated(Exiting)	0.00%	0.00%	0.00%	36.91%	0.00%	20.70%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%
Percent Office Trips Generated(Entering)	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	46.95%	18.69%	0.00%	0.00%
Percent Office Trips Generated(Exiting)	0.00%	0.00%	0.00%	46.95%	0.00%	18.69%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%
Total Trips Generated	0	0	0	259	0	138	0	0	231	129	0	0
Subtotal PM Pk Hr. BUILD Volumes	0	0	0	259	0	138	0	1,709	231	129	1,086	0
Pass-by Trip Adjustments	0	0	0	81	0	157	0	-96	96	75	-75	0
Total PM Peak Hour BUILD Volumes	0	0	0	340	0	295	0	1,613	327	204	1,011	0

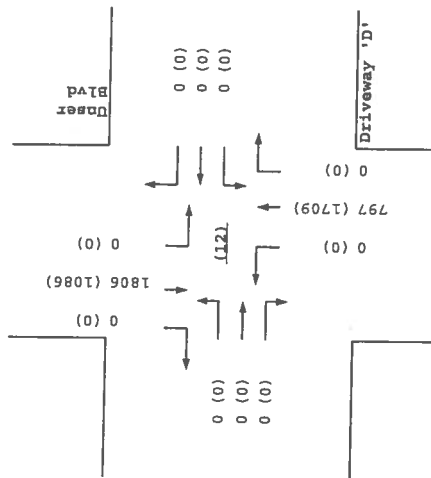
	Entering	Exiting		
Number of Commercial Trips Generated	499	378	A.M.	100% Commercial Development
	602	580	P.M.	
Number of Office Trips Generated	68	9	A.M.	100% Office Development
	20	96	P.M.	

	Eastbound (Driveway 'D')			Westbound (Driveway 'D')			Northbound (Unser Blvd)			Southbound (Unser Blvd)		
2007 AM Peak Hr. Volumes	0	0	0	0	0	0	0	701	0	0	1,588	0
2007 PM Peak Hr. Volumes	0	0	0	0	0	0	0	1,520	0	0	966	0

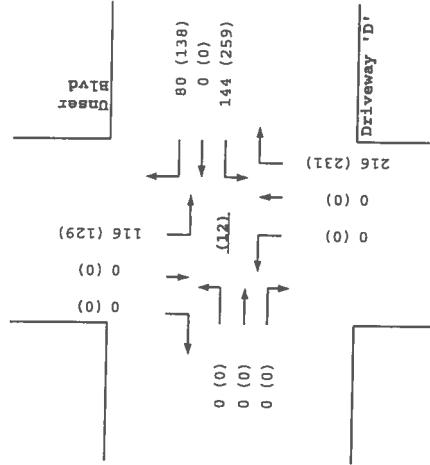
Pass-by Trip Calculations:

PM Pass-by Trips	Eastbound (Driveway 'D')			Westbound (Driveway 'D')			Northbound (Unser Blvd)			Southbound (Unser Blvd)		
Percent Entering	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	-36.00%	36.00%	28.00%	-28.00%	0.00%
Volume Entering	0	0	0	0	0	0	0	-96	96	75	-75	0
Percent Exiting	0.00%	0.00%	0.00%	28.00%	0.00%	54.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%
Volume Exiting	0	0	0	81	0	157	0	0	0	0	0	0
Net PM Passby Trips	0	0	0	81	0	157	0	-96	96	75	-75	0
Entering	0	0	0	81	0	157	0	-96	96	75	-75	0
Exiting	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips	0	0	0	81	0	157	0	-96	96	75	-75	0
	267	0	0	290	0	290	0	290	0	290	0	0

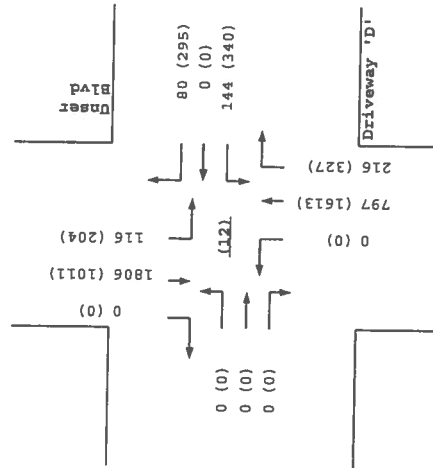
2010
NO BUILD



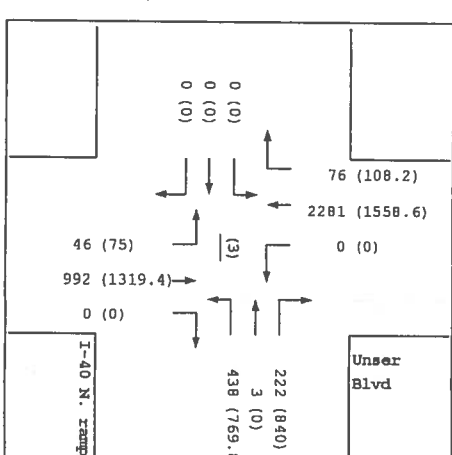
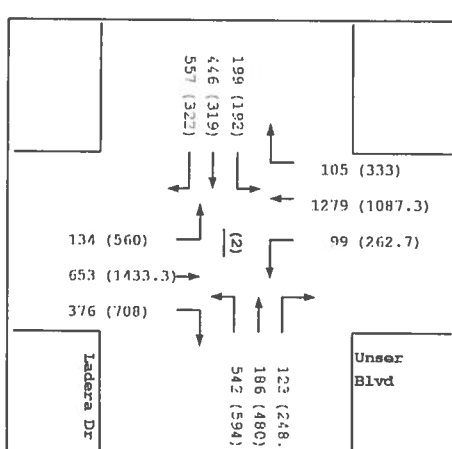
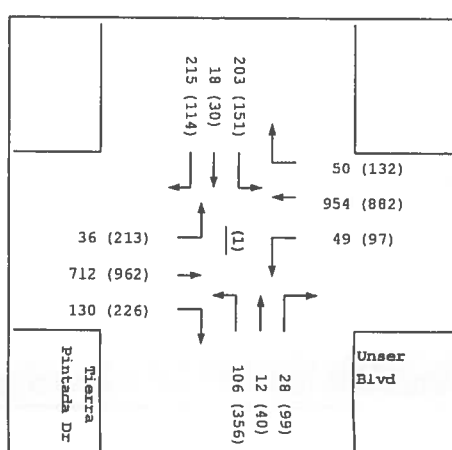
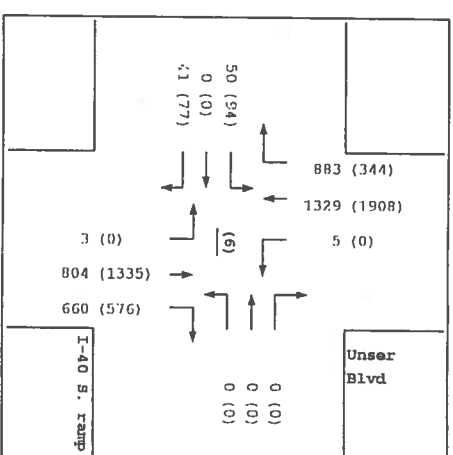
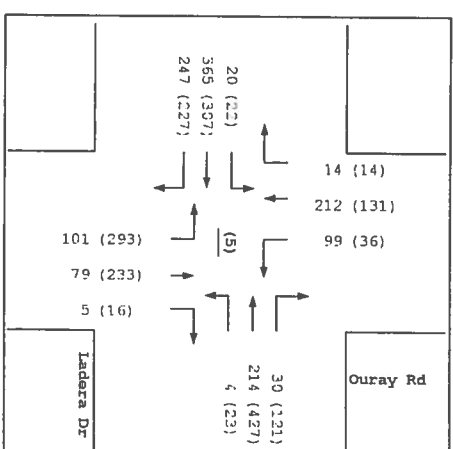
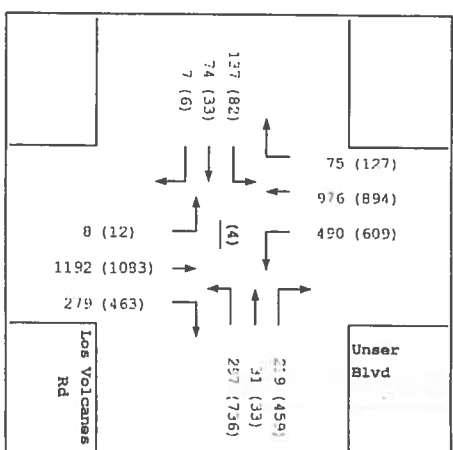
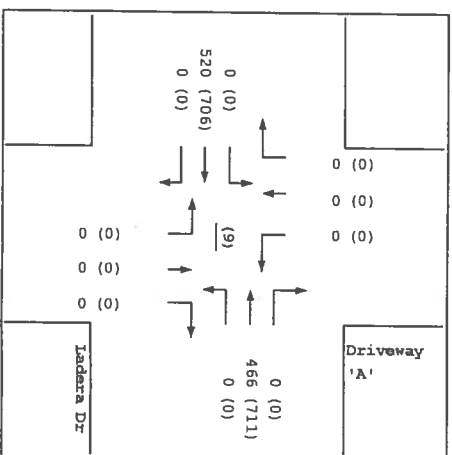
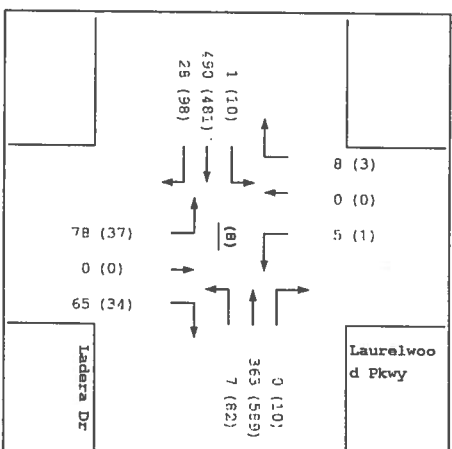
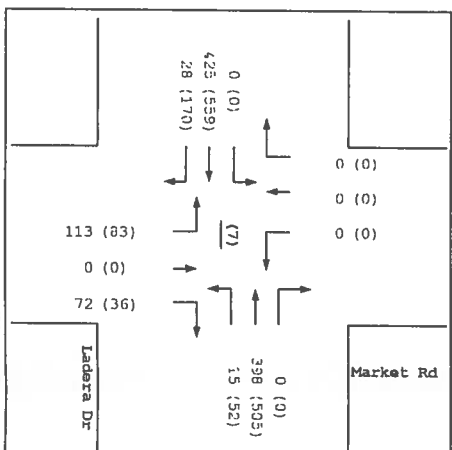
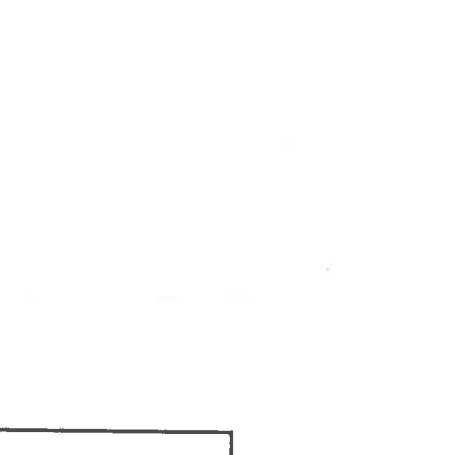
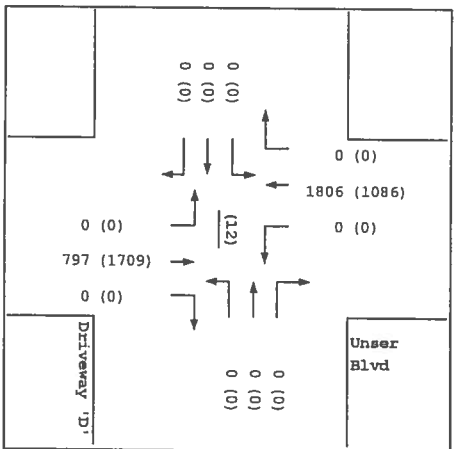
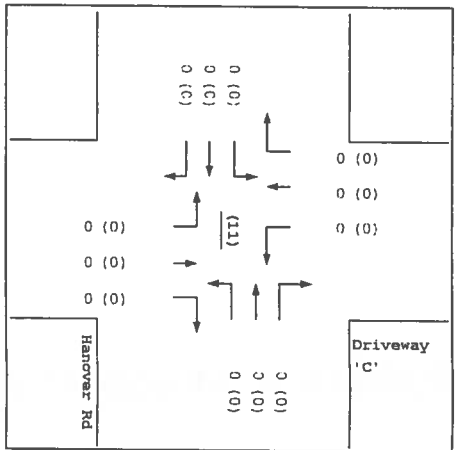
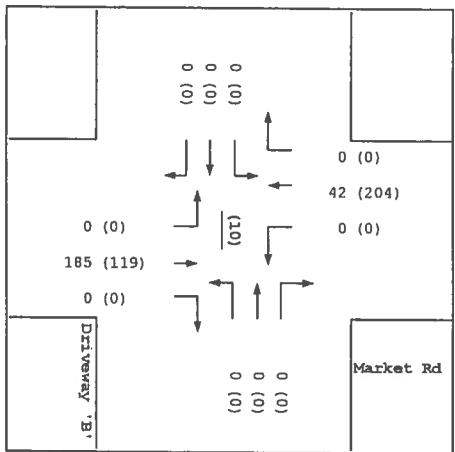
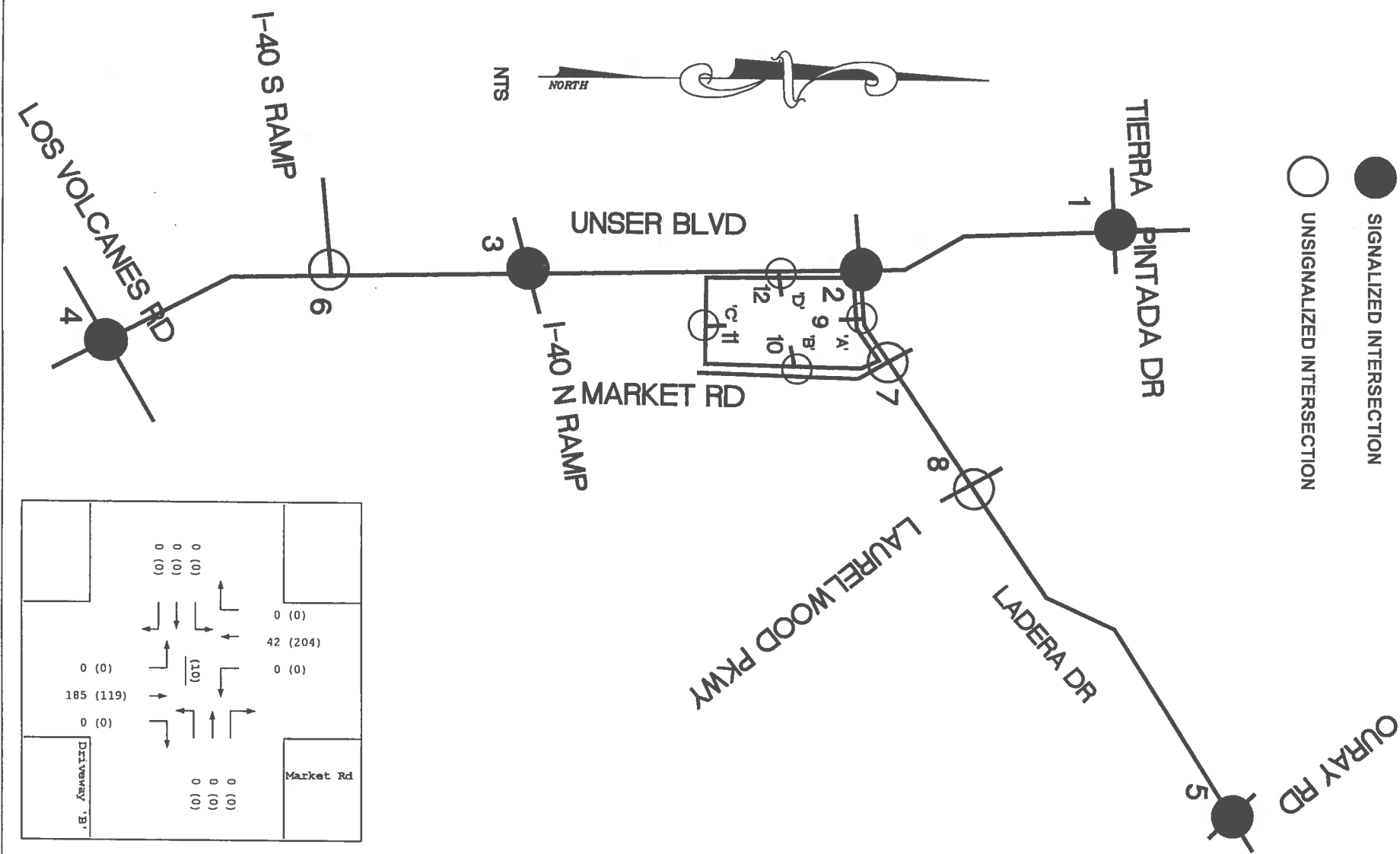
Trips



2010
BUILD

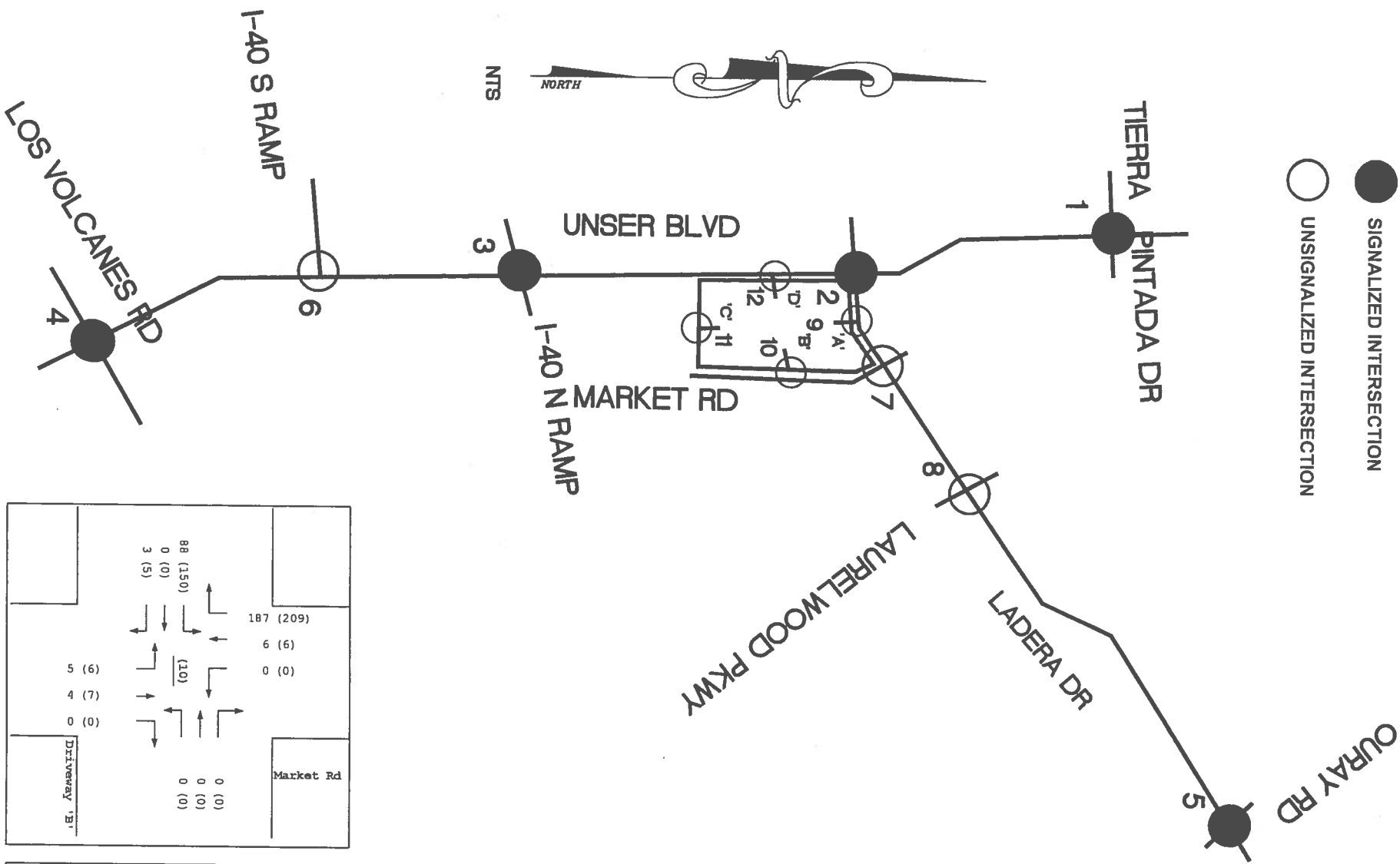


Driveway 'D' / Unser Blvd



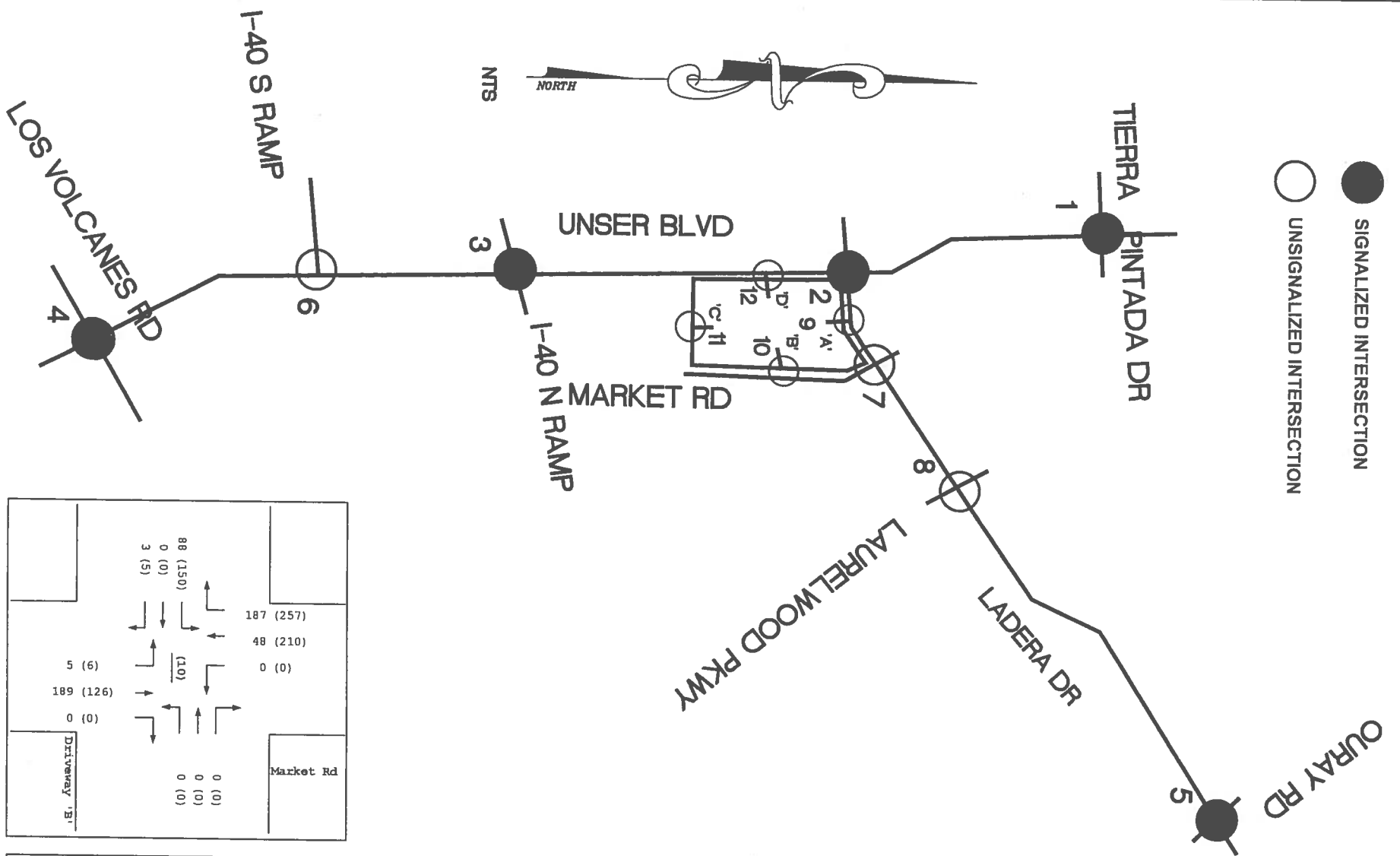
Heritage Neighborhood Center (Ladera Dr / Unser Blvd) 2010 NO BUILD Volumes - AM(PM)

Terry O. Braum, P.E.
P.O. Box 92051
Albuquerque, NM 87199-2051
(505)883-8807 (Voice)



Heritage Neighborhood Center (Ladera Dr / Unser Blvd) Trips Generated Volumes - AM(PM)

Terry O. Brown, P.E.
 P.O. Box 92051
 Albuquerque, NM 87199-2051
 (505)883-8807 (Voice)



<p>Market Rd</p> <p>187 (257) 48 (210) 0 (0)</p> <p>0 (0) 0 (0) 0 (0)</p> <p>88 (150) 0 (0) 3 (5)</p> <p>5 (6) 189 (126) 0 (0)</p> <p>Driveway 'B'</p>	<p>Unser Blvd</p> <p>50 (132) 1016 (945) 49 (97)</p> <p>28 (99) 12 (40) 112 (362)</p> <p>203 (151) 18 (30) 249 (154)</p> <p>61 (253) 751 (1037) 134 (232)</p> <p>Tierra Pintada Dr</p>	<p>Unser Blvd</p> <p>105 (333) 1355 (1169.3) 125 (289.7)</p> <p>140 (278.7) 196 (496) 542 (594)</p> <p>199 (192) 460 (334) 597 (369)</p> <p>163 (607) 704 (1524.3) 376 (708)</p> <p>Ladera Dr</p>	<p>Unser Blvd</p> <p>83 (120.2) 2417 (1805.6) 0 (0)</p> <p>291 (905) 3 (0) 438 (769.8)</p> <p>0 (0) 0 (0) 0 (0)</p> <p>46 (75) 1138 (1485.4) 0 (0)</p> <p>I-40 N. ramp</p>	<p>Unser Blvd</p> <p>883 (344) 1425 (2069) 45 (86)</p> <p>0 (0) 0 (0) 0 (0)</p> <p>60 (105) 0 (0) 41 (77)</p> <p>3 (0) 939 (1489) 660 (576)</p> <p>I-40 S. ramp</p>	<p>Laurelwood Pkwy</p> <p>0 (10) 526 (769) 7 (82)</p> <p>2 (11) 602 (674) 44 (124)</p> <p>9 (4) 0 (0) 5 (1)</p> <p>101 (63) 0 (0) 65 (34)</p> <p>Ladera Dr</p>	<p>Ouray Rd</p> <p>30 (121) 291 (506) 4 (23)</p> <p>24 (27) 414 (400) 287 (292)</p> <p>19 (20) 212 (131) 99 (36)</p> <p>156 (358) 79 (233) 5 (16)</p> <p>Ladera Dr</p>	<p>Unser Blvd</p> <p>255 (500) 31 (33) 297 (736)</p> <p>142 (87) 74 (33) 7 (6)</p> <p>8 (12) 1287 (1190) 279 (463)</p> <p>Los Volcanes Rd</p>	<p>Market Rd</p> <p>0 (0) 399 (505) 201 (260)</p> <p>0 (0) 489 (669) 34 (176)</p> <p>140 (129) 0 (0) 136 (146)</p> <p>Ladera Dr</p>	<p>Driveway 'C'</p> <p>11 (11) 0 (0) 0 (0)</p> <p>0 (0) 0 (0) 0 (0)</p> <p>0 (0) 0 (0) 0 (0)</p> <p>7 (12)</p> <p>Hanover Rd</p>	<p>Unser Blvd</p> <p>80 (295) 0 (0) 144 (340)</p> <p>0 (0) 0 (0) 0 (0)</p> <p>0 (0) 0 (0) 0 (0)</p> <p>1806 (1011) 116 (204)</p> <p>797 (1613) 216 (327)</p> <p>Driveway 'D'</p>
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Heritage Neighborhood Center
(Ladera Dr / Unser Blvd)
2010 BUILD Volumes - AM(PM)

Terry O. Brown, P.E.
P.O. Box 92051
Albuquerque, NM 87199-2051
(505)883-8807 (Voice)

Analysis of Intersection #1

Tierra Pintada Dr / Unser Blvd

Timings

1. Tierra Pintada Dr & Unser Blvd

Terry O. Brown, P.E.
10/30/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	203	18	215	106	12	28	36	715	130	48	954	50
Volume (vph)	Perm	4	4	4	8	8	2	2	2	6	6	6
Turn Type	Perm	4	4	4	8	8	2	2	2	6	6	6
Permitted Phases	4	4	4	4	8	8	2	2	2	6	6	6
Detector Phases	4	4	4	4	8	8	2	2	2	6	6	6
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	21.0	21.0	21.0	21.0	21.0	21.0	21.0	21.0	21.0	21.0	21.0	21.0
Total Split (s)	51.0	51.0	51.0	51.0	51.0	51.0	51.0	51.0	51.0	51.0	51.0	51.0
Total Split (%)	46.4%	46.4%	46.4%	46.4%	46.4%	46.4%	46.4%	46.4%	46.4%	46.4%	46.4%	46.4%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lead/Lag												
Recall Mode	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min
Act Effect Green (s)	25.7	25.7	25.7	25.7	25.7	25.7	25.7	25.7	25.7	25.7	25.7	25.7
Actuated g/C Ratio	0.23	0.23	0.23	0.23	0.23	0.23	0.23	0.23	0.23	0.23	0.23	0.23
v/c Ratio	0.78	0.05	0.62	0.44	0.04	0.09	0.15	0.30	0.12	0.14	0.41	0.05
Control Delay	55.5	29.3	31.9	38.8	28.8	9.3	6.5	4.7	0.8	7.9	7.9	2.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	55.5	29.3	31.9	38.8	28.8	9.3	6.5	4.7	0.8	7.9	7.9	2.2
LOS	E	C	C	D	C	A	A	A	A	A	A	A
Approach Delay	42.8											
Approach LOS	D											

Intersection Summary

Cycle Length	110
Offset	90 (82%), Referenced to phase 2 NBT and 6 SBT, Start of Green
Natural Cycle	45
Control Type	Actuated-Coordinated
Maximum v/c Ratio	0.78
Intersection Signal Delay	15.1
Intersection Capacity Utilization	58.5%
Analysis Period (min)	15

Splits and Phases 1. Tierra Pintada Dr & Unser Blvd

a2	53 s
a4	51 s
a6	51 s

HCM Signalized Intersection Capacity Analysis

1. Tierra Pintada Dr & Unser Blvd

Terry O. Brown, P.E.
10/30/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Total Lost time (s)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1752	1845	1568	1752	1845	1568	1752	1845	1568	1752	1845	1568
Flt Permitted	0.75	1.00	1.00	0.74	1.00	1.00	0.74	1.00	1.00	0.74	1.00	1.00
Satd. Flow (perm)	1378	1845	1568	1371	1845	1568	1371	1845	1568	1371	1845	1568
Volume (vph)	203	18	215	106	12	28	36	715	130	48	954	50
Peak-hour factor, PHF	0.81	0.81	0.81	0.75	0.75	0.75	0.97	0.97	0.97	0.94	0.94	0.94
Adj. Flow (vph)	251	22	265	141	16	37	737	737	737	52	1015	53
RTOR Reduction (vph)	0	0	59	0	0	28	0	0	39	0	0	15
Lane Group Flow (vph)	251	22	206	141	16	9	37	737	95	52	1015	38
Turn Type	Perm	4	4	8	8	8	2	2	2	6	6	6
Protected Phases	4	4	4	8	8	8	2	2	2	6	6	6
Permitted Phases	4	4	4	8	8	8	2	2	2	6	6	6
Actuated Green, G (s)	23.7	23.7	23.7	23.7	23.7	23.7	23.7	23.7	23.7	23.7	23.7	23.7
Effective Green, g (s)	25.7	25.7	25.7	25.7	25.7	25.7	25.7	25.7	25.7	25.7	25.7	25.7
Actuated g/C Ratio	0.23	0.23	0.23	0.23	0.23	0.23	0.23	0.23	0.23	0.23	0.23	0.23
Clearance Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	322	431	366	320	431	366	322	431	366	322	431	366
v/s Ratio Prot	0.01	0.01	0.13	0.10	0.01	0.01	0.08	0.06	0.08	0.06	0.08	0.06
v/s Ratio Perm	0.18	0.05	0.56	0.44	0.04	0.02	0.11	0.30	0.09	0.11	0.41	0.03
v/c Ratio	0.78	0.05	0.62	0.44	0.04	0.02	0.11	0.30	0.09	0.11	0.41	0.03
Uniform Delay, d1	39.5	32.7	37.2	36.0	32.6	32.5	5.0	5.8	4.9	5.0	6.4	4.7
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.70	0.67	0.46	1.00	1.00	1.00
Incremental Delay, d2	11.3	0.0	2.0	1.0	0.0	0.0	0.3	0.1	0.5	0.5	0.1	0.1
Delay (s)	50.8	32.7	39.2	37.0	32.6	32.5	4.1	4.2	2.4	5.5	6.9	4.7
Level of Service	D	C	D	D	C	C	A	A	A	A	A	A
Approach Delay (s)	44.3											
Approach LOS	D											

Intersection Summary

HCM Average Control Delay	15.2	HCM Level of Service	B
HCM Volume to Capacity ratio	0.50	Sum of lost time (s)	6.0
Actuated Cycle Length (s)	110.0	ICU Level of Service	B
Intersection Capacity Utilization	58.5%		
Analysis Period (min)	15		
c Critical Lane Group			

Timings

1: Tierra Pintada Dr & Unser Blvd

Terry O. Brown, P.E.
10/30/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Total Lost time (s)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lane Util. Factor	0.95	1.00	0.85	1.00	1.00	0.85	1.00	0.85	1.00	0.85	1.00	0.85
Flt Protected	1752	1845	1568	1752	1845	1568	1752	1845	1568	1752	1845	1568
Satd. Flow (prot)	0.75	1.00	1.00	0.74	1.00	1.00	0.74	1.00	1.00	0.74	1.00	1.00
Flt Permitted	1378	1845	1568	1371	1845	1568	1371	1845	1568	1371	1845	1568
Satd. Flow (perm)	203	18	249	112	12	28	61	751	134	49	1016	50
Volume (vph)	0.81	0.81	0.81	0.75	0.75	0.75	0.97	0.97	0.97	0.94	0.94	0.94
Peak-hour factor, PHF	251	22	307	149	16	37	63	774	138	52	1081	53
Adj. Flow (vph)	0	0	56	0	0	28	0	0	0	0	0	15
RTOR Reduction (vph)	251	22	251	149	16	9	63	774	98	52	1081	38
Lane Group Flow (vph)	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm
Turn Type	4	4	4	8	8	8	2	2	2	6	6	6
Protected Phases	4	4	4	8	8	8	2	2	2	6	6	6
Permitted Phases	24.1	24.1	24.1	24.1	24.1	24.1	24.1	24.1	24.1	24.1	24.1	24.1
Actuated Green, G (s)	26.1	26.1	26.1	26.1	26.1	26.1	26.1	26.1	26.1	26.1	26.1	26.1
Effective Green, g (s)	0.24	0.24	0.24	0.24	0.24	0.24	0.24	0.24	0.24	0.24	0.24	0.24
Actuated g/C Ratio	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Clearance Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Vehicle Extension (s)	327	438	372	325	438	372	293	2482	1110	430	2482	1110
Lane Grp Cap (vph)	c0.18	0.01	0.16	0.11	0.11	0.01	0.15	0.06	0.09	0.09	0.02	0.02
v/s Ratio Prot	0.77	0.05	0.67	0.46	0.46	0.04	0.22	0.31	0.09	0.12	0.44	0.03
v/s Ratio Perm	39.1	32.4	38.1	35.9	32.3	32.2	5.5	6.0	5.1	6.8	4.8	4.8
Uniform Delay, d1	1.00	1.00	1.00	1.00	1.00	1.00	0.69	0.70	0.50	1.00	1.00	1.00
Progression Factor	10.3	0.0	4.8	1.0	0.0	0.0	1.4	0.3	0.1	0.6	0.6	0.1
Incremental Delay, d2	49.4	32.4	42.8	36.9	32.3	32.2	5.2	4.5	2.6	5.7	7.3	4.9
Delay (s)	D	C	D	D	C	C	A	A	A	A	A	A
Level of Service	D	C	D	D	C	C	A	A	A	A	A	A
Approach Delay (s)	45.3	D	35.7	D	D	D	4.3	A	A	A	7.1	A
Approach LOS	D	D	D	D	D	D	A	A	A	A	A	A

Intersection Summary

HCM Average Control Delay	15.7	HCM Level of Service	B
HCM Volume to Capacity ratio	0.52		
Actual Cycle Length (s)	110.0	Sum of lost time (s)	6.0
Intersection Capacity Utilization	60.2%	ICU Level of Service	B
Analysis Period (min)	15		
c Critical Lane Group			

Splits and Phases: 1: Tierra Pintada Dr & Unser Blvd



2010 AM Peak BUILD Conditions

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2010 AM Peak BUILD Conditions

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Case F - full access at Intersection 12

Timings

1: Tierra Pintada Dr & Unser Blvd

Terry O. Brown, P.E.

