

City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

27 January 1999

Benny E. McMillan Chavez-Grieves Engineering 5639 Jefferson St. NE Albuquerque, New Mexico 87109

RE: ENGINEER CERTIFICATION FOR CERTIFICATE OF OCCUPANCY, MENAUL HIGH

SCHOOL (H-15/D45) CERTIFICATION DATED 12-15-98

Dear Mr McMillan:

Based on the information provided on your December 15, 1998 submittal, Engineer certification for the above referenced site is acceptable.

If I can be of further assistance, please feel free to contact me at 924-3986.

Sincerely,

Scott Davis

PWD Hydrology Div.

c: Andrew Garcia file



July 31, 1998

James Millington, P.E. Chavez-Grieves 5639 Jefferson Street NE Albuquerque, NM 87109

Attn: Christina Ehrsam

RE: MENAUL HIGH SCHOOL, GYM & DINING HALL ADDITIONS (H15-D45).

DRAINAGE REPORT FOR BUILDING PERMIT APPROVAL. ENGINEER'S STAMP DATED JULY 8, 1998.

Dear Mr. Millington:

Based on the information provided on your July 9, 1998 submittal, the above referenced project is approved for Building Permit.

The current Flood Insurance Rate Map is 35001C0332 D, dated Sept. 20, 1996. A copy is enclosed for your information. Please use this reference in the Report.

Please attach a copy of this approved plan to the construction sets prior to sign-off by Hydrology.

Prior to Certificate of Occupancy approval, an Engineer's Certification per DPM will be required.

If I can be of further assistance, please feel free to contact me at 924-3984.

* Sheats DB2 & DB3

Sincerely,

Yohn P. Murray, P.E.

Hydrology

c: Andrew Garcia

Good for You, Albuquerque!





5639 JEFFERSON STREET NE • ALBUQUERQUE, NEW MEXICO 87109 • PHONE (505) 344-4080 • FAX (505) 343-8759

GRADING AND DRAINAGE PLAN

FOR

MENUAL HIGH SCHOOL

GYM ADDN + DINING ADDN

BYM ADDN + DINING ADDN

Albuquerque, New Mexico

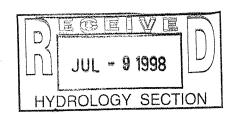
MENUAL HIGH SCHOOL

GYM ADDN + DINING ADDN

Albuquerque, New Mexico



JULY 1998



LOCATION

This site is located on the northeast corner of Menual Boulevard NE and Broadway Boulevard NE on city Zone Atlas page H-15. The site consists of 44.5972 acres.

LEGAL DESCRIPTION

Lands of the Menual School situate within the City of Albuquerque, Bernalillo County, New Mexico.

ZONING AND SURROUNDING DEVELOPMENT

The site is zone SU-1. Much of the surrounding area is developed making this an infill 3500\C0332\D site.

FLOOD HAZARD ZONES

As shown on overall Basin Maps in Appendix B, Panel 350002 0023 of the National Flood Insurance Rate Maps for the City of Albuquerque, dated Oct. 14, 1983, the site is in a floodplain, zone C. Zone C indicates areas of minimal flooding.

RELATED REPORTS

The Menual School Gymnasium grading and drainage plan developed by Jeff Mortensen was approved by City of Albuquerque October 10, 1986. This report refers to a master grading and drainage plan which was unavailable at the City Hydrology section. In the report by Jeff Mortensen a detention pond capable of containing 36,025 cf of storm water runoff was proposed just west of the gymnasium. The detention pond has a controlled discharge to a sidewalk culvert along Broadway Boulevard via three 6" dia. PVC pipes. The discharge at Broadway Boulevard from this detention pond is 6.0

