CITY OF ALBUQUERQUE

PLANNING DEPARTMENT – Development Review Services



August 25, 2015

Reza Afaghpour, P.E. SBS Construction and Engineering P.O. Box 10264 Albuquerque, NM 87184 Richard J. Berry, Mayor

PO Box 1293

Albuquerque

New Mexico 87103

www.cabq.gov

## RE: Western United Electric Conceptual Grading and Drainage Plan Engineer's Stamp Date 8-9-2015 (H10D029)

Dear Mr. Afaghpour:

Based upon the information provided in your submittal received 8-18-2015, the above referenced plan is approved for action by the DRB on the Site Plan for Building Permit. Prior to Building Permit the following comments must be addressed:

- While the DMP states free discharge to La Morada is allowed for this site, the site is still limited to the discharge used in the original analysis. The AHYMO run shows 8.13 cfs. A copy of this information should be provided as part of the excepts, but I will include it in the file for you. It is based on a D=80%
  - 2. DMP shows that offsite basin 6-A was intended to flow thru this site to La Morada. Please address.
  - 3. In the runoff calculations shown on plan, do not include the AHYMO input/output for the 10yr storm as it is not needed and confusing.
  - 4. Site plan shows that crushed gravel is used in the north portion of the lot and should be noted on the grading plan. Is the intent to pave in the future? The AHYMO input shows D= 88%. Should it be lowered?
  - 5. How are flows from Pond A getting to Pond B? Show a channel or swale and a section cut.
  - 6. SW culverts are sized using the orifice equation but they are above the pond. Use the weir equation.
  - 7. Show how flows are conveyed from Pond B to the SW culvert. Provide additional spot elevations around Pond B.
  - 8. Provide SO-19 notes if this is not going thru the Work Order process.
  - 9. The 1ft retaining curb does not appear to be sufficient. Show slope ties.

If you have any questions, you can contact me at 924-3695.

Sincerely,

Rita Harmon, P.E. Senior Engineer, Planning Dept. Development Review Services

Orig: Drainage file c.pdf: via Email: Shawn Biazar



## City of Albuquerque

Planning Department

Development & Building Services Division

DRAINAGE AND TRANSPORTATION INFORMATION SHEET

(REV 02/2013)

Project Title:	Building Permit #:	City Drainage #:	
DRB#: EPC#:		Work Order#:	
Legal Description:			
City Address:			
Engineering Firm:		Contact:	
Address:			
Phone#: Fax#:		E-mail:	
Owner:		Contact:	
Address:			
Phone#: Fax#:		E-mail:	
Architect:		Contact:	
Address:			
Phone#: Fax#:		E-mail:	
Surveyor:		Contact:	
Address:			
Phone#: Fax#:		E-mail:	
Contractor:		Contact:	
Address:			
Phone#: Fax#:		E-mail:	
TYPE OF SUBMITTAL:	CHECK TYPE OF APPROV	AL/ACCEPTANCE SOUGHT:	
DRAINAGE REPORT	SIA/FINANCIAL GUARAN	TEE RELEASE	
DRAINAGE PLAN 1st SUBMITTAL	PRELIMINARY PLAT APPI	ROVAL	
DRAINAGE PLAN RESUBMITTAL	S. DEV. PLAN FOR SUB'D	APPROVAL	
CONCEPTUAL G & D PLAN	S. DEV. FOR BLDG. PERMI	IT APPROVAL	
GRADING PLAN	SECTOR PLAN APPROVAL		
EROSION & SEDIMENT CONTROL PLAN (ESC)	FINAL PLAT APPROVAL		
ENGINEER'S CERT (HYDROLOGY)	CERTIFICATE OF OCCUPA	ANCY (PERM)	
CLOMR/LOMR	CERTIFICATE OF OCCUPANCY (TCL TEMP)		
TRAFFIC CIRCULATION LAYOUT (TCL)	FOUNDATION PERMIT APPROVAL		
ENGINEER'S CERT (TCL)	BUILDING PERMIT APPRO	DVAL	
ENGINEER'S CERT (DRB SITE PLAN)	GRADING PERMIT APPRO	VAL SO-19 APPROVAL	
ENGINEER'S CERT (ESC)	PAVING PERMIT APPROV	AL ESC PERMIT APPROVAL	
SO-19	WORK ORDER APPROVAL	ESC CERT. ACCEPTANCE	
OTHER (SPECIFY)	GRADING CERTIFICATION	N OTHER (SPECIFY)	
WAS A PRE-DESIGN CONFERENCE ATTENDED:	Yes No Co	ppy Provided	
DATE SUBMITTED:	By:		

Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a drainage submittal. The particular nature, location, and scope to the proposed development defines the degree of drainage detail. One or more of the following levels of submittal may be required based on the following

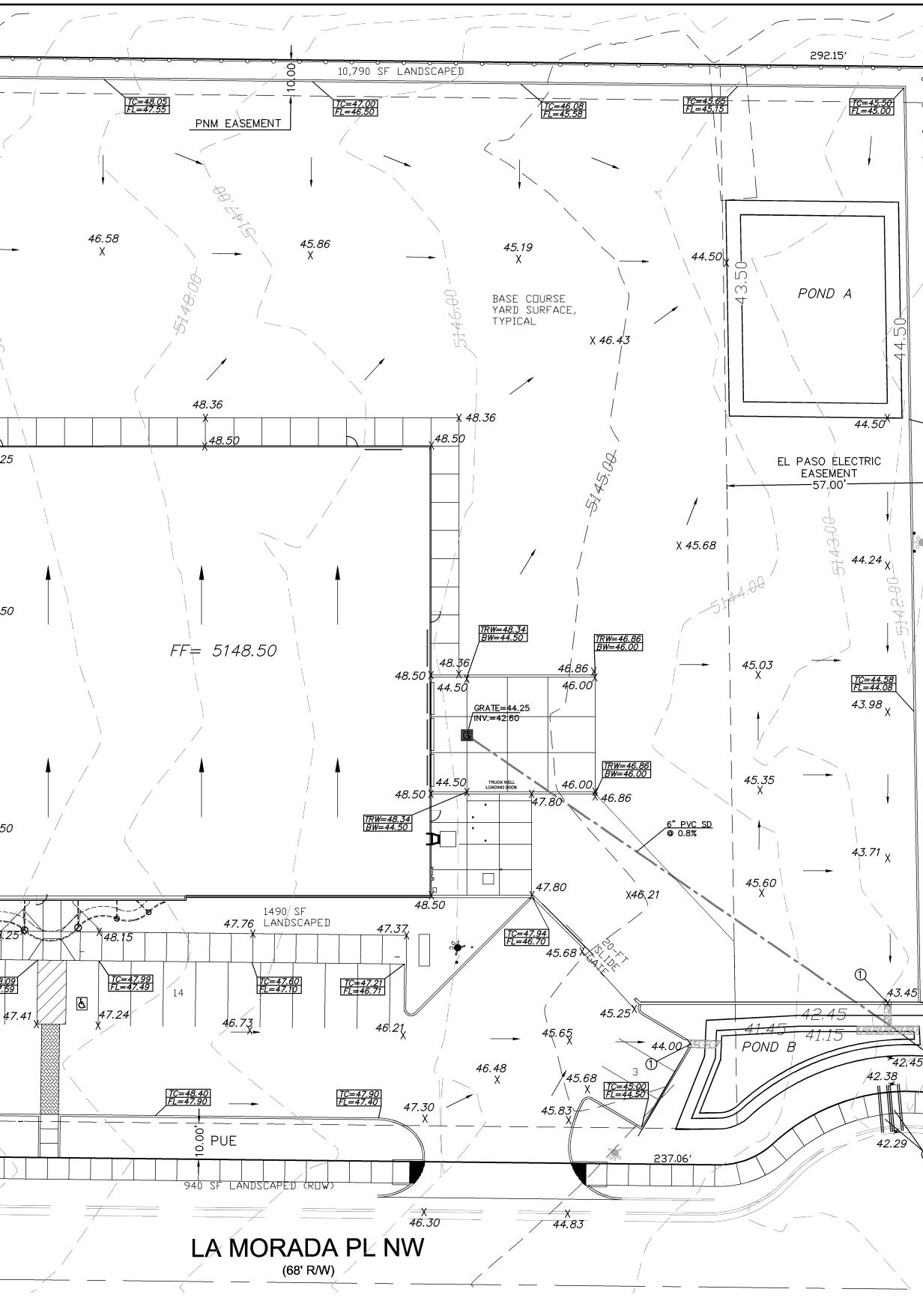
1. Conceptual Grading and Drainage Plan: Required for approval of Site Development Plans greater than five (5) acres and Sector Plans