10/30/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Total Lost time (s)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lane Util. Factor	0.95	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.85	1.00
Flt Protected	1752	1845	1568	1752	1845	1568	1752	1845	1568	1752	1845	1568
Satd. Flow (prot)	0.72	1.00	1.00	0.74	1.00	1.00	0.72	1.00	1.00	0.74	1.00	1.00
Flt Permitted	1333	1845	1568	1333	1845	1568	1333	1845	1568	1333	1845	1568
Satd. Flow (perm)	151	30	114	356	40	99	213	962	226	97	882	132
Volume (vph)	0.90	0.90	0.90	0.75	0.75	0.75	0.94	0.94	0.94	0.88	0.88	0.88
Peak-hour factor, PHF	168	33	127	475	53	132	227	1023	240	110	1002	150
Adj. Flow (vph)	0	0	74	0	0	71	0	0	98	0	0	61
RTOR Reduction (vph)	168	33	53	475	53	61	227	1023	142	110	1002	89
Lane Group Flow (vph)	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm
Turn Type	4	4	4	8	8	8	2	2	2	6	6	6
Protected Phases	4	4	4	8	8	8	2	2	2	6	6	6
Permitted Phases	4	4	4	8	8	8	2	2	2	6	6	6
Actuated Green, G (s)	41.0	41.0	41.0	41.0	41.0	41.0	69.0	69.0	69.0	69.0	69.0	69.0
Effective Green, g (s)	43.0	43.0	43.0	43.0	43.0	43.0	71.0	71.0	71.0	71.0	71.0	71.0
Actuated g/C Ratio	0.36	0.36	0.36	0.36	0.36	0.36	0.59	0.59	0.59	0.59	0.59	0.59
Clearance Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	478	661	562	486	561	562	236	2074	928	228	2074	928
v/s Ratio Prot	0.13	0.02	0.03	0.03	0.03	0.03	0.29	0.29	0.29	0.28	0.28	0.28
v/s Ratio Perm	0.35	0.05	0.09	0.98	0.08	0.11	0.86	0.49	0.15	0.48	0.48	0.10
Uniform Delay, d1	28.3	25.2	25.6	38.0	25.4	25.7	23.2	14.1	11.0	14.0	14.0	10.6
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.51	0.54	0.18	1.00	1.00	1.00
Incremental Delay, d2	0.4	0.0	0.1	34.6	0.1	0.1	29.7	0.4	0.1	7.1	0.8	0.2
Delay (s)	28.7	25.2	25.6	72.6	25.5	25.8	41.5	8.0	2.1	21.1	14.8	10.8
Level of Service	C	C	C	E	C	C	D	A	A	C	B	B
Approach Delay (s)	27.2	C	C	59.5	E	E	12.2	B	B	14.9	B	B
Approach LOS	C	C	C	E	E	E	B	B	B	C	B	B

Intersection Summary	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Cycle Length, 120	43.0	43.0	43.0	43.0	43.0	43.0	71.0	71.0	71.0	71.0	71.0	71.0
Actuated Cycle Length, 120	0.36	0.36	0.36	0.36	0.36	0.36	0.59	0.59	0.59	0.59	0.59	0.59
Offset: 28 (24%), Referenced to phase 2 NBTL and 6 SBTL, Start of Green	0.35	0.05	0.20	0.98	0.08	0.21	0.97	0.49	0.23	0.48	0.48	0.15
Natural Cycle: 65	30.9	25.6	6.6	73.9	26.0	7.7	47.4	8.1	0.5	22.9	15.0	2.0
Control Type: Actuated-Coordinated	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Maximum v/c Ratio: 0.98	30.9	25.6	6.6	73.9	26.0	7.7	47.4	8.1	0.5	22.9	15.0	2.0
Intersection Signal Delay: 21.8	C	C	A	E	C	A	D	A	A	C	B	A
Intersection Capacity Utilization 72.6%	21.0	C	C	56.8	E	E	12.9	B	B	14.1	B	B
Analysis Period (min) 15	C	C	C	E	E	E	B	B	B	C	B	B

Spits and Phases	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
EBL	151	30	114	356	40	99	213	962	226	97	882	132
EBT	4	4	4	8	8	8	2	2	2	6	6	6
EBR	4	4	4	8	8	8	2	2	2	6	6	6
WBL	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
WBT	21.0	21.0	21.0	21.0	21.0	21.0	21.0	21.0	21.0	21.0	21.0	21.0
WBR	46.0	46.0	46.0	46.0	46.0	46.0	46.0	46.0	46.0	46.0	46.0	46.0
NBL	38.3%	38.3%	38.3%	38.3%	38.3%	38.3%	61.7%	61.7%	61.7%	61.7%	61.7%	61.7%
NBT	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
NBR	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0

Intersection Summary	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Cycle Length, 120	43.0	43.0	43.0	43.0	43.0	43.0	71.0	71.0	71.0	71.0	71.0	71.0
Actuated Cycle Length, 120	0.36	0.36	0.36	0.36	0.36	0.36	0.59	0.59	0.59	0.59	0.59	0.59
Offset: 28 (24%), Referenced to phase 2 NBTL and 6 SBTL, Start of Green	0.35	0.05	0.20	0.98	0.08	0.21	0.97	0.49	0.23	0.48	0.48	0.15
Natural Cycle: 65	30.9	25.6	6.6	73.9	26.0	7.7	47.4	8.1	0.5	22.9	15.0	2.0
Control Type: Actuated-Coordinated	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Maximum v/c Ratio: 0.98	30.9	25.6	6.6	73.9	26.0	7.7	47.4	8.1	0.5	22.9	15.0	2.0
Intersection Signal Delay: 21.8	C	C	A	E	C	A	D	A	A	C	B	A
Intersection Capacity Utilization 72.6%	21.0	C	C	56.8	E	E	12.9	B	B	14.1	B	B
Analysis Period (min) 15	C	C	C	E	E	E	B	B	B	C	B	B

2010 PM Peak NOBUILD Conditions
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2010 PM Peak NOBUILD Conditions
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Timings

1: Tierra Pintada Dr & Unser Blvd

Terry O. Brown, P.E.
10/30/2007

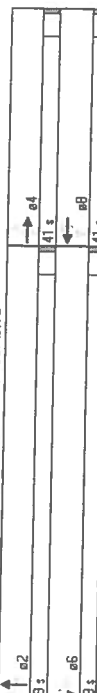
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	151	30	154	362	40	99	253	1037	232	97	945	132
Volume (vph)	Perm	4	Perm	Perm	8	Perm	Perm	Perm	Perm	Perm	Perm	Perm
Turn Type	4	4	4	4	8	8	2	2	2	2	6	6
Protected Phases	4	4	4	4	8	8	2	2	2	2	6	6
Detector Phases	4	4	4	4	8	8	2	2	2	2	6	6
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	21.0	21.0	21.0	21.0	21.0	21.0	21.0	21.0	21.0	21.0	21.0	21.0
Total Split (s)	41.0	41.0	41.0	41.0	41.0	41.0	79.0	79.0	79.0	79.0	79.0	79.0
Total Split (%)	34.2%	34.2%	34.2%	34.2%	34.2%	34.2%	65.8%	65.8%	65.8%	65.8%	65.8%	65.8%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min
Act Eff Green (s)	38.0	38.0	38.0	38.0	38.0	38.0	76.0	76.0	76.0	76.0	76.0	76.0
Actuated g/C Ratio	0.32	0.32	0.32	0.32	0.32	0.32	0.63	0.63	0.63	0.63	0.63	0.63
v/c Ratio	0.40	0.06	0.30	1.12	0.09	0.23	1.12	0.50	0.23	0.48	0.48	0.14
Control Delay	35.6	29.0	12.4	120.0	29.5	9.2	87.1	6.3	0.2	20.0	12.5	1.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	35.6	29.0	12.4	120.0	29.5	9.2	87.1	6.3	0.2	20.0	12.5	1.7
LOS	D	C	B	F	C	A	F	A	A	C	B	A
Approach Delay												
Approach LOS												

Intersection Summary

Cycle Length: 120
 Actuated Cycle Length: 120
 Offset: 22 (18%), Referenced to phase 2 NBT and 6 SBT, Start of Green
 Natural Cycle: 70
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 1.12
 Intersection Signal Delay: 29.1
 Intersection Capacity Utilization 76.9%
 Analysis Period (min) 15

Intersection LOS: C
 ICU Level of Service D

Splits and Phases: 1: Tierra Pintada Dr & Unser Blvd



HCM Signalized Intersection Capacity Analysis

1: Tierra Pintada Dr & Unser Blvd

Terry O. Brown, P.E.
10/30/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Total Lost Time (s)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lane Util. Factor	0.85	1.00	0.85	1.00	0.85	1.00	0.85	1.00	0.85	1.00	0.85	1.00
Flt Protected	1752	1845	1568	1752	1845	1568	1752	1845	1568	1752	1845	1568
Satd. Flow (prot)	0.72	1.00	0.72	1.00	0.72	1.00	0.72	1.00	0.72	1.00	0.72	1.00
Flt Permitted	1333	1845	1568	1333	1845	1568	1333	1845	1568	1333	1845	1568
Satd. Flow (perm)	151	30	154	362	40	99	253	1037	232	97	945	132
Volume (vph)	Perm	4	Perm	Perm	8	Perm	Perm	Perm	Perm	Perm	Perm	Perm
Peak-hour factor, PHF	0.90	0.90	0.90	0.75	0.75	0.75	0.94	0.94	0.94	0.88	0.88	0.88
Adj. Flow (vph)	168	33	171	483	53	132	269	1103	247	110	1074	150
RTOR Reduction (vph)	0	0	79	0	0	74	0	0	91	0	0	55
Lane Group Flow (vph)	168	33	92	483	53	58	269	1103	156	110	1074	95
Turn Type	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm
Protected Phases	4	4	4	8	8	8	2	2	2	2	6	6
Permitted Phases	4	4	4	8	8	8	2	2	2	2	6	6
Actuated Green, G (s)	36.0	36.0	36.0	36.0	36.0	36.0	74.0	74.0	74.0	74.0	74.0	74.0
Effective Green, g (s)	38.0	38.0	38.0	38.0	38.0	38.0	76.0	76.0	76.0	76.0	76.0	76.0
Actuated g/C Ratio	0.32	0.32	0.32	0.32	0.32	0.32	0.63	0.63	0.63	0.63	0.63	0.63
Clearance Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	422	584	487	430	584	497	241	2220	993	231	2220	993
v/s Ratio Prot	0.02	0.06	0.06	0.36	0.03	0.03	0.31	0.31	0.10	0.30	0.31	0.06
v/s Ratio Perm	0.13	0.06	0.18	1.12	0.09	0.12	1.12	0.50	0.16	0.48	0.48	0.10
Uniform Delay, d1	32.1	28.5	29.8	41.0	28.8	29.1	22.0	11.8	9.0	11.6	11.6	8.6
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.51	0.51	0.07	1.00	1.00	1.00
Incremental Delay, d2	0.6	0.0	0.2	81.4	0.1	0.1	69.7	0.3	0.1	6.9	0.8	0.2
Delay (s)	32.7	28.6	29.9	122.4	28.9	29.2	80.9	6.2	0.7	18.4	12.4	8.8
Level of Service	C	C	C	F	C	C	F	A	A	B	B	A
Approach Delay (s)	31.1			86.6			17.8					
Approach LOS	C			F			B					

Intersection Summary

HCM Average Control Delay 30.4 HCM Level of Service C
 HCM Volume to Capacity ratio 1.12
 Actuated Cycle Length (s) 120.0 Sum of lost time (s) 6.0
 Intersection Capacity Utilization 76.9% ICU Level of Service D
 Analysis Period (min) 15
 Critical Lane Group

2010 PM Peak BUILD Conditions

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2010 PM Peak BUILD Conditions

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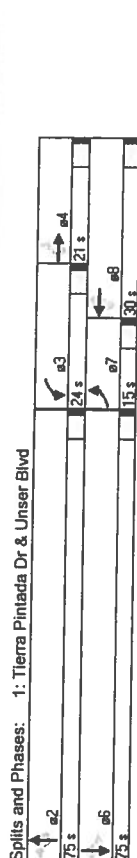
Case F - full access at Intersection 12

Case F - full access at Intersection 12

Timings Terry O. Brown, P.E. 10/30/2007
1: Tierra Pintada Dr & Unser Blvd

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBR
Lane Configurations	151	30	114	358	40	89	213	962	226	97	882
Volume (vph)	pm+pt		Perm	pm+pt		Perm	Perm		Perm	Perm	Perm
Turn Type	7	4	4	3	8	2	2	2	2	6	6
Permitted Phases	4	4	4	8	8	8	8	8	8	8	8
Detector Phases	7	4	4	3	8	2	2	2	2	6	6
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Spk (s)	10.0	21.0	21.0	10.0	21.0	21.0	21.0	21.0	21.0	21.0	21.0
Total Split (s)	15.0	21.0	21.0	24.0	30.0	30.0	75.0	75.0	75.0	75.0	75.0
Total Split (%)	12.5%	17.5%	17.5%	20.0%	25.0%	25.0%	62.5%	62.5%	62.5%	62.5%	62.5%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lead
Lead-Lag Optimizes?											
Recall Mode	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min
Act Eff Green (s)	21.9	10.0	10.0	34.0	19.1	19.1	80.0	80.0	80.0	80.0	80.0
Actuated g/C Ratio	0.18	0.08	0.08	0.28	0.16	0.16	0.67	0.67	0.67	0.67	0.67
v/c Ratio	0.59	0.21	0.52	1.10	0.18	0.37	0.84	0.44	0.21	0.42	0.43
Control Delay	43.8	53.5	18.2	111.9	44.4	10.1	23.9	5.5	0.4	16.1	10.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	43.8	53.5	18.2	111.9	44.4	10.1	23.9	5.5	0.4	16.1	10.3
LOS	D	D	B	F	D	B	C	A	A	B	B
Approach Delay	34.9				86.1						9.8
Approach LOS	C				F						A

Intersection Summary
Cycle Length: 120
Actuated Cycle Length: 120
Offset: 29 (24%), Referenced to phase 2:NBT and 6:SBTL, Start of Green
Natural Cycle: 90
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 1.10
Intersection Signal Delay: 24.5
Intersection Capacity Utilization 72.6%
Analysis Period (min) 15
Intersection LOS: C
ICU Level of Service C



2010 PM Peak NOBUILD Conditions Case F - full access at Intersection 12
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HCM Signalized Intersection Capacity Analysis Terry O. Brown, P.E. 10/30/2007
1: Tierra Pintada Dr & Unser Blvd

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBR
Lane Configurations	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Total Lost time (s)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Flt Protected	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Satd. Flow (prot)	1752	1845	1568	1752	1845	1568	1752	1845	1568	1752	1845
Flt Permitted	0.72	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Satd. Flow (perm)	1333	1845	1568	1333	1845	1568	1333	1845	1568	1333	1845
Volume (vph)	151	30	114	358	40	89	213	962	226	97	882
Peak-hour factor, PHF	0.90	0.90	0.90	0.75	0.75	0.75	0.94	0.94	0.94	0.88	0.88
Adj. Flow (vph)	168	33	127	475	53	132	227	1023	240	110	1002
RTOR Reduction (vph)	0	0	111	0	0	111	0	0	80	0	50
Lane Group Flow (vph)	188	33	16	475	53	21	227	1023	180	110	1002
Turn Type	pm+pt	7	4	Perm	pm+pt	3	8	Perm	Perm	Perm	Perm
Protected Phases	4	4	4	8	8	8	8	8	8	8	8
Permitted Phases	4	4	4	8	8	8	8	8	8	8	8
Actuated Green, G (s)	17.9	8.0	8.0	32.0	17.1	17.1	78.0	78.0	78.0	78.0	78.0
Effective Green, g (s)	21.9	10.0	10.0	34.0	19.1	19.1	80.0	80.0	80.0	80.0	80.0
Actuated g/C Ratio	0.18	0.08	0.08	0.28	0.16	0.16	0.67	0.67	0.67	0.67	0.67
Clearance Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	285	154	131	420	294	250	292	2337	1045	284	2337
v/s Ratio Prot	0.06	0.02		c0.20	0.03		0.01	c0.52	0.10	0.28	0.29
v/s Ratio Perm	0.05			0.12	0.13	0.18	0.08	0.78	0.44	0.15	0.39
Uniform Delay, d1	44.4	51.3	50.9	41.5	43.7	43.0	13.8	9.4	7.4	9.0	9.3
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.45	0.53	0.19	1.00
Incremental Delay, d2	3.1	0.7	0.4	84.7	0.3	0.1	8.4	0.3	0.1	4.0	0.6
Delay (s)	47.5	52.0	51.4	126.2	44.0	43.1	14.6	5.3	1.8	12.9	9.9
Level of Service	D	D	D	F	D	D	B	A	A	B	A
Approach Delay (s)	49.4				103.0			8.1			9.9
Approach LOS	D				F			A			A

Intersection Summary
HCM Average Control Delay 28.3 HCM Level of Service C
HCM Volume to Capacity ratio 0.88
Actuated Cycle Length (s) 120.0 Sum of lost time (s) 6.0
Intersection Capacity Utilization 72.6% ICU Level of Service C
Analysis Period (min) 15
Critical Lane Group

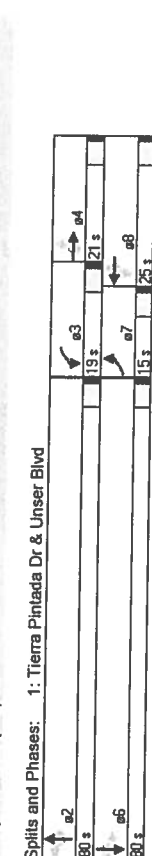
2010 PM Peak NOBUILD Conditions Case F - full access at Intersection 12
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Timings
1: Tierra Pintada Dr & Unser Blvd

Terry O. Brown, P.E.
10/30/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBR
Lane Configurations	151	30	154	362	40	99	253	1037	232	97	945
Volume (vph)	pm+pt	7	4	3	8	8	2	2	2	6	6
Turn Type	Perm	pm+pt	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm
Protected Phases	4	4	4	4	4	4	4	4	4	4	4
Permitted Phases	7	4	4	4	4	4	4	4	4	4	4
Detector Phases	7	4	4	4	4	4	4	4	4	4	4
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	10.0	21.0	21.0	10.0	21.0	21.0	21.0	21.0	21.0	21.0	21.0
Total Split (s)	15.0	21.0	21.0	19.0	25.0	25.0	80.0	80.0	80.0	80.0	80.0
Total Split (%)	12.5%	17.5%	15.8%	20.8%	20.8%	20.8%	66.7%	66.7%	66.7%	66.7%	66.7%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min
Recall Mode	24.1	12.2	31.2	16.2	16.2	82.8	82.8	82.8	82.8	82.8	82.8
Act Eff Green (s)	0.20	0.10	0.10	0.26	0.14	0.14	0.69	0.69	0.69	0.69	0.69
Actuated g/C Ratio	0.54	0.18	0.85	1.27	0.21	0.40	1.01	0.46	0.21	0.43	0.44
w/c Ratio	43.0	49.3	30.1	176.8	46.5	11.1	48.9	4.6	0.2	15.9	9.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Queue Delay	43.0	49.3	30.1	176.8	46.5	11.1	48.9	4.6	0.2	15.9	9.5
LOS	D	D	C	F	D	B	D	A	A	B	A
Approach Delay	37.6										
Approach LOS	D										

Intersection Summary	Min	Max	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Cycle Length: 120	24.1	12.2	31.2	16.2	16.2	82.8	82.8	82.8	82.8	82.8	82.8
Actuated Cycle Length: 120	0.20	0.10	0.10	0.26	0.14	0.14	0.69	0.69	0.69	0.69	0.69
Offset: 22 (18%), Referenced to phase 2:NBT and 6:SBLT, Start of Green	0.54	0.18	0.85	1.27	0.21	0.40	1.01	0.46	0.21	0.43	0.44
Natural Cycle: 120	43.0	49.3	30.1	176.8	46.5	11.1	48.9	4.6	0.2	15.9	9.5
Control Type: Actuated-Coordinated	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Maximum v/c Ratio: 1.27	43.0	49.3	30.1	176.8	46.5	11.1	48.9	4.6	0.2	15.9	9.5
Intersection Signal Delay: 33.5	D	D	C	F	D	B	D	A	A	B	A
Intersection Capacity Utilization 76.9%	37.6										
Analysis Period (min) 15	D										



2010 PM Peak BUILD Conditions - MITIGATED Case F - full access at Intersection 12
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HCM Signalized Intersection Capacity Analysis
1: Tierra Pintada Dr & Unser Blvd

Terry O. Brown, P.E.
10/30/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBR
Lane Configurations	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Total Lost time (s)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Flt	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Satd. Flow (prot)	1752	1845	1568	1752	1845	1568	1752	1845	1568	1752	1568
Flt Permitted	0.72	1.00	1.00	0.59	1.00	1.00	0.72	1.00	1.00	0.72	1.00
Satd. Flow (perm)	1333	1845	1568	1089	1845	1568	1333	1845	1568	1089	1568
Volume (vph)	151	30	154	362	40	99	253	1037	232	97	945
Peak-hour factor, PHF	0.90	0.90	0.90	0.75	0.75	0.75	0.94	0.94	0.94	0.88	0.88
Adj. Flow (vph)	168	33	171	483	53	132	269	1103	247	110	1074
RTOR Reduction (vph)	0	0	103	0	0	114	0	0	77	0	47
Lane Group Flow (vph)	168	33	68	483	53	18	269	1103	170	110	1074
Turn Type	Perm	pm+pt	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm
Protected Phases	4	4	4	4	4	4	4	4	4	4	4
Permitted Phases	7	4	4	4	4	4	4	4	4	4	4
Actuated Green, G (s)	20.1	10.2	10.2	28.3	14.3	14.3	80.8	80.8	80.8	80.8	80.8
Effective Green, g (s)	24.1	12.2	12.2	31.2	16.3	16.3	82.8	82.8	82.8	82.8	82.8
Actuated g/C Ratio	0.20	0.10	0.10	0.26	0.14	0.14	0.69	0.69	0.69	0.69	0.69
Clearance Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	309	188	159	372	251	213	282	2418	1082	270	2418
v/s Ratio Prot	0.05	0.02		c0.17	0.03						
v/s Ratio Perm	0.06			0.04	c0.16						
v/c Ratio	0.54	0.18	0.43	1.30	0.21	0.08	0.95	0.46	0.11	0.28	0.07
Uniform Delay, d1	42.4	49.3	50.6	43.2	46.1	45.3	16.9	8.4	6.5	8.0	0.10
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	2.0	0.4	1.8	152.8	0.4	0.2	21.2	0.2	0.1	4.5	0.2
Delay (s)	44.3	49.7	52.4	195.9	46.8	45.5	28.9	4.3	0.5	12.5	8.9
Level of Service	D	D	D	F	D	D	C	A	A	B	A
Approach Delay (s)	48.5										
Approach LOS	D										

Intersection Summary	Min	Max	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
HCM Average Control Delay	36.5										
HCM Volume to Capacity ratio	1.05										
Actual Cycle Length (s)	120.0										
Intersection Capacity Utilization	76.9%										
Analysis Period (min)	15										
c Critical Lane Group											

2010 PM Peak BUILD Conditions - MITIGATED Case F - full access at Intersection 12
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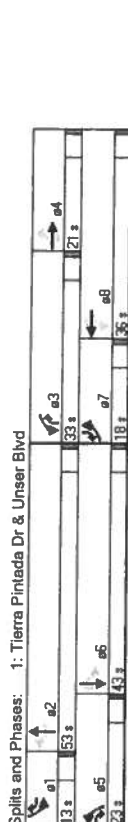
Timings

1: Tierra Pintada Dr & Unser Blvd

Terry O. Brown, P.E.

10/30/2007

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	151	30	154	362	40	99	253	1037	232	97	945	132
Volume (vph)	pm+pt	pm+ov	pm+pt	pm+ov	pm+pt	pm+ov	pm+pt	pm+ov	pm+pt	pm+ov	pm+pt	pm+ov
Turn Type	7	4	5	3	8	1	5	2	3	1	6	7
Permitted Phases	4	4	4	8	8	2	2	2	2	2	6	6
Detector Phases	7	4	5	3	8	1	5	2	3	1	6	7
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	10.0	21.0	10.0	10.0	21.0	10.0	10.0	10.0	10.0	21.0	10.0	10.0
Total Split (s)	18.0	21.0	23.0	33.0	36.0	13.0	23.0	53.0	33.0	13.0	43.0	18.0
Total Split (%)	15.0%	17.5%	19.2%	27.5%	30.0%	10.8%	19.2%	44.2%	27.5%	10.8%	35.8%	15.0%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lead/Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lead	Lag
Lead-Lag Optimize?	Min	None	Min	None	Min	None	Min	None	Min	None	Min	None
Recall Mode	23.6	9.6	33.1	42.1	25.1	39.1	71.9	57.9	90.3	59.5	48.4	65.4
Act Enrd Green (s)	0.20	0.08	0.28	0.35	0.21	0.33	0.60	0.48	0.75	0.50	0.40	0.54
Actuated g/c Ratio	0.54	0.22	0.35	0.87	0.14	0.22	0.72	0.65	0.20	0.43	0.76	0.16
Control Delay	36.0	54.7	18.5	52.8	39.3	6.0	34.1	27.1	0.2	18.9	36.5	3.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	36.0	54.7	18.5	52.8	39.3	6.0	34.1	27.1	0.2	18.9	36.5	3.0
LOS	D	D	B	D	D	A	C	C	A	B	D	A
Approach Delay	29.6	C							24.2	C		
Approach LOS	C								C			



Splits and Phases: 1: Tierra Pintada Dr & Unser Blvd

HCM Signalized Intersection Capacity Analysis

1: Tierra Pintada Dr & Unser Blvd

Terry O. Brown, P.E.