EXISTING SITE CONDITIONS AND DRAINAGE PATTERN

The 44.5972 acre site is the home of Menual High School. The school is developed with various buildings, athletic areas, sidewalks, parking and open space. The site is divided into four basin areas. See Appendix B. Basin A contains 4.1065 acres and is undeveloped with exception of a single lane paved service road. The basin slopes south to north at 2 percent. This basin historically discharges into Claremont Avenue via sheet flow. Basin B contains 13.8473 acres and is a undeveloped vacant lot that has been compacted by human activity. The slope on this basin varies from 1 percent near the faculty housing to 13 to 20 percent at the northern basin edge. This basin historically flows to the M.R.G.C.D. lateral which drains into the City of Albuquerque Detention pond formerly known as Menual School Detention pond. Basin C contains 15.3726 acres and is developed. This basin slopes southeast to southwest at a 1 to 2 percent. The athletic facilities and a portion of the school building and faculty housing are found within this basin. Land treatments vary between lawns to paved parking. The western portion of this area historically flows to the existing detention pond which discharges through three-6" PVC pipes into Broadway Boulevard. The northern portion drains into a small retention pond north of the existing football field. Basin D contains 12.05563 acres and is developed. The development contains classroom buildings, administrative buildings and housing. The land treatments also vary between compacted soil, lawns and paved parking. Basin D slopes from east to west at 1 to 2 percent. This basin historically flows to the southwest corner of the property and discharges over the curb into Broadway Boulevard.

PROPOSED SITE CONDITIONS AND DRAINAGE PATTERN

An 807 square foot addition to the existing Davidson Hall, a 4264 square foot addition to the existing gymnasium, tennis courts, and baseball fields are proposed for this site. This drainage report will address the addition of Davidson Hall, the gymnasium and the tennis courts. All of these additions are located in Basin C. The baseball fields, located in Basin B will be addressed in a separate report. The northern portion of Davidson Hall historically drains to the existing detention pond just west of the existing gymnasium. The new addition will be attached to the north face of the existing building. The roof of this addition slopes to the north to two roof drains. The roof drains then continue under the proposed sidewalk to daylight. The storm runoff will then be channeled through sheet flow to the existing detention pond located west of the existing gymnasium. Also proposed is a 4264 square foot addition to the existing gymnasium and within the next 10 years the school proposes to add another 4000 square feet to the gymnasium. The proposed roof slopes to two roof drains; one on the east side and one on the west side of the proposed gymnasium. The runoff captured in the roof drains then discharges outside of the proposed building and drains to daylight. The proposed additions to Menual School including the future addition to the gymnasium create an increase of 1.2 cfs in storm water flow rate and an increase in storm water volume of 2789 cf. AHYMO was used to calculate the size of the detention pond with the controlled discharge of 6 cfs into Broadway Boulevard. A detention pond of 1.1413 acre-ft (49,715 cf) will be required and we have provided a detention of 49750 cf. Analysis was performed to calculate the increase in storm water runoff contributed to the retention pond due to this addition. See Appendix A.

HYDROLOGY/HYDRAULICS

The runoff calculations and design have been done in accordance with Section 22.2 of the Development Process Manual of the City of Albuquerque, January 1993.

CITY OF ALBUQUERQUE PUBLIC WORKS DEPARTMENT DEVELOPMENT SERVICE / HYDROLOGY SECTION

CONFERENCE RECAP

DRAINAGE FILE/ZONE ATLAS PAGE NO. H15-D45

DATE: 4-24-98

PLANNING DIVISION NO'S:

EPC:

DRB:

SUBJECT: Menaul High School -(Gym & Cafeteria)

STREET ADDRESS (IF KNOWN): 301 Menaul NE

SUBDIVISION NAME:

APPROVAL REQUESTED: Site Plan/Building Permit (approximately 3000 sq-ft)

ATTENDANCE:

Fred J. Aguirre-City Hydrologist

James Alarid-Chavez Grieves

FINDINGS:

An approved drainage report is required for building permit approval. If a site plan is required, an approved conceptual grading and drainage plan is required for site plan sign-off by Hydrology.

The grading plan must show the proposed and existing grades for the entire site.

The proposed discharge from the site must be justified. An infill approach may be appropriate if the majority of the basin is developed and this is the only remaining piece.

Vacation of the retained easements under VO 134-1967 may be required. Recommend a sketch site plan review by DRB to surface site plan and vacation requirements.

THE UNDERSIGNED AGREES THAT THE ABOVE FINDINGS ARE SUMMARIZED ACCURATELY AND ARE SUBJECT TO CHANGE IF FURTHER INVESTIGATION REVEALS THAT THEY ARE NOT REASONABLE OR THAT THEY ARE BASED ON INACCURATE INFORMATION.

