HYDROLOGY H13/D16 2 PLANS

H13/D16

2 PLANS

DRAINAGE INFORMATION SHEET

PROJECT TITLE: PASEO DEL PRADO ZONE ATLAS/DRNG. FILE #: H-13/D16

LEGAL DESCRIPTION: TRACT A & B OF THE PUCETTI TRACT

CITY ADDRESS: INDIAN SCH. RD. & MEADOW VIEW DR. N.W.

ENGINEERING FIRM: ISAACSON & ARFMAN CONTACT: FRED C. ARFMAN

ADDRESS: 128 MONROE ST. N.E. PHONE: 268-8828

OWNER: HENRY & JANE PUCETTI CONTACT: MARK FELDMAN

ADDRESS: PHONE: 243-4729

ARCHITECT: MARK FELDMAN CONTACT: MARK "

ADDRESS: 1821 MEADOW VIEW DR. NW PHONE: 243-4729

SURVEYOR: RON FORSTBAUER SURVEY CO CONTACT: RON FORSTBAUER

ADDRESS: 810 CHAMA. PHONE: 268-0248

CONTRACTOR: GREG BACZEK CONTACT: GREG

ADDRESS: PHONE: 243-4729

RECEIVED
JUN 13 1985
HYDROLOGY SECTION

PRE-DESIGN MEETING:

___ YES

___ NO

___ COPY OF CONFERENCE RECAP SHEET PROVIDED

DRB NO. 85-5 3/19/85

EPC NO. DNA.

PROJ. NO. _____

TYPE OF SUBMITTAL:

X DRAINAGE REPORT

___ DRAINAGE PLAN

___ CONCEPTUAL GRADING & DRAINAGE PLAN

___ GRADING PLAN

___ EROSION CONTROL PLAN

___ ENGINEER'S CERTIFICATION

CHECK TYPE OF APPROVAL SOUGHT:

___ SKETCH PLAT APPROVAL

___ PRELIMINARY PLAT APPROVAL

___ SITE DEVELOPMENT PLAN APPROVAL

___ FINAL PLAT APPROVAL

___ BUILDING PERMIT APPROVAL

___ FOUNDATION PERMIT APPROVAL

___ CERTIFICATE OF OCCUPANCY APPROVAL

___ ROUGH GRADING PERMIT APPROVAL

X GRADING/PAVING PERMIT APPROVAL

X OTHER REVISED EXHIBIT (SPECIFY)
TO BE INSERTED INTO THE PUBLIC ENGR. DWGS. STORM DRAIN DETAILS DELETED DUE TO CHANGE OF DESIGN. SEE ATTACHED APPROVED S.O.19 E-37.

DATE SUBMITTED: 6-12-85

BY: FRED C. ARFMAN

City of Albuquerque
P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

DESIGN HYDROLOGY SECTION
123 Central NW, Albuquerque, NM 87102
(505) 766-7644

May 28, 1985

Mr. Fred Arfman
Isaacson & Arfman, P.A.
128 Monroe Street NE
Albuquerque, NM 87108

REF: REVISED DRAINAGE PLAN FOR PASEO DEL PRADO (H13-D16) RECEIVED MAY 1, 1985

Dear Fred:

The above referenced submittal has been reviewed and is hereby approved for the purpose of obtaining preliminary and final plat approvals once the other DRB requirements are satisfied.

Please be advised that since this development falls under the Subdivision Ordinance the required infrastructure improvements will be handled by the Work Order process. Whereby a building permit, as such, is not issued for this project.

If you have any questions or comments regarding the process or if I can be of further assistance, please call me at 766-7644.

Sincerely,

Billy J. Goolsby

Billy J. Goolsby, PE
City/County Flood Plain Admin.

BJG:mrk

DRAINAGE INFORMATION SHEET

PROJECT TITLE: PASEO DEL PRADO ZONE ATLAS/DRNG. FILE #: H-13 / D16

LEGAL DESCRIPTION: TRACT A & B OF THE PUCETTI TRACT

CITY ADDRESS: INDIAN SCH. RD. & MEADOW VIEW DR. N.W.

ENGINEERING FIRM: ISAACSON & ARFMAN CONTACT: FRED C. ARFMAN

ADDRESS: 128 MONROE ST. N.E. PHONE: 268-8828

OWNER: HENRY & JANE PUCETTI CONTACT: MARK FELDMAN

ADDRESS: PHONE: 243-4729

ARCHITECT: MARK FELDMAN CONTACT: MARK "

ADDRESS: 1821 MEADOW VIEW DR. NW PHONE: 243-4729

SURVEYOR: RON FORSTBAUER SURVEY CO. CONTACT: RON FORSTBAUER

ADDRESS: 610 CHAMA. PHONE: 268-0248

CONTRACTOR: GREG BACZEK CONTACT: GREG

ADDRESS: PHONE: 243-4729

PRE-DESIGN MEETING:

___ YES

___ NO

___ COPY OF CONFERENCE RECAP SHEET PROVIDED

RECEIVED MAY 01 1985 HYDROLOGY SECTION

DRB NO. 85-5 1-8-85

EPC NO. DNA.

PROJ. NO. _____

TYPE OF SUBMITTAL:

X DRAINAGE REPORT

___ DRAINAGE PLAN

___ CONCEPTUAL GRADING & DRAINAGE PLAN

___ GRADING PLAN

___ EROSION CONTROL PLAN

___ ENGINEER'S CERTIFICATION

CHECK TYPE OF APPROVAL SOUGHT:

___ SKETCH PLAT APPROVAL

___ PRELIMINARY PLAT APPROVAL

___ SITE DEVELOPMENT PLAN APPROVAL

X FINAL PLAT APPROVAL

X BUILDING PERMIT APPROVAL

___ FOUNDATION PERMIT APPROVAL

___ CERTIFICATE OF OCCUPANCY APPROVAL

___ ROUGH GRADING PERMIT APPROVAL

___ GRADING/PAVING PERMIT APPROVAL

___ OTHER _____ (SPECIFY)

DATE SUBMITTED: 5-1-85

BY: FRED C. ARFMAN

Isaacson & Arfman, P.A.

Consulting Engineering Associates

Letter of Transmittal

To CITY HYDROLOGY
CITY OF ALBUQUERQUE

Date 5-1-85

Job No.

Attn: BILLY GOOLSBY

Reference PASEO DEL PRADO

RECEIVED MAY 08 1985 HYDROLOGY SECTION

Gentlemen:

We transmit to you 1 copy(ies) of the following

- [ ] Plats
- [x] DRAINAGE Plans
- [ ] Specifications
- [x] SUPPLEMENTAL MATL Report
- [x] COPY OF PLAT
- [x] COPY OF APPROVED S.O. 19 (PRVT. STORM DRAIN)
- [ ] Shop Drawings
- [ ] Submittals
- [ ] Material Specifications
- [ ] Copy of Letter

This information is transmitted:

- [x] As per your request
- [x] For your review & approval
- [ ] For your information
- [ ] For your attention
- [ ] For your signature
- [ ] For your files
- [ ] For your use
- [ ] Please review & return
- [ ] For return to your files
- [x] Please advise

Remarks: AS PER YOUR COMMENTS OF 3-5-85, PLEASE NOTE ROOF DRAIN LOCATIONS, LEGAL DESCRIPTIONS AND REFER TO NOTE 11 OF SUBDIVISION PLAT (MINOR DRAINAGE TO REAR YARDS)

By: FRED C. ARFMAN

Copies To

128 Monroe, NE • Albuquerque, NM 87108 • (505) 268-8828

DRAINAGE INFORMATION SHEET

PROJECT TITLE: PASEO DEL PRADO ZONE ATLAS/DRNG. FILE #: H-13

LEGAL DESCRIPTION: TRACT A & B OF THE PUCETTI TRACT

CITY ADDRESS: INDIAN SCHL. RD. & MEADOW VIEW DR. N.W.

ENGINEERING FIRM: ISAACSON & ARFMAN CONTACT: FRED C. ARFMAN

ADDRESS: 128 MONROE ST. N.E. PHONE: 268-8828

OWNER: HENRY & JANE PUCETTI CONTACT: MARK FELDMAN

ADDRESS: PHONE: 243-4729

ARCHITECT: MARK FELDMAN CONTACT: MARK "

ADDRESS: 1821 MEADOW VIEW DR. NW PHONE: 243-4729

SURVEYOR: RON FORSTBAUER SURVEY CO CONTACT: RON FORSTBAUER

ADDRESS: 610 CHAMA. PHONE: 268-0248

CONTRACTOR: GREG BACZEK CONTACT: GREG

ADDRESS: PHONE: 243-4729

RECEIVED
19
MAR 1985
HYDROLOGY SECTION

PRE-DESIGN MEETING:

___ YES

___ NO

___ COPY OF CONFERENCE RECAP SHEET PROVIDED

DRB NO. 85-5

EPC NO. DNA.

PROJ. NO. _____

TYPE OF SUBMITTAL:

X DRAINAGE REPORT

___ DRAINAGE PLAN

___ CONCEPTUAL GRADING & DRAINAGE PLAN

___ GRADING PLAN

___ EROSION CONTROL PLAN

___ ENGINEER'S CERTIFICATION

CHECK TYPE OF APPROVAL SOUGHT:

___ SKETCH PLAT APPROVAL

___ PRELIMINARY PLAT APPROVAL

___ SITE DEVELOPMENT PLAN APPROVAL

X FINAL PLAT APPROVAL

___ BUILDING PERMIT APPROVAL

___ FOUNDATION PERMIT APPROVAL

___ CERTIFICATE OF OCCUPANCY APPROVAL

___ ROUGH GRADING PERMIT APPROVAL

___ GRADING/PAVING PERMIT APPROVAL

___ OTHER _____ (SPECIFY)

DATE SUBMITTED: 3-27-85

BY: FRED C. ARFMAN

Isaacson &
Arfman, P.A.

