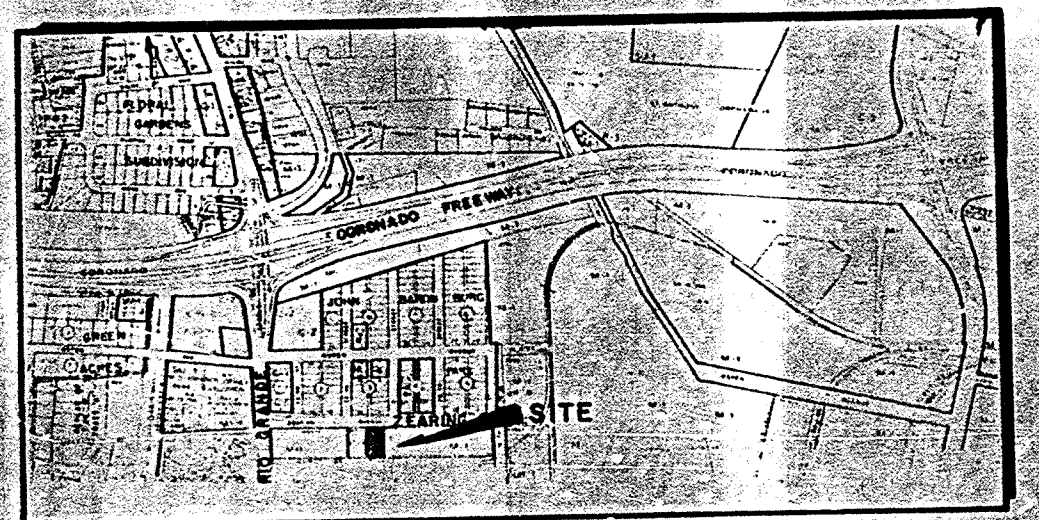


RESUBMITTAL 7-24-84 MOVED BLDG. 2
NORTH FROM SOUTH E. HIGH - POINTED
FLOW NORTH & SOUTH CONTINUED
SWALE ALONG SOUTH SIDE OF BLDG.
TO OUTLET ONTO PARKING/REGRADED
SWALE IN ASPHALT TO FLOW TO POND
FROM S.E. CORNER OF BLDG.

SCALE: 1" = 20'