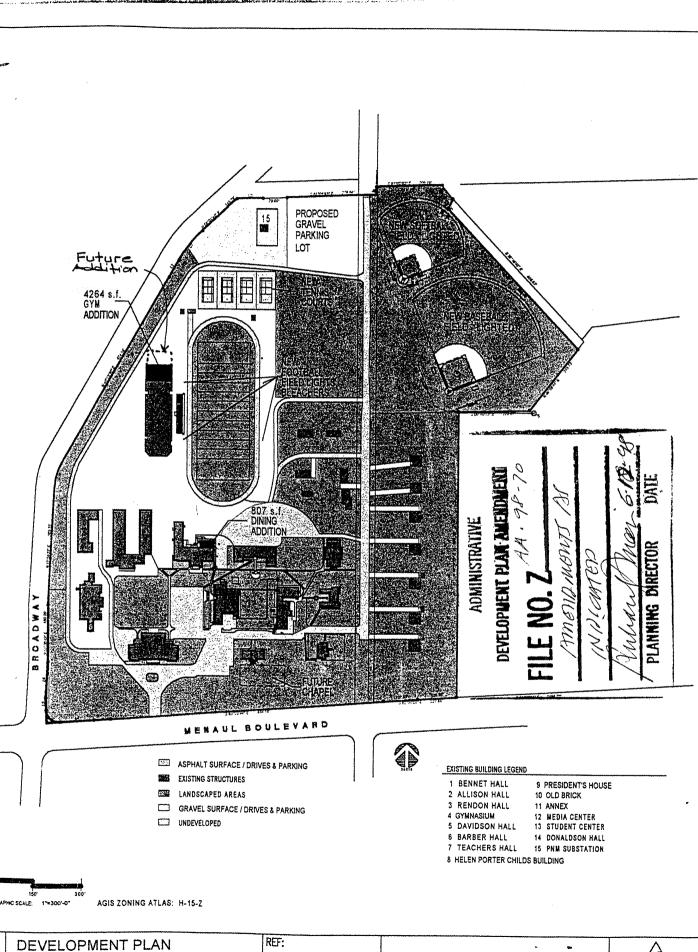
SIGNED:Fred J. Aguirre

TITLE :City Hydrologist

SIGNED:

TITLE:

NOTE PLEASE PROVIDE A COPY OF THIS RECAP WITH YOUR DRAINAGE SUBMITTAL.



MENAUL SCHOOL

1

JOB #

97052

ARCHITECTS • PLANNERS • INTERIOR DESIGNERS
115 AMHERST DRIVE SE.

Author: Chris Ehrsam at Chavez-Grieves

Date: 6/29/98 1:25 PM

Priority: Normal

TO: faguirre@cabq.gov at INTERNET

Subject: Hydrology Recap

----- Message Contents

Hi Fred,

I just want to recap what we talked about on Thursday June 25, 1998.

Menual High School:

- 1. A master drainage plan is not need a
- 2. Show a basin map of the entire area.
- 3. In the drainage report refer to the basin map and discuss existing and proposed runoff conditions.
- 4. Discuss any historical flows such as the Southwest corner historically flows over the curb into the City R.O.W. and that any improvements in this area will need to be review by the city.
- 5. Clarify that any additional site development will have to addressed by the city.
- 6. The existing pond will be upgraded to its original capacity and increased if necessary, as long as the discharge to Broadway is the same amount as historically. 6 cfs.
- 7. Because we are improving the conditions at the baseball fields and the 100 year storm volume is less that existing it will not be necessary to create a pond.

Old Town

- 1. The city will let the owner tie into the city storm drain inlet on San Felipe.
- 2. The City will require a SO19 and a plan showing improvements.
- 3. An easement between property owners will be necessary for the drainage improvements along the west property line.

BASIN	EXISTING Q (CFS)	DEVELOPED Q (CFS)	DISCHARGE POINT
С	47.800	49.000	DETENTION POND

BASIN	EXISTING VOLUME (CF)	DEVELOPED VOLUME (CF)
С	66,509	69,298

APPENDIX A HYDRAULIC COMPUTATIONS

CHAVEZ - GRIEVES / CONSULTING ENGINEERS, Inc.

5639 Jefferson Street NE, Albuquerque, New Mexico 87109

Phone (505) 344-4080 - Fax (505) 343-8759

RUNOFF CALCULATIONS - SIMPLIFIED PROCEDURE

By: Christina A. Ehrsam	Date: 6-12-98
Project: Menual High School-Davidson, Gymnasium and	Tennis Courts
Zone Atlas: <u>H-15/D45</u>	
This procedure is in accordance with the <u>City of Albuquerq</u> 22.2, "Hydrology", peak discharge rate for small watersh	•
Precipitation Zone from Figure A-1: 2	

1. RUNOFF RATE COMPUTATION

Land treatment descriptions are in Table A-4.