Consulting Engineering
Associates

APPROVED BY the DEVELOPMENT REVIEW BOARD on 6/27/85

March 14, 1985

Mr. Fred Aguirre
Design Review Board
City of Albuquerque
P.O. Box 1293
Albuquerque, N.M. 87103

RE: Paseo del Prado Subdivision

Dear Mr. Aguirre:

As per your request and in accordance with the subdivision procedures, we are supplying you with the following list of public and private infrastructure improvements:

PUBLIC

--4" & 6" water line and services
--Fire hydrant at end of line
--8" sanitary sewer and services. Line to connect to existing 8" line in Meadow View Dr. via manhole connection.

- PAVEMENT WIDTH 28'
- SIDEWALK ONE SIDE 4'
- SIDEWALK ON 2ND SCH 6'
- SIDEWALK ON MEADOW VIEW DR. 4'

PRIVATE

--12" storm drain and accompanying catch basin.
--Asphaltic paving of common accessway with P.C.C. curb and gutter

These items will be incorporated into the Subdivision Improvements Agreement and surety provided prior to final plat sign-off.

Very truly yours,
ISAACSON & ARFMAN, P.A.

Fred C. Arfman, P.E.

DRB-85-5/S-85-3

FCA/naf

128 Monroe, NE • Albuquerque, NM 87108 • (505) 268-8828

Isaacson &
Arfman, P.A.

Consulting Engineering
Associates

APPROVED BY the DEVELOPMENT REVIEW BOARD on 6/27/85

DRB 85-5
S85-3
H13-D16

March 14, 1985

Mr. Fred Aguirre
Design Review Board
City of Albuquerque
P.O. Box 1293
Albuquerque, N.M. 87103

RE: Paseo del Prado Subdivision

Dear Mr. Aguirre:

As per your request and in accordance with the subdivision procedures, we are supplying you with the following list of public and private infrastructure improvements:

PUBLIC

--4" & 6" water line and services
--Fire hydrant at end of line
--8" sanitary sewer and services. Line to connect to existing 8" line in Meadow View Dr. via manhole connection.

- PAVEMENT WIDTH 22'
- SIDEWALK ONE SIDE 4'
- SIDEWALK ON IRD SCH 6'
- SIDEWALK ON MEADOW VIEW DR. 4'

PRIVATE

--12" storm drain and accompanying catch basin.
--Asphaltic paving of common accessway with P.C.C. curb and gutter

These items will be incorporated into the Subdivision Improvements Agreement and surety provided prior to final plat sign-off.

Very truly yours,
ISAACSON & ARFMAN, P.A.

Fred C. Arfman, P.E.

FCA/naf

DRB-85-5/S-85-3

128 Monroe, NE • Albuquerque, NM 87108 • (505) 268-8828

Scotch® 7664 "Post-it" Routing-Request Pad

ROUTING - REQUEST

Please
- ☐ READ
- ☐ HANDLE
- ☐ APPROVE

and
- ☐ FORWARD
- ☐ RETURN
- ☐ KEEP OR DISCARD
- ☐ REVIEW WITH ME

To BARBARA, PLEASE ROUTE the ATTACHED INFRASTRUCTURE LISTING to the REVIEWERS OF the REPORTS. PLEASE HAVE BILLY REVIEW the HERITAGE EAST LISTING BY TUESDAY. "IMPORTANT"

Date _____ From _____

Billy:
Does this listing change your comments of the Subd. Imp Agreement? This is a correct listing. However, it is not included in the S.I.A. Please advise if the requested bottom is not.

City of Albuquerque
P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

DESIGN HYDROLOGY SECTION
123 Central NW, Albuquerque, NM 87102
(505) 766-7644

March 5, 1985

Fred Arfman
Isaacson & Arfman, P.A.
128 Monroe Street, NE
Albuquerque, New Mexico 87108

RE: DRAINAGE PLAN FOR PASEO DEL PRADO SUBDIVISION (H-13/D16)
RECEIVED FEBRUARY 12, 1985

Dear Fred:

I have reviewed the referenced submittal and have the following comments:

1. Please provide the legal description on the plan for record.

2. I can see no need to have any roof drains discharge to the rear yards. Therefore, the roof drains will be required to discharge to the front to a pervious surface. Please show the locations of all the roof drains on the plan.

Should you have any questions or comments, please call me at 766-7644.

Cordially,

Billy J. Goolsby

Billy J. Goolsby, P.E.
Design Hydrology Section

BJG/bsj

CITY OF ALBUQUERQUE
MUNICIPAL DEVELOPMENT DEPARTMENT
ENGINEERING DIVISION

HYDROLOGY SECTION PROJ. NO. H-13 DATE: 9/27/84

PLANNING DIVISION NO. _____

CONFERENCE RECAP

SUBJECT: Puccetti Subdivision
Tracts A & B of the Puccetti Tract

WHO	REPRESENTING
ATTENDANCE: Fred Arfman	
Billy Gootsby	

FINDINGS: ① Approved Drainage Report required per DPM for Hydrology sign-off.
② Discharge to be determined by analysis of downstream capacity
③ Subdivisions Improvements Agreement required for Final Platting.

The undersigned agrees that the above findings are summarized accurately and are only subject to change if further investigation reveals that they are not reasonable or that they are based on inaccurate information.

SIGNED: Billy G. Gootsby
TITLE: CE/Hydrology
DATE: 9/27/84

SIGNED: [illegible]
TITLE: [illegible]
DATE: 9/27/84

Barbara

No response necessary
only update of drama-file

BM

DRAINAGE INFORMATION SHEET

PROJECT TITLE: PASEO DEL PRADO ZONE ATLAS/DRNG. FILE #: H-13 / D16

LEGAL DESCRIPTION: TRACT A & B OF THE PUCETTI TRACT

CITY ADDRESS: INDIAN SCHOOL RD. & MEADOW VIEW DR. N.W.

ENGINEERING FIRM: ISAACSON & ARFMAN CONTACT: FRED C. ARFMAN

ADDRESS: 128 MONROE ST. N.E. PHONE: 268-8828

OWNER: HENRY & [illegible] PUCETTI CONTACT: MARK FELDMAN

ADDRESS: PHONE: 243-4729

ARCHITECT: MARK FELDMAN CONTACT: MARK "

ADDRESS: 1821 MEADOW VIEW DR. NW PHONE: 243-4729

SURVEYOR: RON FORSTBAUER SURVEY CO CONTACT: RON FORSTBAUER

ADDRESS: 610 CHAMA. PHONE: 268-0248

CONTRACTOR: GREG BACZEK CONTACT: GREG

ADDRESS: PHONE: 243-4729

PRE-DESIGN MEETING:

___ YES

___ NO

X COPY OF CONFERENCE RECAP SHEET PROVIDED

DRB NO. 85-5 - 1/8/85

EPC NO. DNA.

AA NO. ___

RECEIVED FEB 12 1985 HYDROLOGY SECTION

TYPE OF SUBMITTAL:

X DRAINAGE REPORT

___ DRAINAGE PLAN

___ CONCEPTUAL GRADING & DRAINAGE PLAN

___ GRADING PLAN

___ EROSION CONTROL PLAN

___ ENGINEER'S CERTIFICATION

CHECK TYPE OF APPROVAL SOUGHT:

___ SKETCH PLAT APPROVAL

X PRELIMINARY PLAT APPROVAL

___ SITE DEVELOPMENT PLAN APPROVAL

___ FINAL PLAT APPROVAL

___ BUILDING PERMIT APPROVAL

___ FOUNDATION PERMIT APPROVAL

___ CERTIFICATE OF OCCUPANCY APPROVAL

___ ROUGH GRADING PERMIT APPROVAL

___ GRADING/PAVING PERMIT APPROVAL

___ OTHER _____ (SPECIFY)

DATE SUBMITTED: 2-12-85

BY: FRED C. ARFMAN

DRAINAGE STUDY

FOR THE

PASEO DEL PRADO SUBDIVISION

FEBRUARY, 1985

RECEIVED
FEB 12 1985
HYDROLOGY SECTION

Prepared by:

ISAACSON & ARMAN, P.A.
Consulting Engineering Assoc.
128 Monroe Street N.E.
Albuquerque, N.M. 87108
(505) 268-8828

I. INTRODUCTION

Isaacson & Arfman, P.A., N.M.P.E. No. 7322, is under contract to Baczek/Feldman Joint Development Venture to provide engineering services related to the site development of Tracts A and B of the Puccetti Tract. This development shall be known as Paseo del Prado and consist of thirteen new townhome sites and one existing residential dwelling all on 1.9 acres. More particularly, the site is situated at the northeast quadrant of Meadow View Dr. and Indian School Rd. in Albuquerque's North Valley area.

The site was surveyed for both boundary and topographical requirements by Ron Forstbauer Survey Co., N.M.P.L.S. No. 6126 in October of 1984. Existing site conditions, except for the existing structures in Tract A, were that of an irrigated field.

This site is zoned for a townhouse development and no rezoning is required.

II. SITE CONDITIONS

The site is typical of lands within the North Valley area of Albuquerque. Due to its prior use as an irrigated corn field, a majority of the site (southerly two-thirds) has no storm water outfall. The remaining northerly portion currently discharges flows to the right-of-way of Meadow

View Dr. or has minor on-site retention.

By inspection of Community Panel Number 350002-0022 of the FLOOD BOUNDARY AND FLOODWAY MAP for the City of Albuquerque, dated October 14, 1983; it was apparent that this site is not subject to any flood hazard areas (see attachment).

