

CITY OF ALBUQUERQUE

Planning Department
Brennon Williams, Director



Mayor Timothy M. Keller

June 19, 2020

Shawn Biazar
SBS Construction and Engineering, LLC
10209 Snowflake Ct. NW
Albuquerque, NM 87114

**RE: 2818 4th Street NW Apartments
Grading & Drainage Plan
Engineer's Stamp Date: 06/12/20
Hydrology File: H14D111**

Dear Mr. Biazar:

PO Box 1293

Based upon the information provided in your resubmittal received 06/18/2020, the Grading & Drainage Plan is approved for Building Permit, Grading Permit, SO-19 Permit, and for action by the DRB on Site Plan for Building Permit.

Albuquerque

Please attach a copy of this approved plan in the construction sets for Building Permit processing along with a copy of this letter. Prior to approval in support of Permanent Release of Occupancy by Hydrology, Engineer Certification per the DPM checklist will be required.

NM 87103

As a reminder, if the project total area of disturbance (including the staging area and any work within the adjacent Right-of-Way) is 1 acre or more, then an Erosion and Sediment Control (ESC) Plan and Owner's certified Notice of Intent (NOI) is required to be submitted to the Stormwater Quality Engineer (Doug Hughes, PE, jhughes@cabq.gov, 924-3420) 14 days prior to any earth disturbance.

www.cabq.gov

Also as a reminder, please provide Drainage Covenant for the stormwater quality ponds per Chapter 17 of the DPM prior to Permanent Release of Occupancy. Please submit this on the 4th floor of Plaza de Sol. A \$25 fee will be required.

If you have any questions, please contact me at 924-3995 or rbrissette@cabq.gov.

Sincerely,

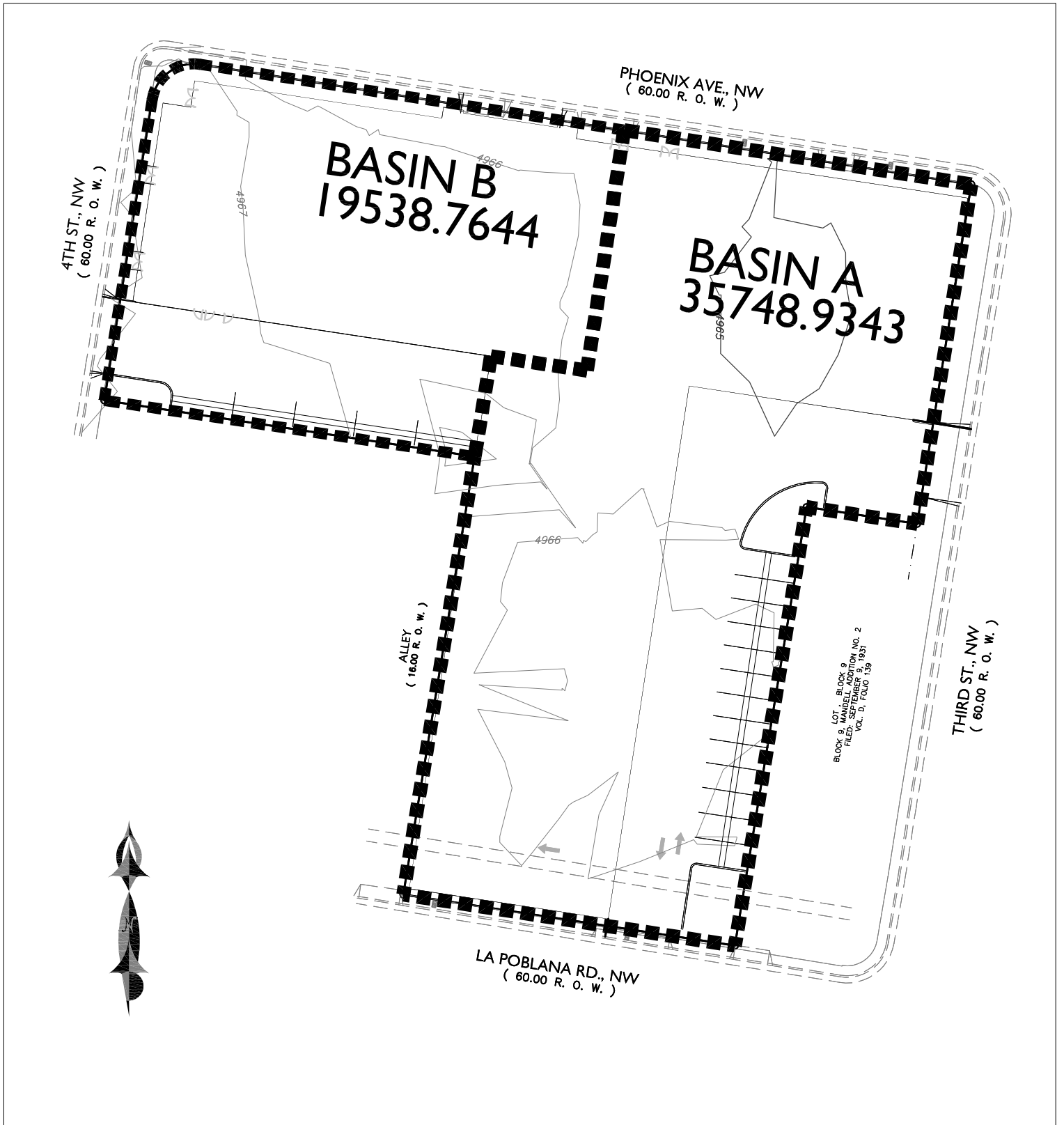
Renée C. Brissette, P.E. CFM
Senior Engineer, Hydrology
Planning Department

2818 4TH STREET, NW

DRAINAGE CALCULATIONS



6/12/2020



NTS

BASIN MAP

RUNOFF CALCULATION RESULTS

BASIN	AREA (SF)	AREA (AC)	AREA (MI ²)
A	35748.93	0.82068	0.001282
B	19538.76	0.44855	0.000701

EXISTING / HISTORICAL

BASIN	Q-100 CFS	Q-10 CFS	TREATMENT A, B, C, D
A	3.22	1.96	0%, 20%, 20%, 60%
B	1.77	1.08	0%, 20%, 20%, 60%

PROPOSED

BASIN	Q-100 CFS	Q-10 CFS	TREATMENT A, B, C, D
A	3.77	2.47	0%, 5%, 0%, 95%
B	2.07	1.35	0%, 5%, 0%, 95%

Pond A Volume & Discharge Calculations

Actual Elev. (ft)	Depth (ft)	Surface Area (sf)	Volume (cf)	Volume (ac-ft)	Q (cfs) (cfs)
64.50	0.00	519.00	0.00	0.000000	0.000
64.75	64.75	675.21	149.28	0.003427	0.210
65.00	65.00	2211.70	510.14	0.011711	0.297
65.50	65.50	5150.33	2350.65	0.053963	0.420
66.00	66.00	7602.04	5538.74	0.127152	0.514

$$Q = CA \sqrt{2gH}$$

C 0.6

Orifice Size (in) 4 (dia)

A = 0.087

Pond B Volume & Discharge Calculations

Actual Elev. (ft)	Depth (ft)	Surface Area (sf)	Volume (cf)	Volume (ac-ft)	Q (cfs) (cfs)
66.48	0.00	211.52	0.00	0.000000	0.000
66.75	66.75	823.87	139.78	0.003209	1.746
67.00	67.00	1346.14	411.03	0.009436	2.423
67.15	67.15	4356.2	838.70	0.019254	2.750

$$Q = CA \sqrt{2gH}$$

C 0.6

Orifice Size (in) 8 (dia)

A = 0.698 2-8"