Use Equation a-10: $Q_P = Q_{PA} A_A + Q_{PB} A_B + Q_{PC} A_C + Q_{PD} A_D$

Values of Q_{pi} are from Table A-9, and are in CFS/acre. Area values are in acres.

BASIN	Q_{PA}	$\mathbf{A}_{\mathbf{A}}$	Q_{PB}	A_{B}	Q_{PC}	$\mathbf{A}_{\mathbf{C}}$	Q_{PD}	$\mathbf{A}_{\mathbf{D}}$	$\mathbf{Q}_{\mathtt{P}}$
EXISTIN	EXISTING RATE OF RUNOFF (CFS)								
Basin C	1.560	0.000	2.280	6.406	3.140	5.742	4.700	3.225	47.793
Total									47.800
DEVELO	PED RATE	OF RUN	OFF (CFS	5)					
Basin C	1.560	0.000	2.280	6.406	3.140	4.966	4.700	4.001	49.004
Total									49.000

2. RUNOFF VOLUME COMPUTATION

Use Equation a-5 to compute weighted excess precipitation:

Weighted E = "E" =
$$(E_A A_A + E_B A_B + E_C A_C + E_D A_D)/(A_A + A_B + A_C + A_D)$$

 $(A_A + A_B + A_C + A_D) = \sum A_i$

Use Equation a-6 to compute the volume:

$$V_{360} = \text{"E"} \times (A_A + A_B + A_C + A_D) \times 3630 \text{ feet}^3/\text{acre-inch}$$

Values of E_i are from Table A-8, and are in inches. Area values are in acres.

BASIN	$\mathbf{E}_{\mathbf{A}}$	$\mathbf{A}_{\mathbf{A}}$	$\mathbf{E}_{\mathbf{B}}$	$\mathbf{A_B}$	$\mathbf{E}_{\mathbf{C}}$	$\mathbf{A}_{\mathbf{C}}$	$\mathbf{E}_{\mathbf{D}}$	$\mathbf{A}_{\mathtt{D}}$	$\sum \mathbf{A_i}$	"E"	V ₃₆₀
EXISTIN	EXISTING VOLUME OF RUNOFF (CUBIC FEET)										
Basin C	0.530	0.000	0.780	6.406	1.130	5.742	2.120	3.225	15.373	1.192	66509.368
Total											66509
DEVELOPED VOLUME OF RUNOFF (CUBIC FEET)											
Basin C	0.530	0.000	0.780	6.406	1.130	4.966	2.120	4.001	15.373	1.242	69298.079
Total											69298

