CITY OF ALBUQUERQUE

Planning Department Brennon Williams, Director



Mayor Timothy M. Keller

December 6, 2019

Fred Arfman, P.E. Isaacson & Arfman, P.A. 128 Monroe St. N.E Albuquerque, NM 87108

RE: AIS - Avanyu Retail 12th & Menual NW Conceptual Grading and Drainage Plan Stamp Date: 11/26/19 Hydrology File: H13D113

Dear Mr. Arfman:

	Based on the submittal received on 11/27/19 the above-referenced submittal is approved for Site
PO Box 1293	Plan for Building Permit.

If you have any questions, please contact me at 924-3695 or dpeterson@cabq.gov.

Albuquerque

NM 87103

Sincerely,

www.cabq.gov

Dana Peterson, P.E. Senior Engineer, Planning Dept. Development Review Services



City of Albuquerque

Planning Department Development & Building Services Division DRAINAGE AND TRANSPORTATION INFORMATION SHEET (REV 11/2018)

Project Title: Avanyu Retail	Building Permit #:	Hydrol	ogy File #: <u>H13</u>
DRB#:			
Legal Description: Tract A, Plat US			
City Address: <u>12th Street & Menau</u>		••••	· · · ·
Applicant: Isaacson & Arfman, PA	A	Contact:	Fred C. Arfman
Address: 128 Monroe Street NE	- Albuquerque, NM 87108		Bryan J. Bobrick
Phone#: (505) 268-8828	Fax#:	E-mail:	freda@iacivil.com
			bryanb@iacivil.com
Owner: US Indian Service and Bur			
Address: 1015 Indian School Road			
Phone#:	Fax#:	E-mail:	
Check all that Apply: TYPE OF SUBMITTAL: ENGINEER/ARCHITECT CERTIFI PAD CERTIFICATION X CONCEPTUAL G & D PLAN GRADING PLAN DRAINAGE MASTER PLAN DRAINAGE REPORT FLOODPLAIN DEVELOPMENT PI ELEVATION CERTIFICATE CLOMR/LOMR	CATION B CATION C P SI SI ERMIT APPLIC F G G	OF APPROVAL/ACCE UILDING PERMIT APPI ERTIFICATE OF OCCU RELIMINARY PLAT AF ITE PLAN FOR SUB'D ITE PLAN FOR BLDG. I NAL PLAT APPROVAL (A/ RELEASE OF FINAN OUNDATION PERMIT RADING PERMIT APPI O-19 APPROVAL	ROVAL PANCY PROVAL APPROVAL PERMIT APPROVAL L NCIAL GUARANTEE APPROVAL ROVAL
TRAFFIC CIRCULATION LAYOU TRAFFIC IMPACT STUDY (TIS) OTHER (SPECIFY) PRE-DESIGN MEETING?	T (TCL)P G W C F	AVING PERMIT APPRO RADING/ PAD CERTIF ORK ORDER APPROVAL LOMR/LOMR LOODPLAIN DEVELOP THER (SPECIFY)	ICATION L

DATE SUBMITTED: November 26, 2019 By: Fred C. Arfman

COA STAFF:

ELECTRONIC SUBMITTAL RECEIVED:

FEE PAID:

UTILITY NOTES

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- A. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE COMPLETE INSTALLATION OF ALL WORK RELATED TO PROPOSED UTILITIES SHOWN ON THIS PLAN INCLUDING: TRENCHING, BACKFILL, SUPPORTS, CLEANOUT PADS, SERVICE STOPS AND BOXES, SERVICE LINES, TESTING, CLEANING, AND STERILIZING. ANY WORK NOT ACCEPTED BY THE ARCHITECT OR ENGINEER DUE TO IMPROPER WORKMANSHIP OR LACK OF PROPER COORDINATION SHALL BE REMOVED AND CORRECTLY INSTALLED AT THE CONTRACTOR'S EXPENSE, AS DIRECTED.
- B. MINIMUM COVER SHALL BE 36" FOR WATERLINES AND 48" FOR SANITARY SEWER, EXCEPT AT BUILDING CONNECTIONS.
- C. UTILITY LINES SHALL BE INSTALLED AFTER COMPLETION OF THE SITE ROUGH GRADING.
- D. UTILITY LINES SHALL BE INSTALLED PRIOR TO SURFACE IMPROVEMENTS SUCH AS PAVEMENT, SIDEWALKS, AND LANDSCAPING.
- E. CONTRACTOR SHALL BE RESPONSIBLE FOR CONNECTIONS TO BUILDING PLUMBING AND ALL NECESSARY FITTINGS. FITTING COSTS SHALL BE INCIDENTAL. REFER TO THE MECHANICAL AND/OR PLUMBING PLANS FOR SERVICE CONNECTIONS
- F. DRY UTILITY LOCATIONS AND DESIGN ARE NOT A PART OF THIS PLAN. CONTRACTOR SHALL COORDINATE WITH THE LOCAL DRY UTILITY COMPANIES TO DETERMINE THE SIZE, DEPTH, LOCATION, FITTINGS AND REQUIRED APPURTENANCES FOR THE DRY UTILITY SERVICE LINES ON THE SITE. REFER TO MECHANICAL AND ELECTRICAL PLANS FOR SERVICE CONNECTIONS.
- G. TRENCHING, BORING, AND JACKING SHALL BE CONSTRUCTED IN ACCORDANCE WITH COA SPEC. SECT. 700. ALL BACKFILL SHALL BE COMPACTED TO A MINIMUM 95% DENSITY PER ASTM D-1557.
- H. ALL WATER VALVE BOXES, MANHOLE RINGS & COVERS, AND OTHER SURFACE ITEMS FOR THE UTILITIES SHALL BE ADJUSTED TO FINISHED GRADE.
- I. ALL CROSSINGS OF WATER AND SEWER LINES SHALL HAVE 12" MIN CLEARANCE. IF 12" CLEARANCE IS NOT POSSIBLE, BOTH PIPES SHALL BE ENCASED IN CONCRETE OR AS DIRECTED BY THE ENGINEER.
- J. VALVES, METERS, SERVICE LINES, METER AND VALVE BOXES, TAPPING SLEEVES, HYDRANTS, AND OTHER WATER APPURTENANCES SHALL BE CONSTRUCTED IN ACCORDANCE WITH COA SPEC. SECT. 800.
- K. WATERLINES LESS THAN 4" DIAMETER SHALL BE COPPER TYPE K MEETING ASTM B 88 REQUIREMENTS. WATERLINES 4" IN DIAMETER OR LARGER SHALL BE PVC PIPE MEETING AWWA C900 DR-18 REQUIREMENTS.
- L. ALL FITTINGS AND COUPLINGS FOR WATERLINES LESS THAN 4" IN DIAMETER ARE TO BE COPPER, SOLDER JOINT FITTINGS IN ACCORDANCE WITH ASME 16.18 OR ASME B16.22.
- M. ALL FITTINGS AND COUPLINGS FOR WATERLINES 4" IN DIAMETER OR LARGER ARE TO BE MEGA LUG MECHANICAL JOINTS OR ENGINEER APPROVED EQUIVALENT.
- N. JOINTS SHALL BE RESTRAINED BY MEGA LUG HARNESSES, OR ENGINEER APPROVED EQUIVALENT. JOINT RESTRAINTS SHALL BE INSTALLED AT DISTANCES FROM THE FITTINGS AS SHOWN ON THE JOINT RESTRAINT TABLE IN THESE PLANS.
- O. BACKFLOW PREVENTERS SHALL BE INSTALLED IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS.
- P. FIRE LINES SHALL USE PIPE MATERIALS LISTED AND APPROVED FOR FIRE SERVICE BY UNDERWRITERS LABORATORIES.
- Q. FIRE DEPARTMENT CONNECTIONS SHALL MEET UL 405, NFPA 1963, LOCAL FIRE DEPARTMENT REQUIREMENTS, AND IFC 2015.
- R. ADJUST WATER AND FIRE LINES TO AVOID FOOTINGS, SEWER LINES, AND OTHER CONDUITS. INSTALL FITTINGS AS NEEDED.
- S. SEWER MANHOLES, CLEANOUTS, SEWER SERVICE TAPS, AND OTHER SEWER APPURTENANCES SHALL BE CONSTRUCTED IN ACCORDANCE WITH COA SPEC. SECT. 900 / APWA SPEC. SECT. 900 / LOCAL UTILITY COMPANY SPECIFICATIONS
- T. SEWER SERVICE LINES SHALL BE INSTALLED AT A 1% MINIMUM SLOPE, UNLESS OTHERWISE SPECIFIED ON THE PLANS. THE PIPE SHALL DRAIN AT A CONSTANT SLOPE BETWEEN FITTINGS. THE PIPE SHALL DRAIN TOWARD THE SEWER MAIN AT ALL LOCATIONS.
- U. ALL SANITARY SEWER LINE MATERIALS SHALL BE PVC SDR-35 PIPE OR PVC SCH 40 PIPE.