AHUMO INPUT FILE

```
* ZONE 2
*****
*          100-YEAR,  6-HR STORM (UNDER EXISITNG CONDITIONS)      *
*****
START          TIME=0.0
RAINFALL       TYPE=1 RAIN QUARTER=0.0 IN
               RAIN ONE=2.01 IN RAIN SIX=2.35 IN
               RAIN DAY=2.75 IN DT=0.03333 HR

* BASIN A
COMPUTE NM HYD   ID=1 HYD NO=101.0 AREA=0.001282 SQ MI
                 PER A=0.00 PER B=20.00 PER C=20.00 PER D=60.00
                 TP=0.1333 HR MASS RAINFALL=-1
*****
*          10-YEAR,   6-HR STORM (UNDER EXISTING CONDITIONS)      *
*****
START          TIME=0.0
RAINFALL       TYPE=1 RAIN QUARTER=0.0 IN
               RAIN ONE=1.34 IN RAIN SIX=1.57 IN
               RAIN DAY=1.83 IN DT=0.03333 HR

* BASIN A
COMPUTE NM HYD   ID=2 HYD NO=111.0 AREA=0.001282 SQ MI
                 PER A=0.00 PER B=20.00 PER C=20.00 PER D=60.00
                 TP=0.1333 HR MASS RAINFALL=-1
*****
*          100-YEAR,  6-HR STORM (UNDER PROPOSED CONDITIONS)      *
*****
START          TIME=0.0
RAINFALL       TYPE=1 RAIN QUARTER=0.0 IN
               RAIN ONE=2.01 IN RAIN SIX=2.35 IN
               RAIN DAY=2.75 IN DT=0.03333 HR

* ON-STIE
COMPUTE NM HYD   ID=3 HYD NO=101.1 AREA=0.001282 SQ MI
                 PER A=0.00 PER B=5.00 PER C=0.00 PER D=95.00
                 TP=0.1333 HR MASS RAINFALL=-1
*****
*          10-YEAR,   6-HR STORM (UNDER PROPOSED CONDITIONS)      *
*****
START          TIME=0.0
RAINFALL       TYPE=1 RAIN QUARTER=0.0 IN
               RAIN ONE=1.34 IN RAIN SIX=1.57 IN
               RAIN DAY=1.83 IN DT=0.03333 HR

* ON-SITE
COMPUTE NM HYD   ID=4 HYD NO=111.1 AREA=0.001282 SQ MI
                 PER A=0.00 PER B=5.00 PER C=0.00 PER D=95.00
                 TP=0.1333 HR MASS RAINFALL=-1
*****
*          PONDING CONDITION                                         *
*****
ROUTE RESERVOIR ID=20 HYD NO=501.1 INFLOW ID=3 CODE=24
                OUTFLOW(CFS)      STORAGE(AC-FT)  ELEVATION(FT)
                0.00              0.000000      64.50
                0.210             0.003427      64.75
                0.297             0.011711      65.00
                0.420             0.053963      65.50
                0.514             0.127152      66.00
*****
*          100-YEAR,  6-HR STORM (UNDER EXISITNG CONDITIONS)      *
*****
START          TIME=0.0
RAINFALL       TYPE=1 RAIN QUARTER=0.0 IN
               RAIN ONE=2.01 IN RAIN SIX=2.35 IN
               RAIN DAY=2.75 IN DT=0.03333 HR

* BASIN B
COMPUTE NM HYD   ID=5 HYD NO=102.0 AREA=0.000701 SQ MI
                 ER A=0.00 PER B=20.00 PER C=20.00 PER D=60.00
                 TP=0.1333 HR MASS RAINFALL=-1
*****
*          10-YEAR,   6-HR STORM (UNDER EXISTING CONDITIONS)      *
*****
START          TIME=0.0
RAINFALL       TYPE=1 RAIN QUARTER=0.0 IN
               RAIN ONE=1.34 IN RAIN SIX=1.57 IN
               RAIN DAY=1.83 IN DT=0.03333 HR

* BASIN A
```

```

COMPUTE NM HYD      ID=6 HYD NO=112.0 AREA=0.000701 SQ MI
                    ER A=0.00 PER B=20.00 PER C=20.00 PER D=60.00
                    TP=0.1333 HR MASS RAINFALL=-1
*****
*      100-YEAR,    6-HR STORM (UNDER PROPOSED CONDITIONS)      *
*****
START              TIME=0.0
RAINFALL           TYPE=1 RAIN QUARTER=0.0 IN
                   RAIN ONE=2.01 IN RAIN SIX=2.35 IN
                   RAIN DAY=2.75 IN DT=0.03333 HR

* ON-STIE
COMPUTE NM HYD      ID=7 HYD NO=102.1 AREA=0.000701 SQ MI
                    PER A=0.00 PER B=5.00 PER C=0.00 PER D=95.00
                    TP=0.1333 HR MASS RAINFALL=-1
*****
*      10-YEAR,    6-HR STORM (UNDER PROPOSED CONDITIONS)      *
*****
START              TIME=0.0
RAINFALL           TYPE=1 RAIN QUARTER=0.0 IN
                   RAIN ONE=1.34 IN RAIN SIX=1.57 IN
                   RAIN DAY=1.83 IN DT=0.03333 HR

* ON-SITE
COMPUTE NM HYD      ID=8 HYD NO=112.1 AREA=0.000701 SQ MI
                    PER A=0.00 PER B=5.00 PER C=0.00 PER D=95.00
                    TP=0.1333 HR MASS RAINFALL=-1
*****
*      PONDING CONDITION                                          *
*****
ROUTE RESERVOIR    ID=30 HYD NO=601.1 INFLOW ID=7 CODE=24
                   OUTFLOW(CFS)      STORAGE (AC-FT)  ELEVATION (FT)
                   0.000              0.000000       66.48
                   1.746              0.003209       66.75
                   2.423              0.009436       67.00
                   2.750              0.019254       67.15
*****

*
ADD HYD             ID=40 HYD NO=110.10 ID=20 ID=30
**
FINISH

```

AHYMO PROGRAM SUMMARY TABLE (AHYMO_97) -
INPUT FILE = 201836.TXT

RUN DATE (MON/DAY/YR) =05/18/2020

USER NO.= AHYMO-I-9702c01000R31-AH

[illegible]

AHYMO PROGRAM (AHYMO 97) -

- Version: 1997.02d

RUN DATE (MON/DAY/YR) = 05/18/2020

START TIME (HR:MIN:SEC) = 11:11:21

USER NO.= AHYMO-I-9702c01000R31-AH

INPUT FILE = 201836.TXT

* ZONE 2

* 100-YEAR, 6-HR STORM (UNDER EXISTING CONDITIONS) *

START TIME=0.0

RAINFALL TYPE=1 RAIN QUARTER=0.0 IN

RAIN ONE=2.01 IN RAIN SIX=2.35 IN

RAIN DAY=2.75 IN DT=0.03333 HR

COMPUTED 6-HOUR RAINFALL DISTRIBUTION BASED ON NOAA ATLAS 2 - PEAK AT 1.40 HR.

DT = .033330 HOURS END TIME = 5.999400 HOURS

.0000	.0016	.0033	.0049	.0066	.0084	.0102
.0120	.0139	.0158	.0178	.0199	.0219	.0241
.0263	.0286	.0309	.0333	.0358	.0384	.0411
.0439	.0467	.0497	.0529	.0561	.0596	.0631
.0669	.0709	.0751	.0807	.0866	.0930	.1066
.1371	.1840	.2514	.3434	.4644	.6186	.8106
1.0449	1.2624	1.3533	1.4300	1.4982	1.5602	1.6174
1.6704	1.7200	1.7664	1.8102	1.8514	1.8904	1.9273
1.9622	1.9953	2.0268	2.0566	2.0850	2.0915	2.0976
2.1033	2.1088	2.1140	2.1191	2.1239	2.1285	2.1329
2.1373	2.1414	2.1454	2.1494	2.1531	2.1568	2.1604
2.1639	2.1673	2.1706	2.1739	2.1771	2.1802	2.1832
2.1862	2.1891	2.1919	2.1947	2.1975	2.2002	2.2028
2.2054	2.2080	2.2105	2.2130	2.2154	2.2178	2.2202
2.2225	2.2248	2.2270	2.2293	2.2315	2.2336	2.2358
2.2379	2.2399	2.2420	2.2440	2.2460	2.2480	2.2500
2.2519	2.2538	2.2557	2.2576	2.2594	2.2612	2.2631
2.2648	2.2666	2.2684	2.2701	2.2718	2.2735	2.2752
2.2769	2.2785	2.2802	2.2818	2.2834	2.2850	2.2866
2.2881	2.2897	2.2912	2.2928	2.2943	2.2958	2.2973
2.2987	2.3002	2.3017	2.3031	2.3045	2.3060	2.3074
2.3088	2.3102	2.3115	2.3129	2.3143	2.3156	2.3169
2.3183	2.3196	2.3209	2.3222	2.3235	2.3248	2.3261
2.3273	2.3286	2.3298	2.3311	2.3323	2.3335	2.3348
2.3360	2.3372	2.3384	2.3396	2.3408	2.3419	2.3431
2.3443	2.3454	2.3466	2.3477	2.3488	2.3500	