Ahymo.in

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*SS****** CHAVEZ-GRIEVES CONSULTING ENGINEERS, INC.
*SS* HYDROLOGIC CALCULATIONS USING THE COMPUTERIZED HYDROLOGIC
*SS* MODEL AHYMO, IN ACCORDANCE WITH SECTION 22.2, HYDROLOGY OF **
     THE CITY OF ALBUQUERQUE'S DEVELOPMENT PROCESS MANUAL, JAN., **
*SS*
*SS*
    1993.
*SS********************
*SS*
     AHYMO RUN FOR MENUAL HIGH SCHOOL
*SS*
     ALBUQUERQUE, NEW MEXICO
*SS*
     FILENAME: G:\S05\206\DOCUMENT\AHYMO.IN
     100-YEAR, 24-HOUR STORM
*SS*
*SS* DATE: AUGUST 15, 1997
*SS*
START
                  0.00
RAINFALL
                  TYPE=1 RAIN QUARTER=0.0 RAIN ONE=2.01
                  RAIN SIX=2.35 RAIN DAY=2.75 DT=0.03333
*SS COMPUTE BASIN C RUNOFF
                  ID=1 HYD=BASIN C DA=.02402 SQ MI
COMPUTE NM HYD
                  %A=0.0 %B=42.0 %C=37.0 %D=21.0
                  TP=0.1333 RAINFALL=-1
                  ID=1 CODE=1
PRINT HYD
*SS ROUTE BASIN C
                  ID=2 HYD=POND INFLOW ID=1 CODE=5
ROUTE RESERVOIR
                  OUTFLOW(CFS) STORAGE(AC-FT)
                                               ELEV(FT)
                                                87.0
                   0
                                   0
                                   0.750
                                                87.5
                   2.0
                                   0.950
                   4.0
                                                88.0
                                   1.150
                                                88.5
                   6.0
                  ID=2 CODE=1
PRINT HYD
FINISH
```

AHYMO PROGRAM (AHYMO 97) -

- Version:

1997.02c

RUN DATE (MON/DAY/YR) = 07/07/1998

START TIME (HR:MIN:SEC) = 14:49:21 USER NO.= AHYMO-I3

Chavez-Grieves-C

INPUT FILE = G:\S05\206\DOCUMENTS\AHYMO.IN

*SS****** CHAVEZ-GRIEVES CONSULTING ENGINEERS, INC. ******

HYDROLOGIC CALCULATIONS USING THE COMPUTERIZED HYDROLOGIC

SS MODEL AHYMO, IN ACCORDANCE WITH SECTION 22.2, HYDROLOGY OF

THE CITY OF ALBUQUERQUE'S DEVELOPMENT PROCESS MANUAL, JAN., *SS*

SS 1993.

*SS********************

* *

SS AHYMO RUN FOR MENUAL HIGH SCHOOL

ALBUQUERQUE, NEW MEXICO *SS*

SS FILENAME: G:\S05\206\DOCUMENT\AHYMO.IN

100-YEAR, 24-HOUR STORM *SS*

DATE: AUGUST 15, 1997 *SS*

SS

.0631

START

0.00

RAINFALL

TYPE=1 RAIN QUARTER=0.0 RAIN ONE=2.01

RAIN SIX=2.35 RAIN DAY=2.75 DT=0.03333

COMPUTED 6-HOUR RAINFALL DISTRIBUTION BASED ON NOAA ATLAS 2 - PEAK AT 1.40 HR. .033330 HOURS END TIME = 5.999400 DT =HOURS .0000 .0016 .0033 .0049 .0066 .0084 .0102 .0158 .0120 .0139 .0178 .0199 .0219 .0241 .0309 .0333 .0358 .0384 .0263 .0286 .0411 .0497 .0529 .0561 .0596 .0439 .0467

Ahymo.out

	.0669	.0709	.0751	.0807	.0866	.0930	
.1066	.1371	.1840	.2514	.3434	.4644	.6186	
.8106	1.0449	1.2624	1.3533	1.4300	1.4982	1.5602	1
.6174	1.6704	1.7200	1.7664	1.8102	1.8514	1.8904	1
.9273	1.9622	1.9953	2.0268	2.0566	2.0850	2.0915	2
.0976	2.1033	2.1088	2.1140	2.1191	2.1239	2.1285	2
.1329	2.1373	2.1414	2.1454	2.1494	2.1531	2.1568	2
.1604	2.1639	2.1673	2.1706	2.1739	2.1771	2.1802	2
.1832	2.1862	2.1891	2.1919	2.1947	2.1975	2.2002	2
.2028	2.2054	2.2080	2.2105	2.2130	2.2154	2.2178	2
.2202	2.2225	2.2248	2.2270	2.2293	2.2315	2.2336	2
.2358	2.2379	2.2399	2.2420	2.2440	2.2460	2.2480	2
.2500	2.2519	2.2538	2.2557	2.2576	2.2594	2.2612	2
.2631	2.2648	2.2666	2.2684	2.2701	2.2718	2.2735	2
.2752	2.2769	2.2785	2.2802	2.2818	2.2834	2.2850	2
.2866	2.2881	2.2897	2.2912	2.2928	2.2943	2.2958	2
.2973	2.2987	2.3002	2.3017	2.3031	2.3045	2.3060	2
.3074	2.3088	2.3102	2.3115	2.3129	2.3143	2.3156	2
.3169	2.3183	2.3196	2.3209	2.3222	2.3235	2.3248	2