PROJECT DATA

<u>PROPERTY</u>: THE SITE IS A PREVIOUSLY DEVELOPED PROPERTY LOCATED WITHIN C.O.A. VICINITY MAP H-13. THE SITE IS BOUND TO THE EAST AND SOUTH BY DEVELOPED COMMERCIAL PROPERTY, TO THE NORTH BY MENAUL BLVD. NW AND TO THE WEST BY 12TH STREET NW.

PROPOSED IMPROVEMENTS: THE PROPOSED IMPROVEMENTS INCLUDE MULTIPLE COMMERCIAL BUILDINGS WITH ASSOCIATED ASPHALT PAVED ACCESS, PARKING, AND LANDSCAPING.

LEGAL: A PORTION OF TRACT 84E, MRGCD MAP 35 & ADJ. VAC PORTION OF 9TH STREET.

ADDRESS: 2400 12TH STREET NW, ALBUQUERQUE, NM 87104.

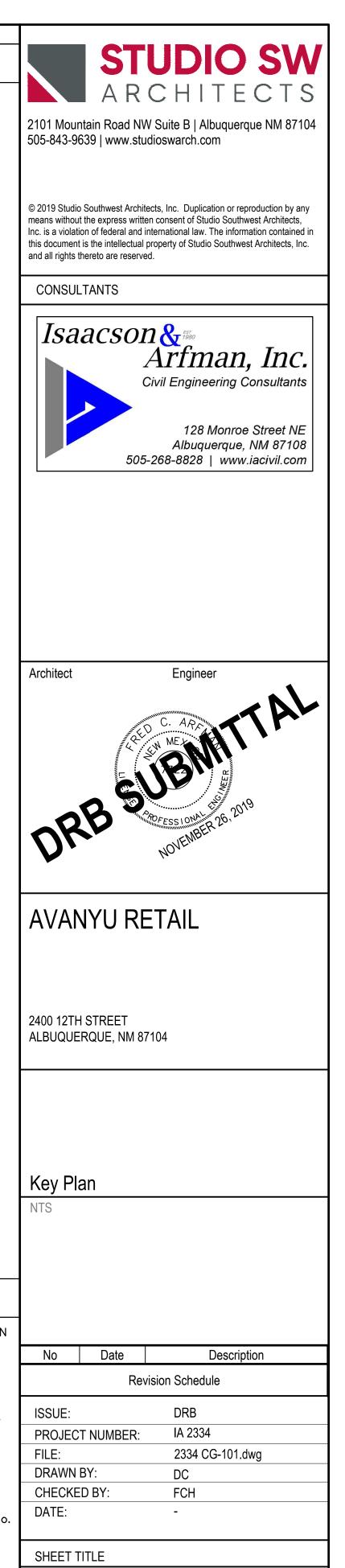
BENCHMARK: ELEVATIONS ARE BASED ON CITY OF ALBUQUERUQUE STATION No. "7-H13", HAVING AN ELEVATION OF 4964.364, NAVD 1988

<u>OFF-SITE</u>: THIS PROPERTY HOUSES A STORM LIFT STATION THAT HAS BEEN DESIGNED TO SERVE THIS PROPERTY AS WELL AS THE PLAZA AND RETAIL PROPERTIES TO THE SOUTH.

FLOOD HAZARD: PER BERNALILLO COUNTY FIRM MAP #35001C0331H, THE SITE IS LOCATED WITHIN FLOODZONE 'X' SHADED DESIGNATED AS AREAS WITH REDUCED FLOOD RISK DUE TO LEVEE.