* BASIN A

COMPUTE NM HYD

ID=1 HYD NO=101.0 AREA=0.001282 SQ MI

PER A=0.00 PER B=20.00 PER C=20.00 PER D=60.00

TP=0.1333 HR MASS RAINFALL=-1

7.106420 K = .072649HR TP = .133300HR K/TP RATIO = .545000 SHAPE CONSTANT, N =

UNIT PEAK = 3.0368 CFS UNIT VOLUME = .9955 B = 526.28 P60 = 2.0100

AREA = .000769 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR

RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

3.944947 K = .119767HR TP = .133300HR K/TP RATIO = .898476 SHAPE CONSTANT, N =

UNIT PEAK = 1.3521 CFS UNIT VOLUME = .9902 B = 351.48 P60 = 2.0100

AREA = .000513 SQ MI IA = .42500 INCHES INF = 1.04000 INCHES PER HOUR

RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

* 10-YEAR, 6-HR STORM (UNDER EXISTING CONDITIONS) *

START TIME=0.0

RAINFALL TYPE=1 RAIN QUARTER=0.0 IN

RAIN ONE=1.34 IN RAIN SIX=1.57 IN

RAIN DAY=1.83 IN DT=0.03333 HR

COMPUTED 6-HOUR RAINFALL DISTRIBUTION BASED ON NOAA ATLAS 2 - PEAK AT 1.40 HR.

DT = .033330 HOURS END TIME = 5.999400 HOURS

.0000	.0011	.0022	.0034	.0046	.0058	.0070
.0083	.0095	.0109	.0122	.0136	.0150	.0165
.0180	.0196	.0212	.0229	.0246	.0263	.0282
.0301	.0321	.0341	.0363	.0385	.0408	.0433
.0459	.0486	.0515	.0552	.0591	.0634	.0725
.0928	.1241	.1690	.2304	.3110	.4138	.5418
.6980	.8430	.9036	.9547	1.0002	1.0416	1.0797
1.1150	1.1481	1.1790	1.2082	1.2357	1.2617	1.2863
1.3096	1.3316	1.3526	1.3725	1.3914	1.3958	1.3999
1.4037	1.4074	1.4109	1.4143	1.4175	1.4206	1.4236
1.4265	1.4293	1.4320	1.4346	1.4372	1.4397	1.4421
1.4444	1.4467	1.4490	1.4511	1.4533	1.4554	1.4574
1.4594	1.4614	1.4633	1.4652	1.4670	1.4688	1.4706
1.4724	1.4741	1.4758	1.4775	1.4791	1.4807	1.4823
1.4839	1.4854	1.4869	1.4884	1.4899	1.4914	1.4928
1.4943	1.4957	1.4970	1.4984	1.4998	1.5011	1.5024
1.5037	1.5050	1.5063	1.5075	1.5088	1.5100	1.5113
1.5125	1.5137	1.5148	1.5160	1.5172	1.5183	1.5195
1.5206	1.5217	1.5228	1.5239	1.5250	1.5261	1.5271
1.5282	1.5292	1.5303	1.5313	1.5323	1.5333	1.5344
1.5354	1.5363	1.5373	1.5383	1.5393	1.5402	1.5412
1.5421	1.5431	1.5440	1.5449	1.5458	1.5467	1.5476
1.5485	1.5494	1.5503	1.5512	1.5521	1.5529	1.5538
1.5547	1.5555	1.5564	1.5572	1.5580	1.5589	1.5597
1.5605	1.5613	1.5621	1.5629	1.5637	1.5645	1.5653
1.5661	1.5669	1.5677	1.5685	1.5692	1.5700	

* BASIN A

COMPUTE NM HYD

ID=2 HYD NO=111.0 AREA=0.001282 SQ MI
PER A=0.00 PER B=20.00 PER C=20.00 PER D=60.00
TP=0.1333 HR MASS RAINFALL=-1

7.106420 K = .072649HR TP = .133300HR K/TP RATIO = .545000 SHAPE CONSTANT, N =
UNIT PEAK = 3.0368 CFS UNIT VOLUME = .9955 B = 526.28 P60 = 1.3400
AREA = .000769 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR
RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

3.812062 K = .123717HR TP = .133300HR K/TP RATIO = .928113 SHAPE CONSTANT, N =
UNIT PEAK = 1.3174 CFS UNIT VOLUME = .9897 B = 342.44 P60 = 1.3400
AREA = .000513 SQ MI IA = .42500 INCHES INF = 1.04000 INCHES PER HOUR
RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

* 100-YEAR, 6-HR STORM (UNDER PROPOSED CONDITIONS) *

START TIME=0.0
RAINFALL TYPE=1 RAIN QUARTER=0.0 IN
 RAIN ONE=2.01 IN RAIN SIX=2.35 IN
 RAIN DAY=2.75 IN DT=0.03333 HR

COMPUTED 6-HOUR RAINFALL DISTRIBUTION BASED ON NOAA ATLAS 2 - PEAK AT 1.40 HR.

DT = .033330 HOURS END TIME = 5.999400 HOURS

.0000	.0016	.0033	.0049	.0066	.0084	.0102
.0120	.0139	.0158	.0178	.0199	.0219	.0241
.0263	.0286	.0309	.0333	.0358	.0384	.0411
.0439	.0467	.0497	.0529	.0561	.0596	.0631
.0669	.0709	.0751	.0807	.0866	.0930	.1066
.1371	.1840	.2514	.3434	.4644	.6186	.8106
1.0449	1.2624	1.3533	1.4300	1.4982	1.5602	1.6174
1.6704	1.7200	1.7664	1.8102	1.8514	1.8904	1.9273
1.9622	1.9953	2.0268	2.0566	2.0850	2.0915	2.0976
2.1033	2.1088	2.1140	2.1191	2.1239	2.1285	2.1329
2.1373	2.1414	2.1454	2.1494	2.1531	2.1568	2.1604
2.1639	2.1673	2.1706	2.1739	2.1771	2.1802	2.1832

2.1862	2.1891	2.1919	2.1947	2.1975	2.2002	2.2028
2.2054	2.2080	2.2105	2.2130	2.2154	2.2178	2.2202
2.2225	2.2248	2.2270	2.2293	2.2315	2.2336	2.2358
2.2379	2.2399	2.2420	2.2440	2.2460	2.2480	2.2500
2.2519	2.2538	2.2557	2.2576	2.2594	2.2612	2.2631
2.2648	2.2666	2.2684	2.2701	2.2718	2.2735	2.2752
2.2769	2.2785	2.2802	2.2818	2.2834	2.2850	2.2866
2.2881	2.2897	2.2912	2.2928	2.2943	2.2958	2.2973
2.2987	2.3002	2.3017	2.3031	2.3045	2.3060	2.3074
2.3088	2.3102	2.3115	2.3129	2.3143	2.3156	2.3169
2.3183	2.3196	2.3209	2.3222	2.3235	2.3248	2.3261
2.3273	2.3286	2.3298	2.3311	2.3323	2.3335	2.3348
2.3360	2.3372	2.3384	2.3396	2.3408	2.3419	2.3431
2.3443	2.3454	2.3466	2.3477	2.3488	2.3500	

* ON-STIE

COMPUTE NM HYD ID=3 HYD NO=101.1 AREA=0.001282 SQ MI
 PER A=0.00 PER B=5.00 PER C=0.00 PER D=95.00
 TP=0.1333 HR MASS RAINFALL=-1

7.106420 K = .072649HR TP = .133300HR K/TP RATIO = .545000 SHAPE CONSTANT, N =
 UNIT PEAK = 4.8083 CFS UNIT VOLUME = .9969 B = 526.28 P60 = 2.0100
 AREA = .001218 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR
 RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