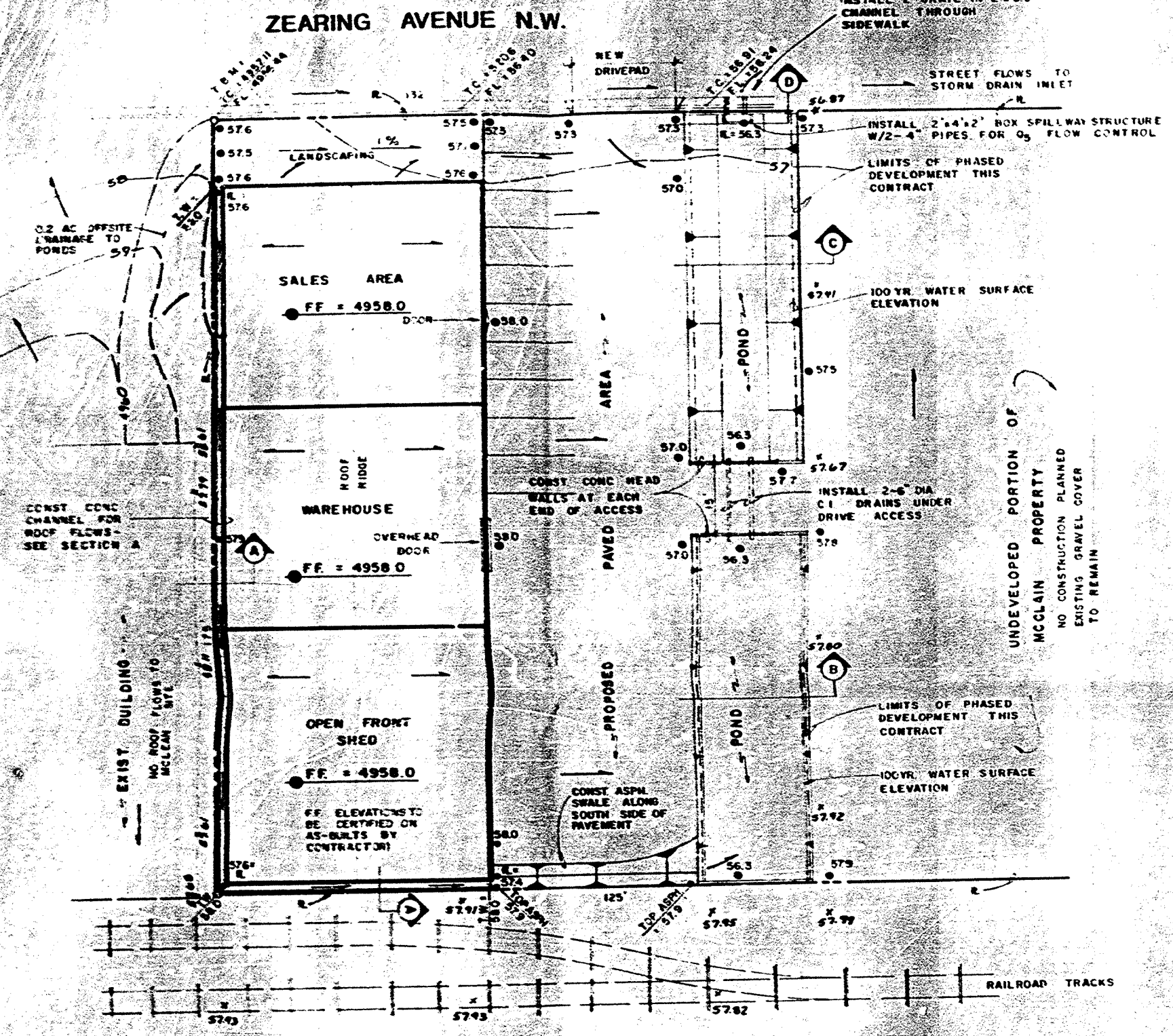
LEGEND

- ASPHALT PAVING
- LANDSCAPED AREA
- PROPERTY LINE
- EXISTING CURB & GUTTER W/SIDEWALK
- EXISTING SPOT ELEVATION
- EXISTING CONTOUR
- PROPOSED SPOT ELEVATION
- PROPOSED FINISHED CONTOUR
- SURFACE FLOW DIRECTION
- BUILDING STRUCTURE



VICINITY MAP H-13

SEP 07 1984
LUSTIG
HYDROLOGY SECTION

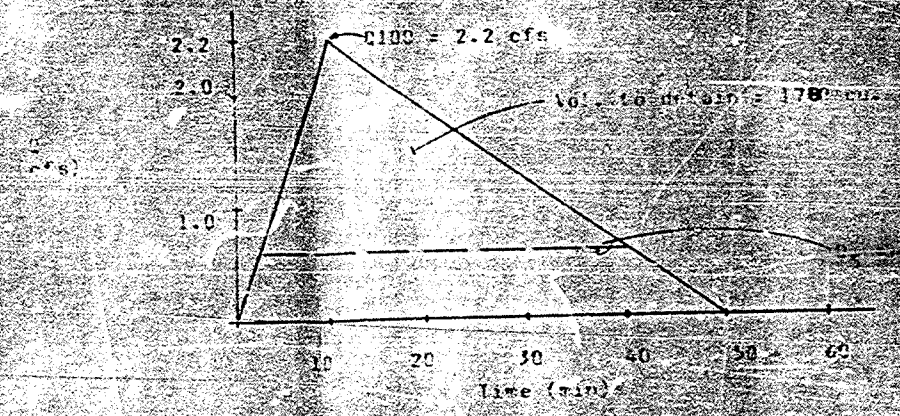


- NOTES:
- Legal: Western most 110' of Tract 340-2-1, M.H.C.D. Map 37, Albuquerque, N.M.
 - Survey: Ken Miller, P.E. A square, chiseled on top of concrete curb at N.E. curb return, located at the intersection of Rio Grande Boulevard and 11th Ave., N.W. N.E. Quarter of intersection. Elevation: 5799.20'
 - 1-21-84, one west lot line projection to Zearing Ave. Elevation: 5757.11'. 4" x 4" channel in walk.
 - Soil: S.M.S. (Silt & Clay) 12.5% clay, 87.5% silt. Soil hydrology from 1-21-84.
 - Flood Hazard: Site is located adjacent to a "Flood Hazard Area" (See Map).
 - Survey: Ken Miller, P.E. and surrounding terrain was flat. Flood of late drainage from northern portion of adjacent site to west to be accepted on-site in front of proposed building.
 - Drainage Criteria: For pre-design meeting with City Hydrology on 1-21-84, the following criteria was established:
 - Drainage plan per D.P. required.
 - Min. F.F. elevation to be established at above flood elevations of 4794'. Ponding required.
 - Special Order #19 document required for sidewalk culvert.
 - Flow Rates/Volumes:
 - Undeveloped site, presently covered with gravel. $Q_{100} = (2.15)(2.2)(0.92)(0.50 \text{ Ac.}) = 2.2 \text{ cfs}$
 - Developed site, draining to pond. $Q_{100} = (2.15)(2.2)(0.92)(0.50 \text{ Ac.}) = 2.2 \text{ cfs}$
 - $Q_{100} = (2.15)(2.2)(0.92)(0.50 \text{ Ac.}) = 2.2 \text{ cfs}$
 - Pond Volume Calculations:

Depth	Area	Avg. Depth	Volume
56.60'	3300	0.27'	908
56.85'	3770	0.47'	1785
 - Channel Calculations: $W=2'$, $S=0.01$, $n=0.02$ (conservative to account for turbulent entrance)
 $Q = 0.331(0.5)^{3/2}$

100' Ponding and Outlet Control:

- from triangular design hydrograph and $Q_{100} = 2.2 \text{ cfs}$, resulting pond detention is 1785 cu. ft.