DRAINAGE PLAN CONCEPT: ON-SITE RUNOFF WILL BE ROUTED TO SURFACE STORMWATER QUALITY / DETENTION PONDS AND A SUB-SURFACE DETENTION SYSTEM. A PRIVATE STORM DRAIN SYSTEM WILL SLOWLY DISCHARGE TO AN EXISTING STORM LIFT STATION PRESET TO PUMP TO THE PUBLIC STORM DRAIN SYSTEM AT THE PREVIOUSLY APPROVED RATE OF 2.0 CFS WHICH INCLUDES THIS PROJECT AS WELL AS THE PLAZA AND RETAIL PROPERTIES TO THE SOUTH - SAME OWNER). SEE SHEET CG-501 FOR ADDITIONAL INFORMATION.

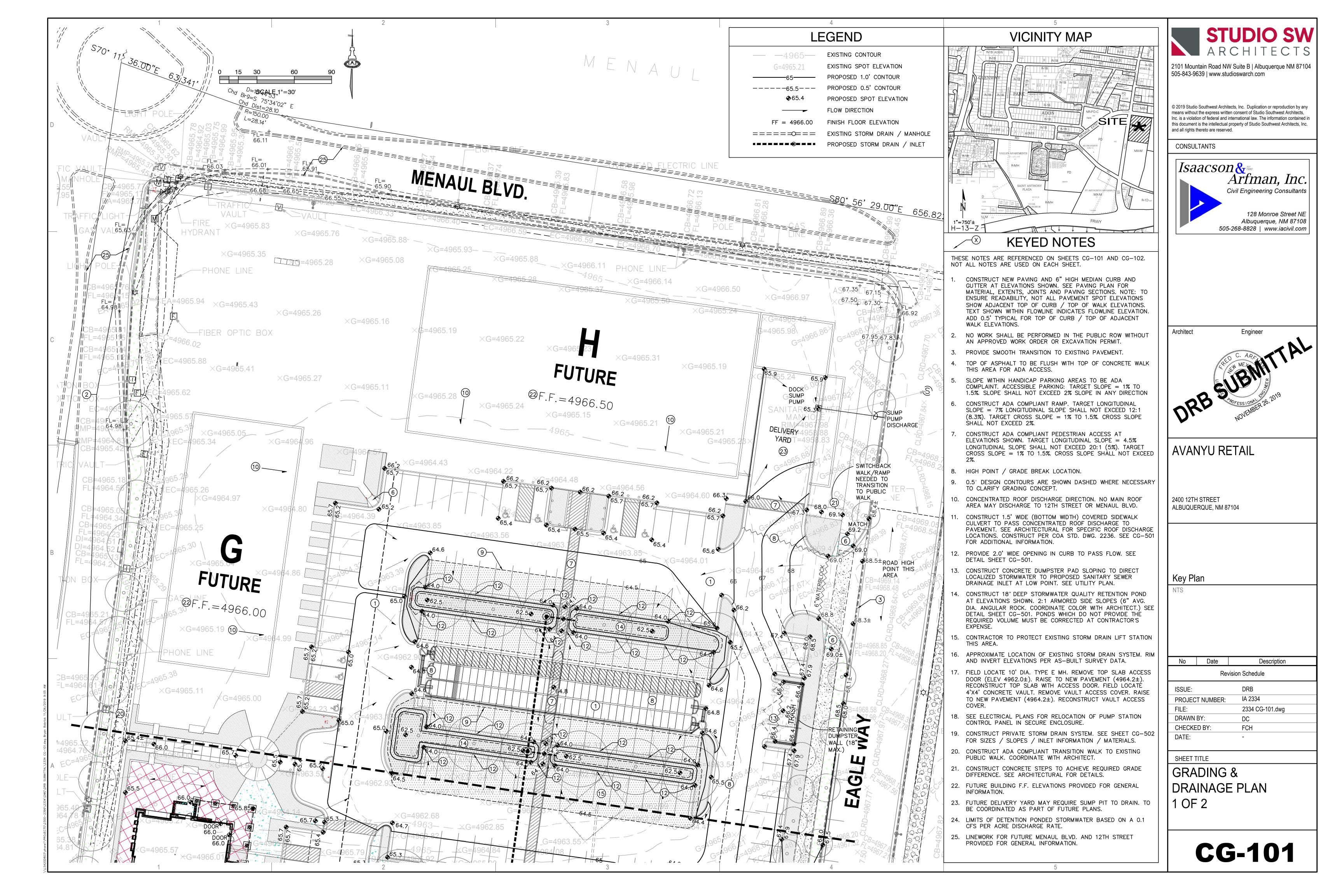
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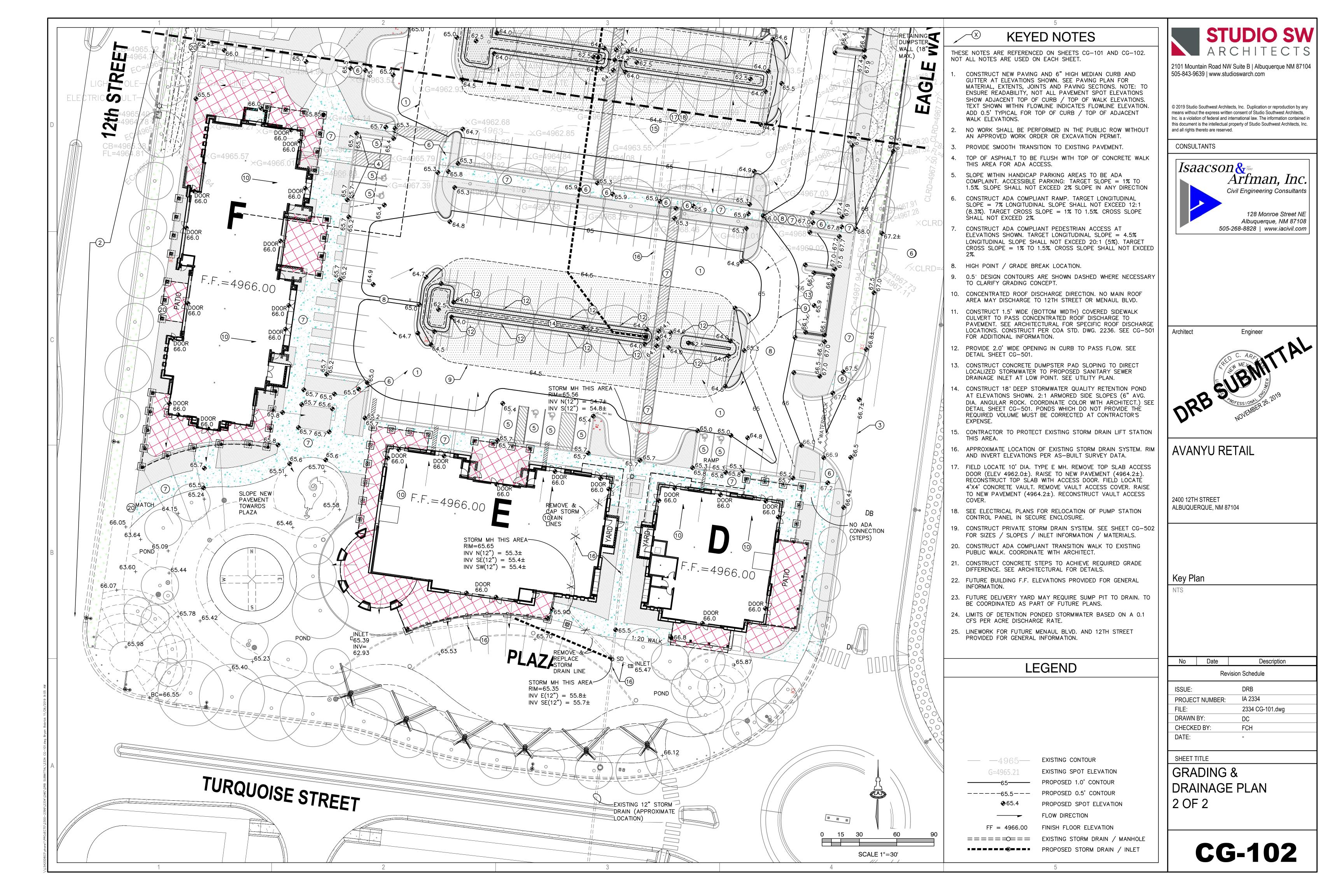