.3261	2.3273	2.3286	2.3298	2.3311	2.3323	2.3335	2
.3348	2.3360	2.3372	2.3384	2.3396	2.3408	2.3419	2
.3431	2.3443	2.3454	2.3466	2.3477	2.3488	2.3500	

*SS COMPUTE BASIN C RUNOFF

COMPUTE NM HYD ID=1 HYD=BASIN_C DA=.02402 SQ MI %A=0.0 %B=42.0 %C=37.0 %D=21.0 TP=0.1333 RAINFALL=-1

K = .072649HR TP = .133300HR K/TP RATIO = .545000

SHAPE CONSTANT, N = 7.106420

UNIT PEAK = 19.915 CFS UNIT VOLUME = .9988 B

= 526.28 P60 = 2.0100

AREA = .005044 SQ MI IA = .10000 INCHES

.04000 INCHES PER HOUR

RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER ME THOD - DT = .033330

K = .120547HR TP = .133300HR K/TP RATIO = .904326

SHAPE CONSTANT, N = 3.917844

UNIT PEAK = 49.775 CFS UNIT VOLUME = 1.000

349.66 P60 = 2.0100

AREA = .018976 SQ MI IA = .42975 INCHES INF =

1.05329 INCHES PER HOUR

RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER ME

THOD - DT = .033330

PRINT HYD ID=1 CODE=1

HYDROGRAPH FROM AREA BASI

 N_C

RUNOFF VOLUME = 1.17561 INCHES = 1.5060 ACRE-FEE

PEAK DISCHARGE RATE = 47.56 CFS AT 1.500 HOURS BASIN

AREA = .0240 SQ. MI.

*SS ROUTE BASIN C

ROUTE RESERVOIR ID=2 HYD=POND INFLOW ID=1 CODE=5

OUTFLOW(CFS) STORAGE(AC-FT) ELEV(FT)

0 0 87.

0 2.0 0.750 87.5

> 4.0 0.950 88.0

> 1.150 88.5 6.0

INFLOW ELEV VOLUME OUTFLOW TIME

TIME (HRS) (CFS) (FEET) (AC-FT) (CFS) 9.33 .00 87.15 .225 .60 9.50 .00 87.14 .217 .58 9.67 .00 87.14 .209 .56 9.83 .00 87.13 .202 .54 10.00 .00 87.13 .195 .52 10.17 .00 87.13 .188 .50 10.33 .00 87.12 .181 .48 10.50 .00 87.11 .168 .45 10.83 .00 87.11 .162 .43 11.00 .00 87.11 .162 .43 11.00 .00 87.10 .156 .42 11.17 .00 87.10 .156 .42 11.17 .00 87.10 .156 .42 11.17 .00 87.10 .156 .42 11.17 .00 87.09 .140 .37 11.67 .00 87.09 .140 .37 11.67 .00 87.09 .130 .35 12.00 .00 87.09 .130 .35 12.83 .00 87.09 .125 .33 12.17 .00 87.08 .121 .32 12.33 .00 87.09 .130 .35 12.83 .00 87.07 .108 .29 12.83 .00 87.07 .108 .29 12.83 .00 87.07 .108 .29 12.83 .00 87.07 .108 .29 12.83 .00 87.07 .108 .29 12.83 .00 87.07 .108 .29 12.83 .00 87.07 .104 .28 13.00 .00 87.06 .093 .25 13.50 .00 87.06 .093 .25 13.50 .00 87.06 .093 .25 13.50 .00 87.06 .093 .25 13.50 .00 87.06 .097 .26 13.33 .00 87.06 .093 .25 13.50 .00 87.06 .093 .25 13.50 .00 87.06 .093 .25 13.50 .00 87.06 .097 .26 13.33 .00 87.06 .097 .26 13.33 .00 87.06 .097 .26 13.33 .00 87.06 .097 .26 13.43 .00 87.06 .097 .26 13.53 .00 87.06 .097 .26 13.83 .00 87.06 .097 .26 13.83 .00 87.06 .097 .26 13.83 .00 87.06 .097 .29 14.50 .00 87.05 .075 .20 14.50 .00 87.05 .075 .20 14.50 .00 87.05 .075 .20 14.50 .00 87.04 .065 .17 15.33 .00 87.04 .065 .17 15.33 .00 87.04 .065 .15 15.67 .00 87.04 .056 .15 15.67 .00 87.04 .056 .15 15.67 .00 87.03 .055 .13	8.33 8.50 8.67 8.83 9.00 9.17	.00	87.19 87.18 87.17 87.17 87.16 87.16	.281 .271 .261 .252 .243 .234	.75 .72 .70 .67 .65
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	9.50 9.67 9.83 10.00 10.17 10.33 10.50 10.67 10.83 11.00 11.17 11.33 11.50 11.67 11.83 12.50 12.67 12.83 13.00 13.17 13.33 13.50 13.67 13.83 14.00 14.17 14.33 14.67 14.83 15.67 14.83 15.67 15.67 15.83 15.67 15.83 16.00	.00 .00 .00 .00 .00 .00 .00 .00 .00 .00	87.14 87.14 87.13 87.13 87.12 87.12 87.11 87.10 87.10 87.10 87.09 87.09 87.09 87.07 87.07 87.07 87.07 87.06 87.07 87.06 87.05 87.05 87.05 87.04 87.04 87.04 87.04 87.04 87.04 87.04 87.04 87.04 87.03	.217 .209 .202 .195 .188 .181 .174 .168 .162 .156 .150 .145 .140 .135 .121 .116 .112 .108 .104 .100 .097 .093 .090 .087 .075 .075 .075 .075 .075 .075 .075 .07	.58 .520 .546 .453 .409 .335 .331 .329 .221 .221 .219 .119 .117 .116 .114 .14

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                                                                .046
             16.67