Site soils are Agua (Af) and Gila (Ge) as indicated on Plate 20 of the Soils Survey of Bernalillo County. Both of the soil types are in the Hydrological Soil Group 'B' classification (see attached soil map and tables).

III. OFF-SITE DRAINAGE CHARACTERISTICS

This site does not accept any appreciable off-site storm waters. The site is bordered by the following properties:

North - Fully developed residential housing

East - Alameda Drain. Due to the drain's raised elevation, a minor berm is required along its westerly right-of-way. The west face of this berm contributes a very minor sheet flow of storm waters onto the site.

South - Indian School Rd. Handles its storm waters within its driving surface.

West - Meadow View Drive. Handles its storm waters within its driving surface and has a 30" RCP storm sewer and catch basins. This storm sewer is at the tail

the contributing basin. Its outfall point into the Alameda Drain is approximately 250 feet to the east of the intersection of Meadow View Dr. and Indian School Road.

IV. ON-SITE DRAINAGE CHARACTERISTICS

The undeveloped drainage characteristics are as follows and are based upon a discharge onto public right-of-way.

Area: 1.967 Ac. = 85,674 sq. ft.
Tc: 10 minutes
I: 4.75
D: 2.2" = 0.183'
Soil: Type 'B'
'C': 0.38 (based on 10% impervious)

Q_{100} = (0.38)(4.75)(1.967) = 3.55 cfs

Q_{10} = (0.657)(3.55) = 2.33 cfs

V_{100} = (0.38)(0.183)(85,674) = 5,958 cf

V_{10} = (0.657)(5,958) = 3,914 cf

As stated earlier, a large portion of the storm waters are retained onsite due to the irrigated field's perimeter berm. If the berm were breached along Meadow View Dr., the above rates could be expected to discharge from the site.

The developed conditions for this site are typical of townhouse developments. Most of the waters generated by the developed site shall be directed to the site's interior accessway and storm water easement. These flows shall be collected by a private storm drain system and conveyed by a 12" pipe to the existing 30" RCP storm sewer in Meadow View Dr. The private storm drain will collect the on-site flows at the accessway's low point, approximately 150 feet east of the site's primary entrance off of Meadow View Dr. A private catch basin shall serve as the single storm drain inlet. The projected flows which contribute to this system are as follows:

Area: 1.12 Ac. = 48,787 sq. ft.

Tc: 10 minutes

I: 4.75

D: 2.2" = 0.183'

'C': 0.78

Q_{100} = (0.78)(4.75)(1.12) = 4.15 cfs

Q_{10} = (0,657)(4.15) = 2.73 cfs

V_{100} = (0.78)(0.183)(48,787) = 6,964 cf

V_{10} = (0.657)(6,964) = 4,575 cf

The remaining areas are those around the inside of the site's perimeter. They primarily consist of backyards, which can not release flows to the private storm drain. In those areas which do not accept any runoff from impervious

surfaces, no storm water discharge point will be provided. Those areas will accept and retain those storm waters which fall on them.

Lots which border the public rights-of-way may have the option of either retaining those storm waters which fall on them or discharging to the public right-of-way. Where storm waters are allowed to exit onto those public rights-of-way; they must be conveyed past the public sidewalks by either: 1. a driveway pad, 2. a sidewalk culvert, or 3. a sidewalk drain pipe.

Finally, in those lots where roof drains are discharging onto the rear yard areas, one of the following provisions must be satisfied.

1. Storm waters must be ponded a minimum of 15 feet away from the building's and adjacent building's foundations.

2. An outlet to the public right-of-way or private accessway be provided.

3. A french drain be situated within that area as far as feasible from foundations. These drains must be capable of handling the 10 year volume from those corresponding roof areas.

V. PRIVATE STORM DRAIN

This private system is necessary in order to provide a

means of discharging storm waters from the relatively flat site. This system is consistent with the Conference Recap of September 27, 1984. This statement allowed free discharge of storm waters via either the planned private storm drain or by rear lot discharge into public right-of-way. The design of this system is found on the Drainage & Grading Exhibit accompanying this report. The support calculations may be found in the report's appendix.

APPENDIX

RECEIVED
FEB 12 1985
HYDROLOGY SECTION

CITY OF ALBUQUERQUE
MUNICIPAL DEVELOPMENT DEPARTMENT
ENGINEERING DIVISION

HYDROLOGY SECTION PROJ. NO. H-13 DATE: 9/27/84

PLANNING DIVISION NO. _____

CONFERENCE RECAP

SUBJECT: Puccetti Subdivision
Tracts A & B of the Puccetti Tract

WHO	REPRESENTING
ATTENDANCE: Fred Artman	
Billy Goolsby	

RECEIVED
FEB 12 1985
HYDROLOGY SECTION

FINDINGS: (1) ~~B~~ Approved Drainage Report required per DPM for Hydrology sign-off
(2) Discharge to be determined by analysis of downstream capacity
(3) Subdivisions Improvements Agreement required for Final Platting.

also Northgate

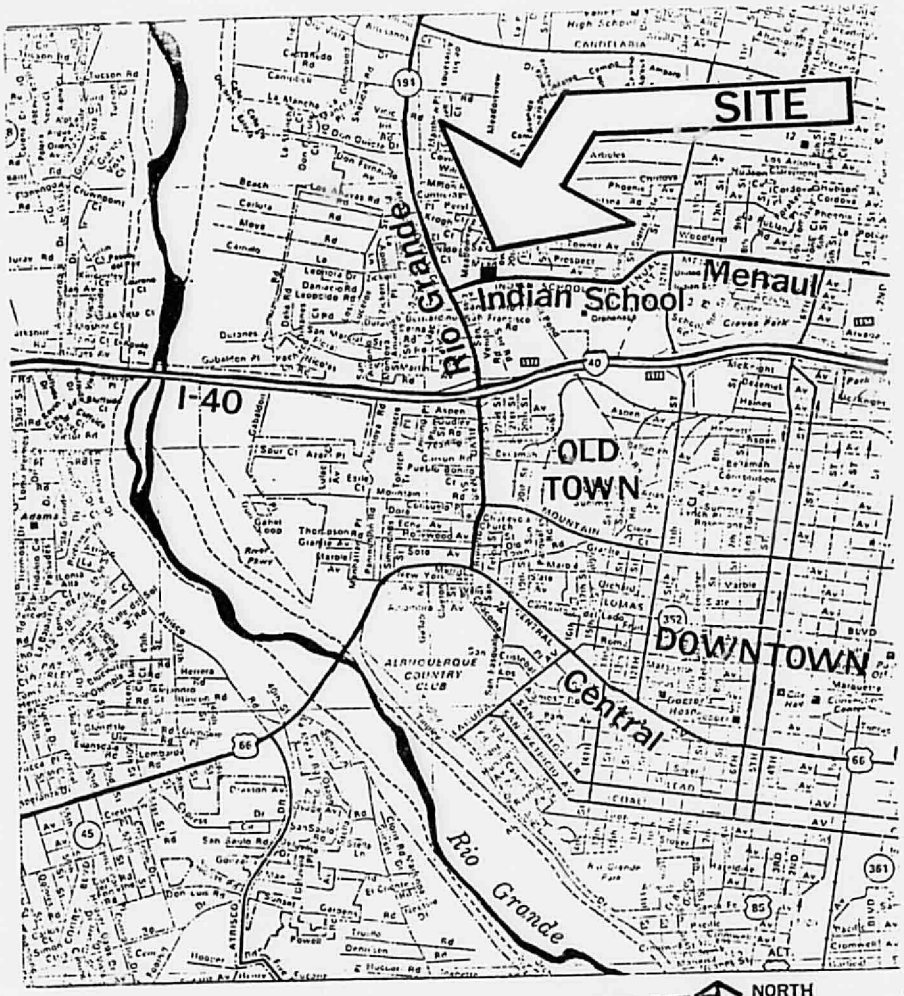
(4) Free discharge - using private storm drain to connect to city storm drain in Meadow View appears to be method of site discharge. Minor private free discharge to street is acceptable

The undersigned agrees that the above findings are summarized accurately and are only subject to change if further investigation reveals that they are not reasonable or that they are based on inaccurate information.

SIGNED: Billy J. Goolsby SIGNED: [illegible]

TITLE: C.E./Hydrology TITLE: [illegible]

DATE: 9/27/84 DATE: [illegible]



Site Location

RECEIVED
FEB 12 1985
HYDROLOGY SECTION

Proposed Townhouse Development
Land of Henry & Jane Puccetti
Albuquerque, New Mexico

A JOINT DEVELOPMENT VENTURE
GREG BACZEK / MARK FELDMAN
1821 Meadow View Dr. N.W.
Albuquerque, N.M.
243-4729