10/30/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Total Lost time (s)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Flt Protected	1752	1845	1568	1752	1845	1568	1752	1845	1568	1752	1845	1568
Flt Permitted	0.72	1.00	1.00	0.56	1.00	1.00	0.10	1.00	1.00	0.17	1.00	1.00
Satd. Flow (perm)	1333	1845	1568	1034	1845	1568	178	3505	1568	307	3505	1568
Volume (vph)	151	30	154	362	40	99	253	1037	232	97	945	132
Peak-hour factor, PHF	0.90	0.90	0.90	0.75	0.75	0.75	0.94	0.94	0.94	0.88	0.88	0.88
Adj. Flow (vph)	168	33	171	483	53	132	289	1103	247	110	1074	150
RTOR Reduction (vph)	0	0	61	0	0	88	0	0	67	0	0	72
Lane Group Flow (vph)	168	33	110	483	53	44	269	1103	180	110	1074	76
Turn Type	pm+pt	pm+ov	pm+pt	pm+ov	pm+pt	pm+ov	pm+pt	pm+ov	pm+pt	pm+ov	pm+pt	pm+ov
Protected Phases	7	4	5	3	8	1	5	2	3	1	6	7
Permitted Phases	4	4	4	8	8	2	2	2	2	2	6	6
Actuated Green, G (s)	19.6	7.6	26.1	40.1	23.1	32.2	68.9	55.8	83.3	55.5	46.4	58.4
Effective Green, g (s)	23.6	9.6	30.1	42.1	25.1	36.2	71.9	57.8	87.3	59.5	48.4	62.4
Actuated g/c Ratio	0.20	0.08	0.25	0.35	0.21	0.30	0.60	0.48	0.73	0.50	0.40	0.52
Clearance Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	311	148	433	539	386	512	376	1688	1180	286	1414	855
v/s Ratio Prot	0.06	0.02	0.04	0.22	0.03	0.01	0.12	0.31	0.04	0.04	0.31	0.01
v/s Ratio Perm	0.04	0.03	0.03	0.09	0.14	0.09	0.72	0.65	0.15	0.38	0.76	0.09
v/c Ratio	0.54	0.22	0.25	0.90	0.36	0.30	28.2	23.5	5.0	17.9	30.8	14.5
Uniform Delay, d1	42.8	51.7	36.0	35.0	38.6	30.0	28.2	23.5	5.0	17.9	30.8	14.5
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.23	1.07	0.16	1.00	1.00	1.00
Incremental Delay, d2	1.9	0.8	0.3	17.3	0.2	0.1	2.1	0.6	0.0	0.9	3.9	0.0
Delay (s)	44.7	52.5	36.3	52.3	38.8	30.1	36.9	25.9	0.8	18.7	34.7	14.6
Level of Service	D	D	D	D	D	C	D	C	A	B	C	B
Approach Delay (s)	41.5	D							23.9	C		
Approach LOS	D								C			

Intersection Summary

HCM Average Control Delay	31.8	HCM Level of Service	C
HCM Volume to Capacity ratio	0.80		
Actuated Cycle Length (s)	120.0	Sum of lost time (s)	9.0
Intersection Capacity Utilization	76.9%	ICU Level of Service	D
Analysis Period (min)	15		
c Critical Lane Group			

2010 PM Peak BUILD Conditions - MITIGATED

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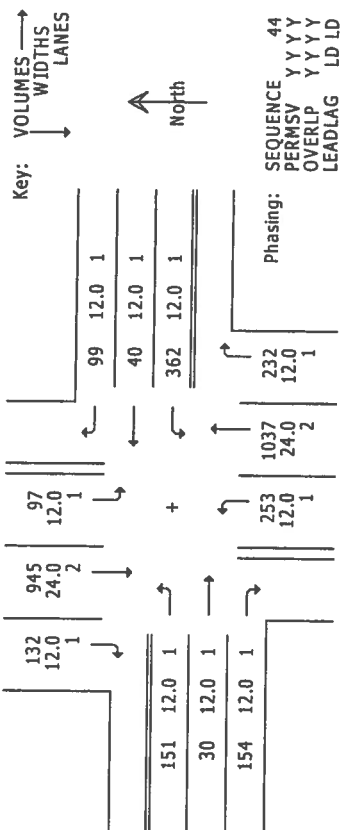
Case F - full access at Intersection 12

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SIGNAL2006/TEAPAC[Ver 2.80.00] - HCM Input Worksheet

Intersection # 1 -

Area Location Type: NONCBD



	SB			WB			NB			EB		
	RT	TH	LT	RT	TH	LT	RT	TH	LT	RT	TH	LT
Heavy veh, %HV	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Pk-hr fact, PHF	.88	.88	.88	.75	.75	.75	.94	.94	.94	.90	.90	.90
Pretimed or Act	A	A	A	A	A	A	A	A	A	A	A	A
Strutp lost, l1	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Ext eff grn, e	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Arrival typ, AT	3	3	3	3	3	3	3	3	3	3	3	3
Ped vol, vped	0	0	0	0	0	0	0	0	0	0	0	0
Bike vol, vbic	0	0	0	0	0	0	0	0	0	0	0	0
Parking locatns	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO	NO
Park mnvrs, Nm	0	0	0	0	0	0	0	0	0	0	0	0
Bus stops, NB	0	0	0	0	0	0	0	0	0	0	0	0
Grade, %G	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0

	Phase 1	Phase 2	Phase 3	Phase 4	Phase 5	Phase 6
Sq 44 LD/LD	←	←	←	←	←	←
North	↑	↑	↑	↑	↑	↑
C=120"	G= 15.1" Y+R= 5.0"	G= 48.4" Y+R= 5.0"	G= 27.1" Y+R= 5.0"	G= 9.4" Y+R= 5.0"	G= 0.0" Y+R= 0.0"	G= 0.0" Y+R= 0.0"

































































































































SIGNAL2006/TEAPAC[Ver 2.80.00] - Capacity Analysis Summary

Intersection Averages for Int # 1 -

V/C 0.668 (Critical V/C 0.847)

Control Delay 35.1

Level of Service D+

Sq 44 LD/LD	Phase 1	Phase 2	Phase 3	Phase 4
				
				
				
				
				
				
				
				
				
				
				
				
				
				
				
				
				
				
				
				
				
				
				
				
				
				
				
				
				
				
				
				

C=120 sec G=100.0 sec = 83.3% Y=20.0 sec = 16.7% Ped= 0.0 sec = 0.0%

Lane Group	Width/Lanes	Reqd	q/c Used	Service Rate @C (vph) @E	Adj Volume	v/c	HCM Delay	L	S	Queue Model 1
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SB Approach

	RT	TH	LT
12/1	0.279	0.391	0.047
24/2	0.047	0.126	0.126
12/1	0.143	0.759	0.369
29.1 C	150	1074	110
7.3 A	102 ft	794 ft	115 ft

NB Approach

	RT	TH	LT
12/1	0.308	0.670	0.126
24/2	0.397	0.403	0.126
12/1	0.159	0.229	0.229
33.3 C	1051	1415	302
7.9 A	177 ft	826 ft	459 ft

WB Approach

	RT	TH	LT
12/1	0.273	0.246	0.226
12/1	0.249	0.079	0.079
12/1	0.282	0.352	0.352
50.8 D	371	116	536
37.8 D+	195 ft	95 ft	815 ft

EB Approach

	RT	TH	LT
12/1	0.285	0.246	0.226
12/1	0.244	0.079	0.079
12/1	0.161	0.342	0.342
35.6 D+	371	116	519
39.1 D+	257 ft	58 ft	220 ft

Analysis of Intersection #2

Ladera Dr / Unser Blvd

Timings
2: Ladera Dr & Unser Blvd

Terry O. Brown, P.E.
10/30/2007

Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	NBR	SBL	SBT
Lane Configurations	198	446	557	542	186	134	653	376	98	1279
Volume (vph)	pm+pt	7	4	5	3	8	5	2	3	6
Turn Type	pm+ov	Prot	pm+ov	Prot	pm+ov	Prot	pm+ov	Prot	pm+ov	Prot
Protected Phases	4	4	4	4	4	4	4	4	4	4
Detector Phases	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Initial (s)	10.0	21.0	10.0	21.0	10.0	21.0	10.0	21.0	10.0	21.0
Minimum Split (s)	12.0	30.0	10.0	23.0	41.0	10.0	57.0	23.0	47.0	47.0
Total Split (%)	10.9%	27.3%	9.1%	20.9%	37.3%	8.1%	51.8%	20.9%	42.7%	42.7%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lead-Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lead	Lag
Lead-Lag Optimize?	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min
Recall Mode	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min
Act Eff Green (s)	36.0	27.0	37.0	20.0	38.0	54.0	54.0	77.0	44.0	44.0
Actuated g/c Ratio	0.33	0.25	0.34	0.18	0.35	0.49	0.49	0.70	0.40	0.40
v/c Ratio	0.69	1.13	0.68	1.11	0.63	0.88	0.45	0.40	0.45	1.12
Control Delay	34.5	122.7	35.2	110.3	32.9	72.1	8.6	4.8	26.2	91.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	34.5	122.7	35.2	110.3	32.9	72.1	8.6	4.8	26.2	91.0
LOS	C	F	D	F	C	E	A	A	C	F
Approach Delay	67.6	E			82.2	F	14.7	B		86.7
Approach LOS	E				F					F

Intersection Summary

Cycle Length: 110
Actuated Cycle Length: 110
Offset: 90 (62%), Referenced to phase 2: NBT and 6: SBT, Start of Green
Natural Cycle: 120
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 1.13
Intersection Signal Delay: 83.1
Intersection Capacity Utilization 98.4%
Analysis Period (min) 15

Intersection LOS: E
ICU Level of Service F

Splits and Phases: 2: Ladera Dr & Unser Blvd



HCM Signalized Intersection Capacity Analysis

2: Ladera Dr & Unser Blvd

Terry O. Brown, P.E.
10/30/2007

Movement	EBL	EBT	EBR	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Total Lost time (s)	1.00	1.00	0.88	0.97	1.00	1.00	0.95	1.00	0.85	1.00	0.95
Lane Util. Factor	1.00	1.00	0.85	1.00	0.94	1.00	1.00	0.85	1.00	0.95	1.00
Flt Protected	0.95	1.00	1.00	0.95	1.00	0.95	1.00	1.00	0.95	1.00	0.95
Satd. Flow (prot)	1752	1845	2760	3400	1734	1752	3505	1568	1752	3465	1568
Flt Permitted	0.41	1.00	1.00	0.95	1.00	0.09	1.00	1.00	0.33	1.00	0.09
Satd. Flow (perm)	762	1845	2760	3400	1734	157	3505	1568	616	3465	1568
Volume (vph)	199	446	557	542	186	134	653	376	98	1279	105
Peak-hour factor, PHF	0.87	0.87	0.87	0.79	0.79	0.85	0.85	0.85	0.89	0.89	0.89
Adj. Flow (vph)	229	513	640	686	235	156	768	442	111	1437	118
RTOR Reduction (vph)	0	0	9	0	0	0	0	0	22	0	5
Lane Group Flow (vph)	229	513	631	686	389	0	158	768	420	111	1550
Turn Type	pm+pt	7	4	5	3	8	5	2	3	6	6
Protected Phases	4	4	4	4	4	4	4	4	4	4	4
Permitted Phases	32.0	25.0	30.0	18.0	35.0	52.0	52.0	70.0	42.0	42.0	42.0
Actuated Green, G (s)	36.0	27.0	34.0	20.0	38.0	54.0	54.0	74.0	44.0	44.0	44.0
Actuated g/c Ratio	0.33	0.25	0.31	0.18	0.35	0.48	0.48	0.67	0.40	0.40	0.40
Clearance Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	330	453	928	618	588	179	1721	1098	246	1386	1386
v/s Ratio Prot	0.06	0.28	0.04	0.20	0.21	0.08	0.22	0.07	0.07	0.45	0.45
v/s Ratio Perm	0.17	0.19	0.19	0.19	0.19	0.38	0.38	0.20	0.18	0.38	0.38
v/c Ratio	0.69	1.13	0.68	1.11	0.62	0.68	0.45	0.38	0.45	1.12	1.12
Uniform Delay, d1	30.0	41.5	33.2	45.0	29.9	53.0	18.3	7.9	24.2	33.0	33.0
Progression Factor	1.00	1.00	1.00	0.94	1.00	1.53	0.42	0.65	0.81	0.86	0.86
Incremental Delay, d2	6.2	63.8	2.0	70.2	1.9	35.3	0.8	0.2	5.5	62.9	62.9
Delay (s)	36.2	125.3	35.2	112.5	32.0	116.5	8.5	5.4	25.0	81.3	81.3
Level of Service	D	F	D	F	C	F	A	A	C	F	F
Approach Delay (s)	68.8	E			83.3	F	19.9	B		86.9	86.9
Approach LOS	E				F					F	F

Intersection Summary

HCM Average Control Delay: 65.0
HCM Volume to Capacity ratio: 1.07
Actuated Cycle Length (s): 110.0
Intersection Capacity Utilization: 98.4%
Analysis Period (min): 15
c Critical Lane Group

2010 AM Peak NOBUILD Conditions

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Case F - full access at Intersection 12
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Timings 2: Ladera Dr & Unser Blvd

Terry O. Brown, P.E.
10/30/2007

Movement	EBL	EBT	EBR	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	189	460	557	542	196	163	704	376	125	1355	1900
Volume (vph)	pm+pt	7	4	4	3	8	5	2	2	6	6
Turn Type	pm+pt	7	4	4	3	8	5	2	2	6	6
Protected Phases	7	4	4	3	8	5	2	2	2	6	6
Permitted Phases	7	4	4	3	8	5	2	2	2	6	6
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	10.0	21.0	21.0	21.0	21.0	21.0	21.0	21.0	21.0	21.0	21.0
Total Split (s)	14.0	30.0	30.0	22.0	38.0	10.0	58.0	58.0	48.0	48.0	48.0
Total Split (%)	12.7%	27.3%	27.3%	20.0%	34.5%	9.1%	52.7%	52.7%	43.6%	43.6%	43.6%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lead/Lag	Lead	Lag	Lag	Lag	Lag	Lag	Lead	Lead	Lag	Lag	Lag
Lead-Lag Optimize?	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min
Recall Mode	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min
Act Eff Green (s)	38.0	27.0	27.0	19.0	35.0	55.0	55.0	55.0	45.0	45.0	45.0
Actuated g/c Ratio	0.35	0.25	0.25	0.17	0.32	0.50	0.50	0.50	0.41	0.41	0.41
v/c Ratio	0.76	1.17	0.85	1.17	0.74	1.08	0.47	0.45	0.61	1.15	1.15
Control Delay	38.7	135.1	41.0	132.6	40.4	119.6	12.0	2.1	32.8	104.7	104.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	38.7	135.1	41.0	132.6	40.4	119.6	12.0	2.1	32.8	104.7	104.7
LOS	D	F	D	F	D	F	B	A	C	F	F
Approach Delay	75.1	E			97.3	F	23.1	C		99.1	F
Approach LOS		E			F						

Intersection Summary

Cycle Length: 110	
Actuated Cycle Length: 110	
Offset: 84 (75%), Referenced to phase 2.NBTL and 6.SBTL, Start of Green	
Natural Cycle: 120	
Control Type: Actuated-Coordinated	
Maximum v/c Ratio: 1.17	
Intersection Signal Delay: 73.6	
Intersection Capacity Utilization: 102.8%	
Analysis Period (min): 15	

Spills and Phases: 2: Ladera Dr & Unser Blvd



2010 AM Peak BUILD Conditions
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Case F - full access at Intersection 12

HCM Signalized Intersection Capacity Analysis 2: Ladera Dr & Unser Blvd

Terry O. Brown, P.E.
10/30/2007

Movement	EBL	EBT	EBR	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1890	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Total Lost time (s)	1.00	1.00	0.88	0.97	1.00	1.00	0.95	1.00	0.85	1.00	0.95
Lane Util. Factor	1.00	1.00	0.85	0.95	1.00	0.95	1.00	0.85	1.00	0.95	1.00
Flt Protected	1752	1845	2760	3400	1729	1752	3505	1568	1752	3467	1752
Flt Permitted	0.28	1.00	1.00	0.95	1.00	0.08	1.00	1.00	0.31	1.00	0.31
Satd. Flow (perm)	521	1845	2760	3400	1729	154	3505	1568	563	3467	1568
Volume (vph)	199	460	587	542	196	140	163	704	376	125	1355
Peak-hour factor, PHF	0.87	0.87	0.87	0.79	0.79	0.85	0.85	0.85	0.89	0.89	0.89
Adj. Flow (vph)	229	529	686	686	248	177	192	828	442	140	1522
RTOR Reduction (vph)	0	0	128	0	23	0	0	0	198	0	5
Lane Group Flow (vph)	229	529	558	686	402	0	182	828	245	140	1635
Turn Type	pm+pt	7	4	4	3	8	5	2	2	6	6
Protected Phases	7	4	4	3	8	5	2	2	2	6	6
Permitted Phases	7	4	4	3	8	5	2	2	2	6	6
Actuated Green, G (s)	34.0	25.0	25.0	17.0	33.0	53.0	53.0	53.0	43.0	43.0	43.0
Effective Green, g (s)	38.0	27.0	27.0	19.0	35.0	55.0	55.0	55.0	45.0	45.0	45.0
Actuated g/c Ratio	0.35	0.25	0.25	0.17	0.32	0.50	0.50	0.50	0.41	0.41	0.41
Clearance Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	303	463	677	587	550	179	1753	784	230	1418	1418
v/s Ratio Prot	0.08	0.29	0.20	0.20	0.23	0.07	0.24	0.16	0.25	0.25	0.25
v/s Ratio Perm	0.19					0.47		0.31	0.61	0.61	0.61
v/c Ratio	0.76	1.17	0.82	1.17	0.73	1.07	0.47	0.31	0.61	1.15	1.15
Uniform Delay, d1	28.0	41.5	39.3	45.5	33.3	53.7	18.0	16.3	25.6	32.5	32.5
Progression Factor	1.00	1.00	1.00	0.96	1.02	1.39	0.61	0.27	0.80	0.87	0.87
Incremental Delay, d2	10.3	97.0	8.0	93.2	5.0	87.2	0.9	1.0	10.6	76.7	76.7
Delay (s)	38.3	138.5	47.3	138.6	39.0	162.1	11.9	5.4	31.2	105.0	105.0
Level of Service	D	F	D	F	D	F	B	A	C	F	F
Approach Delay (s)	79.3	E			99.3	F	29.6			99.2	F
Approach LOS		E			F		C				

Intersection Summary

HCM Average Control Delay	76.7	HCM Level of Service	E
HCM Volume to Capacity ratio	1.15		
Actuated Cycle Length (s)	110.0	Sum of lost time (s)	12.0
Intersection Capacity Utilization	102.8%	ICU Level of Service	G
Analysis Period (min)	15		

c Critical Lane Group

2010 AM Peak BUILD Conditions
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Case F - full access at Intersection 12

Timings

2: Ladera Dr & Unser Blvd

Terry O. Brown, P.E.
10/30/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Total Lost time (s)	1.00	1.00	0.88	0.97	1.00	1.00	0.95	1.00	0.85	1.00	0.95	0.95
Lane Util. Factor	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95
Flt Protected	1752	1845	2760	3400	1745	1752	1845	2760	3400	1745	1752	1845
Satd. Flow (prot)	0.17	1.00	1.00	0.95	1.00	0.17	1.00	1.00	0.95	1.00	0.17	1.00
Flt Permitted	307	1845	2760	3400	1745	307	1845	2760	3400	1745	307	1845
Satd. Flow (perm)	192	334	369	594	496	192	334	369	594	496	192	334
Volume (vph)	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Peak-hour factor, PHF	206	359	397	639	533	206	359	397	639	533	206	359
Adj. Flow (vph)	0	0	318	0	17	0	0	318	0	17	0	0
RTOR Reduction (vph)	206	359	397	639	533	206	359	397	639	533	206	359
Lane Group Flow (vph)	pm+pt	pm+pt	Prot	Prot	Prot	pm+pt	pm+pt	Prot	Prot	Prot	pm+pt	pm+pt
Turn Type	7	4	4	3	8	7	4	4	3	8	7	4
Protected Phases	4					4					4	
Permitted Phases	27.0	22.0	22.0	10.0	27.0	27.0	22.0	22.0	10.0	27.0	27.0	22.0
Actuated Green, G (s)	31.0	24.0	24.0	12.0	29.0	31.0	24.0	24.0	12.0	29.0	31.0	24.0
Effective Green, g (s)	0.26	0.20	0.20	0.10	0.24	0.26	0.20	0.20	0.10	0.24	0.26	0.20
Actuated g/C Ratio	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Clearance Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Vehicle Extension (s)	164	369	552	340	422	164	369	552	340	422	164	369
Lane Grp Cap (vph)	0.07	0.19	0.03	0.19	0.47	0.07	0.19	0.03	0.19	0.47	0.07	0.19
v/s Ratio Prot	0.25					0.25					0.25	
v/s Ratio Perm	1.26	0.97	0.14	1.88	1.93	1.26	0.97	0.14	1.88	1.93	1.26	0.97
v/c Ratio	58.7	47.7	39.5	54.0	45.5	58.7	47.7	39.5	54.0	45.5	58.7	47.7
Uniform Delay, d1	1.00	1.00	1.00	0.99	0.96	1.00	1.00	1.00	0.99	0.96	1.00	1.00
Progression Factor	155.2	39.4	0.1	406.6	429.0	155.2	39.4	0.1	406.6	429.0	155.2	39.4
Incremental Delay, d2	213.9	87.1	39.7	460.1	472.7	213.9	87.1	39.7	460.1	472.7	213.9	87.1
Delay (s)	F	F	D	F	F	F	F	D	F	F	F	D
Level of Service	F	F	D	F	F	F	F	D	F	F	F	D
Approach Delay (s)	94.7			467.3		94.7			467.3		94.7	
Approach LOS	F			F		F			F		F	

Intersection Summary												
Cycle Length, 120												
Actuated Cycle Length, 120												
Offset: 25 (21%), Referenced to phase 2 NBTL and 6 SBTL, Start of Green												
Natural Cycle: 75												
Control Type: Actuated-Coordinated												
Maximum v/c Ratio: 3.28												
Intersection Signal Delay: 203.3												
Intersection Capacity Utilization 143.7%												
Analysis Period (min) 15												

Splits and Phases: 2: Ladera Dr & Unser Blvd

78 s	a2	a3	a4
15 s	a5	a6	a7
10 s	a8	a9	a10

HCM Signalized Intersection Capacity Analysis

2: Ladera Dr & Unser Blvd

Terry O. Brown, P.E.
10/30/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Total Lost time (s)	1.00	1.00	0.88	0.97	1.00	1.00	0.95	1.00	0.85	1.00	0.95	0.95
Lane Util. Factor	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95
Flt Protected	1752	1845	2760	3400	1745	1752	1845	2760	3400	1745	1752	1845
Satd. Flow (prot)	0.17	1.00	1.00	0.95	1.00	0.17	1.00	1.00	0.95	1.00	0.17	1.00
Flt Permitted	307	1845	2760	3400	1745	307	1845	2760	3400	1745	307	1845
Satd. Flow (perm)	192	334	369	594	496	192	334	369	594	496	192	334
Volume (vph)	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Peak-hour factor, PHF	206	359	397	639	533	206	359	397	639	533	206	359
Adj. Flow (vph)	0	0	318	0	17	0	0	318	0	17	0	0
RTOR Reduction (vph)	206	359	397	639	533	206	359	397	639	533	206	359
Lane Group Flow (vph)	pm+pt	pm+pt	Prot	Prot	Prot	pm+pt	pm+pt	Prot	Prot	Prot	pm+pt	pm+pt
Turn Type	7	4	4	3	8	7	4	4	3	8	7	4
Protected Phases	4					4					4	
Permitted Phases	27.0	22.0	22.0	10.0	27.0	27.0	22.0	22.0	10.0	27.0	27.0	22.0
Actuated Green, G (s)	31.0	24.0	24.0	12.0	29.0	31.0	24.0	24.0	12.0	29.0	31.0	24.0
Effective Green, g (s)	0.26	0.20	0.20	0.10	0.24	0.26	0.20	0.20	0.10	0.24	0.26	0.20
Actuated g/C Ratio	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Clearance Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Vehicle Extension (s)	164	369	552	340	422	164	369	552	340	422	164	369
Lane Grp Cap (vph)	0.07	0.19	0.03	0.19	0.47	0.07	0.19	0.03	0.19	0.47	0.07	0.19
v/s Ratio Prot	0.25					0.25					0.25	
v/s Ratio Perm	1.26	0.97	0.14	1.88	1.93	1.26	0.97	0.14	1.88	1.93	1.26	0.97
v/c Ratio	58.7	47.7	39.5	54.0	45.5	58.7	47.7	39.5	54.0	45.5	58.7	47.7
Uniform Delay, d1	1.00	1.00	1.00	0.99	0.96	1.00	1.00	1.00	0.99	0.96	1.00	1.00
Progression Factor	155.2	39.4	0.1	406.6	429.0	155.2	39.4	0.1	406.6	429.0	155.2	39.4
Incremental Delay, d2	213.9	87.1	39.7	460.1	472.7	213.9	87.1	39.7	460.1	472.7	213.9	87.1
Delay (s)	F	F	D	F	F	F	F	D	F	F	F	D
Level of Service	F	F	D	F	F	F	F	D	F	F	F	D
Approach Delay (s)	94.7			467.3		94.7			467.3		94.7	
Approach LOS	F			F		F			F		F	

Intersection Summary

HCM Average Control Delay	214.1	HCM Level of Service	F
HCM Volume to Capacity ratio	2.64		
Actuated Cycle Length (s)	120.0	Sum of lost time (s)	9.0
Intersection Capacity Utilization	143.7%	ICU Level of Service	H
Analysis Period (min)	15		

c Critical Lane Group

2010 PM Peak BUILD Conditions

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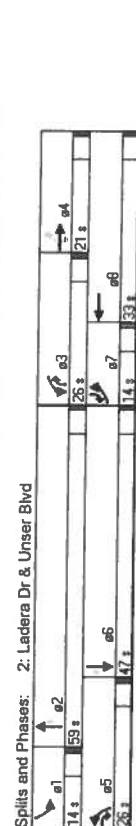
2010 PM Peak BUILD Conditions
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Case F - full access at Intersection 12

Timings

2: Ladera Dr & Unser Blvd

Terry O. Brown, P.E.
10/30/2007

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	192	334	369	594	495	117	117	117	117	117	117	117
Volume (vph)	192	334	369	594	495	117	117	117	117	117	117	117
Turn Type	pm+pt	pm+ov	pm+ov	Prot	pm+ov	Prot	pm+ov	Prot	pm+ov	Prot	pm+ov	Prot
Protected Phases	7	4	5	3	8	5	2	3	1	6	7	6
Permitted Phases	4	4	4	4	4	4	4	4	4	4	4	4
Detector Phases	7	4	5	3	8	5	2	3	1	6	7	6
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	10.0	21.0	10.0	10.0	21.0	10.0	10.0	10.0	10.0	21.0	10.0	10.0
Total Split (s)	14.0	21.0	26.0	26.0	33.0	26.0	59.0	26.0	14.0	47.0	14.0	14.0
Total Split (%)	11.7%	17.5%	21.7%	21.7%	27.5%	21.7%	48.2%	21.7%	11.7%	39.2%	11.7%	11.7%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lead/Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lead	Lag
Lead-Lag Optimizer?												
Recall Mode	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min
Act Effrt Green (s)	28.8	17.8	44.0	23.0	29.8	23.2	56.0	82.0	11.2	44.0	58.0	58.0
Actuated g/c Ratio	0.24	0.15	0.37	0.19	0.25	0.19	0.47	0.68	0.09	0.37	0.48	0.48
v/c Ratio	0.93	0.69	0.39	0.98	0.94	0.97	0.98	0.68	0.95	0.95	0.44	0.44
Control Delay	77.1	56.1	27.9	79.1	55.9	81.5	41.1	13.6	81.8	50.8	18.4	18.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	77.1	56.1	27.9	79.1	55.9	81.5	41.1	13.6	81.8	50.8	18.4	18.4
LOS	E	E	C	E	E	F	D	B	F	F	D	B
Approach Delay	48.9											
Approach LOS	D											



Splits and Phases: 2: Ladera Dr & Unser Blvd

Cycle Length: 120	
Actuated Cycle Length: 120	
Offset: 25 (21%), Referenced to phase 2:NBT and 6:SBT, Start of Green	
Natural Cycle: 110	
Control Type: Actuated-Coordinated	
Maximum v/c Ratio: 0.98	
Intersection Signal Delay: 50.1	
Intersection Capacity Utilization 97.0%	
Analysis Period (min) 15	

2010 PM Peak Build Conditions - MITIGATED
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Case F - full access at Intersection 12
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HCM Signalized Intersection Capacity Analysis

2: Ladera Dr & Unser Blvd

Terry O. Brown, P.E.
10/30/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	192	334	369	594	495	117	117	117	117	117	117	117
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Util. Factor	1.00	0.95	0.98	0.97	0.95	0.97	0.95	0.98	0.95	0.97	0.95	0.95
Flt Protected	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95
Satd. Flow (prot)	1752	3505	2760	3400	3316	3400	3505	1568	3400	3505	1568	1568
Flt Permitted	0.22	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00
Satd. Flow (perm)	415	3505	2760	3400	3316	3400	3505	1568	3400	3505	1568	1568
Volume (vph)	192	334	369	594	495	279	607	1524	708	290	1169	333
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Adj. Flow (vph)	206	359	397	639	533	300	639	1604	745	302	1218	347
RTOR Reduction (vph)	0	0	13	0	65	0	0	0	11	0	0	23
Lane Group Flow (vph)	206	359	384	639	768	0	639	1604	734	302	1218	324
Turn Type	pm+pt	pm+ov	pm+ov	Prot	pm+ov	Prot	pm+ov	Prot	pm+ov	Prot	pm+ov	pm+ov
Protected Phases	7	4	5	3	8	5	2	3	1	6	7	6
Permitted Phases	4	4	4	4	4	4	4	4	4	4	4	4
Actuated Green, G (s)	24.8	15.8	37.0	21.0	27.8	21.2	54.0	75.0	9.2	42.0	51.0	51.0
Effective Green, g (s)	28.8	17.8	41.0	23.0	29.8	23.2	56.0	79.0	11.2	44.0	55.0	55.0
Actuated g/c Ratio	0.24	0.15	0.34	0.19	0.25	0.19	0.47	0.68	0.09	0.37	0.48	0.48
Clearance Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	222	520	1012	652	823	657	1636	1071	317	1285	758	758
v/s Ratio Prot	0.08	0.10	0.07	0.18	0.23	0.19	0.46	0.13	0.09	0.35	0.04	0.04
v/s Ratio Perm	0.14					0.34						
v/c Ratio	0.93	0.69	0.38	0.98	0.93	0.97	0.98	0.68	0.95	0.95	0.43	0.43
Uniform Delay, d1	41.3	48.5	29.9	48.3	44.1	48.1	31.5	12.8	54.1	36.9	21.9	21.9
Progression Delay, d2	1.00	1.00	1.00	1.00	0.95	1.12	0.73	0.90	0.91	1.05	0.95	0.95
Incremental Delay, d2	40.5	3.9	0.2	29.9	17.0	27.1	17.4	1.7	30.2	11.8	0.3	0.3
Delay (s)	81.8	52.4	30.1	78.2	58.8	80.9	40.2	13.2	79.3	50.5	21.0	21.0
Level of Service	F	D	C	E	E	F	D	D	B	E	D	C
Approach Delay (s)	49.5					67.2						
Approach LOS	D					E						

Intersection Summary

HCM Average Control Delay	50.1	HCM Level of Service	D
HCM Volume to Capacity ratio	0.96		
Actuated Cycle Length (s)	120.0	Sum of lost time (s)	6.0
Intersection Capacity Utilization	97.0%	ICU Level of Service	F
Analysis Period (min)	15		
Critical Lane Group			

Analysis of Intersection #3

I-40 North ramp / Unser Blvd

Timings

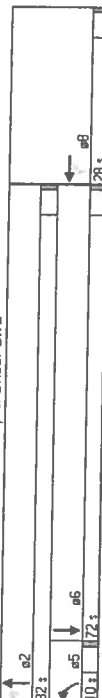
3: I-40 North ramp & Unser Blvd Terry O. Brown, P.E.
10/30/2007

	WBL	WBT	WBR	NBL	NBT	SBT
Lane Group	4	3	222	46	992	2281
Lane Configurations	438	3	222	46	992	2281
Volume (vph)	Perm	8	Free	pm+pt	5	2
Turn Type	8	8	Free	2	2	6
Protected Phases	8	8	Free	2	2	6
Permitted Phases	8	8	Free	2	2	6
Detector Phases	8	8	Free	2	2	6
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	21.0	21.0	10.0	21.0	21.0	21.0
Total Split (s)	28.0	28.0	0.0	10.0	82.0	72.0
Total Split (%)	25.5%	25.5%	0.0%	9.1%	74.5%	65.5%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0
Lead/Lag	Lead	Lead	Lead	Lead	Lead	Lag
Lead-Lag Optimize?	Min	Min	Min	Min	Min	Min
Recall Mode	Min	Min	Min	Min	Min	Min
Act Effct Green (s)	21.6	21.6	110.0	82.4	82.4	72.1
Actuated g/C Ratio	0.20	0.20	1.00	0.75	0.75	0.66
v/c Ratio	0.74	0.74	0.16	0.25	0.39	0.81
Control Delay	55.2	55.4	0.2	18.9	1.8	8.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	55.2	55.4	0.2	18.9	1.8	8.8
LOS	E	E	A	B	A	A
Approach Delay	36.8	36.8	2.6	8.8	8.8	8.8
Approach LOS	D	D	A	A	A	A

Intersection Summary

Cycle Length: 110
 Actuated Cycle Length: 110
 Offset: 20 (18%), Referenced to phase 2 NBTL and 6 SBT, Start of Green
 Natural Cycle: 75
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.81
 Intersection Signal Delay: 11.9
 Intersection Capacity Utilization: 72.1%
 Analysis Period (min): 15

Spills and Phases: 3: I-40 North ramp & Unser Blvd



HCM Signalized Intersection Capacity Analysis

3: I-40 North ramp & Unser Blvd Terry O. Brown, P.E.
10/30/2007

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Movement	1900	1800	1900	1900	1900	1900	1900	1800	1800	1900	1900	1900
Lane Configurations	1900	1800	1900	1900	1900	1900	1900	1800	1800	1900	1900	1900
Ideal Flow (vphpl)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Total Lost time (s)	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Lane Util. Factor	1.00	1.00	1.00	0.85	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Flt. Protected	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Satd. Flow (prot)	1665	1670	1568	1752	3505	5012	5012	1665	1670	1568	1752	3505
Flt. Permitted	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Satd. Flow (perm)	1665	1670	1568	98	3505	5012	5012	1665	1670	1568	98	3505
Volume (vph)	0	0	0	438	3	222	46	992	0	0	2281	76
Peak-hour factor, PHF	0.85	0.85	0.85	0.91	0.91	0.91	0.97	0.97	0.97	0.89	0.89	0.89
Adj. Flow (vph)	0	0	0	481	3	244	47	1023	0	0	2563	85
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	0	0	0	241	243	244	47	1023	0	0	2645	0
Turn Type	Perm	Perm	Perm	Free	pm+pt	Free	pm+pt	Free	pm+pt	Free	pm+pt	Free
Protected Phases	8	8	8	Free	2	2	2	2	2	2	2	2
Permitted Phases	8	8	8	Free	2	2	2	2	2	2	2	2
Actuated Green, G (s)	19.6	19.6	110.0	80.4	80.4	80.4	80.4	80.4	80.4	80.4	80.4	80.4
Effective Green, g (s)	21.6	21.6	110.0	82.4	82.4	82.4	82.4	82.4	82.4	82.4	82.4	82.4
Actuated g/C Ratio	0.20	0.20	1.00	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75
Clearance Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	327	328	1568	183	2626	3285	3285	183	2626	3285	183	2626
v/s Ratio Prot	0.14	0.15	0.16	0.17	0.17	0.17	0.17	0.17	0.17	0.17	0.17	0.17
v/s Ratio Perm	0.74	0.74	0.16	0.26	0.39	0.39	0.39	0.39	0.39	0.39	0.39	0.39
Uniform Delay, d1	41.5	41.6	0.0	14.7	4.9	13.8	13.8	14.7	4.9	13.8	14.7	4.9
Progression Factor	1.00	1.00	1.00	3.84	0.28	0.59	0.59	3.84	0.28	0.59	3.84	0.28
Incremental Delay, d2	8.4	8.7	0.2	0.6	0.3	0.2	0.2	0.6	0.3	0.2	0.6	0.3
Delay (s)	49.9	50.3	0.2	56.9	1.7	8.3	8.3	56.9	1.7	8.3	56.9	1.7
Level of Service	D	D	A	E	A	A	A	E	A	A	E	A
Approach Delay (s)	0.0	0.0	0.0	33.4	4.1	8.3	8.3	33.4	4.1	8.3	33.4	4.1
Approach LOS	A	A	A	C	A	A	A	C	A	A	C	A