2. Drainage Plans: Required for building permits, grading permits, paving permits and site plans less than five (5) acres

3. **Drainage Report**: Required for subdivision containing more than ten (10) lots or constituting five (5) acres or more

4. Erosion and Sediment Control Plan: Required for any new development and redevelopment site with 1-acre or more of land disturbing area, including project less than 1-acre than are part of a larger common plan of development

<u>NOTICE TO CONTRACTORS</u> 1. AN EXCAVATION/CONSTRUCTION PERMIT WILL BE REQUIRED BEFORE BEGINNING AN WORK WITHIN CITY RIGHT-OF-WAY.		N89*22'16"W			
2. ALL WORK DETAILED ON THESE PLANS TO BE PERFORMED, EXCEPT AS OTHERWISE STATED OR PROVIDED HEREON, SHALL BE CONSTRUCTED IN ACCORDANCE WITH CITY OF ALBUQUERQUE INTERIM STANDARD SPECIFICATIONS FOR PUBLIC WORKS CONSTRUCTION,				8 10,790 SF L	ANDSCAPED
1985. 3. TWO WORKING DAYS PRIOR TO ANY EXCAVATION, CONTRACTOR MUST CONTACT LINE LOCATING SERVICE, 765–1234, FOR LOCATION OF EXISTING UTILITIES.		TC=48.75 FL=47.75	<u>TC=48.05</u> FL=47.55 PNM	EASEMENT	
4. PRIOR TO CONSTRUCTION, THE CONTRACTOR SHALL EXCAVATE AND VERIFY THE HORIZONTAL AND VERTICAL LOCATIONS OF ALL CONSTRUCTIONS. SHOULD A CONFLICT EXIST, THE CONTRACTOR SHALL NOTIFY THE ENGINEER SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM AMOUNT OF DELAY.	30				
5. BACKFILL COMPACTION SHALL BE ACCORDING TO TRAFFIC/STREET USE. 6. MAINTENANCE OF THESE FACILITIES SHALL BE THE RESPONSIBILITY OF THE OWNER			46.58	45.86	45.19
OF THE PROPERTY SERVED. 7. WORK ON ARTERIAL STREETS SHALL BE PERFORMED ON A 24-HOUR BASIS.	<u>TC=48.40</u> FL=47.27		* \	- X	X
<ul> <li><u>GENERAL NOTES:</u></li> <li>1: CONTOUR INTERVAL IS HALF (1.00) FOOT.</li> <li>2: ELEVATIONS ARE BASED ON CITY OF ALBUQUERQUE CONTROL STATION 4_H9, HAVING AN ELEVATION OF <u>5209.315</u> FEET ABOVE SEA LEVEL.</li> <li>3: UTILITIES SHOWN HEREON ARE IN THEIR APPROXIMATE LOCATION BASED ONLY ON ABOVE GROUND EVIDENCE FOUND IN THE FIELD AND AS-BUILT INFORMATION PROVIDED BY THE CLIENT. UTILITIES SHOWN HEREON, WHETHER INDICATED AS ABANDONED OR NOT, SHALL BE VERIFIED BY OTHERS FOR EXACT LOCATION AND/ OR DEPTH PRIOR TO EXCAVATION OR DESIGN CON-SIDERATIONS.</li> <li>4: THIS IS <u>NOT</u> A BOUNDARY SURVEY, BEARINGS ARE ASSUMED, DISTANCES</li> </ul>		48.01 ×	48.36 48.36	5b	A 48.36 48.50
AND FOUND PROPERTY CORNERS ARE FOR <u>INFORMATIONAL PURPOSES ONLY.</u> 5: SLOPES ARE AT 3:1 MAXIMUM.	<u>TC=48.56</u> <u>FL=47.56</u>	48.25	\ \		