- Pond shown on plan will detain approximately 1785 cu. ft. at spillway elevation = 56.85'
- Two 4" outlet pipes with invert elevation = 56.3' control discharge below 56.85' until pond reaches elevation 56.85'. Calculation based on King and Braker 5th Ed., Chap. 4.

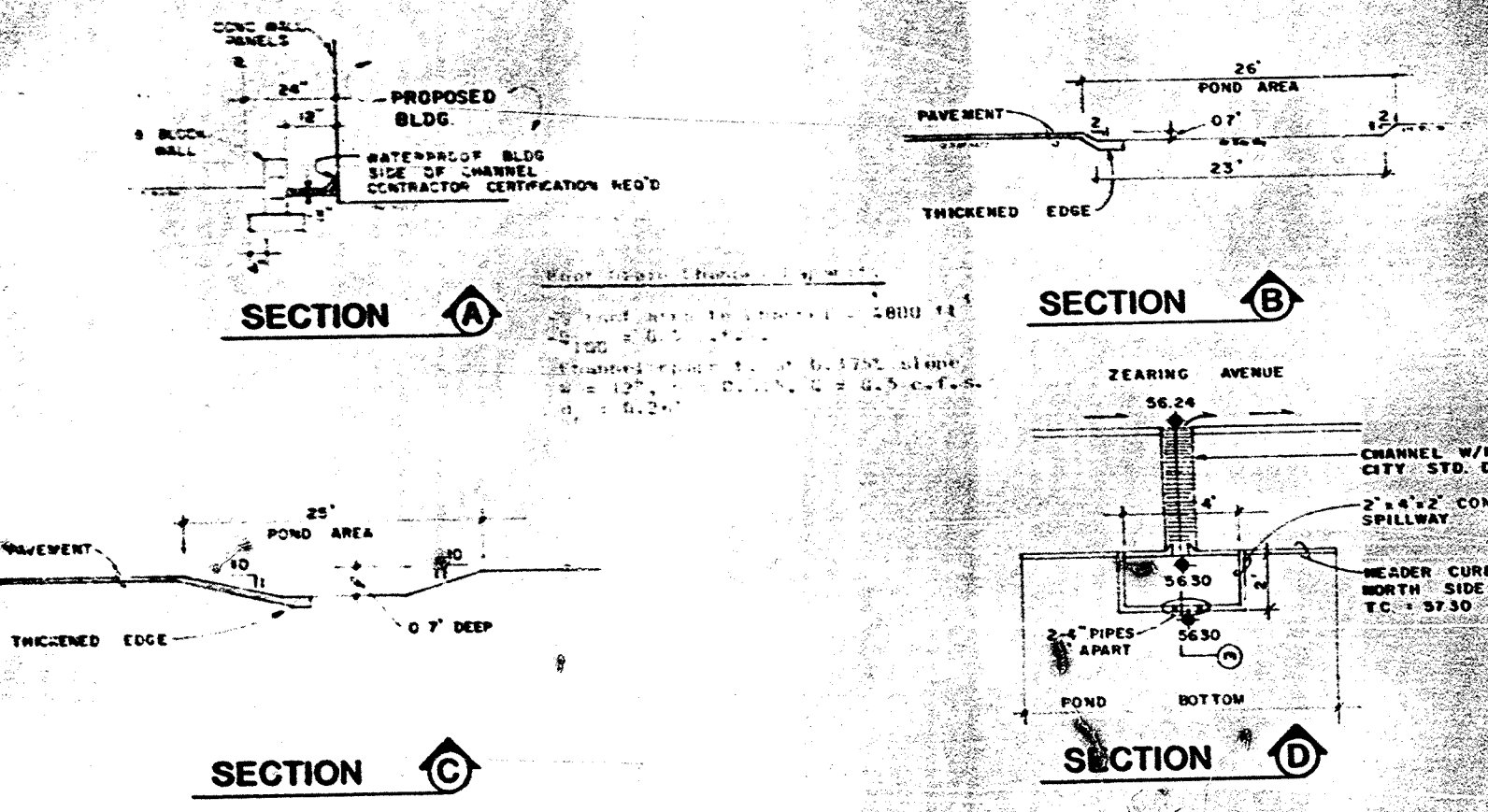
$$Q = C L H^{3/2}$$

$$Q = 2.5(8)(0.25)^{3/2} = 2.5 \text{ cfs}$$
- Box spillway (2' x 2' x 4') with overflow elevation 56.85' with capacity 22 cfs as shown by broad-crested weir formula (King & Braker 5th Ed. Chap. 5).

$$Q = C L H^{3/2}$$

$$Q = 2.5(8)(0.25)^{3/2} = 2.5 \text{ cfs}$$

Resubmittal July 10, 1984



DRAINAGE / GRADING PLAN
OFFICE & WAREHOUSE SITE
ZEARING AVENUE N.W.