AND VALENCIA COUNTIES, NEW MEXICO — SHEET NUMBER 20

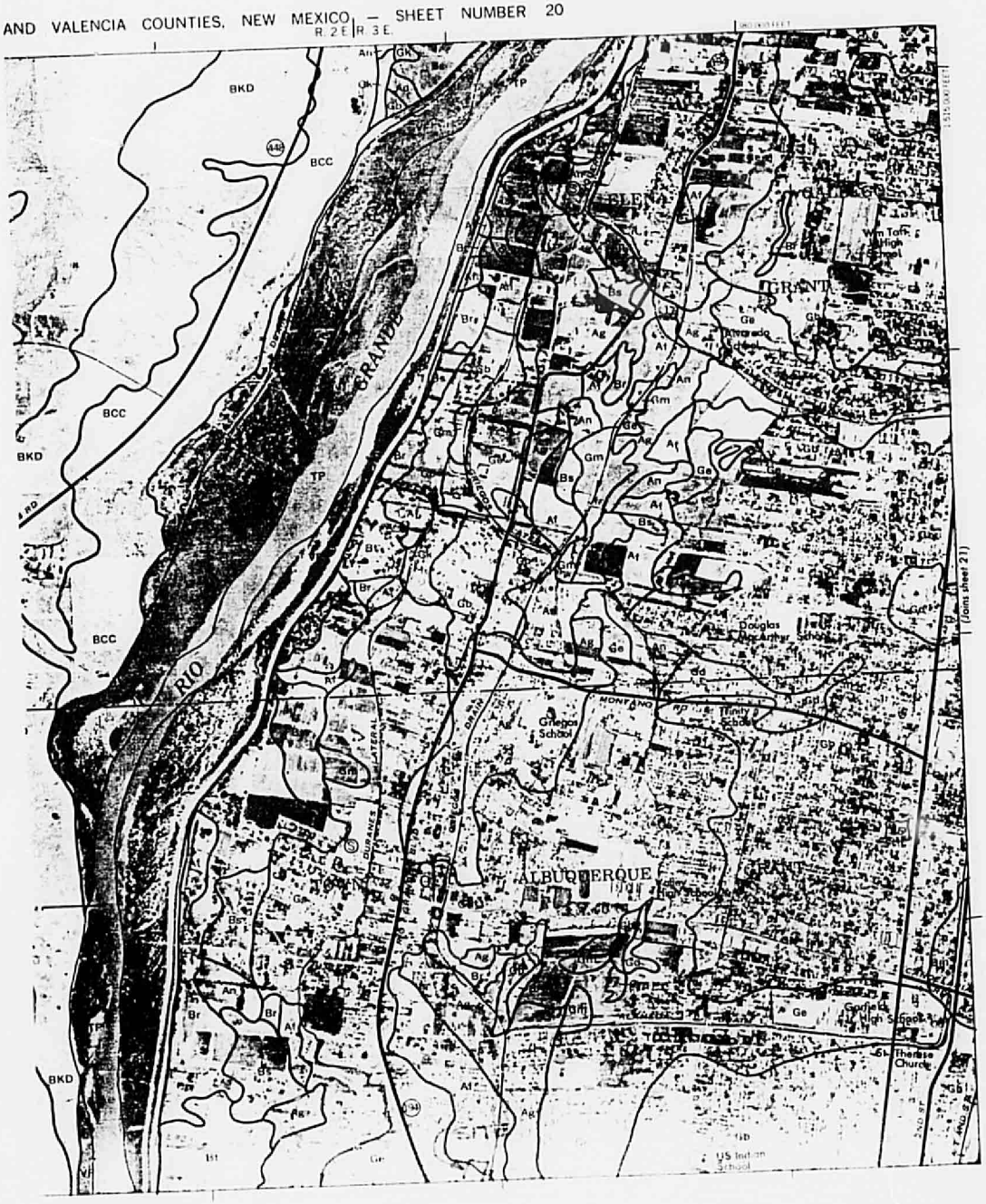


TABLE 8.—*Engineering*

[An asterisk in the first column indicates that at least one mapping unit in this series is made up of two or more kinds of soil. Because the to other series that appear in the first column of this

Soil series and map symbols	Degree and kind of limitations for—					
	Septic tank absorption fields	Sewage lagoons	Shallow excavations	Dwellings without basements	Sanitary landfill (trench type)[1]	Local roads and streets
Agua: Af	Slight	Severe: seepage.	Severe: cut-banks cave	Slight	Severe: seepage.	Slight
Ag	Slight	Severe: seepage.	Severe: cut-banks cave.	Moderate: shrink swell.	Severe: seepage.	Moderate: shrink swell.
Agua variant: Ah	Severe: wet	Severe: wet	Severe: cut-banks cave.	Severe: wet	Severe: wet	Moderate: wet.
*Akela: AkC For Rock outcrop part, see Rock outcrop.	Severe: depth to rock.	Severe: depth to rock.	Severe: depth to rock.	Severe: depth to rock.	Severe: depth to rock.	Severe: depth to rock.
Alemeda: AmB	Severe: depth to rock.	Severe: depth to rock.	Severe: depth to rock.	Severe: depth to rock.	Severe: depth to rock.	Moderate: depth to rock.
Anapra: An, Ao	Slight	Severe: seepage.	Severe: cut-banks cave.	Moderate: shrink swell.	Severe: too sandy; seepage.	Moderate: shrink swell.
Armijo: Ar	Severe: percs slowly.	Slight	Severe: too clayey.	Severe: shrink swell; low strength.	Severe: too clayey.	Severe: shrink swell; low strength.
Badland: Ba	Severe: slope.	Severe: slope.	Severe: slope.	Severe: slope.	Severe: slope.	Severe: slope.
*Bluepoint: Bb, BcA, BCC, Bd3, BKD. For Wink part of Bd3, see Wink series; for Kokan part of BKD, see Kokan series.	Slight if slope is 1 to 8 percent, moderate if 8 to 15.	Severe: seepage.	Severe: cut-banks cave.	Slight if slope is 1 to 8 percent, moderate if 8 to 15.	Moderate: too sandy.	Slight if slope is 1 to 8 percent, moderate if 8 to 15.
*Bond Mapped only with Rock outcrop and Penistaja soils.	Severe: depth to rock.	Severe: depth to rock.	Severe: depth to rock.	Severe: depth to rock.	Severe: depth to rock.	Severe: slope; depth to rock.
*Borolla: BOF For Rock outcrop part, see Rock outcrop.	Severe: slope; depth to rock.	Severe: slope.	Severe: slope.	Severe: slope.	Severe: slope.	Severe: slope.
Brazito: Br, Bs, Bt	Slight	Severe: seepage.	Severe: cutbanks cave.	Slight	Severe: seepage; too sandy.	Slight
Burnac: BUE	Severe: slope; percs slowly.	Severe: slope.	Severe: slope.	Severe: slope; high shrink swell.	Severe: slope; too clayey.	Severe: slope; too clayey.

See footnotes at end of table.

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interpretations

soils in such mapping units can have different properties and limitations, it is necessary to follow carefully the instructions for referring table. Some terms in this table are defined in the glossary]

Suitability as source of—				Soil features affecting—		Hydrologic soil group
Road fill	Sand	Gravel	Topsoil	Pond reservoir areas	Dikes, levees, and other embankments	
Fair: low strength.	Good	Unsuited	Good	Seepage	Piping; low strength; unstable fill.	B
Fair: low strength.	Good	Unsuited	Fair: too clayey.	Seepage	Piping; low strength; unstable fill.	B
Fair: low strength.	Good	Unsuited	Fair: excess salt.	Seepage	Piping; low strength; unstable fill.	B
Poor: thin layer	Unsuited	Unsuited	Poor: thin layer; area reclaim; small stones.	Depth to rock	Thin layer	C
Poor: area reclaim; thin layer.	Unsuited	Unsuited	Fair: thin layer; area reclaim.	Depth to rock	Piping; low strength.	C
Fair: shrink swell.	Good	Unsuited	Fair: too clayey.	Seepage	Piping	B
Poor: shrink swell; low strength.	Unsuited	Unsuited	Poor: too clayey.	Seepage	Compressible; low strength; hard to pack.	D
Poor: slope	Unsuited	Unsuited	Poor: slope	Slope	Hard to pack; thin layer.	C
Good	Fair: excess fines.	Unsuited	Poor: too sandy.	Seepage	Piping; seepage	A
Poor: thin layer	Unsuited	Unsuited	Poor: thin layer.	Depth to rock	Thin layer	D
Poor: slope	Unsuited	Unsuited	Poor: slope	Slope	Thin layer	D
Good	Fair: excess fines.	Unsuited	Fair: thin layer.	Seepage	Seepage; piping; erodes easily.	A
Poor: shrink swell; slope.	Unsuited	Unsuited	Poor: stones; slope.	Slope	Low strength; shrink swell.	C

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TABLE 8.—*Engineering*

Soil series and map symbols	Degree and kind of limitations for—					
	Septic tank absorption fields	Sewage lagoons	Shallow excavations	Dwellings without basements	Sanitary landfill (trench type) [1]	Local road and streets
Carlito: CAF	Severe: slope; percs slowly.	Severe: slope.	Severe: slope; too clayey.	Severe: slope; shrink swell.	Severe: too clayey.	Severe: low strength; shrink swell.
Cut and fill land: Cu	Slight if slope is 1 to 8 percent, moderate if 8 to 15, severe if more than 15.	Severe: seepage.	Moderate: cutbanks cave; slope.	Slight if slope is 1 to 8 percent, moderate if 8 to 15, severe if more than 15.	Severe: seepage.	Slight if slope is 1 to 8 percent, moderate if 8 to 15, severe if more than 15.
*Embudo: EmB, EtC. For Tijeras part of EtC, see Tijeras series.	Slight [2]	Severe: seepage.	Moderate: [3] small stones.	Slight [3]	Severe: seepage.	Slight
Escabosa. Mapped only with Laporte soils and Rock outcrop.	Severe: depth to rock.	Severe: depth to rock.	Severe: depth to rock.	Moderate: depth to rock.	Severe: depth to rock.	Moderate: depth to rock.
*Gila: GA, Gb, Ge, GH. For Hants part of GH, see Hants series.	Moderate: percs slowly.	Moderate: seepage.	Slight	Slight	Slight	Moderate: low strength.
Gc, Gd, GF	Severe: wet	Moderate: seepage; wet.	Moderate: wet.	Slight	Severe: wet	Moderate: low strength.
Glendale: Gk, Gm	Severe: percs slowly.	Slight	Moderate: too clayey.	Moderate: shrink swell.	Moderate: too clayey.	Moderate: shrink swell.
Gs	Severe: percs slowly.	Moderate: wet	Moderate: too clayey.	Moderate: shrink swell.	Severe: wet; too clayey.	Moderate: shrink swell
Hants: Ha	Severe: percs slowly; floods.	Slight	Severe: too clayey; rarely floods.	Severe: shrink swell; floods.	Severe: too clayey; floods.	Severe: shrink swell; floods.
Ildefonso: ILC	Slight if slope is less than 8 percent, moderate if 8 to 15, severe if more than 15.	Severe: seepage.	Severe if slope is more than 15 percent: small stones.	Slight if slope is less than 8 percent, moderate if 8 to 15, severe if more than 15.	Severe: seepage.	Slight if slope is less than 8 percent, moderate if 8 to 15, severe if more than 15.
*Kim: KaB, KbB, KD. For Badland part of KD, see Badland.	Slight	Moderate: seepage.	Slight	Moderate: low strength; shrink swell.	Slight	Moderate: shrink swell.
*Kokan: KOE, KR. For Rock outcrop part of KR see Rock outcrop.	Moderate if slope is 10 to 15 percent, severe if more than 15.	Severe: seepage; slope.	Severe: cutbanks cave; small stones.	Moderate if slope is 10 to 15 percent, severe if more than 15.	Severe: seepage; small stones.	Moderate if slope is 10 to 15 percent, severe if more than 15.