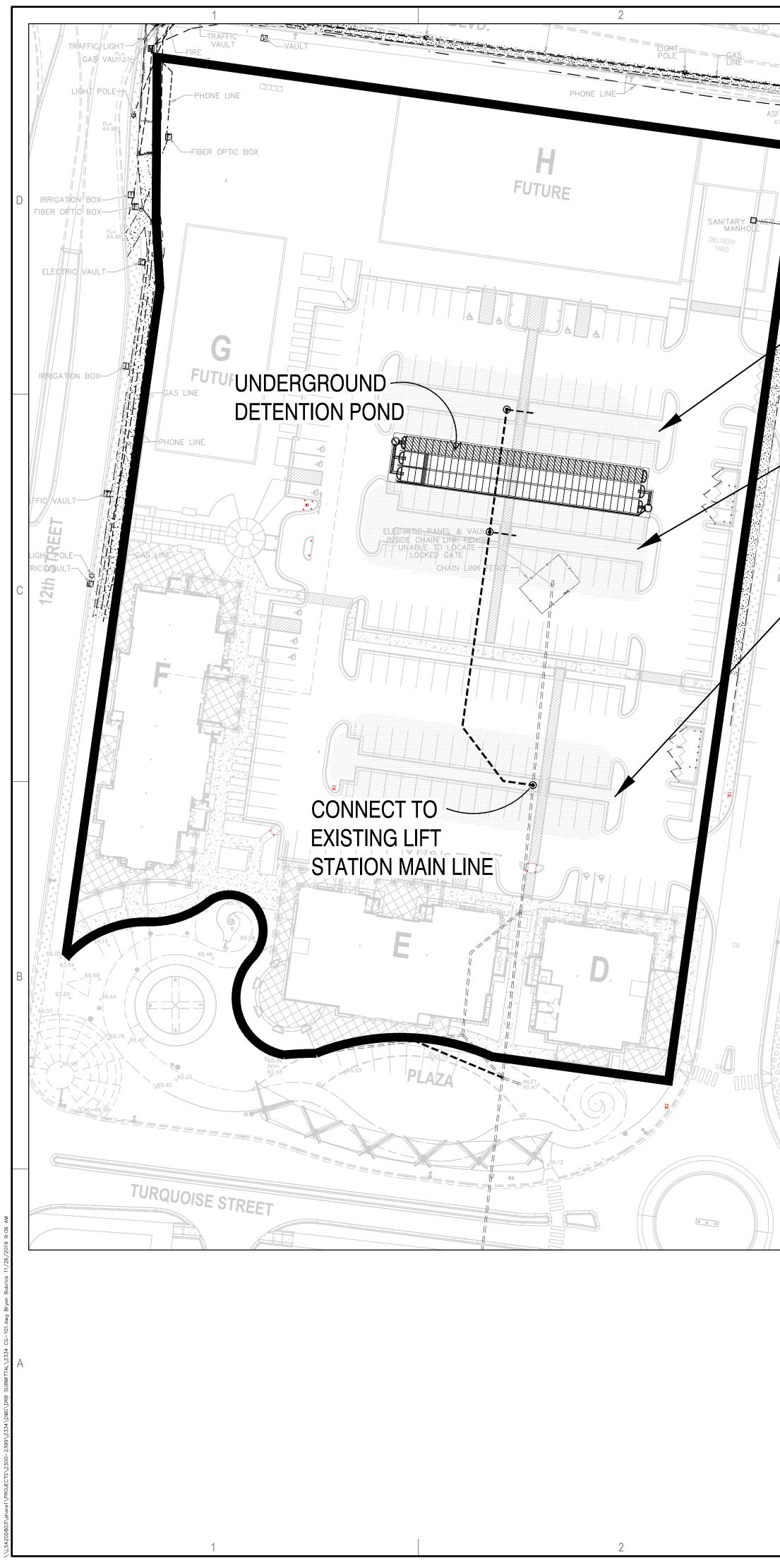
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C-100