Intersection Summary

HCM Average Control Delay: 11.4
 HCM Volume to Capacity ratio: 0.76
 Actuated Cycle Length (s): 110.0
 Intersection Capacity Utilization: 72.1%
 Analysis Period (min): 15
 c Critical Lane Group

Terry O. Brown, P.E.
10/30/2007

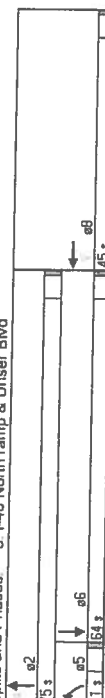
3: I-40 North ramp & Unser Blvd

Lane Group	WBL	WBT	WBR	NBL	NBT	SBT
Lane Configurations						
Volume (vph)	770	0	905	75	1485	1806
Turn Type	Perm		Free	pm+pt		
Protected Phases		8	Free	5	2	6
Permitted Phases		8	Free	2		
Detector Phases		8			2	6
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	21.0	21.0	10.0	21.0	21.0	21.0
Total Split (s)	45.0	45.0	0.0	11.0	75.0	64.0
Total Split (%)	37.5%	37.5%	0.0%	9.2%	62.5%	53.3%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0
Lead						
Lag						
Lead-Lag Optimize?						
Recall Mode	Min	Min	Min	C-Max	C-Max	C-Max
Effective Green (s)	35.5	35.5	120.0	78.5	78.5	67.2
Actuated g/C Ratio	0.30	0.30	1.00	0.65	0.65	0.56
g/C Ratio	0.83	0.83	0.61	0.46	0.74	0.75
Control Delay	54.0	53.8	1.8	35.5	9.8	15.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
total Delay	54.0	53.8	1.8	35.5	9.8	15.0
LOS	D	D	A	D	A	B
Approach Delay		25.8			11.0	15.0
Approach LOS		C			B	C

Intersection Summary

Cycle Length: 120
 Actuated Cycle Length: 120
 Offset: 86 (72%), Referenced to phase 2 NBTL and 6 SBT, Start of Green
 Natural Cycle: 65
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.83
 Intersection Signal Delay: 17.1
 Intersection Capacity Utilization 127.9%
 Analysis Period (min) 15

3: I-40 North ramp & Unser Blvd



2010 PM Peak BUILD Conditions

Case F - full access at Intersection 12
e_Ladera_UnserCaseF2010PBX.sy7

HCM Signalized Intersection Capacity Analysis

Terry O. Brown, P.E.
10/30/2007

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Intersection Summary

	B	H
ICM Average Control Delay	16.2	
ICM Volume to Capacity ratio	0.76	
Actuated Cycle Length (s)	120.0	3.0
Intersection Capacity Utilization	127.9%	
Analysis Period (min)	15	
Critical Lane Group		

2010 PM Peak BUILD Conditions

Case F - full access at Intersection 12
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Analysis of Intersection #4

Los Volcanes Rd / Unser Blvd

Timings

4: Los Volcanes Rd & Unser Blvd

Terry O. Brown, P.E.
10/30/2007

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	137	74	237	31	1192	490	976	75		
Volume (vph)	Prot	7	4	3	8	5	2	23	1	6
Turn Type	Protected Phases									
Permitted Phases	7	4	3	8	5	2	23	1	6	67
Detector Phases										
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	10.0	21.0	10.0	21.0	10.0	21.0	10.0	21.0	10.0	21.0
Total Split (s)	14.0	21.0	21.0	28.0	10.0	39.0	60.0	29.0	58.0	72.0
Total Split (%)	12.7%	19.1%	19.1%	25.5%	9.1%	35.5%	54.5%	25.4%	52.7%	65.5%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lead/Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lead	Lag
Lead-Lag Optimize?										
Recall Mode	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min
Act Eff Green (s)	10.8	13.4	17.4	20.0	7.5	42.2	62.6	25.0	59.6	73.4
Actuated g/C Ratio	0.10	0.12	0.16	0.18	0.07	0.38	0.57	0.23	0.54	0.67
v/c Ratio	0.55	0.48	0.74	0.63	0.04	0.68	0.31	0.77	0.63	0.08
Control Delay	53.8	49.6	53.0	12.6	48.5	31.7	4.1	51.1	15.9	0.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	53.8	49.6	53.0	12.6	48.5	31.7	4.1	51.1	15.9	0.6
LOS	D	D	D	B	D	C	A	D	B	A
Approach Delay	52.3			34.5		26.6			26.4	
Approach LOS	D			C		C			C	
Intersection Summary										
Cycle Length: 110										
Actuated Cycle Length: 110										
Offset: 96 (87%), Referenced to phase 2 NBT and 6 SBT, Start of Green										
Natural Cycle: 80										
Control Type: Actuated-Coordinated										
Maximum v/c Ratio: 0.77										
Intersection Signal Delay: 29.4										
Intersection Capacity Utilization 69.7%										
Analysis Period (min) 15										
Intersection LOS: C										
ICU Level of Service C										
Spills and Phases: 4: Los Volcanes Rd & Unser Blvd										

2010 AM Peak NOBUILD Conditions

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Case F - full access at Intersection 12

HCM Signalized Intersection Capacity Analysis

4: Los Volcanes Rd & Unser Blvd

Terry O. Brown, P.E.
10/30/2007

Movement	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	137	74	237	31	1192	490	976	75		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Util. Factor	0.97	1.00	0.97	1.00	0.97	0.91	1.00	0.97	0.95	1.00
Flt Protected	1.00	0.99	1.00	0.87	1.00	1.00	0.85	1.00	1.00	0.85
Satd. Flow (prot)	3400	1822	3400	1602	3400	5036	1568	3400	3505	1568
Flt Permitted	0.95	1.00	0.95	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	3400	1822	3400	1602	3400	5036	1568	3400	3505	1568
Volume (vph)	137	74	237	31	219	8	1192	279	480	976
Peak-hour factor, PHF	0.75	0.75	0.75	0.75	0.75	0.91	0.91	0.82	0.82	0.82
Adj. Flow (vph)	183	99	396	41	292	9	1310	307	598	1190
RTOR Reduction (vph)	0	4	0	0	239	0	0	107	0	0
Lane Group Flow (vph)	183	104	0	396	94	0	1310	200	598	1190
Turn Type	Prot	7	4	3	8	5	2	23	1	6
Protected Phases										
Permitted Phases										
Actuated Green, G (s)	8.8	11.4	15.4	18.0	5.5	40.2	60.6	23.0	57.7	71.5
Effective Green, g (s)	10.8	13.4	17.4	20.0	7.5	42.2	62.6	25.0	59.7	73.5
Actuated g/C Ratio	0.10	0.12	0.16	0.18	0.07	0.38	0.57	0.23	0.54	0.67
Clearance Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	334	222	538	291	232	1932	892	773	1902	1048
v/s Ratio Prot	0.05	0.06	0.12	0.06	0.00	0.26	0.13	0.18	0.34	0.04
v/s Ratio Perm										
v/c Ratio	0.55	0.47	0.74	0.32	0.04	0.68	0.22	0.77	0.63	0.06
Uniform Delay, d1	47.3	45.0	44.1	39.1	47.9	28.2	11.7	39.8	17.4	6.3
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	1.8	1.6	5.2	0.6	0.1	1.9	0.1	3.8	1.2	0.0
Delay (s)	49.1	46.6	49.3	39.8	48.0	30.2	11.8	43.1	18.6	6.3
Level of Service	D	D	D	D	D	C	B	D	B	A
Approach Delay (s)	48.2			45.0		26.8			25.3	
Approach LOS	D			D		C			C	
Intersection Summary										
HCM Average Control Delay	30.5									
HCM Volume to Capacity ratio	0.68									
Actuated Cycle Length (s)	110.0									
Intersection Capacity Utilization	69.7%									
Analysis Period (min)	15									
c Critical Lane Group										

2010 AM Peak NOBUILD Conditions

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Case F - full access at Intersection 12

Timings

4: Los Volcanes Rd & Unser Blvd

Terry O. Brown, P.E.

10/30/2007

Movement	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	142	74	297	31	1287	1900	1900	1900	1900	1900
Volume (vph)	142	74	297	31	1287	1900	1900	1900	1900	1900
Turn Type	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot
Protected Phases	7	4	3	8	5	2	23	1	6	67
Permitted Phases	7	4	3	8	5	2	23	1	6	67
Deflector Phases	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Initial (s)	10.0	21.0	10.0	21.0	10.0	21.0	10.0	21.0	10.0	21.0
Minimum Split (s)	12.0	21.0	20.0	29.0	10.0	40.0	60.0	29.0	59.0	71.0
Total Split (%)	10.9%	19.1%	18.2%	26.4%	9.1%	36.4%	54.5%	26.4%	53.6%	64.5%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lead/Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lead	Lag
Lead-Lag Optimize?	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min
Recall Mode	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min
Act Eff Green (s)	9.0	13.4	16.7	21.1	7.5	42.2	61.8	25.7	60.4	72.4
Actuated g/C Ratio	0.08	0.12	0.15	0.19	0.07	0.38	0.56	0.23	0.55	0.66
v/c Ratio	0.68	0.48	0.77	0.66	0.04	0.73	0.31	0.79	0.66	0.09
Control Delay	62.2	49.6	55.7	12.1	48.5	32.9	4.3	50.7	15.8	0.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	62.2	49.6	55.7	12.1	48.5	32.9	4.3	50.7	15.8	0.5
LOS	E	D	E	B	D	C	A	D	B	A
Approach Delay	57.6									
Approach LOS	E									

Intersection Summary

Cycle Length: 110
 Actuated Cycle Length: 110
 Offset: 90 (82%), Referenced to phase 2 NBT and 6 SBT, Start of Green
 Natural Cycle: 90
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.79
 Intersection Signal Delay: 30.0
 Intersection Capacity Utilization 74.5%
 Analysis Period (min) 15

Intersection LOS: C
 ICU Level of Service D

Splits and Phases: 4: Los Volcanes Rd & Unser Blvd



2010 AM Peak BUILD Conditions

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2010 AM Peak BUILD Conditions

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HCM Signalized Intersection Capacity Analysis

4: Los Volcanes Rd & Unser Blvd

Terry O. Brown, P.E.

10/30/2007

Movement	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Total Lost time (s)	0.97	1.00	0.97	1.00	0.97	1.00	0.97	1.00	0.97	1.00
Lane Util. Factor	1.00	0.99	1.00	0.87	1.00	1.00	0.85	1.00	0.97	0.95
Frt	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Flt Protected	3400	1822	3400	1598	3400	1598	3400	1598	3400	1598
Satd. Flow (prot)	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Flt Permitted	3400	1822	3400	1598	3400	1598	3400	1598	3400	1598
Satd. Flow (perm)	142	74	297	31	1287	1900	1900	1900	1900	1900
Volume (vph)	142	74	297	31	1287	1900	1900	1900	1900	1900
Peak-hour factor, PHF	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75
Adj. Flow (vph)	189	99	396	41	340	9	1414	307	629	1272
RTOR Reduction (vph)	0	4	0	0	275	0	0	107	0	32
Lane Group Flow (vph)	189	104	0	396	106	0	1414	200	629	1272
Turn Type	Prot	4	Prot	8	Prot	5	2	23	1	6
Protected Phases	7	4	3	8	5	2	23	1	6	67
Permitted Phases	7	4	3	8	5	2	23	1	6	67
Actuated Green, G (s)	7.0	11.4	14.7	19.1	5.5	40.2	59.9	23.7	58.4	70.4
Effective Green, g (s)	9.0	13.4	16.7	21.1	7.5	42.2	61.9	25.7	60.4	72.4
Actuated g/C Ratio	0.08	0.12	0.15	0.19	0.07	0.38	0.56	0.23	0.55	0.66
Clearance Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	278	222	516	307	232	1932	882	794	1925	1032
v/s Ratio Prot	0.06	0.08	0.12	0.07	0.00	0.28	0.13	0.19	0.36	0.04
v/s Ratio Perm	0.68	0.47	0.77	0.35	0.04	0.73	0.23	0.79	0.66	0.06
Uniform Delay, d1	49.1	45.0	44.8	38.5	47.9	29.1	12.1	39.6	17.6	6.7
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	6.5	1.6	6.7	0.7	0.1	2.5	0.1	4.0	1.3	0.0
Delay (s)	55.6	46.6	51.5	39.2	48.0	31.5	12.2	43.6	18.9	6.7
Level of Service	E	D	D	D	D	C	B	D	B	A
Approach Delay (s)	52.3									
Approach LOS	D									

Intersection Summary

HCM Average Control Delay 31.2 HCM Level of Service C
 HCM Volume to Capacity ratio 0.72
 Actuated Cycle Length (s) 110.0
 Intersection Capacity Utilization 74.5%
 Analysis Period (min) 15
 Critical Lane Group

Timings

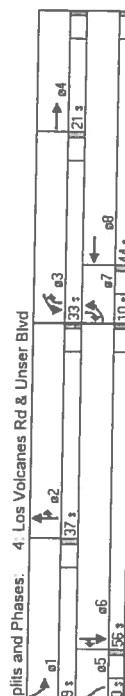
4: Los Volcanes Rd & Unser Blvd

Terry O. Brown, P.E.
10/30/2007

Movement	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Volume (vph)	82	33	736	33	12	1083	463	609	894	127
Turn Type	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot
Protected Phases	7	4	3	8	5	2	2	3	1	6
Permitted Phases	7	4	3	8	5	2	2	3	1	6
Detector Phases	7	4	3	8	5	2	2	3	1	6
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	10.0	21.0	10.0	21.0	10.0	21.0	10.0	21.0	10.0	21.0
Total Split (s)	10.0	21.0	33.0	44.0	10.0	37.0	70.0	29.0	56.0	66.0
Total Split (%)	8.3%	17.5%	27.5%	36.7%	8.3%	30.8%	58.3%	24.2%	46.7%	55.0%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lead/Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lead	Lag
Lead-Lag Optimize?										
Recall Mode	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min
Act Eff Green (s)	7.0	10.5	30.0	33.5	8.0	36.3	69.3	31.2	59.5	69.5
Actuated g/C Ratio	0.06	0.09	0.25	0.28	0.07	0.30	0.58	0.26	0.50	0.58
v/c Ratio	0.50	0.28	0.94	0.75	0.06	0.84	0.51	0.77	0.58	0.15
Control Delay	63.9	49.4	64.0	17.6	52.8	45.5	6.7	48.9	18.1	2.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	63.9	49.4	64.0	17.6	52.8	45.5	6.7	48.9	18.1	2.3
LOS	E	D	E	B	D	D	A	D	B	A
Approach Delay	59.2	45.4	45.4	34.0	34.0	34.0	28.4	28.4	28.4	28.4
Approach LOS	E	D	D	C	C	C	C	C	C	C

Intersection Summary

Cycle Length: 120
 Actuated Cycle Length: 120
 Offset: 51 (43%), Referenced to phase 2 NBT and 6 SBT, Start of Green
 Natural Cycle: 100
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.94
 Intersection Signal Delay: 35.7
 Intersection LOS: D
 Intersection Capacity Utilization 83.4%
 Analysis Period (min) 15



Splits and Phases: 4: Los Volcanes Rd & Unser Blvd

HCM Signalized Intersection Capacity Analysis

4: Los Volcanes Rd & Unser Blvd

Terry O. Brown, P.E.
10/30/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Util. Factor	0.97	1.00	1.00	0.97	1.00	1.00	0.97	1.00	1.00	0.97	1.00	1.00
Flt Protected	1.00	0.98	1.00	1.00	0.86	1.00	1.00	0.85	1.00	1.00	0.95	1.00
Satd. Flow (prot)	3400	1803	3400	1587	3400	1587	3400	1587	3400	1587	3400	1587
Flt Permitted	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	3400	1803	3400	1587	3400	1587	3400	1587	3400	1587	3400	1587
Volume (vph)	82	33	6	736	33	459	12	1083	463	609	894	127
Peak-hour factor, PHF	0.83	0.83	0.83	0.92	0.92	0.92	0.85	0.85	0.85	0.89	0.89	0.89
Adj. Flow (vph)	99	40	7	800	36	499	14	1274	545	684	1004	143
RTOR Reduction (vph)	0	5	0	0	275	0	0	0	0	153	0	60
Lane Group Flow (vph)	99	42	0	800	260	0	14	1274	392	684	1004	83
Turn Type	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot
Protected Phases	7	4	3	8	5	2	2	3	1	6	6	7
Permitted Phases	7	4	3	8	5	2	2	3	1	6	6	7
Actuated Green, G (s)	5.0	8.5	28.0	31.5	28.0	31.5	6.0	34.3	67.3	29.2	57.5	67.5
Effective Green, g (s)	7.0	10.5	30.0	33.5	30.0	33.5	8.0	36.3	69.3	31.2	59.5	69.5
Actuated g/C Ratio	0.06	0.09	0.25	0.28	0.25	0.28	0.07	0.30	0.58	0.26	0.50	0.58
Clearance Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	198	158	850	443	850	443	227	1523	906	884	1738	908
v/s Ratio Prot	0.03	0.02	c0.24	c0.16	c0.24	c0.16	0.00	c0.25	0.25	c0.20	0.29	0.05
v/c Ratio	0.50	0.26	0.94	0.59	0.94	0.59	0.06	0.84	0.43	0.77	0.58	0.09
Uniform Delay, d1	54.8	51.1	44.1	37.3	44.1	37.3	52.5	39.1	14.3	41.1	21.4	11.2
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.07	0.77	0.98
Incremental Delay, d2	2.0	0.9	18.2	2.0	18.2	2.0	0.1	5.6	0.3	3.0	1.0	0.0
Delay (s)	56.8	52.0	62.3	39.3	62.3	39.3	52.6	44.7	14.6	46.9	17.5	11.0
Level of Service	E	D	E	D	E	D	D	D	B	D	B	B
Approach Delay (s)	55.3	53.1	53.1	53.1	53.1	53.1	53.1	53.1	53.1	53.1	53.1	53.1
Approach LOS	E	E	E	D	E	D	D	D	D	D	D	C

Intersection Summary

HCM Average Control Delay 36.1 HCM Level of Service D
 HCM Volume to Capacity ratio 0.81
 Actuated Cycle Length (s) 120.0
 Intersection Capacity Utilization 83.4%
 Analysis Period (min) 15
 Critical Lane Group

2010 PM Peak NOBUILD Conditions

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Case F - full access at Intersection 12

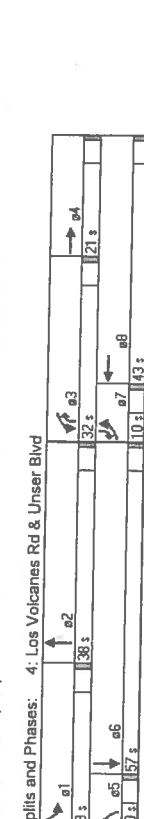
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Timings

4: Los Volcanes Rd & Unser Blvd Terry O. Brown, P.E.
10/30/2007

Movement	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Volume (vph)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Turn Type	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot
Protected Phases	7	4	3	8	5	2	3	1	6	7
Permitted Phases	7	4	3	8	5	2	3	1	6	7
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	10.0	21.0	10.0	21.0	10.0	21.0	10.0	21.0	10.0	21.0
Total Split (s)	10.0	21.0	32.0	43.0	10.0	38.0	32.0	29.0	57.0	10.0
Total Split (%)	8.3%	17.5%	26.7%	35.8%	8.3%	31.7%	26.7%	24.2%	47.5%	8.3%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lead/Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lead	Lag
Lead-Lag Optimize?										
Recall Mode	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min
Act Effct Green (s)	7.0	11.3	29.0	33.3	7.7	35.7	67.7	32.0	60.0	70.0
Actuated g/C Ratio	0.06	0.09	0.24	0.28	0.06	0.30	0.56	0.27	0.50	0.58
v/c Ratio	0.53	0.27	0.97	0.82	0.06	0.93	0.52	0.81	0.64	0.15
Control Delay	65.1	47.5	71.0	23.8	53.7	53.6	6.9	48.7	17.7	1.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	65.1	47.5	71.0	23.8	53.7	53.6	6.9	48.7	17.7	1.5
LOS	E	D	E	C	D	D	A	D	B	A
Approach Delay	59.6		51.2		40.6				28.1	
Approach LOS	E		D		D				C	

Intersection Summary
 Cycle Length: 120
 Actuated Cycle Length: 120
 Offset: 40 (33%), Referenced to phase 2:NBT and 6:SBT, Start of Green
 Natural Cycle: 110
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.97
 Intersection Signal Delay: 39.2
 Intersection Capacity Utilization: 88.8%
 Analysis Period (min): 15



2010 PM Peak BUILD Conditions
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 Case F - full access at Intersection 12

HCM Signalized Intersection Capacity Analysis

4: Los Volcanes Rd & Unser Blvd Terry O. Brown, P.E.
10/30/2007

Movement	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Volume (vph)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Turn Type	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot	Prot
Protected Phases	7	4	3	8	5	2	3	1	6	7
Permitted Phases	7	4	3	8	5	2	3	1	6	7
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	10.0	21.0	10.0	21.0	10.0	21.0	10.0	21.0	10.0	21.0
Total Split (s)	10.0	21.0	32.0	43.0	10.0	38.0	32.0	29.0	57.0	10.0
Total Split (%)	8.3%	17.5%	26.7%	35.8%	8.3%	31.7%	26.7%	24.2%	47.5%	8.3%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lead/Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lead	Lag
Lead-Lag Optimize?										
Recall Mode	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min
Act Effct Green (s)	7.0	11.3	29.0	33.3	7.7	35.7	67.7	32.0	60.0	70.0
Actuated g/C Ratio	0.06	0.09	0.24	0.28	0.06	0.30	0.56	0.27	0.50	0.58
v/c Ratio	0.53	0.27	0.97	0.82	0.06	0.93	0.52	0.81	0.64	0.15
Control Delay	65.1	47.5	71.0	23.8	53.7	53.6	6.9	48.7	17.7	1.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	65.1	47.5	71.0	23.8	53.7	53.6	6.9	48.7	17.7	1.5
LOS	E	D	E	C	D	D	A	D	B	A
Approach Delay	59.6		51.2		40.6				28.1	
Approach LOS	E		D		D				C	

Intersection Summary
 HCM Average Control Delay: 41.7
 HCM Volume to Capacity ratio: 0.87
 Actuated Cycle Length (s): 120.0
 Intersection Capacity Utilization: 88.8%
 Analysis Period (min): 15
 Critical Lane Group: C

2010 PM Peak BUILD Conditions
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 Case F - full access at Intersection 12

Analysis of Intersection #5

Ladera Dr / Ouray Rd

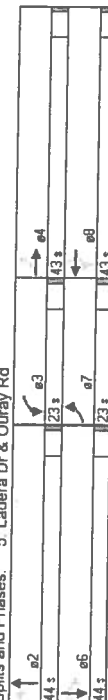
Timings

5: Ladera Dr & Ouray Rd

Terry O. Brown, P.E.
10/30/2007

Lane Group		EBL	EBT	WBL	WBT	WBR	NBL	NBT	SBL	SBT	SBR	
Lane Configurations		↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	✓
Volume (vph)		20	365	4	214	30	101	79	99	212	14	✓
Turn Type		pm+pt	pm+pt	pm+pt	pm+pt	pm+pt	pm+pt	pm+pt	pm+pt	pm+pt	pm+pt	✓
Protected Phases		7	4	3	8	8	2	2	6	6	6	✓
Permitted Phases		4	3	8	8	2	2	2	6	6	6	✓
Detector Phases		5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	✓
Minimum Initial (s)		10.0	21.0	21.0	21.0	21.0	21.0	21.0	21.0	21.0	21.0	✓
Minimum Split (s)		23.0	43.0	23.0	43.0	43.0	44.0	44.0	44.0	44.0	44.0	✓
Total Split (s)		20.9%	39.1%	20.9%	39.1%	39.1%	40.0%	40.0%	40.0%	40.0%	40.0%	✓
Yellow Time (s)		4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	✓
All-Red Time (s)		1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	✓
Lead/Lag		Lead	Lag	Lead	Lag	Lag	Lag	Lag	Lag	Lag	Lag	✓
Lead-Lag Optimize?		Min	C-Max	Min	C-Max	Min	C-Max	Min	C-Max	Min	C-Max	✓
Recall Mode		79.4	71.7	79.2	71.6	71.6	71.6	21.7	21.7	21.7	21.7	✓
Act Effct Green (s)		0.72	0.65	0.72	0.65	0.65	0.65	0.20	0.20	0.20	0.20	✓
Actuated g/C Ratio		0.03	0.30	0.01	0.11	0.03	0.71	0.29	0.46	0.65	0.05	✓
v/c Ratio		2.8	3.3	5.5	8.4	3.5	60.6	36.6	43.5	48.4	13.6	✓
Control Delay		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	✓
Queue Delay		2.8	3.3	5.5	8.4	3.5	60.6	36.6	43.5	48.4	13.6	✓
Total Delay		A	A	A	A	A	E	D	D	D	B	✓
LOS		A	A	A	A	A	E	D	D	D	B	✓
Approach Delay		3.3	7.7	7.7	7.7	49.7	49.7	45.4	45.4	45.4	45.4	✓
Approach LOS		A	A	A	A	D	D	D	D	D	D	✓
Intersection Summary												
Cycle Length, 110												
Actuated Cycle Length, 110												
Offset: 75 (68%), Referenced to phase 4 EBTl and 8 WBTl, Start of Green												
Natural Cycle: 55												
Control Type: Actuated-Coordinated												
Maximum v/c Ratio: 0.71												
Intersection Signal Delay: 20.8												
Intersection Capacity Utilization 44.8%												
Analysis Period (min) 15												

Splits and Phases: 5: Ladera Dr & Ouray Rd



2010 AM Peak NOBUILD Conditions

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Case F - full access at Intersection 12

HCM Signalized Intersection Capacity Analysis

5: Ladera Dr & Ouray Rd

Terry O. Brown, P.E.
10/30/2007

Movement		EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑
Ideal Flow (vphpl)		1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Util. Factor		1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95
Fit Protected		1.00	0.94	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95
Said. Flow (prot)		1752	3293	1752	3505	1568	1752	1829	1752	1845	1568	1752	1845
Fit Permitted		0.60	1.00	0.36	1.00	0.32	1.00	0.32	1.00	0.32	1.00	0.32	1.00
Said. Flow (perm)		1110	3293	667	3505	1568	584	1829	1122	1845	1568	1122	1845
Volume (vph)		20	365	247	4	214	30	101	79	5	99	212	14
Peak-hour factor, PHF		0.91	0.91	0.91	0.89	0.89	0.89	0.79	0.79	0.79	0.89	0.89	0.89
Adj. Flow (vph)		22	401	271	4	240	34	128	100	6	111	238	16
RTOR Reduction (vph)		0	58	0	0	0	0	12	0	2	0	0	0
Lane Group Flow (vph)		22	614	0	4	240	22	128	104	0	111	238	3
Turn Type		pm+pt	pm+pt	pm+pt	pm+pt	pm+pt	pm+pt	pm+pt	pm+pt	pm+pt	pm+pt	pm+pt	pm+pt
Protected Phases		7	4	3	8	8	2	2	6	6	6	6	6
Permitted Phases		4	3	8	8	2	2	2	6	6	6	6	6
Actuated Green, G (s)		75.4	69.7	75.2	69.6	69.6	69.6	19.7	19.7	19.7	19.7	19.7	19.7
Effective Green, g (s)		79.4	71.7	79.2	71.6	71.6	71.6	21.7	21.7	21.7	21.7	21.7	21.7
Actuated g/C Ratio		0.72	0.65	0.72	0.65	0.65	0.65	0.20	0.20	0.20	0.20	0.20	0.20
Clearance Time (s)		5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Vehicle Extension (s)		3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)		846	2146	555	2281	1021	115	361	221	364	309	221	364
v/s Ratio Prot		c0.00	c0.19	0.00	0.07	0.01	c0.22	0.06	0.10	0.13	0.10	0.13	0.10
v/s Ratio Perm		0.02	0.03	0.03	0.01	0.11	0.02	1.11	0.29	0.50	0.65	0.50	0.65
Uniform Delay, d1		4.3	8.2	4.5	7.2	6.8	44.2	37.6	39.3	40.7	35.5	39.3	40.7
Progression Factor		0.52	0.44	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2		0.0	0.3	0.0	0.1	0.0	117.6	0.4	1.8	4.2	0.0	1.8	4.2
Delay (s)		2.3	3.9	4.5	7.3	6.8	161.7	38.0	41.1	44.9	35.5	41.1	44.9
Level of Service		A	A	A	A	A	F	D	D	D	D	D	D
Approach Delay (s)		3.8	7.2	7.2	7.2	105.7	43.3	43.3	43.3	43.3	43.3	43.3	43.3
Approach LOS		A	A	A	A	F	D	D	D	D	D	D	D
Intersection Summary													
HCM Average Control Delay													
HCM Volume to Capacity ratio													
Actuated Cycle Length (s)													
Intersection Capacity Utilization													
Analysis Period (min)													
Critical Lane Group													

2010 AM Peak NOBUILD Conditions
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Case F - full access at Intersection 12

Timings

5: Ladera Dr & Ouray Rd

Terry O. Brown, P.E.
10/30/2007

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	SBR
Lane Configurations	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑
Volume (vph)	24	414	4	291	30	156	79	99	212
Turn Type	pm+pt	4	3	8	Perm	Perm	Perm	Perm	Perm
Protected Phases	4	4	8	8	2	2	6	6	6
Permitted Phases	7	4	3	8	8	2	6	6	6
Detector Phases	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Initial (s)	10.0	21.0	10.0	21.0	21.0	21.0	21.0	21.0	21.0
Minimum Split (s)	18.0	43.0	18.0	43.0	49.0	49.0	49.0	49.0	49.0
Total Split (%)	16.4%	39.1%	16.4%	39.1%	44.5%	44.5%	44.5%	44.5%	44.5%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lead/Lag	Lead	Lag	Lead	Lag	Lag	Lag	Lag	Lag	Lag
Lead-Lag Optimize?									
Recall Mode	Min	C-Max	Min	C-Max	Min	C-Max	Min	C-Max	Min
Act Effct Green (s)	73.2	65.2	72.6	65.0	28.1	28.1	28.1	28.1	28.1
Actuated g/C Ratio	0.67	0.59	0.66	0.59	0.26	0.26	0.26	0.26	0.26
v/c Ratio	0.04	0.38	0.01	0.16	0.04	0.04	0.35	0.51	0.05
Control Delay	6.0	7.0	8.5	12.0	5.0	60.3	30.0	34.2	37.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	6.0	7.0	8.5	12.0	5.0	60.3	30.0	34.2	37.2
LOS	A	A	A	B	A	E	C	C	D
Approach Delay	6.9	A	A	B	A	E	C	C	D
Approach LOS	A	A	A	B	A	E	C	C	D

Intersection Summary

Cycle Length: 110
 Actuated Cycle Length: 110
 Offset: 56 (51%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green
 Natural Cycle: 55
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.80
 Intersection Signal Delay: 20.5
 Intersection Capacity Utilization 50.4%
 Analysis Period (min) 15

Intersection LOS: C
 ICU Level of Service A

Splits and Phases: 5: Ladera Dr & Ouray Rd



2010 AM Peak BUILD Conditions

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2010 AM Peak BUILD Conditions

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HCM Signalized Intersection Capacity Analysis

5: Ladera Dr & Ouray Rd

Terry O. Brown, P.E.
10/30/2007

Movement	EBL	EBT	EBR	WBL	WBT	NBL	NBT	SBL	SBT	SBR
Lane Configurations	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑
Ideal Flow (vphpl)	1800	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Util. Factor	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00
Flt	1.00	0.94	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.85
Flt Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Satd. Flow (prot)	1752	3290	1752	3505	1568	1752	1829	1752	1845	1568
Flt Permitted	0.54	1.00	0.31	1.00	0.40	1.00	0.40	1.00	0.64	1.00
Satd. Flow (perm)	990	3290	565	3505	1568	746	1829	1174	1845	1568
Volume (vph)	24	414	287	4	291	30	156	79	99	212
Peak-hour factor, PHF	0.91	0.91	0.89	0.89	0.79	0.79	0.89	0.89	0.89	0.89
Adj. Flow (vph)	26	455	315	4	327	34	197	100	6	111
RTOR Reduction (vph)	0	72	0	0	0	14	0	2	0	0
Lane Group Flow (vph)	26	698	0	4	327	20	197	104	0	111
Turn Type	pm+pt	7	4	pm+pt	3	8	2	pm	6	pm
Protected Phases	4	4	4	8	8	2	2	6	6	6
Permitted Phases	4	4	4	8	8	2	2	6	6	6
Actuated Green, G (s)	69.2	63.3	63.3	68.6	63.0	63.0	26.1	26.1	26.1	26.1
Effective Green, g (s)	73.2	65.3	65.3	72.6	65.0	65.0	28.1	28.1	28.1	28.1
Actuated g/C Ratio	0.67	0.59	0.59	0.66	0.59	0.59	0.26	0.26	0.26	0.26
Clearance Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	714	1953	455	2071	927	191	467	300	471	401
v/s Ratio Prot	c0.00	c0.21	0.00	0.09	0.01	c0.26	0.06	0.09	0.13	0.00
v/s Ratio Perm	0.02	0.04	0.01	0.01	0.01	0.02	1.03	0.37	0.51	0.01
Uniform Delay, d1	6.3	11.5	6.8	10.2	9.3	40.9	32.3	33.7	35.0	30.6
Progression Factor	0.75	0.65	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.0	0.5	0.0	0.2	0.0	73.6	0.2	0.8	0.9	0.0
Delay (s)	4.7	7.9	6.8	10.3	9.4	114.5	32.6	34.4	35.9	30.6
Level of Service	A	A	A	B	A	F	C	C	D	C
Approach Delay (s)	7.8	A	10.2	B	85.9	F	35.1	D		
Approach LOS	A	A	B	B	F	F	C	D		

Intersection Summary

HCM Average Control Delay: 26.7
 HCM Volume to Capacity ratio: 0.51
 Actuated Cycle Length (s): 110.0
 Intersection Capacity Utilization: 50.4%
 Analysis Period (min): 15
 Critical Lane Group: c

2010 AM Peak BUILD Conditions

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Timings

5: Ladera Dr & Ouray Rd

Terry O. Brown, P.E.