Location TRACT 6, LADERA BUSINESS PARK, UNIT 1 Tracr 6, Ladera Business Park, unit 1 is located at 7311 La Morada Pl., NW, and contains +/- 2.0686 Acres. See attached portion of the Vicinity Map for exact location. Purpose	<u>MATCH EXIST.</u> GRADE				
The purpose of this drainage report is to present a grading and drainage solution for new building and improvements with this tract of land.	28,00				
<ul> <li>Existing Drainage Conditions</li> <li>This site falls within Master Drainage Plan for the Ladra Business Park, Area 1 (H10/D06A) prepared by Mark</li> <li>Goodwin and Associates. Area 1, discharging directly into streets at various locations which eventually drains directly into existing storm drain system desing for this development.</li> </ul>	TC=48.90 FL=47.90	48.36 × 48.50	FF = 5	148.50	TRW=48.34 BW=44.50 48.36
Proposed Conditions and On-Site Drainage Management Plan Since the Master Plan (File H10/D06A) is designed for complete discahrge, we are proposing to pond the 90th Percentile/First Flush requirement which is is 0.34 inches times the impervious area 77,390.18 (2,192.17 cf). Total retention volume provided (3,403.69 cf) far exceeds the ponding requirement for First Flush (2,192.17 cf).				48	8.50 × 44.50 GRATE=44.25
<b>Calculations</b> City of Albuquerque, Development Process Manuel, Section 22.2, Hydrology Section, was used for runoff calculations. See this plan for AHYMO input and Summary output files.					INV.=42 60
POND VOLUME REQUIRED TOTAL PONDING VOLUME REQUIRED (90TH PERCENTILE/FIRST FLUSH) = 0.34 INCHES x (0.34/12 x 77,390.18) = 2,192.17 CF	IMPERVIOUS AREA = <u>MATCH EXIST.</u> GRADE	48.36		48 TRW=48.3 BW=44.50	8.50 44.50 LOADING BOOK 47. 47. 47.
<b>POND CALCULATION</b> TOTAL POND AREA PROVIDED = POND A + B = $3,403.69$ CF > $2,192.17$ CF PONDING CALCULATIONS:	TC=49.25 FL=48.25				
POND A: POND A: $AREA @ 44.50 = 2,881.05, AREA @ 43.50 = 1301.59$ POND A VOLUME = $(2,881.05 + 1,301.59)/2*1.0 = 2,091.32$ AREA @ 42.45 = 1240.57, AREA @ 41.15 = 509.26 POND B: POND B VOLUME = $(1,240.57 + 509.26)/2*1.50 = 1,312.37$		48.00 48.25	48.15	1490/ SF 47.76 LANDSCAPED 47.37	48.50 48.50 TC=47.94 FL=46.70
* ZONE 1 ************************************		$\begin{array}{c} \hline TC = 47.92 \\ \hline FL = 47.42 \\ \hline FL = 47.59 \\ \hline \end{array}$	TC=47.99 FL=47.49 14	$\begin{array}{c c} TC=47.60 \\ FL=47.10 \\ \hline FL=46.71 \\ \hline \end{array}$	
