

### A Transmittal From

### Isaacson & Arfman, P.A.

Consulting Engineering Associates

TO: Shahab Biazar, P.E.

City of Albuquerque

Senior Engineer, Planning Dept.

DATE:

15 May 2012

JOB NO:

1908

FROM: Åsa Weber

REFERENCE: H-12 / D001 Los Duranes Community Center

WE ARE SENDING YOU ATTACHED THE FOLL	OWING ITEMS:
Revised Grading and Drainage Plan	
Revised Supplemental Calculations	
revised supplemental succession	
THIS INFORMATION IS TRANSMITTED:	
As per your request	For your files
For your review and approval	For your use
For your information	☐ Please review and return
For your attention	For return to your files
For your signature	☐ Please advise
COMMENTS	MAY I 9 2012
COMMENTS:	HYDROLOGY
Revisions made based on revised architectural site	plan and client revisions. Overall drainage and grading
concept remain as originally submitted. Calculation	
Please don't hesitate to call me or Bryan Bobrick a	t 268-8828 with any questions
Trease don't hesitate to can like of Diyan Dobliek a	at 200 0020 with any questions.
Thanks	

COPIES TO: File

MAY 15, 2012

### SUPPLEMENTAL INFORMATION

**FOR** 

### LOS DURANES COMMUNITY CENTER

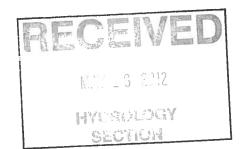
BY

ISAACSON & ARFMAN, P.A.