10/30/2007

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	SBT	SBT
Lane Configurations	22	307	23	427	121	283	233	36	131	14
Volume (vph)	pm+pt	7	4	3	8	8	2	6	6	6
Turn Type	pm+pt	7	4	3	8	8	2	6	6	6
Protected Phases	7	4	3	8	8	2	6	6	6	6
Deflector Phases	7	4	3	8	8	2	6	6	6	6
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	10.0	21.0	10.0	21.0	21.0	21.0	21.0	21.0	21.0	21.0
Total Split (s)	20.0	39.0	20.0	39.0	39.0	39.0	39.0	39.0	39.0	39.0
Total Split (%)	16.7%	32.5%	16.7%	32.5%	32.5%	50.8%	50.8%	50.8%	50.8%	50.8%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lead/Lag	Lead	Lag	Lead	Lag	Lag	Lag	Lag	Lag	Lag	Lag
Lead-Lag Optimize?	Min	C-Max	Min	C-Max	Min	C-Max	Min	C-Max	Min	C-Max
Recall Mode	73.9	65.7	74.1	65.8	37.0	37.0	37.0	37.0	37.0	37.0
Act Effct Green (s)	0.62	0.55	0.62	0.55	0.31	0.31	0.31	0.31	0.31	0.31
Actuated g/c Ratio	0.05	0.32	0.07	0.26	0.15	0.86	0.47	0.14	0.26	0.03
Control Delay	8.2	9.0	11.4	16.5	3.6	60.8	34.2	27.0	30.1	9.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	8.2	9.0	11.4	16.5	3.6	60.8	34.2	27.0	30.1	9.6
LOS	A	A	B	B	A	E	C	C	C	A
Approach Delay	9.0	13.5	13.5	13.5	48.6	48.6	27.9	27.9	27.9	C
Approach LOS	A	B	B	B	D	D	C	C	C	C

Intersection Summary

Cycle Length: 120
 Actuated Cycle Length: 120
 Offset: 27 (23%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green
 Natural Cycle: 55
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.86
 Intersection Signal Delay: 23.4
 Intersection Capacity Utilization 52.2%
 Analysis Period (min) 15

Intersection LOS: C
 ICU Level of Service A

Splits and Phases: 5: Ladera Dr & Ouray Rd

61 s	20 s	33 s	33 s	20 s	33 s
61 s	20 s	33 s	33 s	20 s	33 s

2010 PM Peak NOBUILD Conditions

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Case F - full access at Intersection 12

HCM Signalized Intersection Capacity Analysis

5: Ladera Dr & Ouray Rd

Terry O. Brown, P.E.

10/30/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBT
Lane Configurations	1800	1800	1800	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Total Lost time (s)	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00
Lane Util. Factor	1.00	0.94	1.00	0.95	1.00	0.85	1.00	0.99	1.00	0.85	1.00	0.85
Flt Protected	0.95	1.00	0.95	1.00	1.00	0.95	1.00	0.99	1.00	0.85	1.00	0.85
Satd. Flow (prot)	1752	3281	1752	3505	1568	1752	1827	1752	1845	1568	1752	1845
Flt Permitted	0.43	1.00	0.37	1.00	1.00	0.58	1.00	0.41	1.00	1.00	0.41	1.00
Satd. Flow (perm)	793	3281	688	3505	1568	1079	1827	763	1845	1568	763	1845
Volume (vph)	22	307	227	23	427	121	293	233	36	131	14	14
Peak-hour factor, PHF	0.90	0.90	0.87	0.87	0.87	0.87	0.94	0.94	0.89	0.89	0.89	0.89
Adj. Flow (vph)	24	341	252	26	491	139	312	248	17	40	147	16
RTOR Reduction (vph)	0	72	0	0	0	63	0	3	0	0	0	11
Lane Group Flow (vph)	24	521	0	26	491	76	312	262	0	40	147	5
Turn Type	pm+pt	7	4	pm+pt	3	8	2	2	pm	6	6	6
Protected Phases	7	4	pm+pt	3	8	2	2	2	pm	6	6	6
Permitted Phases	4	8	8	8	8	8	8	8	8	8	8	8
Actuated Green, G (s)	69.9	63.7	70.1	63.8	63.8	35.0	35.0	35.0	35.0	35.0	35.0	35.0
Effective Green, g (s)	73.9	65.7	74.1	65.8	65.8	37.0	37.0	37.0	37.0	37.0	37.0	37.0
Actuated g/c Ratio	0.62	0.55	0.62	0.55	0.55	0.31	0.31	0.31	0.31	0.31	0.31	0.31
Clearance Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	554	1796	498	1922	860	333	563	235	569	483	235	569
v/s Ratio Prot	0.00	c0.16	c0.00	0.14	0.05	c0.29	0.14	0.05	c0.29	0.14	0.05	c0.29
v/s Ratio Perm	0.02	0.03	0.03	0.05	0.05	0.09	0.54	0.05	0.09	0.54	0.05	0.09
v/c Ratio	0.04	0.29	0.05	0.26	0.09	0.54	0.47	0.05	0.26	0.09	0.54	0.47
Uniform Delay, d1	9.1	14.6	9.2	14.2	12.9	40.4	33.5	9.2	14.2	12.9	40.4	33.5
Progression Factor	0.73	0.73	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.0	0.0	0.0	0.3	0.2	33.0	0.6	0.0	0.3	0.2	33.0	0.6
Delay (s)	6.7	10.8	9.3	14.6	13.1	73.3	34.1	9.3	14.6	13.1	73.3	34.1
Level of Service	A	B	A	B	B	E	C	A	B	B	E	C
Approach Delay (s)	10.6	B	14.0	B	55.3	E	31.1	10.6	B	14.0	B	55.3
Approach LOS	B	B	B	B	E	E	C	B	B	B	E	C

Intersection Summary

HCM Average Control Delay: 26.3
 HCM Volume to Capacity ratio: 0.49
 Actuated Cycle Length (s): 120.0
 Intersection Capacity Utilization: 52.2%
 Analysis Period (min): 15
 Critical Lane Group: C

2010 PM Peak NOBUILD Conditions

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Case F - full access at Intersection 12

Timings
5: Ladera Dr & Ouray Rd
Terry O. Brown, P.E.
10/30/2007

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	SBR
Lane Configurations	27	400	23	506	121	358	36	131	20
Volume (vph)	pm-pt	7	4	3	8	8	2	6	6
Turn Type	pm-pt	7	4	3	8	8	2	6	6
Protected Phases	4	3	8	8	2	2	6	6	6
Detector Phases	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Initial (s)	10.0	21.0	10.0	21.0	21.0	21.0	21.0	21.0	21.0
Minimum Split (s)	15.0	40.0	15.0	40.0	40.0	65.0	65.0	65.0	65.0
Total Split (%)	12.5%	33.3%	12.5%	33.3%	33.3%	54.2%	54.2%	54.2%	54.2%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lead-Lag Optimizer?	Lead	Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lead
Recall Mode	Min	C-Max	Min	C-Max	Min	C-Max	Min	C-Max	Min
Act Effct Green (s)	67.4	59.0	67.4	59.0	43.6	43.6	43.6	43.6	43.6
Actuated g/C Ratio	0.56	0.49	0.56	0.49	0.36	0.36	0.36	0.36	0.36
v/c Ratio	0.08	0.45	0.10	0.34	0.17	0.89	0.40	0.11	0.22
Control Delay	12.2	15.0	15.2	21.5	4.5	57.5	28.1	22.1	25.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	12.2	15.0	15.2	21.5	4.5	57.5	28.1	22.1	25.0
LOS	B	B	B	C	A	E	C	C	A
Approach Delay	14.9	18.2	18.2	18.2	45.4	45.4	22.5	22.5	C
Approach LOS	B	B	B	B	D	D	C	C	C

Intersection Summary

Cycle Length: 120
Actuated Cycle Length: 120
Offset: 7 (6%), Referenced to phase 4 EBT and 8 WBT, Start of Green
Natural Cycle: 60
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.89
Intersection Signal Delay: 24.8
Intersection Capacity Utilization 59.2%
Analysis Period (min) 15

Splits and Phases: 5: Ladera Dr & Ouray Rd

a2	a3	a4
55 s	15 s	40 s
a5	a6	a7
55 s	15 s	40 s

HCM Signalized Intersection Capacity Analysis

5: Ladera Dr & Ouray Rd
Terry O. Brown, P.E.
10/30/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1900	1900	1800	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Total Lost time (s)	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00
Lane Util. Factor	1.00	0.94	1.00	0.95	1.00	0.95	1.00	0.99	1.00	0.95	1.00	0.85
Flt Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Satd. Flow (prot)	1752	3283	1752	3505	1568	1752	1827	1752	1845	1568	1752	1845
Flt Permitted	0.36	1.00	0.27	1.00	1.00	0.61	1.00	0.46	1.00	1.00	0.46	1.00
Satd. Flow (perm)	669	3283	500	3505	1568	1117	1827	842	1845	1568	842	1845
Volume (vph)	27	400	292	23	506	121	358	233	16	36	131	20
Peak-hour factor, PHF	0.90	0.90	0.90	0.87	0.87	0.87	0.94	0.94	0.89	0.89	0.89	0.89
Adj. Flow (vph)	30	444	324	26	562	139	381	248	17	40	147	22
RTOR Reduction (vph)	0	81	0	0	0	71	0	3	0	0	0	14
Lane Group Flow (vph)	30	687	0	26	582	68	381	262	0	40	147	8
Turn Type	pm-pt	7	4	pm-pt	3	8	2	2	pm	6	6	6
Protected Phases	4	3	8	8	2	2	6	6	41.6	41.6	41.6	41.6
Permitted Phases	63.4	57.0	63.4	57.0	57.0	41.6	41.6	41.6	43.6	43.6	43.6	43.6
Effective Green, G (s)	67.4	59.0	67.4	59.0	59.0	43.6	43.6	43.6	0.36	0.36	0.36	0.36
Actuated g/C Ratio	0.56	0.49	0.56	0.49	0.49	0.36	0.36	0.36	5.0	5.0	5.0	5.0
Clearance Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	3.0	3.0	3.0	3.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	452	1614	368	1723	771	406	864	306	670	570	0.08	0.01
v/s Ratio Prot	0.00	c0.21	c0.00	0.17	0.04	c0.34	0.14	0.05	0.13	0.22	0.01	0.01
v/s Ratio Perm	0.03	0.03	0.03	0.34	0.09	0.84	0.40	0.13	0.22	0.24	0.01	0.01
Uniform Delay, d1	12.1	19.6	12.6	18.6	16.2	36.9	28.4	25.5	26.4	24.4	1.00	1.00
Progression Factor	0.83	0.83	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.0	0.5	0.1	0.5	0.2	29.2	0.4	0.2	0.2	0.2	0.0	0.0
Delay (s)	10.2	16.8	12.7	19.1	16.4	66.1	28.8	25.7	26.6	24.5	C	C
Level of Service	B	B	B	B	B	E	C	C	C	C	C	C
Approach Delay (s)	16.5	B	18.4	B	50.8	D	26.2	26.2	26.2	26.2	C	C
Approach LOS	B	B	B	B	D	D	C	C	C	C	C	C

Intersection Summary

HCM Average Control Delay 27.2 HCM Level of Service C
HCM Volume to Capacity ratio 0.60
Actuated Cycle Length (s) 120.0
Intersection Capacity Utilization 59.2%
Analysis Period (min) 15
c Critical Lane Group

2010 PM Peak BUILD Conditions
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2010 PM Peak BUILD Conditions
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Analysis of Intersection #6

I-40 South ramp / Unser Blvd

HCM Unsignalized Intersection Capacity Analysis
6: I-40 South ramp & Unser Blvd

Terry O. Brown, P.E.
10/30/2007

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations								 			 	
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Volume (veh/h)	50	0	0	0	0	0	0	804	0	0	1329	0
Peak Hour Factor	0.75	0.75	0.75	0.85	0.85	0.85	0.80	0.80	0.80	0.88	0.88	0.88
Hourly flow rate (vph)	67	0	0	0	0	0	0	1005	0	0	1510	0
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		Raised			Raised							
Median storage veh		1			1							
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	2013	2515	755	1760	2515	502	1510			1005		
vC1, stage 1 conf vol	1510	1510		1005	1005							
vC2, stage 2 conf vol	502	1005		755	1510							
vCu, unblocked vol	2013	2515	755	1760	2515	502	1510			1005		
tC, single (s)	7.6	6.6	7.0	7.6	6.6	7.0	4.2			4.2		
tC, 2 stage (s)	6.6	5.6		6.6	5.6							
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	34	100	100	100	100	100	100			100		
cM capacity (veh/h)	101	116	349	161	116	512	434			679		
Direction, Lane #	EB 1	NB 1	NB 2	SB 1	SB 2							
Volume Total	67	502	502	755	755							
Volume Left	67	0	0	0	0							
Volume Right	0	0	0	0	0							
cSH	101	1700	1700	1700	1700							
Volume to Capacity	0.66	0.30	0.30	0.44	0.44							
Queue Length 95th (ft)	83	0	0	0	0							
Control Delay (s)	92.8	0.0	0.0	0.0	0.0							
Lane LOS	F											
Approach Delay (s)	92.8	0.0		0.0								
Approach LOS	F											
Intersection Summary												
Average Delay			2.4									
Intersection Capacity Utilization			72.1%	ICU Level of Service					C			
Analysis Period (min)			15									


















2010 AM Peak NOBUILD Conditions

Case F - full access at Intersection 12

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HCM Unsignalized Intersection Capacity Analysis
6: I-40 South ramp & Unser Blvd

Terry O. Brown, P.E.
10/30/2007

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations								 			 	
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Volume (veh/h)	60	0	0	0	0	0	0	939	0	0	1425	0
Peak Hour Factor	0.75	0.75	0.75	0.85	0.85	0.85	0.80	0.80	0.80	0.88	0.88	0.88
Hourly flow rate (vph)	80	0	0	0	0	0	0	1174	0	0	1619	0
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		Raised			Raised							
Median storage (veh)		1			1							
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	2206	2793	810	1983	2793	587	1619			1174		
vC1, stage 1 conf vol	1619	1619		1174	1174							
vC2, stage 2 conf vol	587	1174		810	1619							
vCu, unblocked vol	2206	2793	810	1983	2793	587	1619			1174		
tC, single (s)	7.6	6.6	7.0	7.6	6.6	7.0	4.2			4.2		
tC, 2 stage (s)	6.6	5.6		6.6	5.6							
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	6	100	100	100	100	100	100			100		
cM capacity (veh/h)	85	98	321	131	98	450	394			585		
Direction, Lane #	EB 1	NB 1	NB 2	SB 1	SB 2							
Volume Total	80	587	587	810	810							
Volume Left	80	0	0	0	0							
Volume Right	0	0	0	0	0							
cSH	85	1700	1700	1700	1700							
Volume to Capacity	0.94	0.35	0.35	0.48	0.48							
Queue Length 95th (ft)	129	0	0	0	0							
Control Delay (s)	167.6	0.0	0.0	0.0	0.0							
Lane LOS	F											
Approach Delay (s)	167.6	0.0		0.0								
Approach LOS	F											
Intersection Summary												
Average Delay			4.7									
Intersection Capacity Utilization			74.9%			ICU Level of Service				D		
Analysis Period (min)			15									


















2010 AM Peak BUILD Conditions

Case F - full access at Intersection 12

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HCM Unsignalized Intersection Capacity Analysis
6: I-40 South ramp & Unser Blvd

Terry O. Brown, P.E.
10/30/2007

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations								 			 	
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Volume (veh/h)	94	0	0	0	0	0	0	1335	0	0	1908	0
Peak Hour Factor	0.90	0.90	0.90	0.85	0.85	0.85	0.92	0.92	0.92	0.97	0.97	0.97
Hourly flow rate (vph)	104	0	0	0	0	0	0	1451	0	0	1967	0
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		Raised			Raised							
Median storage (veh)		1			1							
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	2693	3418	984	2435	3418	726	1967			1451		
vC1, stage 1 conf vol	1967	1967		1451	1451							
vC2, stage 2 conf vol	726	1451		984	1967							
vCu, unblocked vol	2693	3418	984	2435	3418	726	1967			1451		
tC, single (s)	7.6	6.6	7.0	7.6	6.6	7.0	4.2			4.2		
tC, 2 stage (s)	6.6	5.6		6.6	5.6							
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	0	100	100	100	100	100	100			100		
cM capacity (veh/h)	52	65	246	88	65	365	288			458		
Direction, Lane #	EB 1	NB 1	NB 2	SB 1	SB 2							
Volume Total	104	726	726	984	984							
Volume Left	104	0	0	0	0							
Volume Right	0	0	0	0	0							
cSH	52	1700	1700	1700	1700							
Volume to Capacity	2.01	0.43	0.43	0.58	0.58							
Queue Length 95th (ft)	258	0	0	0	0							
Control Delay (s)	637.2	0.0	0.0	0.0	0.0							
Lane LOS	F											
Approach Delay (s)	637.2	0.0		0.0								
Approach LOS	F											
Intersection Summary												
Average Delay			18.9									
Intersection Capacity Utilization			118.6%	ICU Level of Service					H			
Analysis Period (min)			15									


















2010 PM Peak NOBUILD Conditions

Case F - full access at Intersection 12

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HCM Unsignalized Intersection Capacity Analysis
6: I-40 South ramp & Unser Blvd

Terry O. Brown, P.E.
10/30/2007

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations								 			 	
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Volume (veh/h)	105	0	0	0	0	0	0	1489	0	0	2069	0
Peak Hour Factor	0.90	0.90	0.90	0.85	0.85	0.85	0.92	0.92	0.92	0.97	0.97	0.97
Hourly flow rate (vph)	117	0	0	0	0	0	0	1618	0	0	2133	0
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		Raised			Raised							
Median storage (veh)		1			1							
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	2942	3751	1066	2685	3751	809	2133			1618		
vC1, stage 1 conf vol	2133	2133		1618	1618							
vC2, stage 2 conf vol	809	1618		1066	2133							
vCu, unblocked vol	2942	3751	1066	2685	3751	809	2133			1618		
tC, single (s)	7.6	6.6	7.0	7.6	6.6	7.0	4.2			4.2		
tC, 2 stage (s)	6.6	5.6		6.6	5.6							
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	0	100	100	100	100	100	100			100		
cM capacity (veh/h)	41	53	216	71	53	321	247			394		
Direction, Lane #	EB 1	NB 1	NB 2	SB 1	SB 2							
Volume Total	117	809	809	1066	1066							
Volume Left	117	0	0	0	0							
Volume Right	0	0	0	0	0							
cSH	41	1700	1700	1700	1700							
Volume to Capacity	2.85	0.48	0.48	0.63	0.63							
Queue Length 95th (ft)	322	0	0	0	0							
Control Delay (s)	1042.9	0.0	0.0	0.0	0.0							
Lane LOS	F											
Approach Delay (s)	1042.9	0.0		0.0								
Approach LOS	F											
Intersection Summary												
Average Delay		31.5										
Intersection Capacity Utilization		127.9%				ICU Level of Service				H		
Analysis Period (min)		15										





















2010 PM Peak BUILD Conditions

Case F - full access at Intersection 12

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HCM Unsignalized Intersection Capacity Analysis
7: Ladera Dr & Market Rd

Terry O. Brown, P.E.
10/30/2007

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Volume (veh/h)	0	425	28	15	398	0	113	0	72	0	1	0
Peak Hour Factor	0.88	0.88	0.88	0.79	0.79	0.79	0.86	0.86	0.86	0.85	0.85	0.85
Hourly flow rate (vph)	0	483	32	19	504	0	131	0	84	0	1	0
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type												
Median storage (veh)												
Upstream signal (ft)		865										
pX, platoon unblocked												
vC, conflicting volume	504			515			789	1041	257	867	1057	252
vC1, stage 1 conf vol							499	499		542	542	
vC2, stage 2 conf vol							290	542		325	515	
vCu, unblocked vol	504			515			789	1041	257	867	1057	252
tC, single (s)	4.2			4.2			7.6	6.6	7.0	7.6	6.6	7.0
tC, 2 stage (s)							6.6	5.6		6.6	5.6	
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			98			66	100	89	100	100	100
cM capacity (veh/h)	1050			1040			390	340	739	334	332	745
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	WB 3	NB 1	NB 2	SB 1			
Volume Total	0	322	193	19	336	168	131	84	1			
Volume Left	0	0	0	19	0	0	131	0	0			
Volume Right	0	0	32	0	0	0	0	84	0			
cSH	1700	1700	1700	1040	1700	1700	390	739	332			
Volume to Capacity	0.00	0.19	0.11	0.02	0.20	0.10	0.34	0.11	0.00			
Queue Length 95th (ft)	0	0	0	1	0	0	37	10	0			
Control Delay (s)	0.0	0.0	0.0	8.5	0.0	0.0	18.9	10.5	15.9			
Lane LOS				A			C	B	C			
Approach Delay (s)	0.0			0.3			15.6		15.9			
Approach LOS							C		C			
Intersection Summary												
Average Delay			2.8									
Intersection Capacity Utilization			32.2%				ICU Level of Service		A			
Analysis Period (min)			15									

2010 AM Peak NOBUILD Conditions

Case F - full access at Intersection 12





















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Terry O. Brown, P.E.
10/30/2007

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HCM Unsignalized Intersection Capacity Analysis
7: Ladera Dr & Market Rd

Terry O. Brown, P.E.
10/30/2007

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Volume (veh/h)	1	669	176	260	505	1	129	1	146	1	1	1
Peak Hour Factor	0.93	0.93	0.93	0.80	0.80	0.80	0.88	0.88	0.88	0.85	0.85	0.85
Hourly flow rate (vph)	1	719	189	325	631	1	147	1	166	1	1	1
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type												
Median storage (veh)								Raised			Raised	
Upstream signal (ft)		888						1			1	
pX, platoon unblocked												
vC, conflicting volume	632			909			1784	2099	454	1810	2193	316
vC1, stage 1 conf vol							816	816		1282	1282	
vC2, stage 2 conf vol							967	1282		528	911	
vCu, unblocked vol	632			909			1784	2099	454	1810	2193	316
tC, single (s)	4.2			4.2			7.6	6.6	7.0	7.6	6.6	7.0
tC, 2 stage (s)							6.6	5.6		6.6	5.6	
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			56			0	99	70	95	95	100
cM capacity (veh/h)	939			739			106	98	550	22	25	677
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	WB 3	NB 1	NB 2	SB 1			
Volume Total	1	480	429	325	421	212	147	167	4			
Volume Left	1	0	0	325	0	0	147	0	1			
Volume Right	0	0	189	0	0	1	0	166	1			
cSH	939	1700	1700	739	1700	1700	106	533	34			
Volume to Capacity	0.00	0.28	0.25	0.44	0.25	0.12	1.38	0.31	0.10			
Queue Length 95th (ft)	0	0	0	56	0	0	259	33	8			
Control Delay (s)	8.8	0.0	0.0	13.6	0.0	0.0	292.2	14.8	121.3			
Lane LOS	A			B			F	B	F			
Approach Delay (s)	0.0			4.6			144.5		121.3			
Approach LOS							F		F			
Intersection Summary												
Average Delay			23.0									
Intersection Capacity Utilization			62.3%				ICU Level of Service			B		
Analysis Period (min)			15									

2010 PM Peak BUILD Conditions

Case F - full access at Intersection 12





















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Analysis of Intersection #8

Ladera Dr / Laurelwood Parkway

HCM Unsignalized Intersection Capacity Analysis
8: Ladera Dr & Laurelwood Pkwy

Terry O. Brown, P.E.
10/30/2007

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Volume (veh/h)	1	490	28	7	363	0	78	0	65	5	0	8
Peak Hour Factor	0.90	0.90	0.90	0.75	0.75	0.75	0.89	0.89	0.89	0.75	0.75	0.75
Hourly flow rate (vph)	1	544	31	9	484	0	88	0	73	7	0	11
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type												
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	484			576			834	1065	288	850	1080	242
vC1, stage 1 conf vol							562	562		503	503	
vC2, stage 2 conf vol							271	503		347	578	
vCu, unblocked vol	484			576			834	1065	288	850	1080	242
tC, single (s)	4.2			4.2			7.6	6.6	7.0	7.6	6.6	7.0
tC, 2 stage (s)							6.6	5.6		6.6	5.6	
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			99			76	100	90	98	100	99
cM capacity (veh/h)	1068			987			366	335	706	347	329	756
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	WB 3	NB 1	NB 2	NB 3	SB 1		
Volume Total	1	363	213	9	323	161	88	0	73	17		
Volume Left	1	0	0	9	0	0	88	0	0	7		
Volume Right	0	0	31	0	0	0	0	0	73	11		
cSH	1068	1700	1700	987	1700	1700	366	1700	706	520		
Volume to Capacity	0.00	0.21	0.13	0.01	0.19	0.09	0.24	0.00	0.10	0.03		
Queue Length 95th (ft)	0	0	0	1	0	0	23	0	9	3		
Control Delay (s)	8.4	0.0	0.0	8.7	0.0	0.0	17.9	0.0	10.7	12.2		
Lane LOS	A			A			C	A	B	B		
Approach Delay (s)	0.0			0.2			14.6			12.2		
Approach LOS							B			B		
Intersection Summary												
Average Delay			2.1									
Intersection Capacity Utilization			31.8%				ICU Level of Service			A		
Analysis Period (min)			15									






















2010 AM Peak NOBUILD Conditions

Case F - full access at Intersection 12

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HCM Unsignalized Intersection Capacity Analysis
8: Ladera Dr & Laurelwood Pkwy

Terry O. Brown, P.E.
10/30/2007

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Volume (veh/h)	2	602	44	7	526	1	101	1	65	5	1	9
Peak Hour Factor	0.90	0.90	0.90	0.75	0.75	0.75	0.89	0.89	0.89	0.75	0.75	0.75
Hourly flow rate (vph)	2	669	49	9	701	1	113	1	73	7	1	12
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type												
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	703			718			1080	1419	359	1133	1443	351
vC1, stage 1 conf vol							698	698		721	721	
vC2, stage 2 conf vol							382	721		412	722	
vCu, unblocked vol	703			718			1080	1419	359	1133	1443	351
tC, single (s)	4.2			4.2			7.6	6.6	7.0	7.6	6.6	7.0
tC, 2 stage (s)							6.6	5.6		6.6	5.6	
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			99			60	100	88	97	99	98
cM capacity (veh/h)	884			872			286	256	635	258	250	642
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	WB 3	NB 1	NB 2	NB 3	SB 1		
Volume Total	2	446	272	9	468	235	113	1	73	20		
Volume Left	2	0	0	9	0	0	113	0	0	7		
Volume Right	0	0	49	0	0	1	0	0	73	12		
cSH	884	1700	1700	872	1700	1700	286	256	635	401		
Volume to Capacity	0.00	0.26	0.16	0.01	0.28	0.14	0.40	0.00	0.12	0.05		
Queue Length 95th (ft)	0	0	0	1	0	0	45	0	10	4		
Control Delay (s)	9.1	0.0	0.0	9.2	0.0	0.0	25.6	19.1	11.4	14.4		
Lane LOS	A			A			D	C	B	B		
Approach Delay (s)	0.0			0.1			20.0			14.4		
Approach LOS							C			B		
Intersection Summary												
Average Delay			2.5									
Intersection Capacity Utilization			37.0%				ICU Level of Service			A		
Analysis Period (min)			15									






















2010 AM Peak BUILD Conditions

Case F - full access at Intersection 12

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HCM Unsignalized Intersection Capacity Analysis
8: Ladera Dr & Laurelwood Pkwy

Terry O. Brown, P.E.
10/30/2007

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Volume (veh/h)	10	481	98	82	589	10	37	1	34	1	1	3
Peak Hour Factor	0.89	0.89	0.89	0.91	0.91	0.91	0.85	0.85	0.85	0.75	0.75	0.75
Hourly flow rate (vph)	11	540	110	90	647	11	44	1	40	1	1	4
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type												
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	658			651			1126	1456	325	1166	1506	329
vC1, stage 1 conf vol							618	618		833	833	
vC2, stage 2 conf vol							509	838		333	673	
vCu, unblocked vol	658			651			1126	1456	325	1166	1506	329
tC, single (s)	4.2			4.2			7.6	6.6	7.0	7.6	6.6	7.0
tC, 2 stage (s)							6.6	5.6		6.6	5.6	
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	99			90			84	99	94	99	99	99
cM capacity (veh/h)	919			925			267	226	667	223	207	664
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	WB 3	NB 1	NB 2	NB 3	SB 1		
Volume Total	11	360	290	90	432	227	44	1	40	7		
Volume Left	11	0	0	90	0	0	44	0	0	1		
Volume Right	0	0	110	0	0	11	0	0	40	4		
cSH	919	1700	1700	925	1700	1700	267	226	667	361		
Volume to Capacity	0.01	0.21	0.17	0.10	0.25	0.13	0.16	0.01	0.06	0.02		
Queue Length 95th (ft)	1	0	0	8	0	0	14	0	5	1		
Control Delay (s)	9.0	0.0	0.0	9.3	0.0	0.0	21.1	21.0	10.7	15.1		
Lane LOS	A			A			C	C	B	C		
Approach Delay (s)	0.2			1.1			16.2			15.1		
Approach LOS							C			C		
Intersection Summary												
Average Delay			1.6									
Intersection Capacity Utilization			38.7%			ICU Level of Service				A		
Analysis Period (min)			15									

2010 PM Peak NOBUILD Conditions

Case F - full access at Intersection 12

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HCM Unsignalized Intersection Capacity Analysis

8: Ladera Dr & Laurelwood Pkwy

Terry O. Brown, P.E.
10/30/2007

[illegible]

Analysis of Intersection #9
Ladera Dr / Driveway 'A'

HCM Unsignalized Intersection Capacity Analysis
9: Ladera Dr & 'A'

Terry O. Brown, P.E.
10/30/2007

	→	↘	↙	←	↖	↗
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑			↑↑		↑
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Volume (veh/h)	526	33	0	493	0	65
Peak Hour Factor	0.79	0.79	0.79	0.79	0.85	0.85
Hourly flow rate (vph)	666	42	0	624	0	76
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type					None	
Median storage (veh)						
Upstream signal (ft)	481					
pX, platoon unblocked						
vC, conflicting volume			708		999	354
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			708		999	354
tC, single (s)			4.2		6.9	7.0
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			100		100	88
cM capacity (veh/h)			880		238	640
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	NB 1	
Volume Total	444	264	312	312	76	
Volume Left	0	0	0	0	0	
Volume Right	0	42	0	0	76	
cSH	1700	1700	1700	1700	640	
Volume to Capacity	0.26	0.16	0.18	0.18	0.12	
Queue Length 95th (ft)	0	0	0	0	10	
Control Delay (s)	0.0	0.0	0.0	0.0	11.4	
Lane LOS					B	
Approach Delay (s)	0.0		0.0		11.4	
Approach LOS					B	
Intersection Summary						
Average Delay			0.6			
Intersection Capacity Utilization			26.3%		ICU Level of Service	A
Analysis Period (min)			15			

2010 AM Peak BUILD Conditions

Case F - full access at Intersection 12

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HCM Unsignalized Intersection Capacity Analysis
9: Ladera Dr & 'A'

Terry O. Brown, P.E.
10/30/2007

	→	↘	↙	←	↖	↗
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑			↑↑		↗
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Volume (veh/h)	664	85	0	757	0	111
Peak Hour Factor	0.93	0.93	0.93	0.93	0.85	0.85
Hourly flow rate (vph)	714	91	0	814	0	131
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type					None	
Median storage (veh)						
Upstream signal (ft)	481					
pX, platoon unblocked						
vC, conflicting volume			805		1167	403
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			805		1167	403
tC, single (s)			4.2		6.9	7.0
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			100		100	78
cM capacity (veh/h)			808		185	594
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	NB 1	
Volume Total	476	329	407	407	131	
Volume Left	0	0	0	0	0	
Volume Right	0	91	0	0	131	
cSH	1700	1700	1700	1700	594	
Volume to Capacity	0.28	0.19	0.24	0.24	0.22	
Queue Length 95th (ft)	0	0	0	0	21	
Control Delay (s)	0.0	0.0	0.0	0.0	12.8	
Lane LOS					B	
Approach Delay (s)	0.0		0.0		12.8	
Approach LOS					B	
Intersection Summary						
Average Delay			1.0			
Intersection Capacity Utilization			34.6%		ICU Level of Service	A
Analysis Period (min)			15			

2010 PM Peak BUILD Conditions










Case F - full access at Intersection 12

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Analysis of Intersection #10
Driveway 'B' / Market Rd