See footnotes at end of table.

interpretations—Continued

Suitability as source of—				Soil features affecting—		Hydrologic soil group
Road fill	Sand	Gravel	Topsoil	Pond reservoir areas	Dikes, levees, and other embankments	
Poor: low strength; shrink swell.	Unsuited	Unsuited	Poor: thin layer.	Slope	Hard to pack; low strength; compressible.	C
Good if slope is less than 15 percent, fair if more than 15.	Fair: excess fines.	Unsuited	Poor: small stones.	Seepage	Hard to pack; seepage.	A
Good	Poor: excess fines.	Poor: excess fines.	Poor: small stones.	Seepage	Piping; compressible	B
Poor: thin layer.	Unsuited	Poor: excess fines.	Poor: thin layer; small stones.	Depth to rock; seepage.	Piping; compressible	C
Fair: low strength.	Poor: excess fines.	Poor: excess fines.	Good	Seepage	Piping	B *
Fair: low strength.	Poor: excess fines.	Poor: excess fines.	Poor: excess salts.	Seepage	Piping	B
Fair: shrink swell.	Unsuited	Unsuited	Fair: too clayey.	Favorable	Low strength; piping.	B
Fair: shrink swell.	Unsuited	Unsuited	Poor: excess fines.	Favorable	Low strength; piping.	B
Poor: shrink swell.	Unsuited	Unsuited	Poor: too clayey.	Favorable	Compressible; low strength.	D
Good if slope is less than 15 percent, fair if 15 to 25, poor if more than 25.	Poor: excess fines.	Poor: excess fines.	Poor: small stones.	Seepage	Favorable	B
Fair: low strength.	Unsuited	Unsuited	Good	Seepage	Piping; low strength	B
Good if slope is 10 to 15 percent, fair if 15 to 25, poor if more than 25.	Good	Good	Poor: small stones.	Seepage	Seepage	A

RECOMMENDATIONS FROM SOIL REPORT

REINFORCING

Foundations, slabs and masonry walls should be well reinforced to minimize the effects of differential movement. Refer to the foundation design for details.

BACKFILL AND SURFACE DRAINAGE

Due to the moisture sensitivity of the subsoils, it is important that the recommendations presented in this and the following section are closely followed.

Foundation soils should be prevented from being wetted after construction. Backfill around the foundations should be free of trash and construction debris, and should be moistened and compacted to a minimum of 90% of maximum density as determined by ASTM D-1557. Only enough water should be added to the backfill material to allow proper compaction. Do not puddle or jet backfill.

The final grade should have a good slope away from foundation walls and exterior slabs on all sides. A minimum slope of 1 foot of fall in the first 10 feet from all foundation walls is recommended. Drainage from roofs, and all other surface water, should not be allowed to pond in the backfill areas or within five feet of

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foundations. Downspout extensions and splash blocks should be used to protect the ground surface where the ground is not covered by concrete slabs or asphalt.

Ponding areas should be eliminated if possible. However, if they are required for the control of surface runoff they should be located as far from the foundation walls as possible, a minimum of 15 feet.

IRRIGATION OF LANDSCAPED AREAS

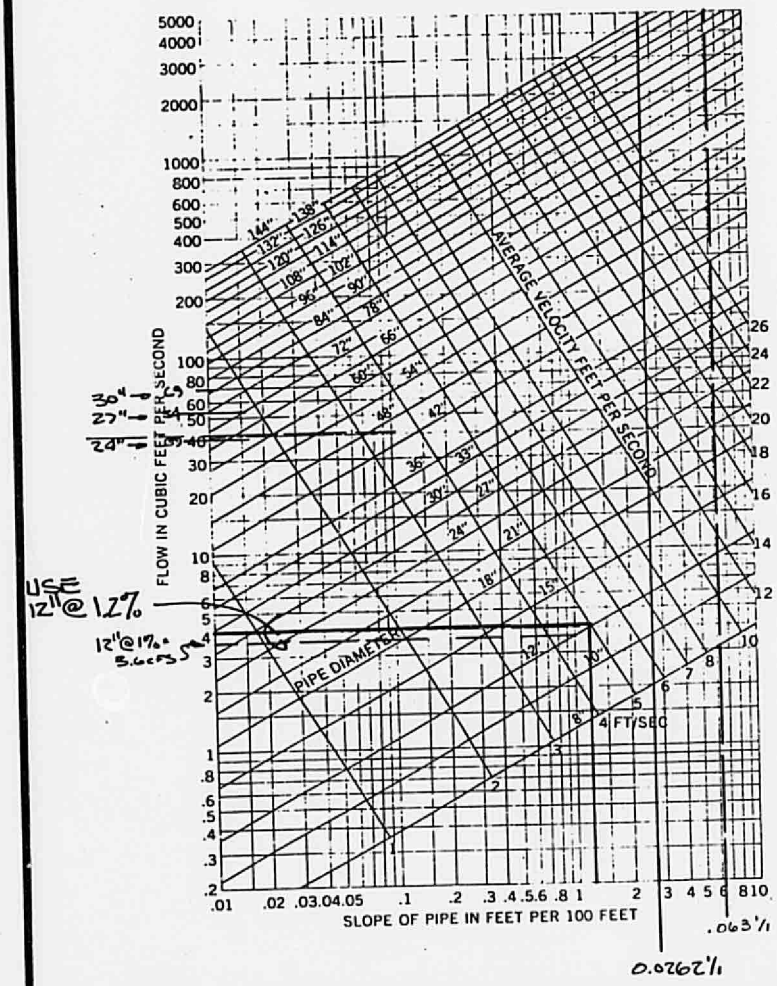
Do not install sprinkler system lines next to foundation walls or exterior slabs. If a sprinkler system is installed, the sprinklers should be placed so that the spray from the heads under full pressure does not fall within 5 feet of foundation walls. Irrigation should be minimized and controlled. Any planters adjacent to the structure should be watertight, and we recommend employing plants native to the area which require minimal irrigation.

EARTH PRESSURES

The lateral earth pressures for the design of underground structures or retaining walls may be approximated by the following equivalent fluid pressure:

FIGURE 5

FLOW FOR CIRCULAR PIPE FLOWING FULL
BASED ON MANNING'S EQUATION n=0.013



Isaacson & Arfman, P.A.

SUBJECT_____ JOB NO.____
BY_____ DATE_____ SHEET NO.____ OF____

CATCH BASIN

GRATE IS SITUATED IS A 0.2' SUMP CONDITION

ASSUMED POND CONDITIONS:

- HEAD = 6" (CURB + SUMP)
- OPEN AREA OF GRATE: USE NEENAH 3067 L
  OPEN AREA = 1.8 ☐
- ON NEENAH CALCULATOR, DETERMINE GRATE CAPACITY FROM DEPTH OF FLOW AND "K" VALUE FOR CHOSEN GRATE

K = 30

∴ Q = 6 CFS > 4.65 ———— O.K.

Isaacson & Arfman, P.A.

SUBJECT _____ JOB NO. ____

BY _____ DATE _____ SHEET NO. ____ OF ____

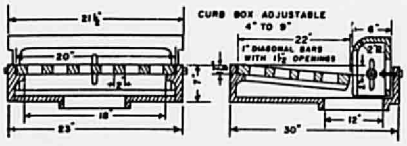
R-3066 Curb Inlet Frame, Grate, Curb Box

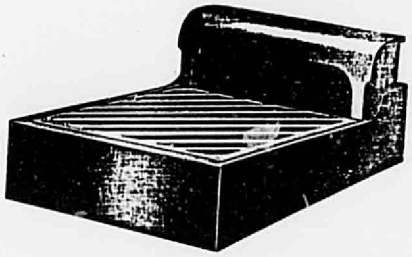
Heavy Duty
Total Weight 400 Pounds

Type DR Reversible grate shown. For opposite hand flip grate top to bottom.

Furnished standard with 12" pan opening. Also available with 18" pan opening on special order. Specify pan opening required.
Specify:

1. Pan opening 12" or 18".





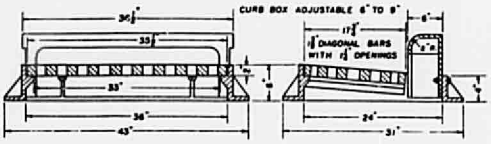
Illustrating R-3066 with Type DR grate

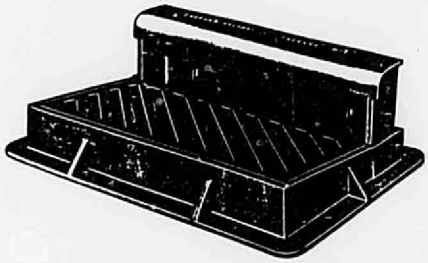
ALTERNATE

R-3067 Curb Inlet Frame, Grate, Curb Box

Heavy Duty
Total Weight 510 Pounds

Type DL Reversible grate shown. For opposite hand flip grate top to bottom.