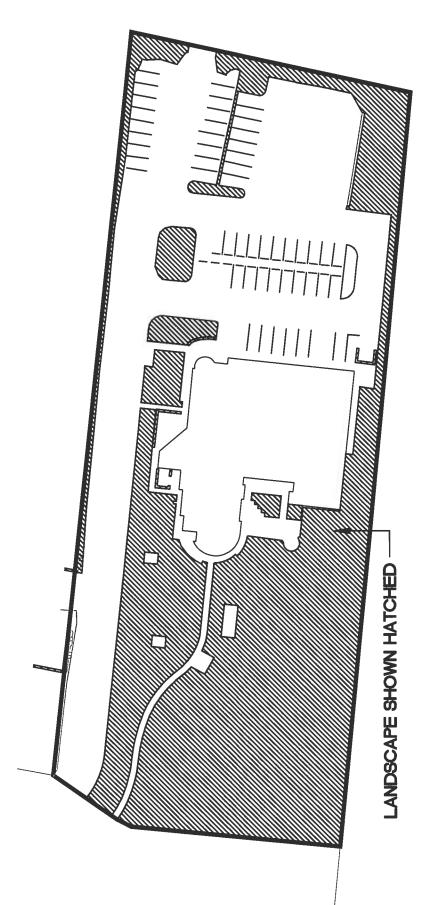
Consulting Engineering Associates

Thomas O. Isaacson, PE(RET.) & LS(RET.) Fred C. Arfman, PE Åsa Nilsson-Weber, PE

IA Project No. 1908







# AREA ANALYZED: 117,483 SF = 2.7 ACRES

IMPERVIOUS (LAND TREATMENT 'D'): - 65,596 SF

LANDSCAPE (LAND TREATMENT 'B'): - 38,387 SF

LANDSCAPE (LAND TREATMENT 'C'): - 13,500 SF

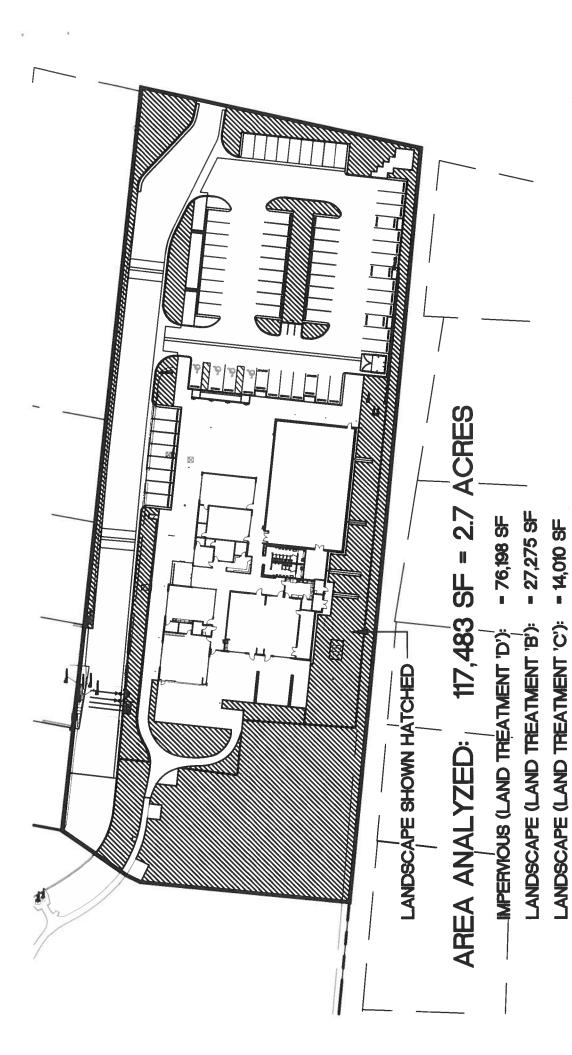
## EXISTING LAND TREATMENT EXHIBIT

LOS DURANES COMMUNITY CENTER MAY 14, 2012





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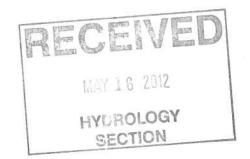
### LOS DURANES COMMUNITY CENTER PROPOSED LAND TREATMENT EXHIBIT

PROPOSED CONDITION:

MAY 14, 2012



		CALCIII ATIO	NG 1	0001 D			1 10	2012
Based on Draina				908 Los Duranes Co Albuquerque Section 2				
Dased on Dramaş	ge Desi	gii Cilicila foi C	ity of A	ON-SITE		ivi, voi 2, dated	Jan., 12	773
AREA OF SITE:				117483	SF	=	2.7	
				100-year, 6-hour				
HISTORIC FLO	OWS:			DEVELOPED FLO	WS:			<b>EXCESS PRECIP:</b>
		Treatment SF	%			Treatment SF	%	Precip. Zone 2
Area A	=	0	0%	Area A	=	0	0%	$E_A = 0.53$
Area B	=	38387	33%	Area B	=	27275	23%	$E_{\rm B} = 0.78$
Area C	=	13500	11%	Area C	=	14010	12%	$E_{\rm C} = 1.13$
Area D	=	65596	56%	Area D	=	76198	65%	$E_D = 2.12$
Total Area	=	117483	100%	Total Area	=	117483	100%	•
On-Site Weighted	d Exces		00-Ye	ar, 6-Hour Storm)				
		Weighted E =		$\underline{E_A A_A} + \underline{E_B A_B} + \underline{E_C A}$		$\underline{A}_{D}$		
				$A_A + A_B + A_C$	+ A <sub>D</sub>			•
Historic E	=	1.57	in.	Developed E	=	1.69	in.	
On-Site Volume	of Runo	off: V360 =		E*A / 12				_
Historic V <sub>360</sub>	=	15355	CF	Developed V <sub>360</sub>	=	16554	CF	
On-Site Peak Dis For Precipitation Q <sub>pA</sub>		Rate: $Qp = Q_{pA}$ 2 1.56	A <sub>A</sub> +Q <sub>p</sub>	$_{\rm B}A_{\rm B}+Q_{\rm pC}A_{\rm C}+Q_{\rm pD}A_{\rm D}$ / $Q_{\rm pC}$	43,560	3.14		-
$Q_{pB}$	=	2.28		$Q_{pD}$	=	4.70		
Historic Q <sub>p</sub>	=	10.1	CFS	Developed Q <sub>n</sub>	=		CFS	]
Ф.		10.11		1= 2: 2: 2 b 2 d d b		10.7		J





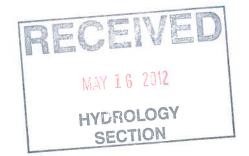
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Consulting Engineering Associates

Los Duranes Community Center

May 15, 2012

Drainage Summary:



Historic discharge patterns will be preserved. The proposed development will discharge stormwater to three separate basins, B1, B2 and B3 (see attached basin map).

Basin B1: Consisting of Leopoldo Rd., approximately 50% of the new parking lot and the majority of the new building. Stormwater from the gym will discharge to the south landscaped area. Excess will pass to the parking area and then combine with flow in Leopoldo Rd. to continue west to a public storm drain system at the west end of the park. 100-year 6-hour discharge = 6.7 cfs.

Basin B2: Consisting of approximately 50% of the new parking lot. Flows from this basin will be directed to one of two water harvesting / infiltration ponding areas. 100-year 6-hour discharge = 1.4 cfs (V100 = 2255 cf). The total ponding provided = 2266 cf with additional storage available within the rock voids. Two 7' long x 2' wide x 7' deep infiltration pits will be constructed within each of the landscaped areas to break through the existing clay lens (approx. depth = 5.5') to provide additional volume and to facilitate infiltration.