HCM Unsignalized Intersection Capacity Analysis
10: 'B' & Market Rd

Terry O. Brown, P.E.
10/30/2007

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Volume (veh/h)	88	3	5	189	48	187
Peak Hour Factor	0.85	0.85	0.85	0.85	0.86	0.86
Hourly flow rate (vph)	104	4	6	222	56	217
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None					
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	399	165	273			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	399	165	273			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	83	100	100			
cM capacity (veh/h)	602	877	1284			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	107	228	273			
Volume Left	104	6	0			
Volume Right	4	0	217			
cSH	608	1284	1700			
Volume to Capacity	0.18	0.00	0.16			
Queue Length 95th (ft)	16	0	0			
Control Delay (s)	12.2	0.2	0.0			
Lane LOS	B	A				
Approach Delay (s)	12.2	0.2	0.0			
Approach LOS	B					
Intersection Summary						
Average Delay		2.2				
Intersection Capacity Utilization		25.8%	ICU Level of Service	A		
Analysis Period (min)		15				










2010 AM Peak BUILD Conditions

Case F - full access at Intersection 12

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HCM Unsignalized Intersection Capacity Analysis
10: 'B' & Market Rd

Terry O. Brown, P.E.
10/30/2007

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Volume (veh/h)	150	5	6	126	210	209
Peak Hour Factor	0.85	0.85	0.88	0.88	0.88	0.88
Hourly flow rate (vph)	176	6	7	143	239	238
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None					
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	514	357	476			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	514	357	476			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	66	99	99			
cM capacity (veh/h)	515	685	1081			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	182	150	476			
Volume Left	176	7	0			
Volume Right	6	0	238			
cSH	519	1081	1700			
Volume to Capacity	0.35	0.01	0.28			
Queue Length 95th (ft)	39	0	0			
Control Delay (s)	15.6	0.4	0.0			
Lane LOS	C	A				
Approach Delay (s)	15.6	0.4	0.0			
Approach LOS	C					
Intersection Summary						
Average Delay		3.6				
Intersection Capacity Utilization		39.1%		ICU Level of Service	A	
Analysis Period (min)		15				

2010 PM Peak BUILD Conditions

Case F - full access at Intersection 12










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Analysis of Intersection #11

Hanover Rd / Driveway 'C'

HCM Unsignalized Intersection Capacity Analysis 11: Hanover Rd & 'C'

Terry O. Brown, P.E.
10/30/2007

						
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Volume (veh/h)	0	0	0	11	7	0
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85
Hourly flow rate (vph)	0	0	0	13	8	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type					None	
Median storage veh						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	13				6	6
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	13				6	6
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	100				99	100
cM capacity (veh/h)	1599				1012	1073
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	0	13	8			
Volume Left	0	0	8			
Volume Right	0	13	0			
cSH	1700	1700	1012			
Volume to Capacity	0.00	0.01	0.01			
Queue Length 95th (ft)	0	0	1			
Control Delay (s)	0.0	0.0	8.6			
Lane LOS			A			
Approach Delay (s)	0.0	0.0	8.6			
Approach LOS			A			
Intersection Summary						
Average Delay			3.3			
Intersection Capacity Utilization			13.3%	ICU Level of Service	A	
Analysis Period (min)			15			

2010 AM Peak BUILD Conditions

Case F - full access at Intersection 12

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HCM Unsignalized Intersection Capacity Analysis
11: Hanover Rd & 'C'

Terry O. Brown, P.E.
10/30/2007

	EBL	EBT	WBT	WBR	SBL	SBR
Movement						
Lane Configurations		↰	↱		↰	↱
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Volume (veh/h)	0	0	0	11	12	0
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85
Hourly flow rate (vph)	0	0	0	13	14	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type					None	
Median storage veh						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	13				6	6
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	13				6	6
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	100				99	100
cM capacity (veh/h)	1599				1012	1073
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	0	13	14			
Volume Left	0	0	14			
Volume Right	0	13	0			
cSH	1700	1700	1012			
Volume to Capacity	0.00	0.01	0.01			
Queue Length 95th (ft)	0	0	1			
Control Delay (s)	0.0	0.0	8.6			
Lane LOS			A			
Approach Delay (s)	0.0	0.0	8.6			
Approach LOS			A			
Intersection Summary						
Average Delay		4.5				
Intersection Capacity Utilization		13.3%		ICU Level of Service		A
Analysis Period (min)		15				

2010 PM Peak BUILD Conditions

Case F - full access at Intersection 12















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Analysis of Intersection #12

Unser Blvd / Driveway 'D'

HCM Unsignalized Intersection Capacity Analysis
12: 'D' & Unser Blvd

Terry O. Brown, P.E.
10/30/2007

								
Movement	WBL	WBR	NBT	NBR	SBL	SBT		
Lane Configurations			 			 		
Sign Control	Stop		Free			Free		
Grade	0%		0%			0%		
Volume (veh/h)	144	80	797	216	116	1806		
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85		
Hourly flow rate (vph)	169	94	938	254	136	2125		
Pedestrians								
Lane Width (ft)								
Walking Speed (ft/s)								
Percent Blockage								
Right turn flare (veh)								
Median type	Raised							
Median storage veh	1							
Upstream signal (ft)	649							
pX, platoon unblocked	0.62							
vC, conflicting volume	2273	469			1192			
vC1, stage 1 conf vol	938							
vC2, stage 2 conf vol	1335							
vCu, unblocked vol	2440	469			1192			
tC, single (s)	6.9	7.0			4.2			
tC, 2 stage (s)	5.9							
tF (s)	3.5	3.3			2.2			
p0 queue free %	0	83			76			
cM capacity (veh/h)	105	538			576			
Direction, Lane #	WB 1	WB 2	NB 1	NB 2	NB 3	SB 1	SB 2	SB 3
Volume Total	169	94	469	469	254	136	1062	1062
Volume Left	169	0	0	0	0	136	0	0
Volume Right	0	94	0	0	254	0	0	0
cSH	105	538	1700	1700	1700	576	1700	1700
Volume to Capacity	1.61	0.17	0.28	0.28	0.15	0.24	0.62	0.62
Queue Length 95th (ft)	324	16	0	0	0	23	0	0
Control Delay (s)	387.2	13.1	0.0	0.0	0.0	13.2	0.0	0.0
Lane LOS	F	B				B		
Approach Delay (s)	253.6		0.0			0.8		
Approach LOS	F							
Intersection Summary								
Average Delay			18.5					
Intersection Capacity Utilization			64.6%	ICU Level of Service		C		
Analysis Period (min)			15					















2010 AM Peak BUILD Conditions

Case F - full access at Intersection 12

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HCM Unsignalized Intersection Capacity Analysis
12: 'D' & Unser Blvd

Terry O. Brown, P.E.
10/30/2007

								
Movement	WBL	WBR	NBT	NBR	SBL	SBT		
Lane Configurations			 			 		
Sign Control	Stop		Free			Free		
Grade	0%		0%			0%		
Volume (veh/h)	340	242	1613	327	204	1011		
Peak Hour Factor	0.85	0.85	0.95	0.95	0.95	0.95		
Hourly flow rate (vph)	400	285	1698	344	215	1064		
Pedestrians								
Lane Width (ft)								
Walking Speed (ft/s)								
Percent Blockage								
Right turn flare (veh)								
Median type	Raised							
Median storage (veh)	1							
Upstream signal (ft)						649		
pX, platoon unblocked								
vC, conflicting volume	2659	849			2042			
vC1, stage 1 conf vol	1698							
vC2, stage 2 conf vol	962							
vCu, unblocked vol	2659	849			2042			
tC, single (s)	6.9	7.0			4.2			
tC, 2 stage (s)	5.9							
tF (s)	3.5	3.3			2.2			
p0 queue free %	0	6			20			
cM capacity (veh/h)	42	302			269			
Direction, Lane #	WB 1	WB 2	NB 1	NB 2	NB 3	SB 1	SB 2	SB 3
Volume Total	400	285	849	849	344	215	532	532
Volume Left	400	0	0	0	0	215	0	0
Volume Right	0	285	0	0	344	0	0	0
cSH	42	302	1700	1700	1700	269	1700	1700
Volume to Capacity	9.60	0.94	0.50	0.50	0.20	0.80	0.31	0.31
Queue Length 95th (ft)	Err	232	0	0	0	155	0	0
Control Delay (s)	Err	76.0	0.0	0.0	0.0	56.0	0.0	0.0
Lane LOS	F	F				F		
Approach Delay (s)	5873.0		0.0			9.4		
Approach LOS	F							
Intersection Summary								
Average Delay		1006.9						
Intersection Capacity Utilization		84.7%			ICU Level of Service		E	
Analysis Period (min)		15						

2010 PM Peak BUILD Conditions

Case F - full access at Intersection 12

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Timings
12: 'D' & Unser Blvd

Terry O. Brown, P.E.
10/31/2007

	WBL	WBR	NBT	NBR	SBL	SBT
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	144	80	797	216	116	1806
Volume (vph)	144	80	797	216	116	1806
Turn Type	pm+ov	pm+ov	pm+ov	pm+ov	pm+ov	pm+ov
Protected Phases	8	1	2	8	1	6
Permitted Phases	8	1	2	8	1	6
Detector Phases	8	1	2	8	1	6
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	21.0	10.0	21.0	21.0	10.0	21.0
Total Split (s)	15.0	15.0	80.0	15.0	15.0	95.0
Total Split (%)	13.6%	13.6%	72.7%	13.6%	13.6%	86.4%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0
Lead/Lag	Lead	Lag	Lead	Lag	Lead	Lag
Lead-Lag Optimize?						
Recall Mode	Min	C-Max	Min	C-Max	Min	C-Max
Act Eff Green (s)	11.4	23.0	81.0	95.4	92.6	92.6
Actuated g/C Ratio	0.10	0.21	0.74	0.87	0.84	0.84
v/c Ratio	0.48	0.23	0.25	0.18	0.27	0.50
Control Delay	51.1	8.8	8.7	0.8	1.8	1.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	51.1	8.8	8.7	0.8	1.8	1.9
LOS	D	A	A	A	A	A
Approach Delay	36.0	A	7.0	A	A	1.9
Approach LOS	D	A	A	A	A	A
Intersection Summary						
Cycle Length: 110						
Actuated Cycle Length: 110						
Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBTL, Start of Green						
Natural Cycle: 55						
Control Type: Actuated-Coordinated						
Maximum v/c Ratio: 0.50						
Intersection Signal Delay: 5.9						
Intersection Capacity Utilization 45.7%						
Analysis Period (min) 15						
Intersection LOS: A						
ICU Level of Service A						
Splits and Phases: 12: 'D' & Unser Blvd						

HCM Signalized Intersection Capacity Analysis
12: 'D' & Unser Blvd

Terry O. Brown, P.E.
10/31/2007

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↔	↔	↔	↔	↔	↔
Ideal Flow (vthpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	3.0	3.0	3.0	3.0	3.0	3.0
Lane Util. Factor	0.97	1.00	0.91	1.00	1.00	0.91
Frt	1.00	0.85	1.00	0.85	1.00	1.00
Flt Protected	0.95	1.00	1.00	1.00	0.95	1.00
Satd. Flow (prot)	3400	1568	5036	1568	1752	5036
Flt Permitted	0.95	1.00	1.00	1.00	0.27	1.00
Satd. Flow (perm)	3400	1568	5036	1568	496	5036
Volume (vph)	144	80	797	216	116	1806
Peak-hour factor, PHF	0.85	0.85	0.85	0.85	0.85	0.85
Adj. Flow (vph)	169	94	938	254	136	2125
RTOR Reduction (vph)	0	77	0	41	0	0
Lane Group Flow (vph)	169	17	938	213	136	2125
Turn Type	pm+ov pm+pt					
Protected Phases	8	1	2	8	1	6
Permitted Phases	2 6					
Actuated Green, G (s)	9.4	16.0	79.0	88.4	90.6	90.6
Effective Green, g (s)	11.4	20.0	81.0	92.4	92.6	92.6
Actuated g/C Ratio	0.10	0.18	0.74	0.84	0.84	0.84
Clearance Time (s)	5.0	5.0	5.0	5.0	5.0	5.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	352	328	3708	1360	516	4239
v/s Ratio Prot	c0.05	0.00	0.19	0.02	0.02	c0.42
v/s Ratio Perm	0.01	0.01	0.12	0.20	0.20	0.50
v/c Ratio	0.48	0.05	0.25	0.16	0.26	0.50
Uniform Delay, d1	46.5	37.2	4.7	1.6	1.8	2.4
Progression Factor	1.00	1.00	1.77	3.59	0.87	0.71
Incremental Delay, d2	1.0	0.1	0.2	0.1	0.1	0.1
Delay (s)	47.5	37.2	8.5	5.9	1.6	1.8
Level of Service	D	D	A	A	A	A
Approach Delay (s)	43.9	7.9	A	A	1.8	A
Approach LOS	D	D	A	A	A	A
Intersection Summary						
HCM Average Control Delay	6.7					
HCM Volume to Capacity ratio	0.50					
Actuated Cycle Length (s)	110.0					
Intersection Capacity Utilization	45.7%					
Analysis Period (min)	15					
c Critical Lane Group						
	HCM Level of Service A					
	Sum of lost time (s) 6.0					
	ICU Level of Service A					

2010 AM Peak BUILD Conditions - MITIGATED Case F - full access at Intersection 12
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2010 AM Peak BUILD Conditions - MITIGATED Case F - full access at Intersection 12
D:\ATOBEP\PROJECTS\Heritage_Neighborhood_Marketplace_Ladera_Unser\CaseF2010AB_Mit.s77

Determination of Warrants for Auxiliary Lanes

Project Name: **Heritage Neighborhood Center**
 Name of Highway: **Unser Blvd**
 Name of Cross Street: **Driveway 'D'**

Determination of Warrants for: Westbound Driveway

Implementation Year Volumes - 2010 Posted Speed Limit: 45

Right Turn Deceleration Lane - Implementation Year Volumes

Condition	Year	Projected Right Turn Volume	Warrant Volume In thru Lane	Projected Volume In thru Lane	✓ If Met	Lane Length (Deceleration)*	Adjustment Factor for Grade**	Lane Length (Storage)***	Total Lane Length	Taper Ratio
AM Peak Hour NO BUILD	2010	-	-	399		N/A		-	N/A	N/A
AM Peak Hour BUILD	2010	216	1	399	✓	400	1.00	-	400	12.5:1
PM Peak Hour NO BUILD	2010	-	-	855		N/A		-	N/A	N/A
PM Peak Hour BUILD	2010	327	1	807	✓	400	1.00	-	400	12.5:1

Based on Table 17.B-2 (Criteria for Deceleration Lanes on Urban Multi-Lane Highways)

Left Turn Deceleration Lane - Implementation Year Volumes

Condition	Year	Projected Left Turn Volume	Warrant Volume In thru Lane	Projected Volume In thru Lane	✓ If Met	Lane Length (Deceleration)*	Adjustment Factor for Grade**	Lane Length (Storage)***	Total Lane Length	Taper Ratio
AM Peak Hour NO BUILD	2010	-	-	903		N/A		N/A	N/A	N/A
AM Peak Hour BUILD	2010	116	1	903	✓	400	1.00	150	550	12.5:1
PM Peak Hour NO BUILD	2010	-	-	543		N/A		N/A	N/A	N/A
PM Peak Hour BUILD	2010	204	1	506	✓	400	1.00	250	650	12.5:1

Based on Table 17.B-2 (Criteria for Deceleration Lanes on Urban Multi-Lane Highways)

* Lane Length Requirements based on Table 18.K-1 (Deceleration and Acceleration Lengths)

** Enter Grade Adjustment Factor from Table 18.K-2 or other criteria.

*** Lane Storage Length Is Based on a calculated 3-minute queue based on average arrival rate per minute.

= Volume/Hr. divided by 60 times three (rounded) times 25 feet per vehicle.

Lane Storage Length for right turn decel lanes is zero unless there is a stop condition.

Notes and Comments:

1. This warrant sheet is for the westbound Driveway 'D' at 100% Development of the Project

Turning Movement Count Data

Date: 6/13/2007
E-W Street: 98th St

Day: Wednesday
N-S Street: Unser Blvd

AM Period

Time	Eastbound					Westbound					Northbound					Southbound					Sum	Hourly Total
	LT	TH	RT	Truck	Sum	LT	TH	RT	Truck	Sum	LT	TH	RT	Truck	Sum	LT	TH	RT	Truck	Sum		
7:00-7:15	8	2	51	0	61	7	0	2	0	9	9	111	3	4	123	3	169	2	0	174	367	1714
7:15-7:30	7	0	62	0	69	2	0	0	0	2	7	161	2	1	170	0	215	2	0	217	458	
7:30-7:45	17	0	55	0	72	5	0	1	0	6	6	163	4	3	173	3	209	4	0	216	467	
7:45-8:00	6	0	42	0	48	1	0	1	0	2	5	160	3	5	168	5	191	8	0	204	422	
8:00-8:15	4	3	36	0	43	6	0	2	0	8	10	147	6	2	163	2	168	7	0	177	391	
8:15-8:30	10	0	35	0	45	6	0	3	0	9	14	121	4	3	139	4	131	2	0	137	330	
8:30-8:45	9	1	40	0	50	11	0	4	0	15	8	126	4	8	138	3	161	3	0	167	370	
8:45-9:00	2	1	28	1	31	5	1	3	0	9	19	99	3	3	121	3	136	3	0	142	303	
9:00-9:15	10	1	23	1	34	4	1	1	0	6	17	71	3	4	91	2	120	2	0	124	255	
9:15-9:30	8	1	27	0	36	6	0	1	0	7	12	69	6	1	87	2	118	6	0	126	256	
9:30-9:45	7	0	26	0	33	7	2	3	0	12	22	96	7	1	125	3	94	4	0	101	271	
9:45-10:00	10	2	24	2	36	4	0	2	0	6	15	93	2	1	110	3	102	8	0	113	265	
Peak Hour	34	3	195	0	232	14	0	4	0	18	28	631	15	11	674	10	783	21	0	814	1738	Peak
PHF	0.50	0.25	0.79		0.81	0.58	#####	0.50		0.56	0.70	0.97	0.63		0.97	0.50	0.91	0.66		0.94	0.93	7:15
Truck %				0%					0%					2%					0%		1%	8:15

Mid-Day Period

Time	Eastbound					Westbound					Northbound					Southbound					Sum	Hourly Total
	LT	TH	RT	Truck	Sum	LT	TH	RT	Truck	Sum	LT	TH	RT	Truck	Sum	LT	TH	RT	Truck	Sum		
11:00-11:15	9	0	22	0	31	4	0	4	0	8	16	74	8	3	98	3	99	7	0	109	246	1048
11:15-11:30	6	0	30	0	36	15	0	2	0	17	20	100	8	2	128	6	84	6	0	96	277	
11:30-11:45	8	1	30	0	39	13	0	3	0	16	12	106	7	3	125	5	105	4	0	114	294	
11:45-12:00	2	0	21	0	23	12	1	3	0	16	14	81	10	2	105	3	78	6	0	87	231	
12:00-12:15	4	1	20	1	25	11	0	4	0	15	24	96	13	4	133	5	86	5	0	96	269	
12:15-12:30	7	2	22	0	31	16	2	8	0	26	33	116	4	2	153	7	120	8	0	135	345	
12:30-12:45	8	1	26	0	35	8	2	4	0	14	24	98	7	1	129	1	78	7	0	86	264	
12:45-1:00	10	0	23	1	33	3	0	11	0	14	20	100	11	0	131	5	85	4	0	94	272	
1:00-1:15	10	2	28	0	40	12	0	9	0	21	24	77	7	2	108	10	96	9	0	115	284	
1:15-1:30	5	1	22	0	28	15	2	6	0	23	23	99	5	1	127	4	85	5	0	94	272	
1:30-1:45	8	1	17	0	26	8	1	4	0	13	23	102	4	4	129	4	89	9	0	102	270	
1:45-2:00	6	1	11	0	18	7	2	1	0	10	22	100	11	2	133	4	93	3	0	100	261	
Peak Hour	35	5	99	1	139	39	4	32	0	75	101	391	29	5	521	23	379	28	0	430	1165	Peak
PHF	0.88	0.63	0.88		0.87	0.61	0.50	0.73		0.72	0.77	0.84	0.66		0.85	0.58	0.79	0.78		0.80	0.84	12:15
Truck %				1%					0%					1%					0%		1%	1:15

PM Period

Time	Eastbound					Westbound					Northbound					Southbound					Sum	Hourly Total
	LT	TH	RT	Truck	Sum	LT	TH	RT	Truck	Sum	LT	TH	RT	Truck	Sum	LT	TH	RT	Truck	Sum		
3:00-3:15	2	1	11	0	14	13	0	5	0	18	27	116	8	6	151	9	122	5	0	136	319	1557
3:15-3:30	11	1	14	0	26	5	0	8	0	13	30	148	14	3	192	5	129	12	0	146	377	
3:30-3:45	6	1	28	0	35	10	0	5	0	15	23	170	6	1	199	4	154	4	0	162	411	
3:45-4:00	12	2	22	0	36	11	1	7	0	19	42	186	10	0	238	2	147	8	0	157	450	
4:00-4:15	8	0	26	0	34	10	2	5	0	17	38	161	7	0	206	5	182	11	0	198	455	
4:15-4:30	3	3	13	0	19	18	4	7	0	29	45	203	10	2	258	9	168	11	0	188	494	
4:30-4:45	6	0	33	0	39	6	3	8	0	17	40	201	6	0	247	6	158	13	0	177	480	
4:45-5:00	4	0	20	0	24	7	1	11	0	19	45	172	17	1	234	3	183	8	0	194	471	
5:00-5:15	8	1	17	0	26	22	1	8	0	31	48	216	8	1	272	6	180	13	0	199	528	
5:15-5:30	7	0	17	0	24	11	0	0	0	11	45	229	3	0	277	10	179	5	0	194	506	
5:30-5:45	8	0	20	0	28	20	0	11	0	31	34	206	6	0	246	6	219	15	0	240	545	
5:45-6:00	8	1	14	0	23	12	0	5	0	17	52	189	5	1	246	6	192	14	0	212	498	
Peak Hour	31	2	68	0	101	65	1	24	0	90	179	840	22	2	1041	28	770	47	0	845	2077	Peak
PHF	0.97	0.50	0.85		0.90	0.74	0.25	0.55		0.73	0.86	0.92	0.69		0.94	0.70	0.88	0.78		0.88	0.95	5:00
Truck %				0%					0%					0%					0%		0%	6:00

Turning Movement Count Data

Date: 6/12/2007
E-W Street: Ladera Dr

Day: Tuesday
N-S Street: Unser Blvd

AM Period

Time	Eastbound					Westbound					Northbound					Southbound					Sum	Hourly Total
	LT	TH	RT	Truck	Sum	LT	TH	RT	Truck	Sum	LT	TH	RT	Truck	Sum	LT	TH	RT	Truck	Sum		
7:00-7:15	46	52	88	3	186	68	16	10	0	94	10	88	42	2	140	9	237	17	1	263	683	
7:15-7:30	44	66	91	1	201	75	15	6	1	96	6	104	49	2	159	10	263	11	2	284	740	
7:30-7:45	44	77	106	0	227	76	43	9	0	128	18	120	69	1	207	17	227	15	0	259	821	
7:45-8:00	41	56	80	2	177	98	31	19	1	148	14	117	64	4	195	9	179	15	0	203	723	2967
8:00-8:15	26	35	80	1	141	61	16	27	1	104	17	109	49	4	175	22	198	10	0	230	650	2934
8:15-8:30	32	49	80	1	161	86	14	8	1	108	17	100	55	3	172	22	179	12	2	213	654	2848
8:30-8:45	33	38	65	2	136	61	19	6	1	86	17	79	46	1	142	11	177	13	5	201	565	2592
8:45-9:00	28	31	63	2	122	45	23	10	1	78	17	76	33	3	126	13	149	14	4	176	502	2371
9:00-9:15	25	24	37	2	88	42	19	7	1	68	14	72	28	1	115	14	128	17	2	159	430	2151
9:15-9:30	21	31	37	0	89	41	15	5	0	61	19	71	36	3	129	8	105	14	2	127	406	1903
9:30-9:45	26	38	31	2	97	40	26	9	3	75	16	88	44	3	151	12	126	17	7	155	478	1816
9:45-10:00	29	62	30	1	122	47	19	9	1	75	18	72	38	1	128	14	100	17	0	131	456	1770
Peak Hour	175	251	365	6	791	317	105	44	2	466	48	429	224	9	701	45	906	58	3	1009	2967	Peak
PHF	0.95	0.81	0.86		0.87	0.81	0.61	0.58		0.79	0.67	0.89	0.81		0.85	0.66	0.86	0.85		0.89	0.90	7:00
Truck %				1%					0%					1%					0%		1%	8:00

Mid-Day Period

Time	Eastbound					Westbound					Northbound					Southbound					Sum	Hourly Total
	LT	TH	RT	Truck	Sum	LT	TH	RT	Truck	Sum	LT	TH	RT	Truck	Sum	LT	TH	RT	Truck	Sum		
11:00-11:15	26	30	32	0	88	43	19	12	0	74	28	80	41	3	149	15	75	16	2	106	417	
11:15-11:30	29	24	29	1	82	41	27	16	3	84	34	73	47	3	154	14	86	14	2	114	434	
11:30-11:45	22	32	29	0	83	38	22	8	1	68	18	74	49	1	141	21	76	18	0	115	407	
11:45-12:00	17	38	24	0	79	46	27	6	2	79	14	71	53	2	138	16	103	19	0	138	434	1692
12:00-12:15	22	26	15	0	63	37	34	8	2	79	53	97	62	2	212	22	98	16	2	136	490	1765
12:15-12:30	17	31	37	0	85	48	43	10	0	101	31	98	58	4	187	13	99	21	1	133	506	1837
12:30-12:45	25	28	31	0	84	39	40	12	0	91	30	81	48	3	159	8	98	18	2	124	458	1888
12:45-1:00	27	29	32	0	88	50	33	7	0	90	31	83	42	2	156	15	98	20	5	133	467	1921
1:00-1:15	26	26	17	1	69	46	26	18	0	90	28	95	46	1	169	16	80	17	2	113	441	1872
1:15-1:30	20	38	24	0	82	53	29	7	1	89	31	92	46	4	169	16	100	15	0	131	471	1837
1:30-1:45	18	39	26	0	83	30	29	5	0	64	27	69	58	0	154	17	91	19	3	127	428	1807
1:45-2:00	15	32	16	1	63	58	25	3	1	86	24	76	71	1	171	16	90	21	0	127	447	1787
Peak Hour	91	114	115	0	320	174	150	37	2	361	145	359	210	11	714	58	393	75	10	528	1921	Peak
PHF	0.84	0.92	0.78		0.91	0.87	0.87	0.77		0.89	0.68	0.92	0.85		0.84	0.66	0.99	0.89		0.97	0.95	12:00
Truck %				0%					1%					2%					2%		1%	1:00

PM Period

Time	Eastbound					Westbound					Northbound					Southbound					Sum	Hourly Total
	LT	TH	RT	Truck	Sum	LT	TH	RT	Truck	Sum	LT	TH	RT	Truck	Sum	LT	TH	RT	Truck	Sum		
3:00-3:15	27	21	32	0	80	49	36	12	1	97	38	116	60	1	214	10	97	30	1	137	528	
3:15-3:30	29	24	28	0	81	43	42	12	3	97	43	123	59	0	225	16	117	32	0	165	568	
3:30-3:45	29	47	33	0	109	57	44	16	1	117	35	174	99	1	308	16	160	22	0	198	732	
3:45-4:00	21	26	17	0	64	50	45	22	0	117	64	154	82	1	300	14	151	35	0	200	681	2509
4:00-4:15	29	38	22	0	89	53	36	21	3	110	56	180	104	2	340	36	145	41	0	222	761	2742
4:15-4:30	20	42	16	0	78	59	62	21	2	142	59	172	67	1	298	20	144	50	0	214	732	2906
4:30-4:45	26	44	32	0	102	61	56	16	1	133	70	209	86	0	365	21	144	36	3	201	801	2975
4:45-5:00	33	45	31	1	109	56	58	16	0	130	65	213	85	4	363	13	128	52	0	193	795	3089
5:00-5:15	32	36	34	0	102	78	58	19	0	155	73	238	89	3	400	33	149	33	0	215	872	3200
5:15-5:30	36	52	33	0	121	83	64	29	1	176	66	215	102	1	383	22	131	54	0	207	887	3355
5:30-5:45	34	50	39	1	123	53	75	27	0	155	68	222	103	3	393	17	124	50	0	191	862	3416
5:45-6:00	38	44	32	0	114	67	67	32	1	166	81	185	78	0	344	22	143	47	0	212	836	3457
Peak Hour	140	182	138	1	460	281	264	107	2	652	288	860	372	7	1520	94	547	184	0	825	3457	Peak
PHF	0.92	0.88	0.88		0.93	0.85	0.88	0.84		0.93	0.89	0.90	0.90		0.95	0.71	0.92	0.85		0.96	0.97	5:00
Truck %				0%					0%					0%					0%		0%	6:00

9/28/2007

Traffic Count Data Sheet

Year Counts Taken: 2007 E-W Street I-40 N. ramp Speed Limit (I-40 N. ramp)= 25 MPH
 N-S Street: Unser Blvd Speed Limit (Unser Blvd)= 45 MPH
 Date of Count: 5/22/07

Begin Time	End Time	Eastbound (I-40 N. ramp)			Westbound (I-40 N. ramp)			Northbound (Unser Blvd)			Southbound (Unser Blvd)		
		L	T	R	L	T	R	L	T	R	L	T	R
7:00 AM	7:15 AM	0	0	0	87	3	38	13	165	0	0	496	19
7:15 AM	7:30 AM	0	0	0	83	0	35	6	175	0	0	498	13
7:30 AM	7:45 AM	0	0	0	93	0	53	3	180	0	0	424	25
7:45 AM	8:00 AM	0	0	0	80	0	62	2	167	0	0	357	11
8:00 AM	8:15 AM	0	0	0	58	0	54	2	134	0	0	356	43
8:15 AM	8:30 AM	0	0	0	74	0	67	4	156	0	0	349	5
8:30 AM	8:45 AM	0	0	0	77	0	64	2	130	0	0	298	9
8:45 AM	9:00 AM	0	0	0	78	0	47	8	92	0	0	264	6
AM Peak Hour Volumes		0	0	0	343	3	188	24	687	0	0	1775	68
% of Total Traffic		0.0%	0.0%	0.0%	11.1%	0.1%	6.1%	0.8%	22.2%	0.0%	0.0%	57.5%	2.2%
% Directional													
AM Peak Hour Factor													

Begin Time	End Time	Eastbound (I-40 N. ramp)			Westbound (I-40 N. ramp)			Northbound (Unser Blvd)			Southbound (Unser Blvd)		
		L	T	R	L	T	R	L	T	R	L	T	R
4:00 PM	4:15 PM	0	0	0	158	3	137	4	178	0	0	276	43
4:15 PM	4:30 PM	0	0	0	177	0	159	4	145	0	0	232	40
4:30 PM	4:45 PM	0	0	0	184	4	143	6	146	0	0	207	46
4:45 PM	5:00 PM	0	0	0	157	0	193	11	166	0	0	229	48
5:00 PM	5:15 PM	0	0	0	153	0	217	6	210	0	0	219	20
5:15 PM	5:30 PM	0	0	0	145	0	190	8	193	0	0	208	14
5:30 PM	5:45 PM	0	0	0	168	0	190	3	148	0	0	232	15
5:45 PM	6:00 PM	0	0	0	160	0	174	7	174	0	0	244	21
PM Peak Hour Volumes		0	0	0	626	0	771	24	725	0	0	903	70
% of Total Traffic		0.0%	0.0%	0.0%	20.1%	0.0%	24.7%	0.8%	23.2%	0.0%	0.0%	29.0%	2.2%
% Directional													
PM Peak Hour Factor													

Traffic Count Data Sheet

Year Counts Taken: **2007** E-W Street Los Volcanes Speed Limit (Los Volcanes)= **35** MPH
 N-S Street: Unser Blvd Speed Limit (Unser Blvd)= **45** MPH
 Date of Count: **5/23/07**

Begin Time	End Time	Eastbound (Los Volcanes)			Westbound (Los Volcanes)			Northbound (Unser Blvd)			Southbound (Unser Blvd)		
		L	T	R	L	T	R	L	T	R	L	T	R
7:00 AM	7:15 AM	32	26	0	32	11	42	1	293	40	62	208	13
7:15 AM	7:30 AM	51	18	4	31	8	42	4	307	17	61	235	17
7:30 AM	7:45 AM	28	20	1	8	1	24	3	297	25	97	256	19
7:45 AM	8:00 AM	26	5	2	24	7	26	0	204	19	40	198	20
8:00 AM	8:15 AM	38	5	2	8	4	28	3	217	46	30	462	43
8:15 AM	8:30 AM	23	4	5	4	3	28	4	233	42	33	439	6
8:30 AM	8:45 AM	44	3	2	4	6	46	4	470	9	34	436	47
8:45 AM	9:00 AM	20	4	9	42	3	23	6	448	6	27	438	44
AM Peak Hour Volumes		137	69	7	95	27	134	8	1101	101	260	897	69
% of Total Traffic		4.7%	2.4%	0.2%	3.3%	0.9%	4.6%	0.3%	37.9%	3.5%	9.0%	30.9%	2.4%
% Directional			7.3%			8.8%			41.7%			42.2%	
AM Peak Hour Factor			0.73			0.75			0.91			0.82	