Illustrating R-3067 with Type DL grate

R-3067-L Curb Inlet Frame, Grate, Curb Box

Heavy Duty

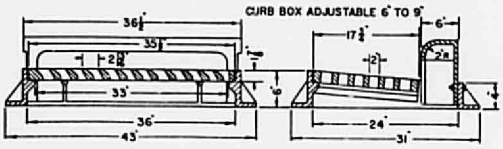
Total Weight 490 Pounds

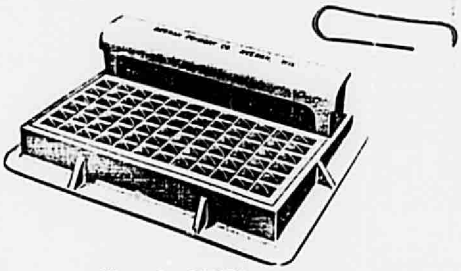
R-3067-V

Same as above except with Type V Grate.

Heavy Duty

Total Weight 465 Pounds





Illustrating R-3067-L

PRIMARY

FREE OPEN AREAS OF NEENAH GRATES (Continued)

CATALOG NO.	TYPE	SQ. FT. OPEN
R-2700-A	G	0.5
R-2700-B	G	0.5
R-2700-C	G	0.7
R-2700-D	G	0.7
R-3010	S	1.0
R-3010	A	1.0
R-3010	D	1.3
R-3015	R	1.3
R-3030	A	1.0
R-3030	D	1.3
R-3030	DR/DL	1.3
R-3031-A	S	0.8
R-3031-B	S	0.8
R-3032-A	S	0.8
R-3032-B	S	0.8
R-3033-A	S	0.8
R-3033-B	S	0.8
R-3034-A	S	1.1
R-3034-B	S	1.1
R-3035-A	S	1.1
R-3036-A	S	1.1
R-3036-B	S	1.1
R-3037-A	S	1.6
R-3038-A	S	1.5
R-3039-A	S	1.5
R-3040	A	1.0
R-3042	A	0.3
R-3050	A	1.0
R-3065	DR/DL	0.9
R-3065-L	L	0.9
R-3065-V	V	1.1
[illegible]	DR/DL	1.4
R-3067	DR/DL	1.9
[illegible]	L	1.9
R-3067-V	V	2.2
R-3069	S	1.0
R-3070	A	0.9
R-3070	D	1.0
R-3070	DR/DL	1.0
R-3075	DR/DL	1.0
R-3075-L	L	0.9
R-3075-V	V	1.1
R-3076	V	1.5
R-3077	V	3.0
R-3077-L	V	1.5
R-3077-R	V	1.5
R-3078	V	1.5
R-3080	A	0.9
R-3080	D	1.0
R-3080	DR/DL	1.0
R-3081	A	0.9
R-3081	D	1.0
R-3081	DR/DL	1.0
R-3085	DR/DL	1.0
R-3090	A	1.0
R-3120	A	0.7
R-3130	A	0.7
R-3141	A	0.7
R-3150	A	0.9
R-3151	A	0.9
R-3152	A	1.0
R-3153	A	2.5
R-3154	D	2.6
R-3155-A	D	1.9
R-3156	C	1.9
R-3157-1	A	2.3
R-3157-2	V	1.5
R-3157-A	K	1.7
R-3159-A	S	1.3
R-3161	S	1.3
R-3165	A	1.4
R-3165	D	1.1
R-3169	B	0.7
R-3170	B	0.7
R-3173	C	1.3
R-3174	A	1.4
R-3175	A	1.8
R-3177	A	1.2
R-3179	D	1.6
R-3180	C	0.9
R-3203-A	A or C	1.0
R-3203-B	A or C	1.0
R-3203-C	A or C	1.0
R-3205	K	0.9

CATALOG NO.	TYPE	SQ. FT. OPEN
R-3206	D	0.9
R-3210	C	1.7
R-3210-A	C	1.6
R-3210-AL	L	1.5
R-3210-L	L	1.5
R-3220	C	1.5
R-3220-L	L	1.5
R-3222-C	C	1.4
R-3222-L	L	1.5
R-3227	D	2.3
R-3227-C	C	1.9
R-3227-D	DR/DL	2.3
R-3228-BD	DR/DL	2.3
R-3228-ED	DR/DL	2.3
R-3228-G	D	2.2
R-3228-H	C	1.9
R-3228-J	D	2.2
R-3228-K	C	1.9
R-3229-A	C	1.7
R-3229-L	L	1.5
R-3230	A	1.4
R-3230-A	C	1.2
R-3232	C	2.0
R-3233	A	2.8
R-3233-D	A	2.8
R-3234	C	0.9
R-3234-1	C	1.2
R-3234-B	C	1.3
R-3234-B1	C	1.3
R-3234-B2	C	1.3
R-1235	C	1.3
R-1235-D	D	1.0
R-3236	C	1.1
R-3236-1	C	1.2
R-3236-A	C	1.1
R-3236-B	C	1.1
R-3237	C	1.3
R-3237-1	C	1.1
R-3237-A	C	1.1
R-3237-B	C	1.1
R-3238	C	1.3
R-3239-A	A	1.0
R-3240	C	2.3
R-3240-A	C	2.9
R-3246	C	1.8
R-3246-A	C	2.5
R-3246-A	DR/DL	2.7
R-3246-AL	L	3.0
R-3246-B	D	2.4
R-3246-C	D	2.2
R-3246-CMC	D	1.8
R-3246-D	S	1.8
R-3246-D	D	1.8
R-3246-E	C	1.8
R-3246-F	C	1.8
R-3246-G	DR/DL	1.9
R-3247-A	DR/DL	5.1
R-3248	B	0.6
R-3249-A	C	1.3
R-3249-B	C	1.3
R-3249-E	S	1.1
R-3249-F	S	1.1
R-3250	K	1.5
R-3250-A	K	1.5
R-3250-1	K	1.5
R-3250-B	K	1.4
R-3250-C	K	1.4
R-3250-BL	L	1.0
R-3250-CL	L	1.0
R-3250-BV	V	1.3
R-3250-CV	V	1.3
R-3251	C	1.0
R-3252-A	V	1.3
R-3253-A	K	1.5
R-3254	G	1.2
R-3259	A	1.7
R-3260-A	A	2.6
R-3261-A	C	1.1
R-3266	V	0.6
R-3267	V	0.3
R-3270	A	0.9
R-3270-B	A	0.9
R-3270-5	A	0.9
R-3272	A	1.2
R-3273-A	C	1.2

CATALOG NO.	TYPE	SQ. FT. OPEN
R-3274	C	1.3
R-3274-A	C	1.4
R-3274-B	C	1.4
R-3275	A	0.9
R-3276	B	0.8
R-3277	A	1.0
R-3278-1	C	1.2
R-3278-A	C	1.3
R-3279	D	1.4
R-3280-A	C	1.8
R-3280-B	C	1.2
R-3281	C	1.4
R-3281-A	C	1.0
R-3281-B	DR/DL	1.0
R-3282	A	0.7
R-3283-A	B	1.4
R-3283-B	B	2.8
R-3283-C	B	4.2
R-3285	C	1.5
R-3285-A-1	D	1.3
R-3285-B	D	1.9
R-3286	C	0.8
R-3286-BV	V	0.7
R-3287	C	1.4
R-3287-10V	V	1.1
R-3287-SB10	S	1.5
R-3288-E	DR/DL	2.6
R-3288-E	V	2.9
R-3288-F	DR/DL	2.6
R-3288-HV	V	3.2
R-3288-J	C	3.8
R-3289-A	D	1.5
R-3289-HV	V	1.6
R-3289-L	C	1.5
R-3289-C	DR/DL	1.3
R-3290	C	1.7
R-3290-A	C	2.6
R-3290-B	D	3.5
R-3290-C	D	3.8
R-3291	C	1.7
R-3292	C	1.8
R-3293	D	2.8
R-3294-C	A	3.1
R-3295-A	D	4.7
R-3295-B	D	7.1
R-3295-AL	L	3.2
R-3295-BL	L	4.8
R-3295-AV	V	4.1
R-3295-BV	V	6.1
R-3296-A	C	3.6
R-3296-B	C	5.4
R-3298	C	1.7
R-3299-A	Steel	4.8
R-3299-C	Steel	3.5
R-3336	A or C	1.8
R-3337-A	C	1.1
R-3338-F	A	1.4
R-3338-G	A	2.8
R-3339	C	1.6
R-3339-A	A or C	1.8
R-3339-B	A or C	2.1
R-3339-D	G	1.1
R-3340-B	C	1.3
R-3340-C	C	1.2
R-3340-D	C	1.2
R-3340-E	C	2.4
R-3341	K	0.3
R-3342	K	0.5
R-3344	K	1.1
R-3345	K	1.1
R-3346	K	1.4
R-3347	K	1.3
R-3347-A	K	2.3
R-3348	K	1.9
R-3349	K	2.4
R-3349-A	K	1.8
R-3351	K	2.7
R-3352	K	3.5
R-3355	K	4.1
R-3356-A	K	1.5
R-3357	K	2.2
R-3359	K	1.1
R-3360-A	K	1.6
R-3361	K	3.4
R-3381	K	1.0