3.563124 K = .132088HR TP = .133300HR K/TP RATIO = .990905 SHAPE CONSTANT, N =
 UNIT PEAK = .15624 CFS UNIT VOLUME = .9154 B = 324.91 P60 = 2.0100
 AREA = .000064 SQ MI IA = .50000 INCHES INF = 1.25000 INCHES PER HOUR
 RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

 * 10-YEAR, 6-HR STORM (UNDER PROPOSED CONDITIONS) *

START TIME=0.0
 RAINFALL TYPE=1 RAIN QUARTER=0.0 IN
 RAIN ONE=1.34 IN RAIN SIX=1.57 IN
 RAIN DAY=1.83 IN DT=0.03333 HR

COMPUTED 6-HOUR RAINFALL DISTRIBUTION BASED ON NOAA ATLAS 2 - PEAK AT 1.40 HR.
 DT = .033330 HOURS END TIME = 5.999400 HOURS

.0000	.0011	.0022	.0034	.0046	.0058	.0070
.0083	.0095	.0109	.0122	.0136	.0150	.0165
.0180	.0196	.0212	.0229	.0246	.0263	.0282
.0301	.0321	.0341	.0363	.0385	.0408	.0433
.0459	.0486	.0515	.0552	.0591	.0634	.0725
.0928	.1241	.1690	.2304	.3110	.4138	.5418
.6980	.8430	.9036	.9547	1.0002	1.0416	1.0797
1.1150	1.1481	1.1790	1.2082	1.2357	1.2617	1.2863
1.3096	1.3316	1.3526	1.3725	1.3914	1.3958	1.3999
1.4037	1.4074	1.4109	1.4143	1.4175	1.4206	1.4236
1.4265	1.4293	1.4320	1.4346	1.4372	1.4397	1.4421
1.4444	1.4467	1.4490	1.4511	1.4533	1.4554	1.4574
1.4594	1.4614	1.4633	1.4652	1.4670	1.4688	1.4706
1.4724	1.4741	1.4758	1.4775	1.4791	1.4807	1.4823
1.4839	1.4854	1.4869	1.4884	1.4899	1.4914	1.4928
1.4943	1.4957	1.4970	1.4984	1.4998	1.5011	1.5024
1.5037	1.5050	1.5063	1.5075	1.5088	1.5100	1.5113
1.5125	1.5137	1.5148	1.5160	1.5172	1.5183	1.5195
1.5206	1.5217	1.5228	1.5239	1.5250	1.5261	1.5271
1.5282	1.5292	1.5303	1.5313	1.5323	1.5333	1.5344
1.5354	1.5363	1.5373	1.5383	1.5393	1.5402	1.5412
1.5421	1.5431	1.5440	1.5449	1.5458	1.5467	1.5476
1.5485	1.5494	1.5503	1.5512	1.5521	1.5529	1.5538
1.5547	1.5555	1.5564	1.5572	1.5580	1.5589	1.5597
1.5605	1.5613	1.5621	1.5629	1.5637	1.5645	1.5653
1.5661	1.5669	1.5677	1.5685	1.5692	1.5700	

* ON-SITE

COMPUTE NM HYD ID=4 HYD NO=111.1 AREA=0.001282 SQ MI
PER A=0.00 PER B=5.00 PER C=0.00 PER D=95.00
TP=0.1333 HR MASS RAINFALL=-1

7.106420 K = .072649HR TP = .133300HR K/TP RATIO = .545000 SHAPE CONSTANT, N =
UNIT PEAK = 4.8083 CFS UNIT VOLUME = .9969 B = 526.28 P60 = 1.3400
AREA = .001218 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR
RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

3.354139 K = .140318HR TP = .133300HR K/TP RATIO = 1.052645 SHAPE CONSTANT, N =
UNIT PEAK = .14884 CFS UNIT VOLUME = .9067 B = 309.53 P60 = 1.3400
AREA = .000064 SQ MI IA = .50000 INCHES INF = 1.25000 INCHES PER HOUR
RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

* PONDING CONDITION *

ROUTE RESERVOIR ID=20 HYD NO=501.1 INFLOW ID=3 CODE=24
OUTFLOW(CFS) STORAGE(AC-FT) ELEVATION(FT)
0.00 0.000000 64.50
0.210 0.003427 64.75
0.297 0.011711 65.00
0.420 0.053963 65.50
0.514 0.127152 66.00

* * * * *

TIME (HRS)	INFLOW (CFS)	ELEV (FEET)	VOLUME (AC-FT)	OUTFLOW (CFS)
---------------	-----------------	----------------	-------------------	------------------

.00	.00	64.50	.000	.00
.80	.00	64.50	.000	.00
1.60	2.59	65.53	.059	.43
2.40	.16	65.75	.091	.47
3.20	.03	65.58	.065	.43
4.00	.02	65.33	.040	.38
4.80	.02	65.08	.018	.32
5.60	.02	64.68	.002	.15
6.40	.00	64.51	.000	.01

PEAK DISCHARGE = .473 CFS - PEAK OCCURS AT HOUR 2.13
MAXIMUM WATER SURFACE ELEVATION = 65.782
MAXIMUM STORAGE = .0953 AC-FT INCREMENTAL TIME= .033330HRS

* 100-YEAR, 6-HR STORM (UNDER EXISITNG CONDITIONS) *

START TIME=0.0
RAINFALL TYPE=1 RAIN QUARTER=0.0 IN
RAIN ONE=2.01 IN RAIN SIX=2.35 IN
RAIN DAY=2.75 IN DT=0.03333 HR

COMPUTED 6-HOUR RAINFALL DISTRIBUTION BASED ON NOAA ATLAS 2 - PEAK AT 1.40 HR.

DT = .033330 HOURS END TIME = 5.999400 HOURS

.0000	.0016	.0033	.0049	.0066	.0084	.0102
.0120	.0139	.0158	.0178	.0199	.0219	.0241
.0263	.0286	.0309	.0333	.0358	.0384	.0411
.0439	.0467	.0497	.0529	.0561	.0596	.0631
.0669	.0709	.0751	.0807	.0866	.0930	.1066
.1371	.1840	.2514	.3434	.4644	.6186	.8106
1.0449	1.2624	1.3533	1.4300	1.4982	1.5602	1.6174
1.6704	1.7200	1.7664	1.8102	1.8514	1.8904	1.9273
1.9622	1.9953	2.0268	2.0566	2.0850	2.0915	2.0976
2.1033	2.1088	2.1140	2.1191	2.1239	2.1285	2.1329
2.1373	2.1414	2.1454	2.1494	2.1531	2.1568	2.1604
2.1639	2.1673	2.1706	2.1739	2.1771	2.1802	2.1832
2.1862	2.1891	2.1919	2.1947	2.1975	2.2002	2.2028
2.2054	2.2080	2.2105	2.2130	2.2154	2.2178	2.2202
2.2225	2.2248	2.2270	2.2293	2.2315	2.2336	2.2358

2.2379	2.2399	2.2420	2.2440	2.2460	2.2480	2.2500
2.2519	2.2538	2.2557	2.2576	2.2594	2.2612	2.2631
2.2648	2.2666	2.2684	2.2701	2.2718	2.2735	2.2752
2.2769	2.2785	2.2802	2.2818	2.2834	2.2850	2.2866
2.2881	2.2897	2.2912	2.2928	2.2943	2.2958	2.2973
2.2987	2.3002	2.3017	2.3031	2.3045	2.3060	2.3074
2.3088	2.3102	2.3115	2.3129	2.3143	2.3156	2.3169
2.3183	2.3196	2.3209	2.3222	2.3235	2.3248	2.3261
2.3273	2.3286	2.3298	2.3311	2.3323	2.3335	2.3348
2.3360	2.3372	2.3384	2.3396	2.3408	2.3419	2.3431
2.3443	2.3454	2.3466	2.3477	2.3488	2.3500	

* BASIN B

COMPUTE NM HYD

ID=5 HYD NO=102.0 AREA=0.000701 SQ MI
ER A=0.00 PER B=20.00 PER C=20.00 PER D=60.00
TP=0.1333 HR MASS RAINFALL=-1

7.106420 K = .072649HR TP = .133300HR K/TP RATIO = .545000 SHAPE CONSTANT, N =
UNIT PEAK = 1.6606 CFS UNIT VOLUME = .9922 B = 526.28 P60 = 2.0100
AREA = .000421 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR
RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