* 100-YEAR, 6-HR STORM (UNDER EXISTING CONDITIONS) * ***********************************	36 <sup>°</sup> E	<u>TC=48.50</u> FL=47.75		6.73 46.21	
RAINFALL TYPE=1 RAIN QUARTER=0.0 IN RAIN ONE=1.87 IN RAIN SIX=2.20 IN RAIN DAY=2.66 IN DT=0.03333 HR * ON-SITE		TC=49.00 FL=48.50			46.48 V X
COMPUTE NM HYD ID=1 HYD NO=101.0 AREA=0.003149 SQ MI PER A=0.00 PER B=100.00 PER C=0.00 PER D=0.00 TP=0.1333 HR MASS RAINFALL=-1		FL=48.50  			47.30 4
**************************************		N89*22′22″W			
START TIME=0.0 RAINFALL TYPE=1 RAIN QUARTER=0.0 IN RAIN ONE=1.25 IN RAIN SIX=1.47 IN RAIN DAY=1.77 IN DT=0.03333 HR			940 SF		
* ON-SITE COMPUTE NM HYD ID=1 HYD NO=111.0 AREA=0.003149 SQ MI PER A=100.00 PER B=100.00 PER C=0.00 PER D=0.00	``````````````````````````````````````			MORADA PL N	
TP=0.1333 HR MASS RAINFALL=-1 ************************************			/ <u>/</u>	(68' R/W) /	
START TIME=0.0 RAINFALL TYPE=1 RAIN QUARTER=0.0 IN RAIN ONE=1.87 IN RAIN SIX=2.20 IN RAIN DAY=2.66 IN DT=0.03333 HR	AHYMO PROGRAM SUMMARY TABLE INPUT FILE = MORADA.TXT	E (AHYMO_97) -	- VERSION:	1997.02d RUN DATE (MON USER NO.= AHYMO-I-	I/DAY/YR) =07/27/2015 -9702c01000R31-AH
* ON-SITE COMPUTE NM HYD ID=1 HYD NO=103.1 AREA=0.003149 SQ MI PER A=0.00 PER B=6.00 PER C=6.00 PER D=88.00 TP=0.1333 HR MASS RAINFALL=-1	HYDROGRAPH COMMAND IDENTIFICATION	FROM TO I ID ID AREA NO. NO. (SQ MI)	PEAK RUNOFF DISCHARGE VOLUME (CFS) (AC–FT) (	TIME TO CFS RUNOFF PEAK PER INCHES) (HOURS) ACRE	NOTATION
* 10-YEAR, 6-HR STORM (UNDER PROPOSED CONDITIONS) * ***********************************	START RAINFALL TYPE= 1 COMPUTE NM HYD 101.00 START	- 1 .00315	4.09 .112	.66738 1.533 2.031 P	TIME=     .00       RAIN6=     2.200       PER IMP=     .00       TIME=     .00
RAINFALL TYPE=1 RAIN QUARTER=0.0 IN RAIN ONE=1.25 IN RAIN SIX=1.47 IN RAIN DAY=1.77 IN DT=0.03333 HR	RAINFALL TYPE= 1 COMPUTE NM HYD 111.00 START	- 1 .00315	.94 .025	.14676 1.533 .468 P	RAIN6= 1.470 PER IMP= .00 TIME= .00
* ON-STIE COMPUTE NM HYD ID=1 HYD NO=111.1 AREA=0.003149 SQ MI PER A=0.00 PER B=6.00 PER C=6.00 PER D=88.00	RAINFALL TYPE= 1 COMPUTE NM HYD 103.10 START	- 1 .00315	8.35 .307	1.82751 1.500 4.145 P	RAIN6= 2.200 PER IMP= 88.00 TIME= .00
TP=0.1333 HR MASS RAINFALL=-1 ************************************	RAINFALL TYPE= 1 COMPUTE NM HYD 111.10 FINISH	- 1 .00315	5.40 .189		RAIN6= 1.470 ER IMP= 88.00