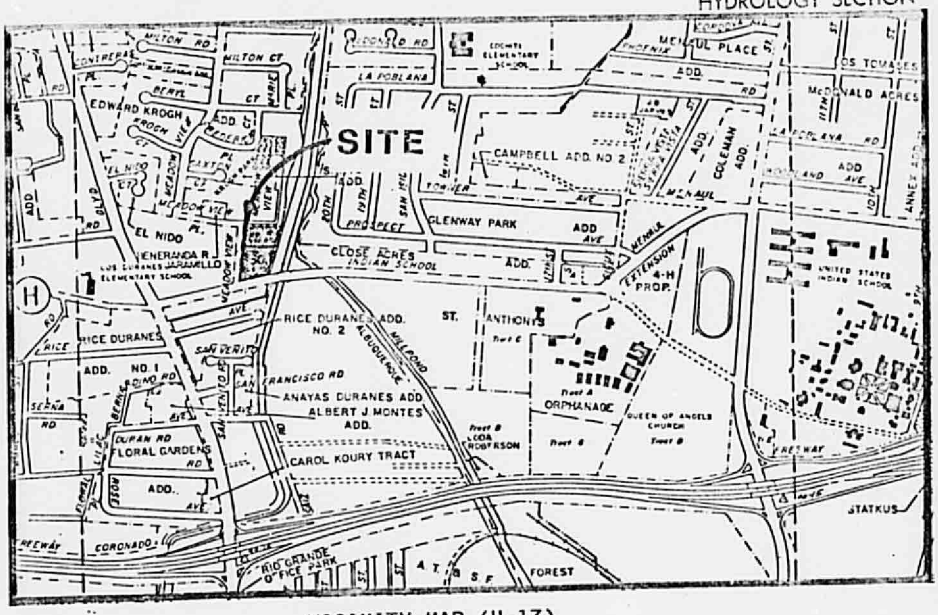
CATALOG NO.	TYPE	SQ. FT. OPEN
R-3382	K	2.3
R-3383-A	K	2.7
R-3383-B	K	4.0
R-3390	K	2.2
R-3392	K	1.8
R-3393-A	K	2.0
R-3396	K	3.2
R-3397	A	1.1
R-3397-1	V	1.5
R-3401	D	1.9
R-3401-B	B	3.8
R-3401-C	B	5.7
R-3402-3	A or C	0.4
R-3402-F	A or C	3.2
R-3403-A	B	1.7
R-3403-F	A	3.2
R-3404	A or C	1.4
R-3406	C	1.8
R-3406-A	C	1.8
R-3407	A or C	5.3
R-3408	D	1.4
R-3408-A	D	2.8
R-3408-B	D	2.8
R-3409	C	1.5
R-3410-1	A	0.2
R-3410	C	0.2
R-3411	B	2.1
R-3412	A	2.0
R-3413	C	2.0
R-3415	A	4.1
R-3416	A	1.4
R-3417	C	3.8
R-3419	B	2.6
R-3420	A	0.8
R-3423	B	2.1
R-3425-A	K	1.6
R-3425-B	K	1.6
R-3429-A	A or C	1.0
R-3430	A or C	0.9
R-3431	A or C	0.9
R-3433	A or C	2.0
R-3434	A or C	1.0
R-3435	A or C	2.7
R-3436	A or C	1.7
R-3437	A or C	3.3
R-3438	A or C	7.0
R-3439-A	A or C	0.4
R-3439-B	A or C	0.8
R-3439-C	A or C	1.3
R-3439-D	A or C	1.4
R-3439-E	A or C	1.9
R-3439-F	A or C	1.9
R-3439-G	A or C	1.8
R-3439-H	A or C	2.2
R-3439-J	A or C	3.0
R-3439-K	A or C	3.7
R-3440	C	0.2
R-3440-A	C	0.6
R-3440-A1	A	0.4
R-3440-S	C	0.8
R-3441	A	1.0
R-3442	B	2.1
R-3443	A	0.8
R-3443-A	A	1.3
R-3443-B	A	1.3
R-3446-A	B	1.3
R-3446-B1	D	2.1
R-3446-C	A	1.6
R-3447-A	A	1.2
R-3448-B	C	1.1
R-3448-C	C	0.9
R-3448-D	S	1.2
R-3449	C	0.9
R-3450	A	0.7
R-3451	C	1.2
R-3451-B	C	1.7
R-3453	A	1.5
R-3454	A	4.1
R-3454-B	D	4.1
R-3455-A	A	2.6
R-3455-C	A	2.6
R-3456	C	2.7
R-3456-A	A	2.6
R-3457	A	4.6
R-3457-B	C	5.4

Type K indicates "Special" grate style and is not among standard types as illustrated.

CITY OF ALBUQUERQUE

DRAINAGE FACILITIES WITHIN CITY RIGHT-OF-WAY

RECEIVED
APR 08 1986
HYDROLOGY SECTION



VICINITY MAP (H-13)

SITE DATA:

LEGAL: TRACTS A AND B OF THE PUCETTI TRACT, 1.92 ACRES
ADDRESS: 1820 MEADOW VIEW LANE N.W.
AGENT: MARK FELDMAN
1821 MEADOW VIEW LANE N.W.
BENCHMARK: CITY OF ALBUQ. CONTROL BM-7-H13 LOCATED AT RIO GRANDE BLVD. AND INDIAN SCHOOL RD.
TEMP. BM.: NO. EDGE OF MH COVER @ INDIAN SCHOOL RD. & MEADOW VIEW. EL: 4959.85

CERTIFIED ARTIFACT NEW MEXICO 7322 REGISTERED PROFESSIONAL ENGINEER
4-8-85
REVISED 3-20-85

NOTICE TO CONTRACTOR

1. An excavation/construction permit will be required before beginning any work within City right-of-way. An approved copy of these plans must be submitted at the time of application for this permit.
2. All work detailed on these plans to be performed, except as otherwise stated or provided hereon, shall be constructed in accordance with PUBLIC WORKS CONTRACT 84-3.
3. Two working days prior to any excavation, contractor must contact Line Locating Service, 765-1234, for location of existing utilities.
4. Prior to construction, the contractor shall excavate and verify the horizontal and vertical locations of all obstructions. Should a conflict exist, the contractor shall notify the engineer so that the conflict can be resolved with a minimum amount of delay.
5. Backfill compaction shall be according to RESIDENTIAL street use.

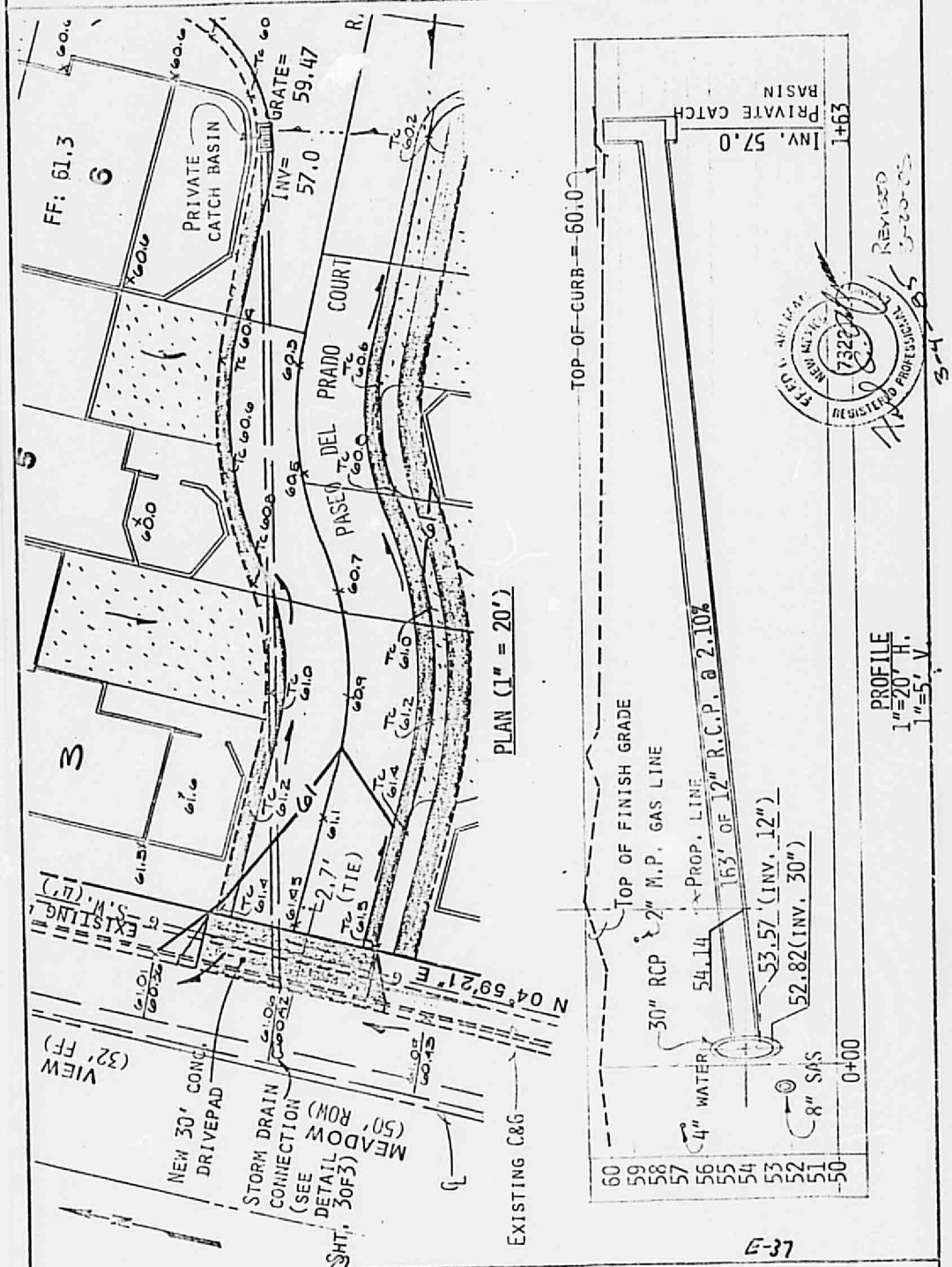
H13-D16

E-37

APPROVALS	NAME	DATE	TITLE:
A.C.E./DESIGN		03/25/85	PASEO DEL PRADO SUBDIVISION PRIVATE STORM DRAIN CONNECTION 1820 MEADOW VIEW LANE N.W.
INSPECTOR	C. S.		