3.944947 K = .119767HR TP = .133300HR K/TP RATIO = .898476 SHAPE CONSTANT, N =
UNIT PEAK = .73935 CFS UNIT VOLUME = .9825 B = 351.48 P60 = 2.0100
AREA = .000280 SQ MI IA = .42500 INCHES INF = 1.04000 INCHES PER HOUR
RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

* 10-YEAR, 6-HR STORM (UNDER EXISTING CONDITIONS) *

START TIME=0.0
RAINFALL TYPE=1 RAIN QUARTER=0.0 IN
RAIN ONE=1.34 IN RAIN SIX=1.57 IN
RAIN DAY=1.83 IN DT=0.03333 HR

COMPUTED 6-HOUR RAINFALL DISTRIBUTION BASED ON NOAA ATLAS 2 - PEAK AT 1.40 HR.
DT = .033330 HOURS END TIME = 5.999400 HOURS

.0000	.0011	.0022	.0034	.0046	.0058	.0070
.0083	.0095	.0109	.0122	.0136	.0150	.0165
.0180	.0196	.0212	.0229	.0246	.0263	.0282
.0301	.0321	.0341	.0363	.0385	.0408	.0433
.0459	.0486	.0515	.0552	.0591	.0634	.0725
.0928	.1241	.1690	.2304	.3110	.4138	.5418
.6980	.8430	.9036	.9547	1.0002	1.0416	1.0797
1.1150	1.1481	1.1790	1.2082	1.2357	1.2617	1.2863
1.3096	1.3316	1.3526	1.3725	1.3914	1.3958	1.3999
1.4037	1.4074	1.4109	1.4143	1.4175	1.4206	1.4236
1.4265	1.4293	1.4320	1.4346	1.4372	1.4397	1.4421
1.4444	1.4467	1.4490	1.4511	1.4533	1.4554	1.4574
1.4594	1.4614	1.4633	1.4652	1.4670	1.4688	1.4706
1.4724	1.4741	1.4758	1.4775	1.4791	1.4807	1.4823
1.4839	1.4854	1.4869	1.4884	1.4899	1.4914	1.4928
1.4943	1.4957	1.4970	1.4984	1.4998	1.5011	1.5024
1.5037	1.5050	1.5063	1.5075	1.5088	1.5100	1.5113
1.5125	1.5137	1.5148	1.5160	1.5172	1.5183	1.5195
1.5206	1.5217	1.5228	1.5239	1.5250	1.5261	1.5271
1.5282	1.5292	1.5303	1.5313	1.5323	1.5333	1.5344
1.5354	1.5363	1.5373	1.5383	1.5393	1.5402	1.5412
1.5421	1.5431	1.5440	1.5449	1.5458	1.5467	1.5476
1.5485	1.5494	1.5503	1.5512	1.5521	1.5529	1.5538
1.5547	1.5555	1.5564	1.5572	1.5580	1.5589	1.5597
1.5605	1.5613	1.5621	1.5629	1.5637	1.5645	1.5653
1.5661	1.5669	1.5677	1.5685	1.5692	1.5700	

* BASIN A

COMPUTE NM HYD

ID=6 HYD NO=112.0 AREA=0.000701 SQ MI
ER A=0.00 PER B=20.00 PER C=20.00 PER D=60.00
TP=0.1333 HR MASS RAINFALL=-1

7.106420 K = .072649HR TP = .133300HR K/TP RATIO = .545000 SHAPE CONSTANT, N =
UNIT PEAK = 1.6606 CFS UNIT VOLUME = .9922 B = 526.28 P60 = 1.3400
AREA = .000421 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR
RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

3.812062 K = .123717HR TP = .133300HR K/TP RATIO = .928113 SHAPE CONSTANT, N =
UNIT PEAK = .72033 CFS UNIT VOLUME = .9822 B = 342.44 P60 = 1.3400
AREA = .000280 SQ MI IA = .42500 INCHES INF = 1.04000 INCHES PER HOUR
RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

* 100-YEAR, 6-HR STORM (UNDER PROPOSED CONDITIONS) *

START TIME=0.0
RAINFALL TYPE=1 RAIN QUARTER=0.0 IN
RAIN ONE=2.01 IN RAIN SIX=2.35 IN
RAIN DAY=2.75 IN DT=0.03333 HR

COMPUTED 6-HOUR RAINFALL DISTRIBUTION BASED ON NOAA ATLAS 2 - PEAK AT 1.40 HR.
DT = .033330 HOURS END TIME = 5.999400 HOURS
.0000 .0016 .0033 .0049 .0066 .0084 .0102
.0120 .0139 .0158 .0178 .0199 .0219 .0241
.0263 .0286 .0309 .0333 .0358 .0384 .0411
.0439 .0467 .0497 .0529 .0561 .0596 .0631
.0669 .0709 .0751 .0807 .0866 .0930 .1066
.1371 .1840 .2514 .3434 .4644 .6186 .8106
1.0449 1.2624 1.3533 1.4300 1.4982 1.5602 1.6174
1.6704 1.7200 1.7664 1.8102 1.8514 1.8904 1.9273
1.9622 1.9953 2.0268 2.0566 2.0850 2.0915 2.0976
2.1033 2.1088 2.1140 2.1191 2.1239 2.1285 2.1329
2.1373 2.1414 2.1454 2.1494 2.1531 2.1568 2.1604
2.1639 2.1673 2.1706 2.1739 2.1771 2.1802 2.1832
2.1862 2.1891 2.1919 2.1947 2.1975 2.2002 2.2028
2.2054 2.2080 2.2105 2.2130 2.2154 2.2178 2.2202
2.2225 2.2248 2.2270 2.2293 2.2315 2.2336 2.2358
2.2379 2.2399 2.2420 2.2440 2.2460 2.2480 2.2500
2.2519 2.2538 2.2557 2.2576 2.2594 2.2612 2.2631
2.2648 2.2666 2.2684 2.2701 2.2718 2.2735 2.2752
2.2769 2.2785 2.2802 2.2818 2.2834 2.2850 2.2866
2.2881 2.2897 2.2912 2.2928 2.2943 2.2958 2.2973
2.2987 2.3002 2.3017 2.3031 2.3045 2.3060 2.3074
2.3088 2.3102 2.3115 2.3129 2.3143 2.3156 2.3169
2.3183 2.3196 2.3209 2.3222 2.3235 2.3248 2.3261
2.3273 2.3286 2.3298 2.3311 2.3323 2.3335 2.3348
2.3360 2.3372 2.3384 2.3396 2.3408 2.3419 2.3431
2.3443 2.3454 2.3466 2.3477 2.3488 2.3500

* ON-STIE
COMPUTE NM HYD ID=7 HYD NO=102.1 AREA=0.000701 SQ MI
PER A=0.00 PER B=5.00 PER C=0.00 PER D=95.00
TP=0.1333 HR MASS RAINFALL=-1

7.106420 K = .072649HR TP = .133300HR K/TP RATIO = .545000 SHAPE CONSTANT, N =
UNIT PEAK = 2.6292 CFS UNIT VOLUME = .9949 B = 526.28 P60 = 2.0100
AREA = .000666 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR
RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

3.563124 K = .132088HR TP = .133300HR K/TP RATIO = .990905 SHAPE CONSTANT, N =
UNIT PEAK = .85431E-01CFS UNIT VOLUME = .8709 B = 324.91 P60 = 2.0100
AREA = .000035 SQ MI IA = .50000 INCHES INF = 1.25000 INCHES PER HOUR
RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

 * 10-YEAR, 6-HR STORM (UNDER PROPOSED CONDITIONS) *

START TIME=0.0
 RAINFALL TYPE=1 RAIN QUARTER=0.0 IN
 RAIN ONE=1.34 IN RAIN SIX=1.57 IN
 RAIN DAY=1.83 IN DT=0.03333 HR

COMPUTED 6-HOUR RAINFALL DISTRIBUTION BASED ON NOAA ATLAS 2 - PEAK AT 1.40 HR.
 DT = .033330 HOURS END TIME = 5.999400 HOURS