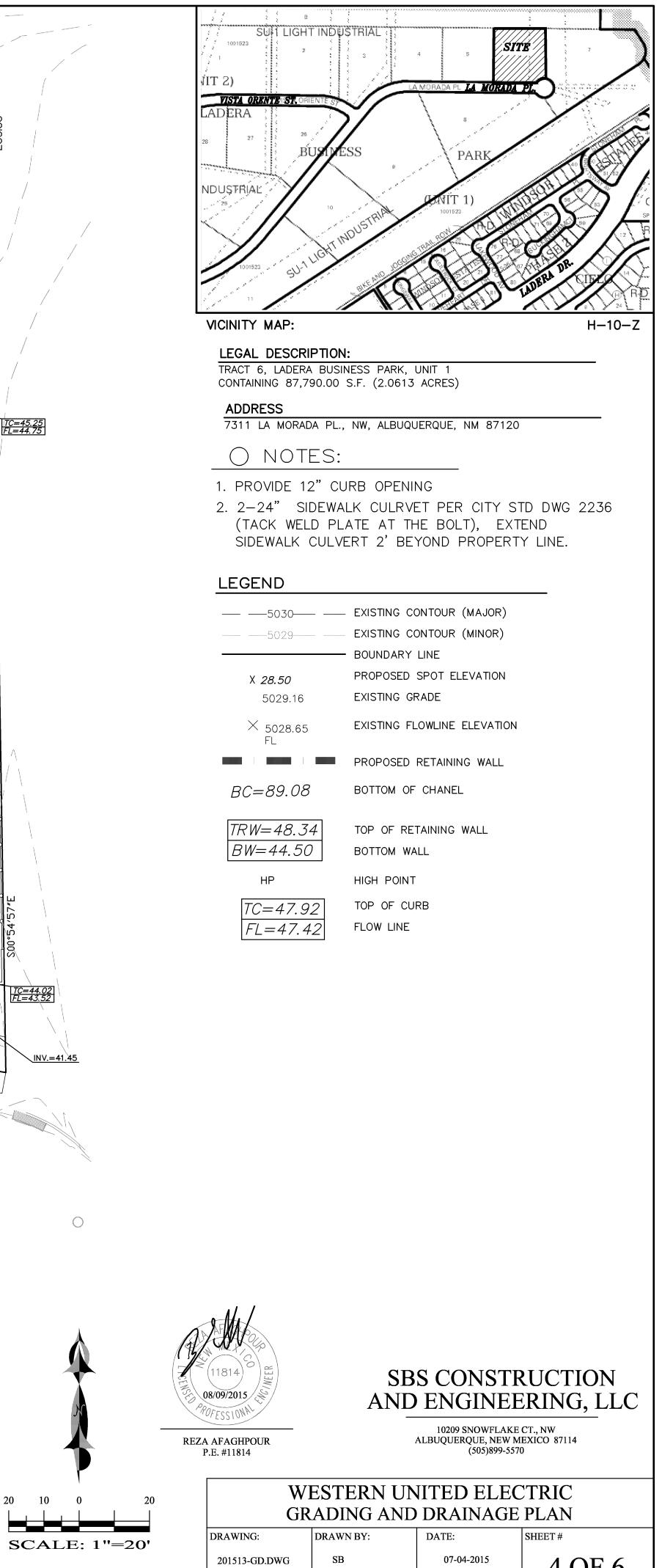
SIDEWALK CULVERT CALCULATIONS

2-24" Sidewalk Culvert Flow Capacity Calculation Using Orifice Equation Orifice Equation: Q=CA(2gh)^0.50

h (head) = 0.67'A = 0.67 sf g = 32.20

Q = 0.60 x 2.68 x(2 x 32.2 x 0.67)^0.50 = 10.56 cfs

<u>10.56 cfs >> 8.35 cfs (Entire runoff generated from site)</u>



LAST REVISION: 08-17-15

TC=45.25 FL=44.75

TC=44.02 FL=43.52

<u>INV.=41.45</u>

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