CITY OF ALBUQUERQUE

DRAINAGE FACILITIES WITHIN CITY RIGHT-OF-WAY



E-37

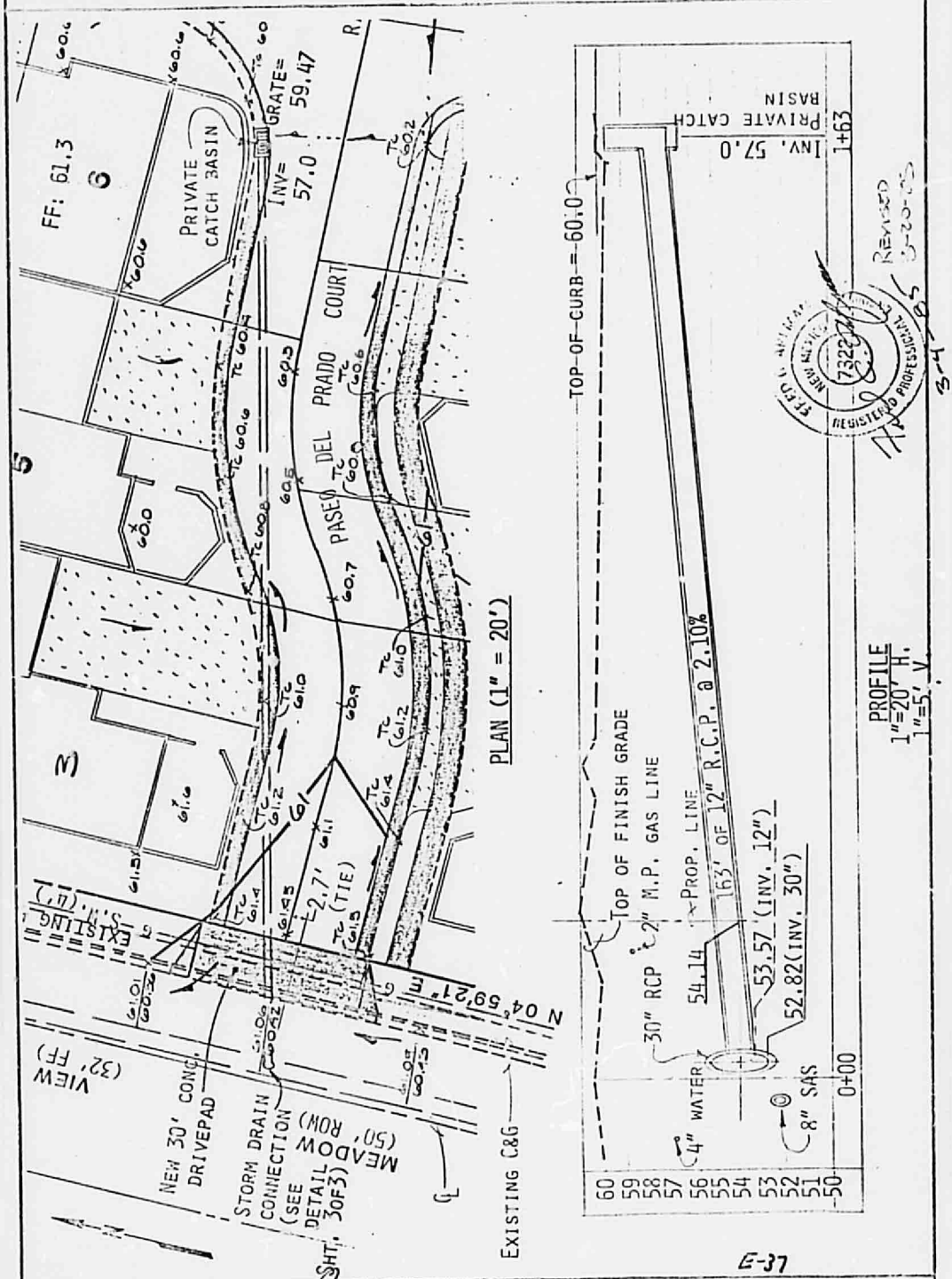
APPROVALS	NAME	DATE
A.C.E./DESIGN		03/25/05
INSPECTOR		

TITLE:
PASEO DEL PRADO SUBDIVISION
PRIVATE STORM DRAIN CONNECTION
1820 MEADOW VIEW LANE N.W.

PROJ. NO. 24155

CITY OF ALBUQUERQUE

DRAINAGE FACILITIES WITHIN CITY RIGHT-OF-WAY



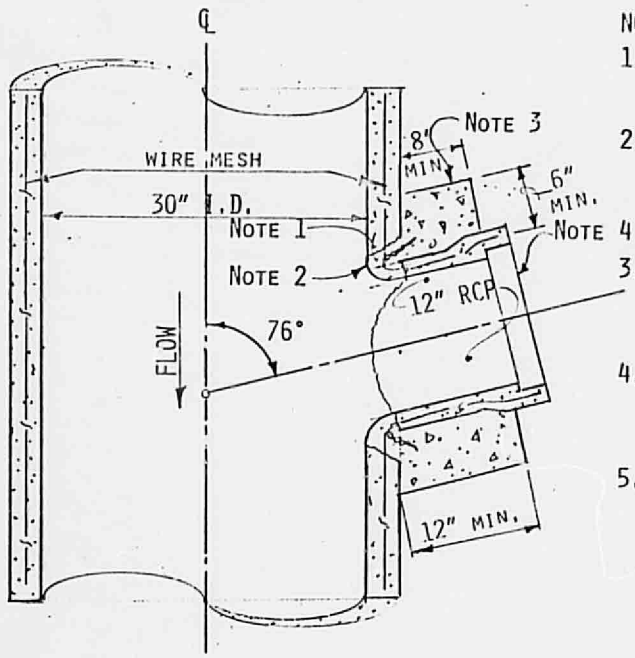
E-37

APPROVALS	NAME	DATE
A.C.E./DESIGN		07/25/85
INSPECTOR		

TITLE:
PASEO DEL PRADO SUBDIVISION
PRIVATE STORM DRAIN CONNECTION
1820 MEADOW VIEW LANE N.W.
PERMIT NO. 24755

CITY OF ALBUQUERQUE

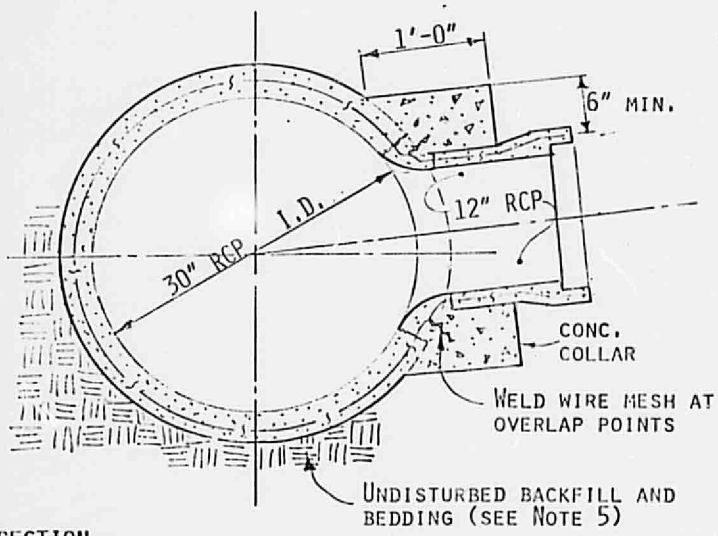
DRAINAGE FACILITIES WITHIN CITY RIGHT-OF-WAY



PLAN
1" = 1'-0"

NOTES

1. BREAK OUT CONCRETE & OVERLAP WIRE MESH (MIN. OF 6")
2. GROUT INSIDE OF INLET JUNCTION STRUCTURE TO FORM A SMOOTH SURFACE EQUAL TO THE INSIDE DIA. OF OUTLET PIPE.
3. CONCRETE COLLAR TO BE 3,000 PSI WITH 3 #4 CIRCUMFERENTIAL AND #3 TIES @ 12"
4. USE BELL END (MAX. LENGTH OF 18") TO ALLOW ACCESS TO SHAPE INSIDE SURFACES OF PIPES.
5. ANY DISTURBED BACKFILL OR BEDDING AROUND THE LOWER TWO QUADRANTS OF THE 30" DIA. PIPE, SHALL BE REMOVED AND REPLACED W/3,000 PSI CONCRETE.



SECTION
1" = 1'-0"

REGISTERED PROFESSIONAL ENGINEER 7322
5-4-85
REVISED 3-20-85

CONNECTION DETAIL

E-37

APPROVALS	NAME	DATE
A.C.E./DESIGN		05/05/85
INSPECTOR	C.S.	

TITLE: PASEO DEL PRADO SUBDIVISION STORM DRAIN CONNECTION 1820 MEADOW VIEW LANE N.W.

PERMIT NO. 2-0755 | MAP