.0000	.0011	.0022	.0034	.0046	.0058	.0070
.0083	.0095	.0109	.0122	.0136	.0150	.0165
.0180	.0196	.0212	.0229	.0246	.0263	.0282
.0301	.0321	.0341	.0363	.0385	.0408	.0433
.0459	.0486	.0515	.0552	.0591	.0634	.0725
.0928	.1241	.1690	.2304	.3110	.4138	.5418
.6980	.8430	.9036	.9547	1.0002	1.0416	1.0797
1.1150	1.1481	1.1790	1.2082	1.2357	1.2617	1.2863
1.3096	1.3316	1.3526	1.3725	1.3914	1.3958	1.3999
1.4037	1.4074	1.4109	1.4143	1.4175	1.4206	1.4236
1.4265	1.4293	1.4320	1.4346	1.4372	1.4397	1.4421
1.4444	1.4467	1.4490	1.4511	1.4533	1.4554	1.4574
1.4594	1.4614	1.4633	1.4652	1.4670	1.4688	1.4706
1.4724	1.4741	1.4758	1.4775	1.4791	1.4807	1.4823
1.4839	1.4854	1.4869	1.4884	1.4899	1.4914	1.4928
1.4943	1.4957	1.4970	1.4984	1.4998	1.5011	1.5024
1.5037	1.5050	1.5063	1.5075	1.5088	1.5100	1.5113
1.5125	1.5137	1.5148	1.5160	1.5172	1.5183	1.5195
1.5206	1.5217	1.5228	1.5239	1.5250	1.5261	1.5271
1.5282	1.5292	1.5303	1.5313	1.5323	1.5333	1.5344
1.5354	1.5363	1.5373	1.5383	1.5393	1.5402	1.5412
1.5421	1.5431	1.5440	1.5449	1.5458	1.5467	1.5476
1.5485	1.5494	1.5503	1.5512	1.5521	1.5529	1.5538
1.5547	1.5555	1.5564	1.5572	1.5580	1.5589	1.5597
1.5605	1.5613	1.5621	1.5629	1.5637	1.5645	1.5653
1.5661	1.5669	1.5677	1.5685	1.5692	1.5700	

* ON-SITE
 COMPUTE NM HYD ID=8 HYD NO=112.1 AREA=0.000701 SQ MI
 PER A=0.00 PER B=5.00 PER C=0.00 PER D=95.00
 TP=0.1333 HR MASS RAINFALL=-1

7.106420 K = .072649HR TP = .133300HR K/TP RATIO = .545000 SHAPE CONSTANT, N =
 UNIT PEAK = 2.6292 CFS UNIT VOLUME = .9949 B = 526.28 P60 = 1.3400
 AREA = .000666 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR
 RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

3.354139 K = .140318HR TP = .133300HR K/TP RATIO = 1.052645 SHAPE CONSTANT, N =
 UNIT PEAK = .81387E-01CFS UNIT VOLUME = .8719 B = 309.53 P60 = 1.3400
 AREA = .000035 SQ MI IA = .50000 INCHES INF = 1.25000 INCHES PER HOUR
 RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

 * PONDING CONDITION *

ROUTE RESERVOIR ID=30 HYD NO=601.1 INFLOW ID=7 CODE=24
 OUTFLOW(CFS) STORAGE(AC-FT) ELEVATION(FT)
 0.000 0.000000 66.48
 1.746 0.003209 66.75
 2.423 0.009436 67.00
 2.750 0.019254 67.15

* * * * *

TIME (HRS)	INFLOW (CFS)	ELEV (FEET)	VOLUME (AC-FT)	OUTFLOW (CFS)
.00	.00	66.48	.000	.00

```

*
ADD HYD                      ID=40  HYD NO=110.10  ID=20  ID=30
**
FINISH

```

NORMAL PROGRAM FINISH END TIME (HR:MIN:SEC) = 11:11:21



City of Albuquerque

Planning Department
Development & Building Services Division

DRAINAGE AND TRANSPORTATION INFORMATION SHEET (REV 6/2018)

Project Title: 2818 4TH STREET, NW, APTS. **Building Permit #:** _____ **Hydrology File #:** H14D111
DRB#: _____ **EPC#:** _____ **Work Order#:** _____
Legal Description: LOTS 1-A, 6,7,8, 9 10, Block 2, CITY REALTY COMPANY'S ADDITION NO. 1 AND LOTS 4,5, 6, BLOCK 9, MANDDLE ADDITION NO. 2.
City Address: 2818 4TH STREET, NW

Applicant: SBS CONSTRUCTION AND ENGINEERING, LLC **Contact:** SHAWN BIAZAR
Address: 10209 SNOWFLAKE CT., NW, ALBUQUERQUE, NM 87114
Phone#: (505) 804-5013 **Fax#:** (505) 897-4996 **E-mail:** AECLLC@AOL.COM

Other Contact: _____ **Contact:** _____
Address: _____
Phone#: _____ **Fax#:** _____ **E-mail:** _____

TYPE OF DEVELOPMENT: _____ PLAT (# of lots) _____ RESIDENCE ☒ DRB SITE ☒ ADMIN SITE

IS THIS A RESUBMITTAL? ☒ Yes _____ No

DEPARTMENT _____ TRANSPORTATION ☒ HYDROLOGY/DRAINAGE

Check all that Apply:

TYPE OF SUBMITTAL:

_____ ENGINEER/ARCHITECT CERTIFICATION
_____ PAD CERTIFICATION
_____ CONCEPTUAL G & D PLAN
☒ GRADING PLAN
☒ DRAINAGE REPORT
_____ DRAINAGE MASTER PLAN
_____ FLOODPLAIN DEVELOPMENT PERMIT APPLIC
_____ ELEVATION CERTIFICATE
_____ CLOMR/LOMR
_____ TRAFFIC CIRCULATION LAYOUT (TCL)
_____ TRAFFIC IMPACT STUDY (TIS)
_____ STREET LIGHT LAYOUT
_____ OTHER (SPECIFY) _____
_____ PRE-DESIGN MEETING?

TYPE OF APPROVAL/ACCEPTANCE SOUGHT:

☒ BUILDING PERMIT APPROVAL
_____ CERTIFICATE OF OCCUPANCY
_____ PRELIMINARY PLAT APPROVAL
_____ SITE PLAN FOR SUB'D APPROVAL
☒ SITE PLAN FOR BLDG. PERMIT APPROVAL
_____ FINAL PLAT APPROVAL
_____ SIA/ RELEASE OF FINANCIAL GUARANTEE
_____ FOUNDATION PERMIT APPROVAL
☒ GRADING PERMIT APPROVAL
☒ SO-19 APPROVAL
_____ PAVING PERMIT APPROVAL
_____ GRADING/ PAD CERTIFICATION
_____ WORK ORDER APPROVAL
_____ CLOMR/LOMR
_____ FLOODPLAIN DEVELOPMENT PERMIT
_____ OTHER (SPECIFY) _____

DATE SUBMITTED: 6-17-2020 **By:** SHAWN BIAZAR

COA STAFF:

ELECTRONIC SUBMITTAL RECEIVED: _____

FEE PAID: _____

SBS CONSTRUCTION AND ENGINEERING, LLC

June 17, 2020

Mrs. Re'ene C. Brissette, PE CFM
Senior Engineer, Planning Dept.
Development Review Services
City of Albuquerque Planning Department
PO Box 1293, 600 Second Street, NW
Albuquerque, NM 87103

RE: 2818 4th Street Apartments Grading Plan, Hydrology File# H14D111

Dear Mrs. Brissette;

Attached please find a copy of the revised grading plan and drainage report for the above referenced site. We have addressed your comments and below are the response to your comments;

- 1) We have included project bench mark, please see note 2 nder General Notes.
- 2) The SO-19 Notes has been added to grading plan.
- 3) The text has been moved to show the outfall of the pipe.
- 4) A cover sheet with engineer's stamp has been added to the drainage calculation.
- 5) The weir calculation with coefficient of 2.70 has been added to the grading plan for the two feet curb opening.
- 6) The owner will prepare Erosion and Sediment Control (ESC) Plan and Owner's certified Notice of Intent (NOI) and submit to the Stormwater Quality Engineer for approval prior any work done on the site.
- 7) We will pay the \$300.00 review fee for the DRB.