Begin Time	End Time	Eastbound (Los Volcanes)			Westbound (Los Volcanes)			Northbound (Unser Blvd)			Southbound (Unser Blvd)		
		L	T	R	L	T	R	L	T	R	L	T	R
4:00 PM	4:15 PM	42	4	0	22	7	44	4	478	9	34	232	48
4:15 PM	4:30 PM	9	3	5	40	4	44	0	450	14	9	452	23
4:30 PM	4:45 PM	43	0	4	42	4	46	4	437	6	22	476	22
4:45 PM	5:00 PM	49	2	3	49	2	26	2	205	44	34	242	26
5:00 PM	5:15 PM	18	3	1	17	7	37	1	186	13	21	255	35
5:15 PM	5:30 PM	18	2	2	17	5	40	0	204	23	20	241	21
5:30 PM	5:45 PM	9	8	0	19	7	29	5	214	51	24	254	30
5:45 PM	6:00 PM	9	2	1	20	4	26	4	189	33	43	272	35
PM Peak Hour Volumes		54	15	4	73	23	132	10	793	120	108	1022	121
% of Total Traffic		2.2%	0.6%	0.2%	2.9%	0.9%	5.3%	0.4%	32.0%	4.8%	4.4%	41.3%	4.9%
% Directional			2.9%			9.2%			37.3%			50.5%	
PM Peak Hour Factor			0.83			0.92			0.85			0.89	

Traffic Count Data Sheet

Year Counts Taken:

2007

E-W Street LADERA
N-S Street: OURAYSpeed Limit (LADERA)= 40 MPH
Speed Limit (OURAY)= 30/35 MPH
Date of Count: 9/20/07

UNSIGNALIZED

Begin Time	End Time	Eastbound (LADERA)			Westbound (LADERA)			Northbound (OURAY)			Southbound (OURAY)		
		L	T	R	L	T	R	L	T	R	L	T	R
7:00 AM	7:15 AM	2	90	59	4	26	2	22	42	2	42	36	2
7:15 AM	7:30 AM	4	84	68	0	30	3	20	16	1	18	42	3
7:30 AM	7:45 AM	5	94	73	0	38	6	23	22	1	22	56	4
7:45 AM	8:00 AM	6	111	57	1	40	3	38	18	1	32	39	3
8:00 AM	8:15 AM	5	76	49	2	37	8	17	21	2	17	54	3
8:15 AM	8:30 AM	0	59	34	3	22	8	6	47	3	5	45	5
8:30 AM	8:45 AM	4	58	42	4	35	4	19	46	4	6	46	4
8:45 AM	9:00 AM	0	60	46	2	34	0	18	45	2	8	49	2
AM Peak Hour Volumes		20	365	247	3	145	20	98	77	5	89	191	13
% of Total Traffic		1.6%	28.7%	19.4%	0.2%	11.4%	1.6%	7.7%	6.0%	0.4%	7.0%	15.0%	1.0%
% Directional			49.6%			13.2%			14.1%			23.0%	
AM Peak Hour Factor			0.91			0.89			0.79			0.89	

Begin Time	End Time	Eastbound (LADERA)			Westbound (LADERA)			Northbound (OURAY)			Southbound (OURAY)		
		L	T	R	L	T	R	L	T	R	L	T	R
4:00 PM	4:15 PM	4	64	37	3	80	23	69	75	3	40	30	2
4:15 PM	4:30 PM	2	58	29	3	73	49	64	69	2	43	36	6
4:30 PM	4:45 PM	4	56	34	4	84	24	66	75	3	45	39	5
4:45 PM	5:00 PM	3	52	37	2	80	22	70	74	2	43	35	6
5:00 PM	5:15 PM	4	84	48	1	83	18	63	67	4	11	34	3
5:15 PM	5:30 PM	4	61	41	4	92	23	78	55	1	12	30	4
5:30 PM	5:45 PM	5	61	52	6	105	35	78	60	6	3	31	2
5:45 PM	6:00 PM	6	65	59	9	99	31	74	51	5	8	28	4
PM Peak Hour Volumes		19	271	200	20	379	107	293	233	16	34	123	13
% of Total Traffic		1.1%	15.9%	11.7%	1.2%	22.2%	6.3%	17.2%	13.6%	0.9%	2.0%	7.2%	0.8%
% Directional			28.7%			29.6%			31.7%			10.0%	
PM Peak Hour Factor			0.90			0.87			0.94			0.89	

Traffic Count Data Sheet

Year Counts Taken: 2007

E-W Street I-40 S. ramp
N-S Street: Unser BlvdSpeed Limit (I-40 S. ramp)= 25 MPH
Speed Limit (Unser Blvd)= 45 MPH
Date of Count: 5/21/07

Begin Time	End Time	Eastbound (I-40 S. ramp)			Westbound (I-40 S. ramp)			Northbound (Unser Blvd)			Southbound (Unser Blvd)		
		L	T	R	L	T	R	L	T	R	L	T	R
7:00 AM	7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0
7:15 AM	7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0
7:30 AM	7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0
7:45 AM	8:00 AM	9	0	6	0	0	0	3	173	172	3	296	145
8:00 AM	8:15 AM	10	0	1	0	0	0	0	126	141	1	206	206
8:15 AM	8:30 AM	9	0	0	0	0	0	0	128	138	0	165	209
8:30 AM	8:45 AM	3	0	2	0	0	0	0	108	129	0	154	180
8:45 AM	9:00 AM	6	0	4	0	0	0	40	96	443	5	488	439
AM Peak Hour Volumes		31	0	9	0	0	0	3	535	580	4	821	740
% of Total Traffic		1.1%	0.0%	0.3%	0.0%	0.0%	0.0%	0.1%	19.6%	21.3%	0.1%	30.2%	27.2%
% Directional			1.5%			0.0%			41.1%			57.5%	
AM Peak Hour Factor			0.67			#DIV/0!			0.80			0.88	

Begin Time	End Time	Eastbound (I-40 S. ramp)			Westbound (I-40 S. ramp)			Northbound (Unser Blvd)			Southbound (Unser Blvd)		
		L	T	R	L	T	R	L	T	R	L	T	R
4:00 PM	4:15 PM	40	0	5	0	0	0	0	444	82	0	297	94
4:15 PM	4:30 PM	19	0	6	0	0	0	0	168	106	0	285	91
4:30 PM	4:45 PM	17	0	5	0	0	0	0	135	77	0	294	80
4:45 PM	5:00 PM	18	0	2	0	0	0	0	162	92	0	272	71
5:00 PM	5:15 PM	15	0	8	0	0	0	0	176	89	0	311	66
5:15 PM	5:30 PM	48	0	5	0	0	0	0	458	95	0	297	55
5:30 PM	5:45 PM	48	0	6	0	0	0	0	438	75	0	324	78
5:45 PM	6:00 PM	42	0	2	0	0	0	0	456	87	0	276	60
PM Peak Hour Volumes		69	0	21	0	0	0	0	641	364	0	1162	308
% of Total Traffic		2.7%	0.0%	0.8%	0.0%	0.0%	0.0%	0.0%	25.0%	14.2%	0.0%	45.3%	12.0%
% Directional			3.5%			0.0%			39.2%			57.3%	
PM Peak Hour Factor			0.90			#DIV/0!			0.92			0.97	

Traffic Count Data Sheet

Year Counts Taken: **2007** E-W Street LADERA Speed Limit (LADERA)= **40** MPH
 N-S Street: MARKET Speed Limit (MARKET)= **30** MPH
 Date of Count: **9/21/07**

UNSIGNALIZED

Begin Time	End Time	Eastbound (LADERA)			Westbound (LADERA)			Northbound (MARKET)			Southbound (MARKET)		
		L	T	R	L	T	R	L	T	R	L	T	R
7:00 AM	7:15 AM	0	409	5	4	69	0	24	0	20	0	0	0
7:15 AM	7:30 AM	0	112	4	2	75	0	25	0	23	0	0	0
7:30 AM	7:45 AM	0	121	7	3	83	0	27	0	16	0	0	0
7:45 AM	8:00 AM	0	95	8	4	116	0	35	0	19	0	0	0
8:00 AM	8:15 AM	0	97	9	5	91	0	26	0	14	0	0	0
8:15 AM	8:30 AM	0	89	43	5	77	0	25	0	8	0	0	0
8:30 AM	8:45 AM	0	94	9	4	79	0	24	0	4	0	0	0
8:45 AM	9:00 AM	0	99	42	7	85	0	24	0	6	0	0	0
AM Peak Hour Volumes		0	425	28	14	365	0	113	0	72	0	0	0
% of Total Traffic		0.0%	41.8%	2.8%	1.4%	35.9%	0.0%	11.1%	0.0%	7.1%	0.0%	0.0%	0.0%
% Directional			44.5%			37.3%			18.2%				
AM Peak Hour Factor			0.88			0.79			0.86				#DIV/0!

Begin Time	End Time	Eastbound (LADERA)			Westbound (LADERA)			Northbound (MARKET)			Southbound (MARKET)		
		L	T	R	L	T	R	L	T	R	L	T	R
4:00 PM	4:15 PM	0	424	20	7	93	0	44	0	5	0	0	0
4:15 PM	4:30 PM	0	429	24	40	96	0	9	0	4	0	0	0
4:30 PM	4:45 PM	0	446	32	5	409	0	15	0	4	0	0	0
4:45 PM	5:00 PM	0	424	45	14	404	0	42	0	7	0	0	0
5:00 PM	5:15 PM	0	153	27	14	115	0	19	0	8	0	0	0
5:15 PM	5:30 PM	0	121	42	16	143	0	22	0	3	0	0	0
5:30 PM	5:45 PM	0	115	41	10	100	0	22	0	12	0	0	0
5:45 PM	6:00 PM	0	124	46	8	105	0	20	0	13	0	0	0
PM Peak Hour Volumes		0	513	156	48	463	0	83	0	36	0	0	0
% of Total Traffic		0.0%	39.5%	12.0%	3.7%	35.6%	0.0%	6.4%	0.0%	2.8%	0.0%	0.0%	0.0%
% Directional			51.5%			39.3%			9.2%				
PM Peak Hour Factor			0.93			0.80			0.88				#DIV/0!

Traffic Count Data Sheet

Year Counts Taken: **2007** E-W Street LADERA Speed Limit (LADERA)= **40** MPH
 N-S Street: LAURELWOOD PKWY Speed Limit (LAURELWOOD PKWY) **25** MPH
 UNSIGNALIZED Date of Count: **9/19/07**

Begin Time	End Time	Eastbound (LADERA)			Westbound (LADERA)			Northbound (LAURELWOOD PKWY)			Southbound (LAURELWOOD PKWY)		
		L	T	R	L	T	R	L	T	R	L	T	R
7:00 AM	7:15 AM	0	126	7	1	47	0	21	0	19	1	0	2
7:15 AM	7:30 AM	0	128	6	2	52	0	19	0	17	2	0	3
7:30 AM	7:45 AM	1	135	8	1	65	0	16	0	19	2	0	2
7:45 AM	8:00 AM	0	101	7	1	91	0	22	0	10	0	0	1
8:00 AM	8:15 AM	4	94	40	2	67	0	20	0	6	2	0	0
8:15 AM	8:30 AM	0	87	8	3	54	0	23	0	9	0	0	2
8:30 AM	8:45 AM	4	94	6	4	57	0	19	0	8	2	0	0
8:45 AM	9:00 AM	0	99	8	5	64	0	22	0	40	0	0	4
AM Peak Hour Volumes		1	490	28	5	255	0	78	0	65	5	0	8
% of Total Traffic		0.1%	52.4%	3.0%	0.5%	27.3%	0.0%	8.3%	0.0%	7.0%	0.5%	0.0%	0.9%
% Directional			55.5%			27.8%			15.3%			1.4%	
AM Peak Hour Factor			0.90			0.71			0.89			0.65	

Begin Time	End Time	Eastbound (LADERA)			Westbound (LADERA)			Northbound (LAURELWOOD PKWY)			Southbound (LAURELWOOD PKWY)		
		L	T	R	L	T	R	L	T	R	L	T	R
4:00 PM	4:15 PM	2	402	49	12	95	0	7	0	10	0	0	4
4:15 PM	4:30 PM	4	400	24	14	94	4	9	4	12	0	0	4
4:30 PM	4:45 PM	4	94	46	13	93	0	12	0	20	0	0	3
4:45 PM	5:00 PM	4	408	7	13	108	0	5	0	12	0	0	4
5:00 PM	5:15 PM	2	118	26	10	117	0	9	0	8	0	0	0
5:15 PM	5:30 PM	2	96	18	21	150	2	6	0	9	1	0	0
5:30 PM	5:45 PM	3	107	22	24	136	4	12	0	9	0	0	2
5:45 PM	6:00 PM	2	101	20	21	143	3	10	0	8	0	0	1
PM Peak Hour Volumes		9	422	86	76	546	9	37	0	34	1	0	3
% of Total Traffic		0.7%	34.5%	7.0%	6.2%	44.6%	0.7%	3.0%	0.0%	2.8%	0.1%	0.0%	0.2%
% Directional			42.3%			51.6%			5.8%			0.3%	
PM Peak Hour Factor			0.89			0.91			0.85			0.50	

Intersection Data SheetIntersection: **Los Volcanes Rd / Unser Blvd.**Posted Speed Limit (E-W Street): 35**Eastbound Approach:** **Los Volcanes Rd**Date: 8/8/2005

Left Turn Lanes	Thru/Left Lanes	Thru Lanes	Thru/Right Lanes	Right Turn Lanes
2	0	0	1	0

Length: 95 feet

Left Turn Arrow?	Thru Green?	Right Turn Arrow?
Y	Y	Y

Is there a right turn slip lane that by-passes the traffic signal?

No

Westbound Approach: **Los Volcanes Rd**

Left Turn Lanes	Thru/Left Lanes	Thru Lanes	Thru/Right Lanes	Right Turn Lanes
1	0	1	0	1

Length: 140 feet

Left Turn Arrow?	Thru Green?	Right Turn Arrow?
Y	Y	Y

Is there a right turn slip lane that by-passes the traffic signal?

No

Posted Speed Limit (N-S Street): 45**Northbound Approach:** **Unser Blvd.**

Left Turn Lanes	Thru/Left Lanes	Thru Lanes	Thru/Right Lanes	Right Turn Lanes
1	stripes	2	0	1

Length: 600 feet

Left Turn Arrow?	Thru Green?	Right Turn Arrow?
Y	Y	Y

Is there a right turn slip lane that by-passes the traffic signal?

Yes

Southbound Approach: **Unser Blvd.**

Left Turn Lanes	Thru/Left Lanes	Thru Lanes	Thru/Right Lanes	Right Turn Lanes
1	stripes	2	0	1

Length: 170 feet

Left Turn Arrow?	Thru Green?	Right Turn Arrow?
Y	Y	Y

Is there a right turn slip lane that by-passes the traffic signal?

No

Signalized Intersection Information Sheet

Intersection: LADERA & OURAY

Speed Limit - E-W Street: 40 M.P.H.

Speed Limit - N-S Street: 30 M.P.H.

Date: 9/20/2007

East Bound Approach:

LADERA

	Left Turn Lanes	Thru / Lefts	Thru Lanes	Thru / Rights	Right Turn Lanes
Length	7	-	1	1	-

Left Turn Arrow?	Thru Green	Right Turn Arrow?
YES	YES	NO

Is there a right turn slip laned that by-passes the traffic signal? NO

West Bound Approach:

LADERA

	Left Turn Lanes	Thru / Lefts	Thru Lanes	Thru / Rights	Right Turn Lanes
Length	5	-	2	-	1

Left Turn Arrow?	Thru Green	Right Turn Arrow?
YES	YES	NO

Is there a right turn slip laned that by-passes the traffic signal? NO

North Bound Approach:

OURAY

	Left Turn Lanes	Thru / Lefts	Thru Lanes	Thru / Rights	Right Turn Lanes
Length	6	-	-	1	-

Left Turn Arrow?	Thru Green	Right Turn Arrow?
NO	YES	NO

Is there a right turn slip laned that by-passes the traffic signal? NO

South Bound Approach:

OURAY

	Left Turn Lanes	Thru / Lefts	Thru Lanes	Thru / Rights	Right Turn Lanes
Length	5	-	1	-	1

Left Turn Arrow?	Thru Green	Right Turn Arrow?
NO	NO	NO

Is there a right turn slip laned that by-passes the traffic signal? NO

NOTE: Existing Geometry

Signalized Intersection Information Sheet

Intersection: LADERA & MARKET

Speed Limit - E-W Street: 40 M.P.H.

Speed Limit - N-S Street: 30 M.P.H.

Date: 9/21/2007

East Bound Approach:

LADERA

	Left Turn Lanes	Thru / Lefts	Thru Lanes	Thru / Rights	Right Turn Lanes						
Length	12	-			-						
	<table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <td style="width: 33%;">Left Turn Arrow?</td> <td style="width: 33%;">Thru Green</td> <td style="width: 33%;">Right Turn Arrow?</td> </tr> <tr> <td style="text-align: center;">NO</td> <td style="text-align: center;">NO</td> <td style="text-align: center;">NO</td> </tr> </table>					Left Turn Arrow?	Thru Green	Right Turn Arrow?	NO	NO	NO
Left Turn Arrow?	Thru Green	Right Turn Arrow?									
NO	NO	NO									

Is there a right turn slip laned that by-passes the traffic signal? NO

West Bound Approach:

LADERA

	Left Turn Lanes	Thru / Lefts	Thru Lanes	Thru / Rights	Right Turn Lanes						
Length	4	-			-						
	<table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <td style="width: 33%;">Left Turn Arrow?</td> <td style="width: 33%;">Thru Green</td> <td style="width: 33%;">Right Turn Arrow?</td> </tr> <tr> <td style="text-align: center;">NO</td> <td style="text-align: center;">NO</td> <td style="text-align: center;">NO</td> </tr> </table>					Left Turn Arrow?	Thru Green	Right Turn Arrow?	NO	NO	NO
Left Turn Arrow?	Thru Green	Right Turn Arrow?									
NO	NO	NO									

Is there a right turn slip laned that by-passes the traffic signal? NO

North Bound Approach:

MARKET

	Left Turn Lanes	Thru / Lefts	Thru Lanes	Thru / Rights	Right Turn Lanes						
Length	0	-	-	-							
	<table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <td style="width: 33%;">Left Turn Arrow?</td> <td style="width: 33%;">Thru Green</td> <td style="width: 33%;">Right Turn Arrow?</td> </tr> <tr> <td style="text-align: center;">NO</td> <td style="text-align: center;">NO</td> <td style="text-align: center;">NO</td> </tr> </table>					Left Turn Arrow?	Thru Green	Right Turn Arrow?	NO	NO	NO
Left Turn Arrow?	Thru Green	Right Turn Arrow?									
NO	NO	NO									

Is there a right turn slip laned that by-passes the traffic signal? NO

South Bound Approach:

MARKET

	Left Turn Lanes	Thru / Lefts	Thru Lanes	Thru / Rights	Right Turn Lanes						
Length	0	-	-	-	-						
	<table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <td style="width: 33%;">Left Turn Arrow?</td> <td style="width: 33%;">Thru Green</td> <td style="width: 33%;">Right Turn Arrow?</td> </tr> <tr> <td style="text-align: center;">NO</td> <td style="text-align: center;">NO</td> <td style="text-align: center;">NO</td> </tr> </table>					Left Turn Arrow?	Thru Green	Right Turn Arrow?	NO	NO	NO
Left Turn Arrow?	Thru Green	Right Turn Arrow?									
NO	NO	NO									

Is there a right turn slip laned that by-passes the traffic signal? NO

NOTE: Existing Geometry

Signalized Intersection Information SheetIntersection: LADERA & LAURELWOOD PKWYSpeed Limit - E-W Street: 40 M.P.H.Speed Limit - N-S Street: 25 M.P.H.

Date:

9/19/2007**East Bound Approach:****LADERA**

	Left Turn Lanes	Thru / Lefts	Thru Lanes	Thru / Rights	Right Turn Lanes
Length	7	-	1	1	0
	<div>Left Turn Arrow? Thru Green Right Turn Arrow?</div> <div>NO NO NO</div>				

Is there a right turn slip laned that by-passes the traffic signal? NO**West Bound Approach:****LADERA**

	Left Turn Lanes	Thru / Lefts	Thru Lanes	Thru / Rights	Right Turn Lanes
Length	4	-	1	1	0
	<div>Left Turn Arrow? Thru Green Right Turn Arrow?</div> <div>NO NO NO</div>				

Is there a right turn slip laned that by-passes the traffic signal? NO**North Bound Approach:****LAURELWOOD PKWY**

	Left Turn Lanes	Thru / Lefts	Thru Lanes	Thru / Rights	Right Turn Lanes
Length	12	-	1	-	10
	<div>Left Turn Arrow? Thru Green Right Turn Arrow?</div> <div>NO NO NO</div>				

Is there a right turn slip laned that by-passes the traffic signal? NO**South Bound Approach:****LAURELWOOD PKWY**

	Left Turn Lanes	Thru / Lefts	Thru Lanes	Thru / Rights	Right Turn Lanes
Length	0	-	1	-	0
	<div>Left Turn Arrow? Thru Green Right Turn Arrow?</div> <div>NO NO NO</div>				

Is there a right turn slip laned that by-passes the traffic signal? NO**NOTE:** Existing Geometry

**Storm Cloud
Turning Movement Tabulation**

INTERSECTION: Ladera and Unser
(existing traffic signal)

AM Peak Hour

Existing* (2002)
Background Growth** (2002-2003)
Background Growth** (2003-2005)
Approved Future Developments:

	Southbound Unser			Westbound Ladera			Northbound Unser			Eastbound Ladera		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
2005 Volumes	75	2,262	14	668	88	36	87	310	262	36	104	488
2007 no Build	8	279	2	76	10	4	10	34	29	4	12	60
Storm Cloud	84	2,540	16	744	98	40	97	344	291	40	116	556
2007 Build	7	208	1	57	7	3	7	25	22	3	9	44
PHF	0.860			0.840			0.810			0.810		
131	28	36	47	44	87	112	142					

PM Peak Hour

Existing* (2002)
Background Growth** (2002-2003)
Background Growth** (2003-2005)
Approved Future Developments:

	Southbound Unser			Westbound Ladera			Northbound Unser			Eastbound Ladera		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
2005 Volumes	67	549	20	257	97	65	229	1,029	660	34	102	139
2007 no Build	3	23	1	10	4	2	8	38	29	1	4	6
Storm Cloud	70	572	21	267	101	67	237	1,067	570	35	106	145
2007 Build	6	47	2	20	8	5	16	79	42	3	8	12
PHF	0.810			0.950			0.830			0.74		
83	91	118	159	54	71	92						

* 2002 Volumes from Middle Rio Grande COG 11/12/02

** Growth rates 7% on the WB approach, and 4% on all the other approaches

growth rates	4.1%	4.1%	4.1%	3.8%	3.8%	3.8%	3.7%	3.7%	3.7%	4.0%	4.0%	4.0%
Residential Trip Distribution % Enter				10.19%		13.12%		7.25%				
Residential Trip Distribution % Exit										10.19%	13.12%	7.25%

Watershed & Inspiration
Existing & Forecast Turning Movement Tabulation

INTERSECTION: Tierra Pintada and Unser
(existing traffic signal)

AM Peak Hour

Existing (2007)
Background Growth* (2007-2012)
Approved Future Developments**:

Storm Cloud

Proposed Developments:
Watershed Residential

Retail

Southbound Unser			Westbound Tierra Pintada			Northbound Unser			Eastbound Tierra Pintada		
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
10	783	21	14	0	4	15	93	2	34	3	195
2	161	4	3	0	1	3	17	0	7	1	39
2012 No Build Volumes											
0	0	10	0	0	0	0	0	0	34	0	0
12	944	35	17	0	5	18	110	2	75	4	234
Enter											
0	0	24	0	0	0	5	0	0	0	0	0
Exit											
0	0	0	0	0	0	0	0	0	78	0	17
Enter											
0	0	0	0	0	0	1	0	0	0	0	0
Exit											
0	0	0	0	0	0	0	0	0	0	0	0
12	944	59	17	0	5	24	110	2	153	4	252
PHF											
0.940			0.560			0.970			0.810		

PM Peak Hour

Existing (2007)
Background Growth* (2007-2012)
Approved Future Developments**:

Storm Cloud

Proposed Developments:
Watershed

Retail

Southbound Unser			Westbound Tierra Pintada			Northbound Unser			Eastbound Tierra Pintada		
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
28	770	47	65	1	24	179	840	22	31	2	68
6	158	10	12	0	5	33	155	4	6	0	14
2012 No Build Volumes											
0	0	30	0	0	0	0	0	0	20	0	0
34	928	87	77	1	29	212	995	26	57	2	82
Enter											
0	0	78	0	0	0	17	0	0	0	0	0
Exit											
0	0	0	0	0	0	0	0	0	44	0	10
Enter											
0	0	0	0	0	0	2	0	0	0	0	0
Exit											
0	0	0	0	0	0	0	0	0	0	0	2
34	928	165	77	1	29	231	995	26	102	2	93
PHF											
0.880			0.730			0.940			0.900		

growth rates	4.1%	4.1%	4.1%	3.8%	3.8%	3.8%	3.7%	3.7%	3.7%	4.0%	4.0%	4.0%
Residential Trip Distribution % Enter			11.45%				2.51%					
Residential Trip Distribution % Exit										11.45%		2.51%
Retail Trip Distribution % Enter							3.00%					
Retail Trip Distribution % Exit												3.00%

* - Background traffic growth estimates

** - Proposed development include Storm Cloud, Target, Ladera Business Park, Vista Oriente, and subdivisions from the southwest valley from appropriate TIA's

**Watershed & Inspiration
Existing & Forecast Turning Movement Tabulation**

INTERSECTION: Ladera and Unser
(existing traffic signal)

AM Peak Hour

Existing (2007)
Background Growth* (2007-2012)
Approved Future Developments**:

Storm Cloud
Vista Oriente Development
Ladera Business Park
Target

Proposed Developments:
Watershed Residential

Retail

	Southbound Unser			Westbound Ladera			Northbound Unser			Eastbound Ladera		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Existing (2007)	45	906	58	317	105	44	48	429	224	175	251	365
Background Growth* (2007-2012)	9	186	12	60	20	8	9	79	41	35	50	73
Approved Future Developments**:												
Storm Cloud	0	0	0	0	13	0	0	0	0	0	44	24
Vista Oriente Development	21	11	7	0	0	25	0	13	0	9	0	0
Ladera Business Park	5	47	0	0	0	3	0	28	0	0	0	0
Target	0	42	0	161	0	0	32	32	121	0	0	43
2012 No Build Volumes	80	1,192	77	538	138	80	89	581	386	219	345	505
Proposed Developments: Watershed Residential												
Enter	0	0	0	0	24	0	0	5	0	0	0	0
Exit	0	17	0	0	0	0	0	0	0	0	78	0
Retail												
Enter	0	0	0	0	0	0	0	1	0	0	0	0
Exit	0	0	0	0	0	0	0	0	0	0	0	0
2012 Build	80	1,209	77	538	162	80	89	587	386	219	424	505
PHF	0.890			0.790			0.850			0.870		

PM Peak Hour

Existing (2007)
Background Growth* (2007-2012)
Approved Future Developments**:

Storm Cloud
Vista Oriente Development
Ladera Business Park
Target

Proposed Developments:
Watershed Residential

Retail

	Southbound Unser			Westbound Ladera			Northbound Unser			Eastbound Ladera		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Existing (2007)	94	547	184	281	264	107	288	860	372	140	182	138
Background Growth* (2007-2012)	19	112	38	53	50	20	53	159	69	28	36	28
Approved Future Developments**:												
Storm Cloud	0	0	0	0	39	0	21	0	0	0	26	14
Vista Oriente Development	41	0	0	113	10	39	0	49	68	3	7	0
Ladera Business Park	23	204	0	0	0	14	0	123	0	0	0	0
Target	0	77	0	292	0	0	77	77	290	0	77	0
2012 No Build Volumes	177	940	222	739	363	180	439	1,268	799	171	328	180
Proposed Developments: Watershed Residential												
Enter	0	0	0	0	78	0	0	17	0	0	0	0
Exit	0	10	0	0	0	0	0	0	0	0	44	0
Retail												
Enter	0	0	0	0	0	0	0	2	0	0	0	0
Exit	0	2	0	0	0	0	0	0	0	0	0	0
2012 Build	177	952	222	739	441	180	439	1,287	799	171	373	180
PHF	0.960			0.930			0.950			0.930		

growth rates	4.1%	4.1%	4.1%	3.8%	3.8%	3.8%	3.7%	3.7%	3.7%	4.0%	4.0%	4.0%
Residential Trip Distribution % Enter					11.45%			2.51%				
Residential Trip Distribution % Exit		2.51%									11.45%	
Retail Trip Distribution % Enter								3.00%				
Retail Trip Distribution % Exit		3.00%										

* - Background traffic growth estimates

** - Proposed development include Storm Cloud, Target, Ladera Business Park, Vista Oriente, and subdivisions from the southwest valley from appropriate TIA's

Watershed & Inspiration
Existing & Forecast Turning Movement Tabulation

INTERSECTION: Unser and I-40 WB Ramp
(existing un-signalized intersection)

AM Peak Hour

Existing (2007)
Background Growth* (2007-2012)
Approved Future Developments**:

Storm Cloud
Southwest Mesa Subdivisions
Target

2012 No Build Volumes

Proposed Developments:
Watershed Residential

Southbound Unser			Westbound I-40			Northbound Unser			Eastbound I-40		
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
0	1,491	54	265	5	215	13	535	0	0	0	0
0	224	8	50	1	41	2	66	0	0	0	0
0	0	0	0	0	0	0	24	0	0	0	0
0	10	0	37	0	0	0	26	0	0	0	0
0	246	0	28	0	0	20	186	0	0	0	0
0	1,971	62	380	6	256	35	837	0	0	0	0
Enter	0	0	0	0	5	0	0	0	0	0	0
Exit	0	17	0	0	0	0	0	0	0	0	0
2012 Build	0	1,988	62	380	6	261	35	837	0	0	0
PHF	0.950		0.940			0.890					

PM Peak Hour

Existing (2007)
Background Growth* (2007-2012)
Approved Future Developments**:

Storm Cloud
Southwest Mesa Subdivisions
Target

2012 No Build Volumes

Proposed Developments:
Watershed Residential

Southbound Unser			Westbound I-40			Northbound Unser			Eastbound I-40		
Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
0	981	86	586	2	754	22	729	0	0	0	0
0	147	13	111	0	143	3	90	0	0	0	0
0	0	0	0	0	0	0	14	0	0	0	0
0	6	31	93	0	0	0	38	0	0	0	0
0	447	0	51	0	0	49	445	0	0	0	0
0	1,581	130	841	2	897	74	1,316	0	0	0	0
Enter	0	0	0	0	17	0	0	0	0	0	0
Exit	0	10	0	0	0	0	0	0	0	0	0
2012 Build	0	1,591	130	841	2	914	74	1,316	0	0	0
PHF	0.99		0.97			0.96					

Residential Trip Distribution % Enter	3.0%	3.0%	3.0%	3.8%	3.8%	3.8%	2.5%	2.5%	2.5%	4.0%	4.0%	4.0%
Residential Trip Distribution % Exit		2.51%				2.51%						

* - Background traffic growth estimates

** - Proposed development include Storm Cloud, Target, Ladera Business Park, Vista Oriente, and subdivisions from the southwest valley from appropriate TIA's

Watershed & Inspiration
Existing & Forecast Turning Movement Tabulation

INTERSECTION: Unser and I-40 EB Ramp
(existing un-signalized intersection)

AM Peak Hour

	Southbound Unser			Westbound I-40			Northbound Unser			Eastbound I-40		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Existing (2007)	0	788	917	0	0	0	0	486	874	53	1	23
Background Growth* (2007-2012)	0	118	138	0	0	0	0	60	108	11	0	5
Approved Future Developments**:												
Storm Cloud	0	0	0	0	0	0	0	24	0	0	0	0
Southwest Mesa Subdivisions	0	77	0	0	0	0	0	26	49	0	0	0
Target	0	274	0	0	0	0	0	207	21	0	0	27
2012 No Build Volumes	0	1,257	1,055	0	0	0	0	803	1,052	64	1	55
Proposed Developments: Watershed Residential												
Enter	0	0	0	0	0	0	0	0	0	0	0	0
Exit	0	0	17	0	0	0	0	0	0	0	0	0
2012 Build	0	1,257	1,072	0	0	0	0	803	1,052	64	1	55
PHF	0.970						0.890			0.690		

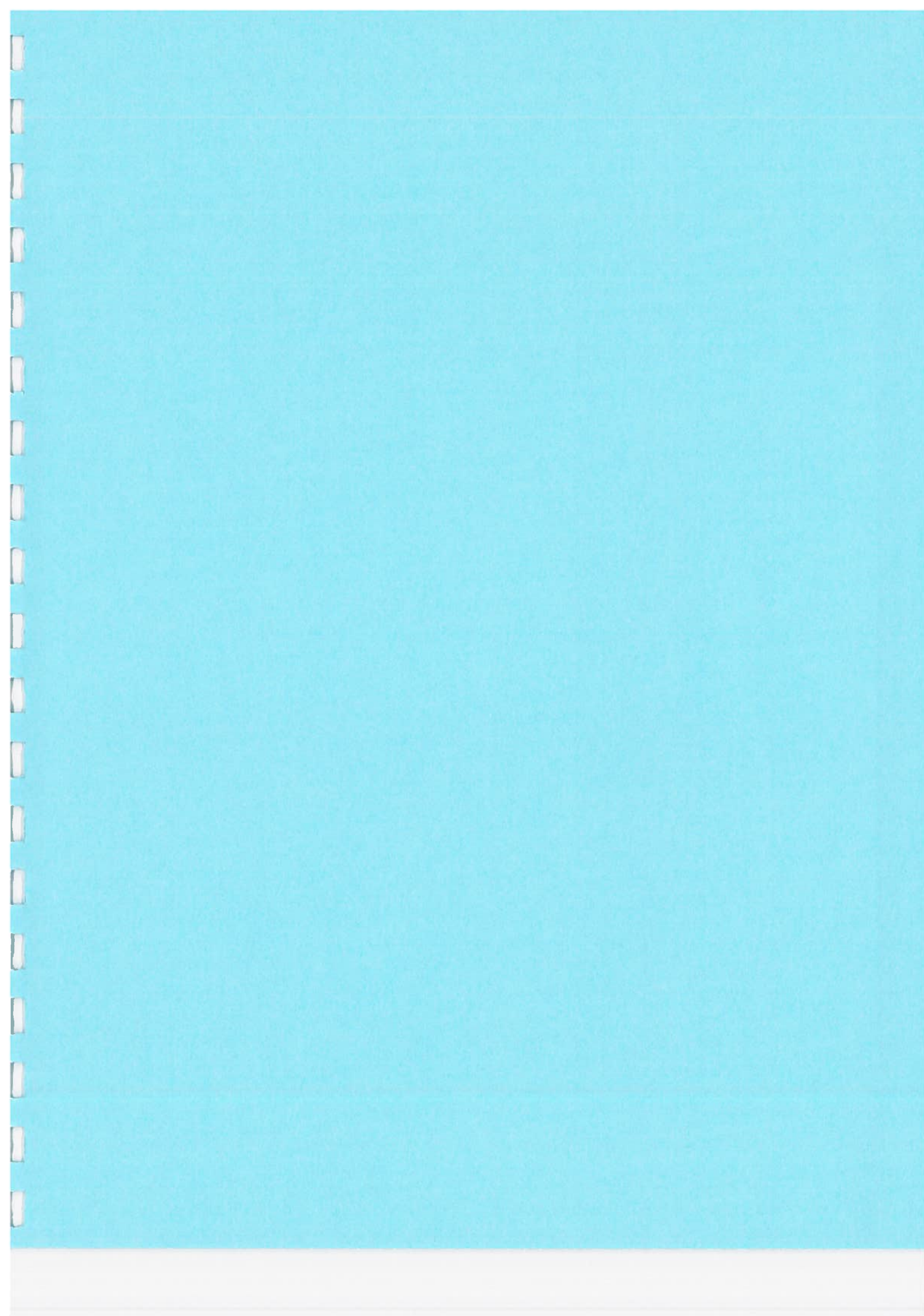
PM Peak Hour

	Southbound Unser			Westbound I-40			Northbound Unser			Eastbound I-40		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Existing (2007)	0	1,238	259	0	0	0	0	663	395	56	0	31
Background Growth* (2007-2012)	0	186	39	0	0	0	0	82	49	11	0	6
Approved Future Developments**:												
Storm Cloud	0	0	0	0	0	0	0	14	0	0	0	0
Southwest Mesa Subdivisions	0	98	0	0	0	0	0	38	96	0	0	0
Target	0	498	0	0	0	0	0	496	51	0	0	49
2012 No Build Volumes	0	2,020	298	0	0	0	0	1,293	591	67	0	86
Proposed Developments: Watershed Residential												
Enter	0	0	0	0	0	0	0	0	0	0	0	0
Exit	0	0	10	0	0	0	0	0	0	0	0	0
2012 Build	0	2,020	308	0	0	0	0	1,293	591	67	0	86
PHF	0.93						0.92			0.62		

Residential Trip Distribution % Enter	3.0%	3.0%	3.0%	3.8%	3.8%	3.8%	2.5%	2.5%	2.5%	4.0%	4.0%	4.0%
Residential Trip Distribution % Exit			2.51%									

* - Background traffic growth estimates

** - Proposed development include Storm Cloud, Target, Ladera Business Park, Vista Oriente, and subdivisions from the southwest valley from appropriate TIA's



Monday, January 14, 2008

Richard H. Dourte, City Engineer
City of Albuquerque
P. O. Box 1293
Albuquerque, NM 87103

Re: Ladera Dr. / Unser Blvd.