NOTES







N		CALCULATIONS: 2334 - IPCC - Menaul & 12th - SE Co	rner : Nov
0		Based on Drainage Design Criteria for City of Albuquerque Section 22.2, DPM,	
2		100-YEAR, 6-HOUR CALCULATIO	
-		AREA OF SITE: 197952 SF	= 4.5
		100-year, 6-hour	
		DEVELOPED FLOWS:	
Ν		Treat	ment SF %
)		Area A =	0 0%
•			9795 109
			9898 5%
			58259 85 ^r
			97952 100
			1932 100
		On-Site Weighted Excess Precipitation (100-Year, 6-Hour Storm)	
		Weighted E = $E_A A_A + E_B A_B + E_C A_C + E_D A_D$	
		$A_{A} + A_{B} + A_{C} + A_{D}$ Developed E =	1.94 in.
			1, 77 111.
	TH POND	On-Site Volume of Runoff: $V360 = E^*A / 12$	
	Area Volu	Developed $V_{360} =$	31945 C
4964.5	6515		
4964.0	1834 2087 C	On-Site Peak Discharge Rate: $Qp = Q_{pA}A_A + Q_{pB}A_B + Q_{pC}A_C + Q_{pD}A_D / 43,560$	
4963.0	992 1413 C	For Precipitation Zone 2	
4962.5	725 429 0	$Q_{pA} = 1.56$ $Q_{pC} = 3.14$	
OND VOL	UME = 3930 C		10.0 CI
CENT	TED DOND	Developed $Q_p =$	19.9 CH
	TER POND		
	Area Volui 8141		
4964.0	1854 2499 C		
4963.0	1125 1490 C	CALCULATIONS: 2334 - IPCC - Menaul & 12th - SE Corner : November 2	20,2019
4962.5	792 479 0	HYDROGRAPH FOR SMALL WATERSHED DPM SECTION 22-2 * PAGE A-13/14	
		DIMORTION 22-2 TAUEA-15/17	
POND VOL	UME = 4468 (Base time, tB, for a small watershed hydrograph is,	
		$tB = (2.107 * E * A / Q_P) - (0.25 * A_D / A)$	
SOU	TH POND	Where $E = 1.94$ inches	
Contour	Area Volu	$A = 4.54 \text{ acres}$ $A_D = 3.86 \text{ acres}$	
4004 5	9043	$\begin{array}{rcl} A_D &=& 3.86 \text{ acres} \\ Q_P &=& 19.9 \text{ cfs} \end{array}$	
4964.5	1087 2533 0	Qr 17.7 CIS	
4964.5 4964.0			
	530 809 0	$t_{\rm B} = 0.72 \text{ hours}$	
4964.0	530 809 C 258 197 C		
4964.0 4963.0		$t_B = 0.72$ hours E is the excess precipitation in inches (from DPM TABLE A-8), Q_P is the peat flow, A_D is the area (acres) of treatment D, and A_T is the total area in acres. Us	

INTERIOR. PERIMETER LANDSCAPE AND PAVEMENT WILL DRAIN TO THE SURROUNDING STREETS.

THIS SITE WILL DETAIN THE 100-YEAR 6-HOUR VOLUME WITHIN SURFACE PONDS AND UNDERGROUND OPEN CHAMBER SYSTEM. THESE DETENTION AREAS WILL BE CONNECTED TO THE EXISTING STORM DRAIN LIFT STATION CONSTRUCTED WITH THE ADJACENT SITE (COA HYDROLOGY NO. H13D106). THIS SYSTEM IS DESIGNED TO PUMP 2.0 CFS TO THE PUBLIC STORM DRAIN.

PER THE LIFT STATION DESIGN, THE TOTAL ACREAGE IMPACTING THE LIFT STATION IS 13.1 ACRES. THE PER ACRE DISCHARGE RATE IS 2.0/13.1=.15 CFS/ACRE. THEREFORE, THE DETENTION VOLUME IS BASED ON A DISCHARGE RATE OF 4.5*.15 =.68 CFS (SEE INFLOW/OUTFLOW HYDROGRAPH).

TOTAL REQUIRED POND VOLUME=32,285 CF

SURFACE PONDS								
•	NORTH	=	3930	CF				
•	CENTER	=	4468	CF				
•	SOUTH	=	3538	CF				

TOTAL SURFACE= 11935 CF

THE REMAINING (32285-11935) 20,350 CF WILL BE COLLECTED IN AN UNDERGROUND STORMTECH MC-4500 STORMWATER DETENTION SYSTEM.

MINOR PERIMETER LANDSCAPING AND PAVEMENT FRONTING MENAUL AND 12TH STREET WILL DISCHARGE TO THE STREETS.

PER THE APPROVED LIFT STATION DESIGN: LEAD PUMP TURNS ON AT 4955.00 LAG PUMP TURNS ON AT 4959.00

Continue the peak for 0.25 * A_D / A_T hours. When A_D is zero, the hydrograph will be triangular. When A_D is not zero, the hyrograph will be trapezoidal. see the graph below:

=

tP