If you require additional information regarding this project, please do not hesitate to contact me at (505) 804-5013.

Sincerely,

Shawn Biazar

Shawn Biazar, Managing Member

DRAINAGE MANAGEMENT PLAN

Location

LOTS 1-A, 6, 7, 8, 9, AND 10, BLOCK 2, CITY REALTY COMPANY'S ADDITION NO. 1; TOGETHER WITH LOTS 4, 5, AND 6, BLOCK 9, OF THE MANDELL ADDITION NO. 2. CONTAINING 1.2690 ACRE. See attached portion of Vicinity Map H-14-Z for exact location.

Purpose

The purpose of this drainage report is to present a grading and drainage solution for the proposed commercial buildings. We are requesting site plan for building permit approval as well as building permit approval.

Existing Site/Drainage Conditions

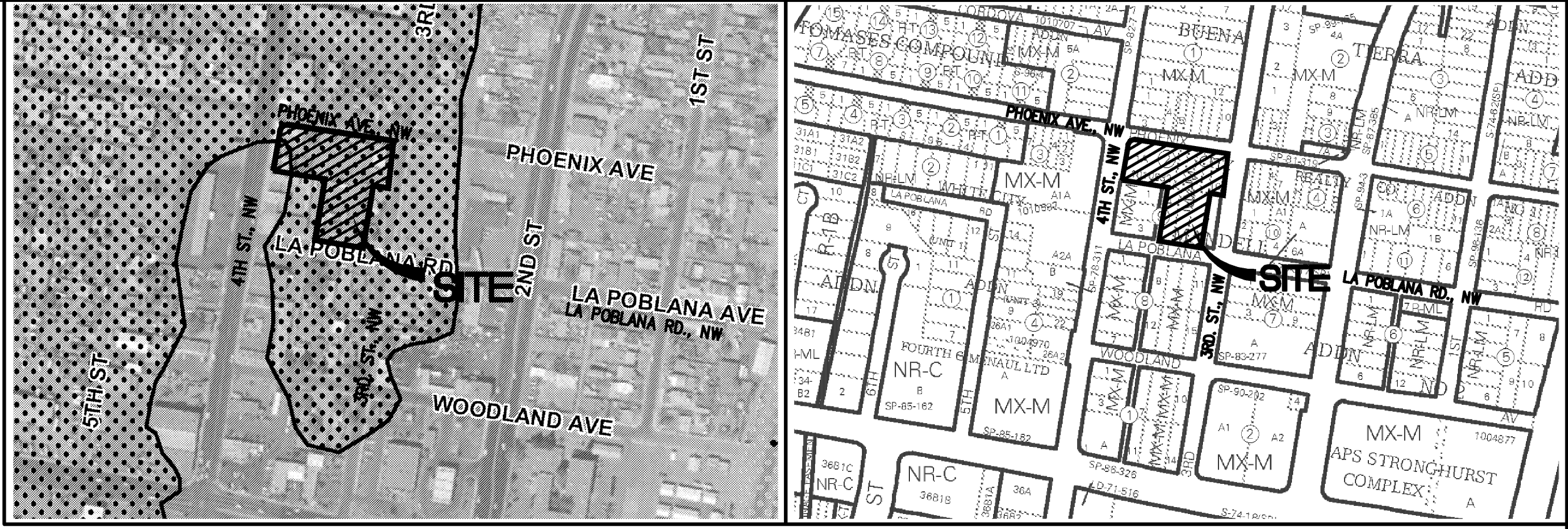
This site contained of existing buildings, concrete pads, asphalt and some gravel. All the existing structures and asphalt and concrete pads have been removed. The site is fairly flat. Most of the site was draining into adjacents streets. No offsite runoff impacts this site.

Proposed Conditions and On-Site Drainage Management Plan

This site is located within zone 2 and in the north valley. We are proposing a building with ground level and underground parking structure. This site will pond most of water and discharge at a control rate. According to grading plan file #G14-D086, the north valley rate of discharge is 2.75 CFS per acre. This site contains 1.2690 acre (55,277.64 sf). Therefore our rate of discharge will be at 3.49 CFS (1.2690 * 2.75). The site was analzred under Basins A and B. The runoff from Basin A is detained in the parking lot and drained at a controlled discharge rate of 0.47 cfs via a 4" pipe to the back of sidewalk culvert to La Poblana Rd. Basin B is discharge at a detained rate of 1.86 cfs and drain out via 2-8" pipe to a sidewalk culvert to 4th St. The total detainee discharge rate is 2.33 cfs which is well below the allowable discharge of 3.49 cfs and less than current discharge under the existing conditions.

NOTICE TO CONTRACTORS

1. AN EXCAVATION/CONSTRUCTION PERMIT WILL BE REQUIRED BEFORE BEGINNING ANY WORK WITHIN BERNALILLO COUNTY RIGHT-OF-WAY.
2. ALL WORK DETAILED ON THESE PLANS TO BE PERFORMED, EXCEPT AS OTHERWISE STATED OR PROVIDED HEREON, SHALL BE CONSTRUCTED IN ACCORDANCE WITH BERNALILLO COUNTY STANDARD SPECIFICATIONS FOR PUBLIC WORKS CONSTRUCTION.
3. TWO WORKING DAYS PRIOR TO ANY EXCAVATION, CONTRACTOR MUST CONTACT LINE LOCATING SERVICE, 260-1990, FOR LOCATION OF EXISTING UTILITIES.
4. PRIOR TO CONSTRUCTION, THE CONTRACTOR SHALL EXCAVATE AND VERIFY THE HORIZONTAL AND VERTICAL LOCATIONS OF ALL CONSTRUCTIONS. SHOULD A CONFLICT EXIST, THE CONTRACTOR SHALL NOTIFY THE ENGINEER SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM AMOUNT OF DELAY.
5. BACKFILL COMPACTION SHALL BE ACCORDING TO TRAFFIC/STREET USE.
6. MAINTENANCE OF THESE FACILITIES SHALL BE THE RESPONSIBILITY OF THE OWNER OF THE PROPERTY SERVED.
7. WORK ON ARTERIAL STREETS SHALL BE PERFORMED ON A 24-HOUR BASIS.



FIRM MAP:

35001C0332G

VICINITY MAP:

H-14-Z

LEGAL DESCRIPTION:

LOTS 1-A, 6, 7, 8, 9, AND 10, BLOCK 2, CITY REALTY COMPANY'S ADDITION NO. 1; TOGETHER WITH

LOTS 4, 5, AND 6, BLOCK 9, OF THE MANDELL ADDITION NO. 2.

CONTAINING: 55,277.54 SF (1.2690 ACRE)

GENERAL NOTES:

- 1: CONTOUR INTERVAL IS HALF (1.00) FOOT.
- 2: ELEVATIONS ARE BASED ON CITY OF ALBUQUERQUE CONTROL STATION A-438, HAVING AN ELEVATION OF 4975.35 FEET ABOVE SEA LEVEL.
- 3: UTILITIES SHOWN HEREON ARE IN THEIR APPROXIMATE LOCATION BASED ONLY ON ABOVE GROUND EVIDENCE FOUND IN THE FIELD AND AS-BUILT INFORMATION PROVIDED BY THE CLIENT. UTILITIES SHOWN HEREON, WHETHER INDICATED AS ABANDONED OR NOT, SHALL BE VERIFIED BY OTHERS FOR EXACT LOCATION AND/ OR DEPTH PRIOR TO EXCAVATION OR DESIGN CONSIDERATIONS.
- 4: THIS IS NOT A BOUNDARY SURVEY, BEARINGS ARE ASSUMED, DISTANCES AND FOUND PROPERTY CORNERS ARE FOR INFORMATIONAL PURPOSES ONLY.
- 5: SLOPES ARE AT 3:1 MAXIMUM.