Dear Richard:

As you and Tony Loyd have requested, I have prepared additional analysis of the intersection of Ladera Dr. / Unser Blvd. for the following conditions:

Case "F" – Assuming a full access signalized Driveway "D" on Unser Blvd. south of Ladera Dr. is approved for the Heritage Neighborhood Marketplace project.

Case "L" – Assuming a right-turn-in, right-turn-out, left-turn-in driveway is approved for the Heritage Neighborhood Marketplace project.

For each of the preceding two cases, the following conditions were analyzed:

Base Geometry – assumes the geometry recommended in the Traffic Impact Study for the project.

w/Triple Left Turn Lanes – assumes the Base Geometry plus triple westbound left turn lanes on Ladera Dr. at Unser Blvd.

w/Left Turn Flyover – assumes the Base Geometry plus a westbound left turn flyover lane in lieu of at-grade left turn lanes.

The Base Geometry for the intersection of Ladera Dr. / Unser Blvd. as recommended in the Traffic Impact Study for Heritage Neighborhood Marketplace is summarized in the following table:

Base Geometry (Ladera Dr. / Unser Blvd.)					
Approach	Left Turn Lanes	Thru/Lefts	Thru Lanes	Thru/Rights	Right Turn Lanes
EB Ladera Dr.	1	0	2	0	2
WB Ladera Dr.	2	0	1	1	0
NB Unser Blvd.	2	0	2	0	1
SB Unser Blvd.	2	0	2	0	1

The volumes utilized in this analysis were all the same as those calculated and utilized in the Traffic Impact Study for the Heritage Neighborhood Marketplace project. The only thing that was changed for each case or the analysis was the geometry. Triple



Timings
2: Ladera Dr & Unser Blvd

Terry O. Brown, P.E.
1/14/2008

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	190	450	597	190	450	597	190	450	597	190	450	597
Volume (vph)	190	450	597	190	450	597	190	450	597	190	450	597
Turn Type	pm+ov	pm+ov	pm+ov	pm+ov	pm+ov	pm+ov	pm+ov	pm+ov	pm+ov	pm+ov	pm+ov	pm+ov
Protected Phases	7	4	4	7	4	4	7	4	4	7	4	4
Permitted Phases	7	4	4	7	4	4	7	4	4	7	4	4
Detector Phases	7	4	4	7	4	4	7	4	4	7	4	4
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	10.0	21.0	10.0	21.0	10.0	21.0	10.0	21.0	10.0	21.0	10.0	21.0
Total Split (s)	21.0	22.0	14.0	20.0	21.0	14.0	20.0	21.0	22.0	14.0	20.0	21.0
Total Split (%)	19.1%	20.0%	12.7%	18.2%	19.1%	12.7%	18.2%	19.1%	20.0%	12.7%	18.2%	19.1%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lead/Lag	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag	Lead	Lag	Lead	Lag
Lead-Lag Optimizes?	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min
Recall Mode	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min
Act Effd Green (s)	35.5	19.0	32.9	17.0	19.5	10.9	54.0	74.0	8.0	51.1	70.8	70.8
Actuated g/c Ratio	0.32	0.17	0.30	0.15	0.18	0.10	0.49	0.67	0.07	0.46	0.64	0.64
v/c Ratio	0.69	0.87	0.82	0.90	0.61	0.57	0.48	0.41	0.57	0.93	0.11	0.11
Control Delay	38.4	61.0	44.3	59.8	32.3	56.9	14.1	7.9	57.9	35.8	1.3	1.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	38.4	61.0	44.3	59.8	32.3	56.9	14.1	7.9	57.9	35.8	1.3	1.3
LOS	D	E	D	E	C	E	B	A	E	D	A	A
Approach Delay	48.5											
Approach LOS	D											



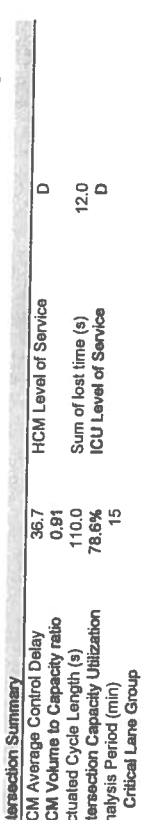
Splits and Phases: 2: Ladera Dr & Unser Blvd
Cycle Length: 110
Offset: 84 (78%), Referenced to phase 2:NBT and 6:SBT, Start of Green
Natural Cycle: 90
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.93
Intersection Signal Delay: 37.1
Intersection Capacity Utilization 78.6%
Analysis Period (min) 15
Intersection LOS: D
ICU Level of Service D

2010 AM Peak BUILD Conditions - MITIGATED w/Triples Lefts
Case F - full access at Intersection 12
D:\ATOBEP\PROJECTS\Heritage_Neighborhood_Marketplace_Ladera_UnserCaseF\Triple_Lefts\2010AB_Mit_TriplesLefts.sy

HCM Signalized Intersection Capacity Analysis
2: Ladera Dr & Unser Blvd

Terry O. Brown, P.E.
1/14/2008

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	190	450	597	190	450	597	190	450	597	190	450	597
Volume (vph)	190	450	597	190	450	597	190	450	597	190	450	597
Turn Type	pm+ov	pm+ov	pm+ov	pm+ov	pm+ov	pm+ov	pm+ov	pm+ov	pm+ov	pm+ov	pm+ov	pm+ov
Protected Phases	7	4	4	7	4	4	7	4	4	7	4	4
Permitted Phases	7	4	4	7	4	4	7	4	4	7	4	4
Detector Phases	7	4	4	7	4	4	7	4	4	7	4	4
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	10.0	21.0	10.0	21.0	10.0	21.0	10.0	21.0	10.0	21.0	10.0	21.0
Total Split (s)	21.0	22.0	14.0	20.0	21.0	14.0	20.0	21.0	22.0	14.0	20.0	21.0
Total Split (%)	19.1%	20.0%	12.7%	18.2%	19.1%	12.7%	18.2%	19.1%	20.0%	12.7%	18.2%	19.1%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lead/Lag	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag	Lead	Lag	Lead	Lag
Lead-Lag Optimizes?	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min
Recall Mode	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min
Act Effd Green (s)	35.5	19.0	32.9	17.0	19.5	10.9	54.0	74.0	8.0	51.1	70.8	70.8
Actuated g/c Ratio	0.32	0.17	0.30	0.15	0.18	0.10	0.49	0.67	0.07	0.46	0.64	0.64
v/c Ratio	0.69	0.87	0.82	0.90	0.61	0.57	0.48	0.41	0.57	0.93	0.11	0.11
Control Delay	38.4	61.0	44.3	59.8	32.3	56.9	14.1	7.9	57.9	35.8	1.3	1.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	38.4	61.0	44.3	59.8	32.3	56.9	14.1	7.9	57.9	35.8	1.3	1.3
LOS	D	E	D	E	C	E	B	A	E	D	A	A
Approach Delay	48.5											
Approach LOS	D											



Splits and Phases: 2: Ladera Dr & Unser Blvd
Cycle Length: 110
Offset: 84 (78%), Referenced to phase 2:NBT and 6:SBT, Start of Green
Natural Cycle: 90
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.93
Intersection Signal Delay: 37.1
Intersection Capacity Utilization 78.6%
Analysis Period (min) 15
Intersection LOS: D
ICU Level of Service D

2010 AM Peak BUILD Conditions - MITIGATED w/Triples Lefts
Case F - full access at Intersection 12
D:\ATOBEP\PROJECTS\Heritage_Neighborhood_Marketplace_Ladera_UnserCaseF\Triple_Lefts\2010AB_Mit_TriplesLefts.sy

Timings

2: Ladera Dr & Unser Blvd

Terry O. Brown, P.E.
1/14/2008

Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	192	334	369	594	496	607	1524	708	290	1169	333
Volume (vph)	pm+pt	7	4	5	3	8	5	2	3	1	6
Turn Type	pm+pt	7	4	5	3	8	5	2	3	1	6
Protected Phases	7	4	5	3	8	5	2	3	1	6	7
Permitted Phases	7	4	5	3	8	5	2	3	1	6	7
Detector Phases	7	4	5	3	8	5	2	3	1	6	7
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	10.0	21.0	10.0	10.0	21.0	10.0	21.0	10.0	10.0	21.0	10.0
Total Split (s)	14.0	24.0	26.0	21.0	31.0	26.0	61.0	21.0	14.0	49.0	14.0
Total Split (%)	11.7%	20.0%	21.7%	17.5%	25.8%	21.7%	50.8%	17.5%	11.7%	40.8%	11.7%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lead/Lag	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag	Lead	Lag	Lead
Lead-Lag Optimizer?	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min
Recall Mode	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min
Act Effct Green (s)	32.0	21.0	47.0	18.0	28.0	23.0	58.0	79.0	11.0	46.0	80.0
Actuated g/C Ratio	0.27	0.18	0.39	0.15	0.23	0.19	0.48	0.66	0.09	0.38	0.50
w/c Ratio	0.83	0.58	0.36	0.86	0.99	0.98	0.95	0.71	0.97	0.91	0.43
Control Delay	77.2	49.9	25.6	61.5	69.6	81.1	33.9	13.8	87.2	40.1	21.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	77.2	49.9	25.6	61.5	69.6	81.1	33.9	13.8	87.2	40.1	21.5
LOS	E	D	C	E	E	F	C	B	F	D	C
Approach Delay	45.7	D	D	66.1	E	E	39.0	D	44.3	D	D
Approach LOS	D	D	D	E	E	E	D	D	D	D	D
Intersection Summary											
Cycle Length: 120											
Actuated Cycle Length: 120											
Offset: 26 (22%), Referenced to phase 2:NBT and 6:SBT; Start of Green											
Natural Cycle: 90											
Control Type: Actuated-Coordinated											
Maximum v/c Ratio: 0.99											
Intersection Signal Delay: 46.7											
Intersection Capacity Utilization 97.0%											
Analysis Period (min) 15											

Splits and Phases: 2: Ladera Dr & Unser Blvd



HCM Signalized Intersection Capacity Analysis

2: Ladera Dr & Unser Blvd

Terry O. Brown, P.E.
1/14/2008

Movement	EBL	EBT	EBR	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Total Lost time (s)	1.00	0.95	0.88	0.94	0.95	0.87	0.85	1.00	0.97	0.95	1.00
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Flt Protected	0.95	1.00	1.00	0.95	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1752	3505	2760	4942	3316	3400	3505	1568	3400	3505	1568
Flt Permitted	0.19	1.00	1.00	0.95	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	351	3505	2760	4942	3316	3400	3505	1568	3400	3505	1568
Volume (vph)	192	334	369	594	496	607	1524	708	290	1169	333
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Adj. Flow (vph)	206	359	397	639	533	300	639	1604	745	302	1218
RTOR Reduction (vph)	0	0	13	0	65	0	0	19	0	0	18
Lane Group Flow (vph)	206	359	384	639	766	0	639	1604	726	302	1218
Turn Type	pm+pt	7	4	5	3	8	5	2	3	1	6
Protected Phases	7	4	5	3	8	5	2	3	1	6	7
Permitted Phases	7	4	5	3	8	5	2	3	1	6	7
Actuated Green, G (s)	28.0	19.0	40.0	16.0	26.0	21.0	56.0	72.0	9.0	44.0	53.0
Effective Green, g (s)	32.0	21.0	44.0	18.0	28.0	23.0	58.0	76.0	11.0	46.0	57.0
Actuated g/C Ratio	0.27	0.18	0.37	0.15	0.23	0.19	0.48	0.63	0.09	0.38	0.48
Clearance Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	222	613	1081	741	774	652	1694	1032	312	1344	784
v/c Ratio Prot	c0.09	0.10	0.07	c0.13	c0.23	c0.19	c0.46	0.11	0.09	0.35	0.04
v/c Ratio Perm	0.16	0.07	0.07	0.06	0.09	0.08	0.99	0.36	0.17	0.17	0.17
Uniform Delay, d1	39.1	45.5	27.7	49.8	45.9	48.3	29.5	14.5	54.3	35.0	20.7
Progression Factor	1.00	1.00	1.00	0.98	0.97	1.06	0.71	0.78	0.94	0.91	1.16
Incremental Delay, d2	40.5	1.4	0.2	10.0	30.0	29.3	12.0	2.1	34.3	7.8	0.3
Delay (s)	79.6	46.9	27.9	58.7	74.5	80.5	33.1	13.4	85.3	38.5	24.3
Level of Service	E	D	C	E	E	F	C	B	F	D	C
Approach Delay (s)	46.1	D	D	67.7	E	38.3	D	D	44.1	D	D
Approach LOS	D	D	D	E	E	D	D	D	D	D	D
Intersection Summary											
HCM Average Control Delay	46.8										
HCM Volume to Capacity ratio	0.94										
Actuated Cycle Length (s)	120.0										
Intersection Capacity Utilization	97.0%										
Analysis Period (min)	15										
c Critical Lane Group											

2010 PM Peak BUILD Conditions - MITIGATED w/Trip Left Turn Lanes Case F - full access at Intersection 12
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Timings

2: Ladera Dr & Unser Blvd

Terry O. Brown, P.E.
1/14/2008

Movement	EBL	EBT	EBR	WBT	WBL	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	199	460	597	196	163	376	704	376	125	1355	105	105
Volume (vph)	199	460	597	196	163	376	704	376	125	1355	105	105
Turn Type	pm+pl	pm+ov	pm+ov	Prot	custom	Prot	pm+ov	pm+ov	Prot	pm+ov	pm+ov	pm+ov
Protected Phases	7	4	5	8	5	2	3	1	6	7	6	6
Permitted Phases	4	4	4	4	4	4	4	4	4	4	4	4
Detector Phases	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Initial (s)	10.0	21.0	10.0	21.0	10.0	21.0	10.0	21.0	10.0	21.0	10.0	10.0
Minimum Split (s)	18.0	29.0	12.0	21.0	12.0	60.0	10.0	11.0	59.0	18.0	18.0	18.0
Total Split (%)	16.4%	26.4%	10.9%	18.1%	10.9%	54.5%	9.1%	10.0%	53.6%	16.4%	16.4%	16.4%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lead/Lag	Lead	Lag	Lag	Lead	Lead	Lead	Lead	Lead	Lead	Lag	Lag	Lag
Lead-Lag Optimize?	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min
Recall Mode	33.7	23.7	36.8	16.0	10.1	58.3	68.3	8.9	57.2	75.0	75.0	75.0
Act Effect Green (s)	0.31	0.22	0.33	0.15	0.09	0.53	0.62	0.08	0.52	0.98	0.98	0.98
Actuated g/C Ratio	0.77	0.70	0.65	0.71	0.62	0.45	0.44	0.51	0.84	0.11	0.11	0.11
Control Delay	48.7	45.0	24.9	37.0	60.7	12.8	9.6	54.6	25.1	1.3	1.3	1.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	48.7	45.0	24.9	37.0	60.7	12.8	9.6	54.6	25.1	1.3	1.3	1.3
LOS	D	D	C	D	E	B	A	D	C	A	A	A
Approach Delay	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0
Approach LOS	D	D	D	D	D	D	D	D	D	D	D	D

Intersection Summary
Cycle Length: 110
Actuated Cycle Length: 110
Offset: 84 (76%), Referenced to phase 2:NBT and 6:SBT, Start of Green
Natural Cycle: 80
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.84
Intersection Signal Delay: 27.4
Intersection Capacity Utilization 76.4%
Analysis Period (min) 15

Splits and Phases: 2: Ladera Dr & Unser Blvd



2010 AM Peak BUILD Conditions - MITIGATED w/Left Turn Flyover
Case F - full access at Intersection 12
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A-4

HCM Signalized Intersection Capacity Analysis

2: Ladera Dr & Unser Blvd

Terry O. Brown, P.E.
1/14/2008

Movement	EBL	EBT	EBR	WBT	WBL	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	199	460	597	196	163	376	704	376	125	1355	105	105
Volume (vph)	199	460	597	196	163	376	704	376	125	1355	105	105
Peak-hour factor, PHF	0.87	0.87	0.87	0.79	0.79	0.79	0.85	0.85	0.85	0.89	0.89	0.89
Adj. Flow (vph)	229	529	686	0	248	177	192	828	442	140	1522	118
RTOR Reduction (vph)	0	0	134	0	120	0	0	25	0	0	0	41
Lane Group Flow (vph)	229	529	552	0	305	0	182	828	417	140	1522	77
Turn Type	pm+pl	pm+ov	pm+ov	Prot	custom	Prot	pm+ov	pm+ov	Prot	pm+ov	pm+ov	pm+ov
Protected Phases	7	4	5	8	5	2	3	1	6	7	6	6
Permitted Phases	4	4	4	4	4	4	4	4	4	4	4	4
Actuated Green, G (s)	31.8	21.8	29.9	14.0	8.1	56.3	61.3	6.9	55.1	67.9	67.9	67.9
Effective Green, g (s)	33.8	23.8	33.9	16.0	10.1	58.3	65.3	8.9	57.1	71.9	71.9	71.9
Actuated g/C Ratio	0.31	0.22	0.31	0.15	0.09	0.53	0.58	0.08	0.52	0.65	0.65	0.65
Clearance Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	303	758	926	478	312	1858	974	275	1819	1068	1068	1068
v/s Ratio Prot	c0.10	0.15	c0.05	0.09	0.06	0.24	0.03	0.04	c0.43	0.01	0.01	0.01
v/s Ratio Perm	c0.13	0.15	0.15	0.54	0.62	0.45	0.43	0.51	0.84	0.07	0.07	0.07
Uniform Delay, d1	31.3	39.8	32.3	44.3	48.1	15.9	12.2	48.5	22.5	6.9	6.9	6.9
Progression Factor	1.00	1.00	1.00	1.02	1.08	0.73	0.84	0.98	0.98	0.93	0.93	0.93
Incremental Delay, d2	10.3	2.8	1.0	2.8	3.6	0.8	0.3	1.4	4.4	0.0	0.0	0.0
Delay (s)	41.6	42.6	33.3	48.0	54.7	12.5	10.6	49.4	24.4	6.5	6.5	6.5
Level of Service	D	D	C	D	D	B	B	D	D	C	C	C
Approach Delay (s)	38.0	38.0	38.0	48.0	48.0	17.4	17.4	25.1	25.1	25.1	25.1	25.1
Approach LOS	D	D	D	D	D	B	B	C	C	C	C	C

Intersection Summary

HCM Average Control Delay	28.5	HCM Level of Service	C
HCM Volume to Capacity ratio	0.77		
Actual Cycle Length (s)	110.0	Sum of lost time (s)	6.0
Intersection Capacity Utilization	76.4%	ICU Level of Service	D
Analysis Period (min)	15		

c Critical Lane Group

2010 AM Peak BUILD Conditions - MITIGATED w/Left Turn Flyover
Case F - full access at Intersection 12
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Timings

2: Ladera Dr & Unser Blvd

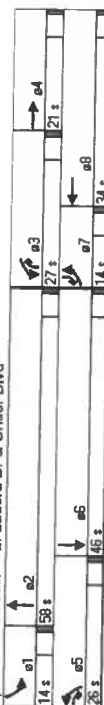
Terry O. Brown, P.E.

1/14/2008

Movement	EBL	EBT	EBR	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	192	334	369	934	496	607	1524	708	290	1169	333
Volume (vph)	192	334	369	934	496	607	1524	708	290	1169	333
Turn Type	pm+ov	pm+ov	pm+ov	pm+ov	pm+ov	pm+ov	pm+ov	pm+ov	pm+ov	pm+ov	pm+ov
Protected Phases	7	4	5	3	8	5	2	3	1	6	7
Permitted Phases	4	4	4	3	8	5	2	3	1	6	7
Detector Phases	7	4	5	3	8	5	2	3	1	6	7
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	10.0	21.0	10.0	21.0	10.0	21.0	10.0	21.0	10.0	21.0	10.0
Total Split (s)	14.0	21.0	26.0	27.0	34.0	26.0	58.0	27.0	14.0	46.0	14.0
Total Split (%)	11.7%	17.5%	21.7%	22.5%	28.3%	21.7%	48.3%	22.5%	11.7%	38.3%	11.7%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lead/Lag	Lead	Lag	Lead	Lead	Lag	Lead	Lag	Lead	Lag	Lead	Lag
Lead-Lag Optimize?	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min
Recall Mode	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min
Act Effd Green (s)	28.4	17.4	43.4	24.0	30.4	23.0	55.0	82.0	11.0	43.0	57.0
Actuated g/c Ratio	0.24	0.15	0.36	0.20	0.25	0.19	0.46	0.69	0.09	0.35	0.48
v/c Ratio	0.93	0.70	0.39	1.01	0.92	0.97	0.99	0.68	0.96	0.95	0.45
Control Delay	77.3	56.8	28.6	78.7	54.4	77.8	53.3	14.7	96.6	56.0	20.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	77.3	56.8	28.6	78.7	54.4	77.8	53.3	14.7	96.6	56.0	20.1
LOS	E	E	C	E	D	E	D	B	F	E	C
Approach Delay	49.5	49.5	28.6	78.7	54.4	77.8	53.3	14.7	96.6	56.0	20.1
Approach LOS	D	D	C	E	E	D	D	B	F	E	C
Intersection Summary											
Cycle Length: 120											
Actuated Cycle Length: 119.4											
Natural Cycle: 120											
Control Type: Semi Act-Uncoord											
Maximum v/c Ratio: 1.01											
Intersection Signal Delay: 55.2											
Intersection Capacity Utilization 97.0%											
Analysis Period (min) 15											

Intersection LOS: E
ICU Level of Service F

Splits and Phases: 2: Ladera Dr & Unser Blvd



HCM Signalized Intersection Capacity Analysis

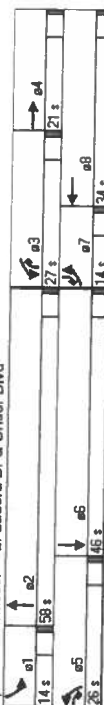
2: Ladera Dr & Unser Blvd

Terry O. Brown, P.E.

1/14/2008

Movement	EBL	EBT	EBR	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	192	334	369	934	496	607	1524	708	290	1169	333
Volume (vph)	192	334	369	934	496	607	1524	708	290	1169	333
Turn Type	pm+ov	pm+ov	pm+ov	pm+ov	pm+ov	pm+ov	pm+ov	pm+ov	pm+ov	pm+ov	pm+ov
Protected Phases	7	4	5	3	8	5	2	3	1	6	7
Permitted Phases	4	4	4	3	8	5	2	3	1	6	7
Detector Phases	7	4	5	3	8	5	2	3	1	6	7
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	10.0	21.0	10.0	21.0	10.0	21.0	10.0	21.0	10.0	21.0	10.0
Total Split (s)	14.0	21.0	26.0	27.0	34.0	26.0	58.0	27.0	14.0	46.0	14.0
Total Split (%)	11.7%	17.5%	21.7%	22.5%	28.3%	21.7%	48.3%	22.5%	11.7%	38.3%	11.7%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lead/Lag	Lead	Lag	Lead	Lead	Lag	Lead	Lag	Lead	Lag	Lead	Lag
Lead-Lag Optimize?	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min
Recall Mode	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min
Act Effd Green (s)	28.4	17.4	43.4	24.0	30.4	23.0	55.0	82.0	11.0	43.0	57.0
Actuated g/c Ratio	0.24	0.15	0.36	0.20	0.25	0.19	0.46	0.69	0.09	0.35	0.48
v/c Ratio	0.93	0.70	0.39	1.01	0.92	0.97	0.99	0.68	0.96	0.95	0.45
Control Delay	77.3	56.8	28.6	78.7	54.4	77.8	53.3	14.7	96.6	56.0	20.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	77.3	56.8	28.6	78.7	54.4	77.8	53.3	14.7	96.6	56.0	20.1
LOS	E	E	C	E	D	E	D	B	F	E	C
Approach Delay	49.5	49.5	28.6	78.7	54.4	77.8	53.3	14.7	96.6	56.0	20.1
Approach LOS	D	D	C	E	E	D	D	B	F	E	C
Intersection Summary											
Cycle Length: 120											
Actuated Cycle Length: 119.4											
Natural Cycle: 120											
Control Type: Semi Act-Uncoord											
Maximum v/c Ratio: 1.01											
Intersection Signal Delay: 55.2											
Intersection Capacity Utilization 97.0%											
Analysis Period (min) 15											

Splits and Phases: 2: Ladera Dr & Unser Blvd



2010 PM Peak BUILD Conditions - MITIGATED w/TripLCase8 TurRphases right-out, left-in access at Intersection 12
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A-7

HCM Signalized Intersection Capacity Analysis 2: Ladera Dr & Unser Blvd

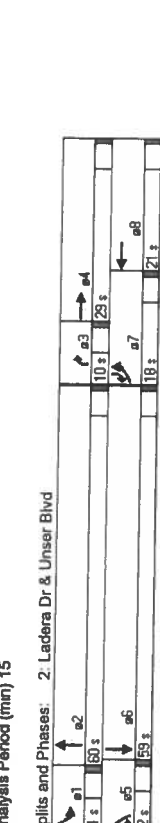
Terry O. Brown, P.E.
 1/14/2008

Timings

2: Ladera Dr & Unser Blvd

Lane Group	EBL	EBT	EBR	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	199	163	704	376	125	1355	105			
Volume (vph)	199	163	704	376	125	1355	105			
Turn Type	pm+pt	pm+ov	pm+ov	pm+ov	pm+ov	pm+ov	pm+ov			
Protected Phases	7	4	5	8	5	2	3	1	6	7
Permitted Phases	4	4	5	8	5	2	3	1	6	7
Detector Phases	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Initial (s)	10.0	21.0	10.0	21.0	10.0	21.0	10.0	10.0	10.0	10.0
Minimum Split (s)	18.0	29.0	12.0	21.0	12.0	60.0	10.0	11.0	59.0	18.0
Total Split (s)	16.4%	28.4%	10.9%	19.1%	10.9%	54.5%	9.1%	10.0%	53.6%	16.4%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lead/Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lead	Lag
Lead-Lag Optimize?	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min
Recall Mode	33.7	23.7	36.8	15.9	10.1	58.3	68.3	9.0	57.2	74.9
Act Eff Green (s)	0.31	0.22	0.33	0.14	0.09	0.53	0.62	0.08	0.52	0.68
Actuated g/C Ratio	0.77	0.70	0.65	0.71	0.61	0.45	0.44	0.51	0.84	0.11
Control Delay	48.7	45.1	24.9	36.5	57.5	17.1	11.2	55.5	28.0	1.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	48.7	45.1	24.9	36.5	57.5	17.1	11.2	55.5	28.0	1.4
LOS	D	D	C	D	E	B	B	E	C	A
Approach Delay	36.1	36.5	36.5	36.5	36.5	36.5	36.5	36.5	36.5	36.5
Approach LOS	D	D	D	D	D	C	C	D	C	C

Intersection Summary
 Cycle Length: 110
 Actuated Cycle Length: 110
 Offset: 84 (76%), Referenced to phase 2:NBT and 6:SBT, Start of Green
 Natural Cycle: 80
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.84
 Intersection Signal Delay: 28.0
 Intersection Capacity Utilization 76.4%
 Analysis Period (min) 15



Splits and Phases: 2: Ladera Dr & Unser Blvd

HCM Signalized Intersection Capacity Analysis 2: Ladera Dr & Unser Blvd

Terry O. Brown, P.E.
 1/14/2008

Timings

2: Ladera Dr & Unser Blvd

Movement	EBL	EBT	EBR	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	199	163	704	376	125	1355	105			
Volume (vph)	199	163	704	376	125	1355	105			
Turn Type	pm+pt	pm+ov	pm+ov	pm+ov	pm+ov	pm+ov	pm+ov			
Protected Phases	7	4	5	8	5	2	3	1	6	7
Permitted Phases	4	4	5	8	5	2	3	1	6	7
Detector Phases	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Initial (s)	10.0	21.0	10.0	21.0	10.0	21.0	10.0	10.0	10.0	10.0
Minimum Split (s)	18.0	29.0	12.0	21.0	12.0	60.0	10.0	11.0	59.0	18.0
Total Split (s)	16.4%	28.4%	10.9%	19.1%	10.9%	54.5%	9.1%	10.0%	53.6%	16.4%
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lead/Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lead	Lag
Lead-Lag Optimize?	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min
Recall Mode	33.7	23.7	36.8	15.9	10.1	58.3	68.3	9.0	57.2	74.9
Act Eff Green (s)	0.31	0.22	0.33	0.14	0.09	0.53	0.62	0.08	0.52	0.68
Actuated g/C Ratio	0.77	0.70	0.65	0.71	0.61	0.45	0.44	0.51	0.84	0.11
Control Delay	48.7	45.1	24.9	36.5	57.5	17.1	11.2	55.5	28.0	1.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	48.7	45.1	24.9	36.5	57.5	17.1	11.2	55.5	28.0	1.4
LOS	D	D	C	D	E	B	B	E	C	A
Approach Delay	36.1	36.5	36.5	36.5	36.5	36.5	36.5	36.5	36.5	36.5
Approach LOS	D	D	D	D	D	C	C	D	C	C

Intersection Summary
 HCM Average Control Delay 30.0
 HCM Volume to Capacity ratio 0.77
 Actuated Cycle Length (s) 110.0
 Intersection Capacity Utilization 76.4%
 Analysis Period (min) 15
 Critical Lane Group C

Splits and Phases: 2: Ladera Dr & Unser Blvd

2010 PM Peak Hour Delays (Case "F" & Case "L") Ladera Dr. / Unser Blvd.