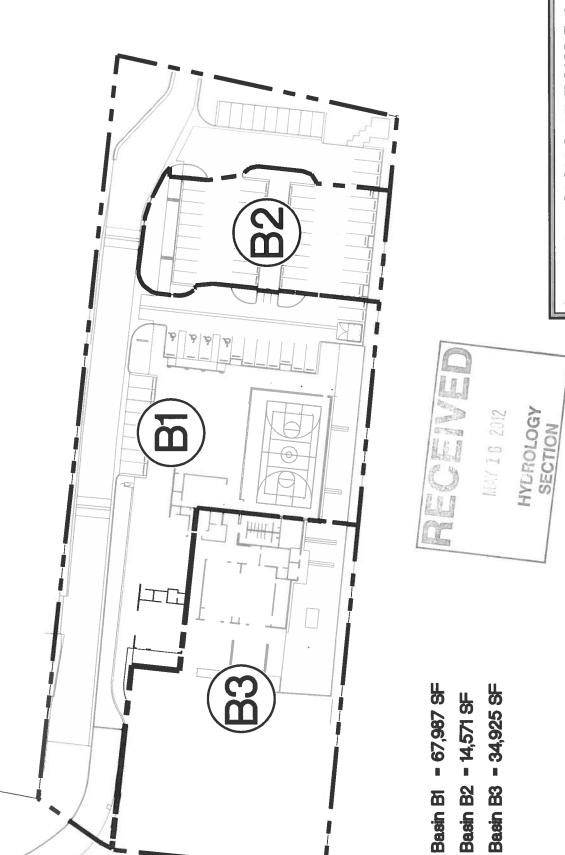
POND - NORTH ISLAND					
Contour	Area	Volume			
4964.50	883				
4964.00	490	343 CF			
4963.50	138	157 CF			
TOTAL V	OL.	<b>500</b> CF			

POND - CENTER ISLAND					
Contour	Area	Volume			
4965.00	4000				
4964.70	1200	780 CF			
4964.00	735	677 CF			
4963.50	500	309 CF			
TOTAL \	OL.	1766 CF			

Basin B3: Consisting of the western portion of the proposed building and the existing grass park area. Flows from this basin will discharge all stormwater to the landscaping / park for infiltration. Excess stormwater may continue west through the park as it does historically. 100-year 6-hour discharge = 2.6 cfs.

In conclusion, 100% of developed flow (1.4 cfs) from Basin B2 are captured in water harvesting / infiltration areas. Depressed landscaped areas are provided throughout the site to further reduce site runoff. Surface mulch (gravel) and rock within these depressions will provide additional storage.

The total discharge from the site is not expected to exceed 9.3 cfs which is less than the historic discharge of 10.1 cfs.



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LOS DURANES COMMUNITY CENTER

MAY 14, 2012

### PROPOSED BASINS

### 1908 DPM Calculations - REVISED.xlsm

BASIN NO. 1		DESC	CRIPTION					
Area of basin flows =	67987	SF			1.0	6 Ac.		
The following calculations are based on Treatment areas as shown in table to the r				ight	LAND TE	REATMENT		
	Sub-basin Weight	ed Excess Preci	pitation (see f	ormula ab	oove)	A =	0%	
	Weighted E	=	1.88	in.		B =	10%	
	Sub-basin Volum	e of Runoff (see formula above)				C =	11%	
	V <sub>360</sub>	=	10635	CF		D =	79%	
	Sub-basin Peak D	ischarge Rate: (	see formula al	ove)				
	$Q_{P}$	=	6.7	cfs				
BASIN NO. 2		DESC	CRIPTION					
Area of basin flows =	14571	SF		=	0			
The following calculation	ns are based on Tr	eatment areas as	s shown in tab	le to the ri	ight	LAND TE	REATMENT	
	Sub-basin Weight	ed Excess Preci	ipitation (see f	ormula ab	oove)	A =	0%	
	Weighted E	=	1.86			B =	10%	
	Sub-basin Volum	e of Runoff (see formula above)				C =	13%	
	V <sub>360</sub>	=	2255	CF		D =	77%	
	Sub-basin Peak Discharge Rate: (see formula above)							
	$Q_{P}$	=	1.4	cfs				
BASIN NO. 3		DESC	CRIPTION					
Area of basin flows =	34925	SF		=	0.			
The following calculations are based on Treatment areas as shown in table to the right				_	LAND TI	REATMENT		
	Sub-basin Weight	ed Excess Preci			oove)	A =	0%	
	Weighted E	=	1.27			B =	54%	
	Sub-basin Volum	e of Runoff (see	formula abov			C =	13%	
	V <sub>360</sub>	=	3690	CF		D =	33%	
	Sub-basin Peak D	ischarge Rate: (	see formula al	oove)				
	$Q_P$	=	2.6	cfs				