POND VOLUME REQUIRED FOR FIRST FLUSH

BASIN A

0.34 INCHES x IMPERVIOUS AREA = (0.34/12 x 33,961.48) = 962.24 CF THIS VOLUME IS PART OF THE PONDING PROVIDED.

BASIN B

0.34 INCHES x IMPERVIOUS AREA = (0.34/12 x 18,561.82) = 525.92 CF THIS VOLUME IS PART OF THE PONDING PROVIDED.

FIRST FLUSH PONDING VOLUME PROVIDED

POND A-1: AREA @ TOP = 464.46, BOTTOM =0, DEPTH=1.50'
POND VOLUME = (464.46+0)/2*1.50' = 348.35 CF

POND A-2: AREA @ TOP = 519.00, BOTTOM =0, DEPTH=2.50'
POND VOLUME = (519.00+0)/2*2.50' = 648.75 CF

POND A-1 + POND A-2 = 348.35 + 648.75 = 997.10 CF > 962.24 CF

POND B: AREA @ TOP = 211.52, BOTTOM =211.52, DEPTH=2.75'
POND VOLUME = (211.52+211.52)/2*2.75' = 581.68 CF > 525.92 CF

NOTICE TO CONTRACTOR

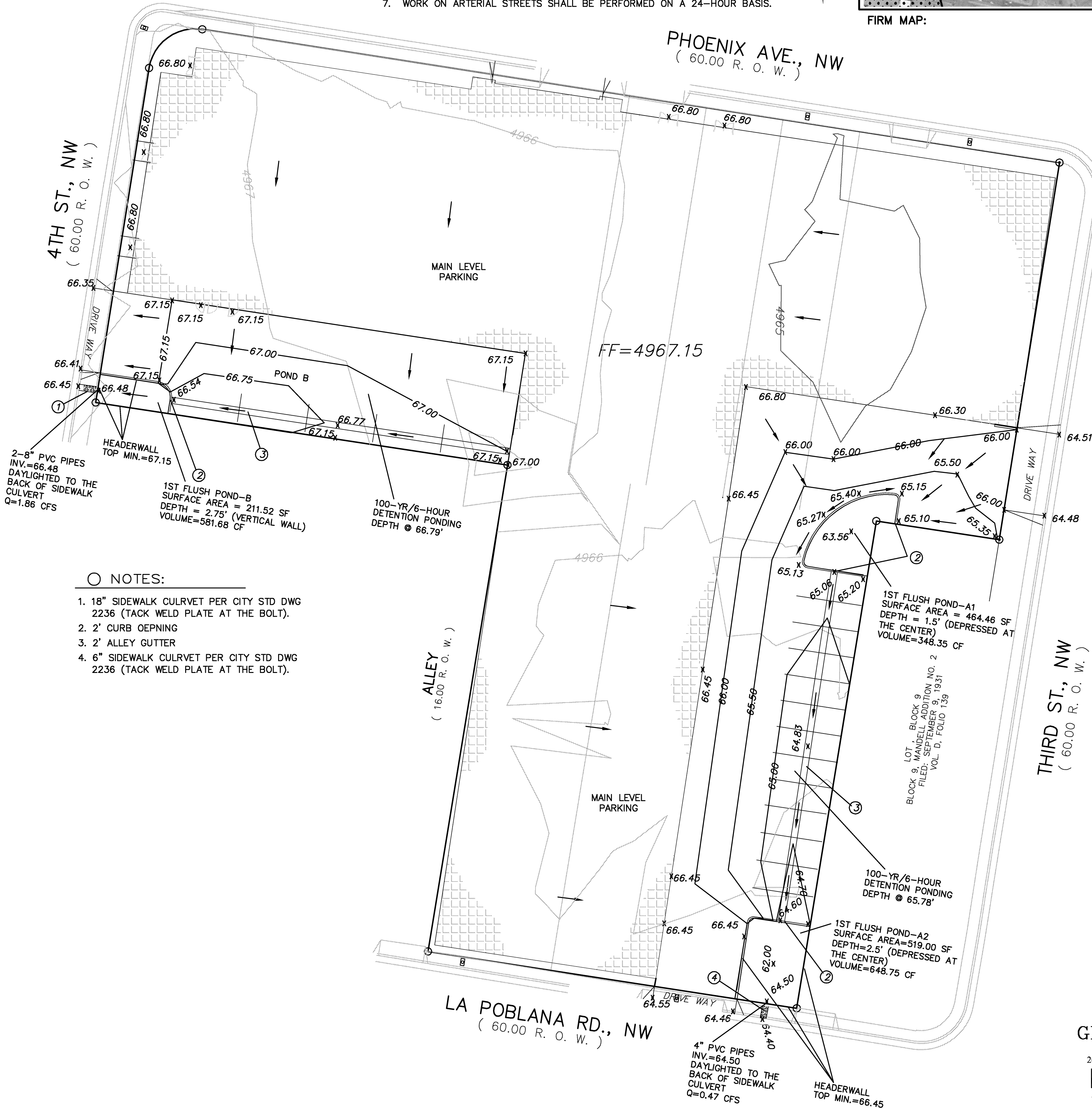
PRIVATE DRAINAGE FACILITIES WITHIN CITY RIGHT-OF-WAY (SO-19')

1. AN EXCAVATION PERMIT WILL BE REQUIRED BEFORE BEGINNING ANY WORK WITHIN CITY RIGHT-OF-WAY.
2. ALL WORK ON THIS PROJECT SHALL BE PERFORMED IN ACCORDANCE WITH APPLICABLE FEDERAL, STATE AND LOCAL LAWS, RULES AND REGULATIONS CONCERNING CONSTRUCTION SAFETY AND HEALTH.
3. TWO WORKING DAYS PRIOR TO ANY EXCAVATION, THE CONTRACTOR MUST CONTACT NEW MEXICO ONE CALL, DIAL "911" [OR (505) 260-1990] FOR THE LOCATION OF EXISTING UTILITIES.
4. PRIOR TO CONSTRUCTION, THE CONTRACTOR SHALL EXCAVATE AND VERIFY THE LOCATIONS OF ALL OBSTRUCTIONS. SHOULD A CONFLICT EXIST, THE CONTRACTOR SHALL NOTIFY THE ENGINEER SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM AMOUNT OF DELAY.
5. BACKFILL COMPACTION SHALL BE ACCORDING TO TRAFFIC/STREET USE.
6. MAINTENANCE OF THE FACILITY SHALL BE THE RESPONSIBILITY OF THE OWNER OF THE PROPERTY BEING SERVED.
7. WORK ON ARTERIAL STREETS MAY BE REQUIRED ON A 24 HOUR BASIS.
8. CONTRACTOR MUST CONTACT AUGIE ARMIJO AT (505) 857-8607 AND CONSTRUCTION COORDINATION AT 924-3416 TO SCHEDULE AN INSPECTION.

APPROVALS	NAME	DATE
INSPECTOR		

NOTES:

1. 18" SIDEWALK CULRVET PER CITY STD DWG 2236 (TACK WELD PLATE AT THE BOLT).
2. 2' CURB OEPNING
3. 2' ALLEY GUTTER
4. 6" SIDEWALK CULRVET PER CITY STD DWG 2236 (TACK WELD PLATE AT THE BOLT).



LEGEND

- 5030 — EXISTING CONTOUR (MAJOR)
- — EXISTING CONTOUR (MINOR)
- BOUNDARY LINE
- 28.50 PROPOSED SPOT ELEVATION
- 5029.16 EXISTING GRADE
- x 5028.65 EXISTING FLOWLINE ELEVATION
- x FL
- PROPOSED RETAINING WALL
- BC=89.08 BOTTOM OF CHANEL
- TC=28.50 TOP OF CURB
- TA=28.00 TOP OF ASPHALT
- HP HIGH POINT
- 86.65 AS-BUILT GRADES
- x 86.65 AS-BUILT SPOT ELEVATIONS

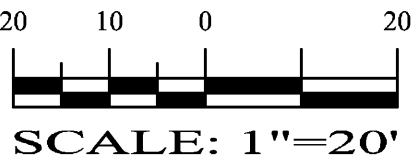


REZA AFAGHPOUR
P.E. #11814

SBS CONSTRUCTION AND ENGINEERING, LLC

10209 SNOWFLAKE CT., NW
ALBUQUERQUE, NEW MEXICO 87114
(505)899-5570

GRAPHIC SCALE



LAST REVISION: 05-18-2020

2818 4TH STREET, NW
GRADING AND DRAINAGE PLAN

DRAWING:	DRAWN BY:	DATE:	SHEET #
201836.DWG	SH-B	02-20-2020	C 101