                                                                 .045
                                                                                  .12
             16.83
                                                                .043
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             17.00
             17.16
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             17.33
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             17.50
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             17.66
                                                                .036
                                                                 .035
             17.83
                                                                                   .09
             18.00
                                                                 .033
                                                                                   .09
             18.16
                                                                .032
                                                                                   .09
                                                                .031
             18.33
                                                                                   .08
             18.50
                                                                 .030
                                                                                   .08
                         INFLOW ELEV VOLUME OUTFLOW (CFS) (FEET) (AC-FT) (CFS)
             TIME
             (HRS)
                               .00 87.02
.00 87.02
.00 87.02
.00 87.02
                                                             .029
.028
             18.66
                                                                                   .08
             18.83
                                                                                   .07
                                                                                   .07
             19.00
                                                               .027
             19.16
                                                                .026
        19.16 .00 87.02 .026 .07
19.33 .00 87.02 .025 .07
19.50 .00 87.02 .024 .06
19.66 .00 87.02 .023 .06
19.83 .00 87.01 .022 .06
PEAK DISCHARGE = 5.913 CFS - PEAK OCCURS AT HOUR 2.07
MAXIMUM WATER SURFACE ELEVATION = 88.478
MAXIMUM STORAGE = 1.1413 AC-FT INCREMENTAL TIME=
.033330HRS
       PRINT HYD
                                    ID=2 CODE=1
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HYDROGRAPH FROM AREA POND

RUNOFF VOLUME = 1.15877 INCHES = 1.4844 ACRE-FEE PEAK DISCHARGE RATE = 5.91 CFS AT 2.066 HOURS BASIN AREA = .0240 SQ. MI.

FINISH

NORMAL PROGRAM FINISH END TIME (HR:MIN:SEC) = 14:49:2



5639 JEFFERSON STREET NE • ALBUQUERQUE, NEW MEXICO 87109 PHONE (505) 344-4080 • FAX (505) 343-8759

SHEET NO.				
JOB MEN	JAC H	ian_	School	
SUBJECT DE	TENTION	<u> </u>	and	
CLIENT SMP	JOB	NO		
BY Chris	E	DATE .	6.23	28
CHECKED BY		DATE		Ţ

ORIFICE EQUATION.

OUTLET EL. 1NV. EL. 85.00

80.35 3.0. POND 68.00 T.O. POND

3~ 6" & PIC PIPES

Q = 0.6 (A) 1/2gh

h = .3.0 ft. $h = 0.1964 \text{ in}^2$ $q = 32.2 \text{ ft/sec}^2$

Q = 1,64 3 pipes = 4,92 cfs. 5639 JEFFERSON STREET NE • ALBUQUERQUE, NEW MEXICO 87109 PHONE (505) 344-4080 • FAX (505) 343-8759

SHEET NO		_ OF	/
JOB Menc	IAL HI	ah Sc	hool
SUBJECT De	ten tini	Pon	
CLIENT SMA	JOB	NO	07 m
BY Chris		_ DATE	· 23.5
CHECKED BY		_ DATE	

ORIFICE EQUATION: Q = 0.6 (A) TZgh

T.O. POND = 88.50

B.O. POND = 87.0

3~ 6" PVC Pipes INV. EL. 85.65

A = 0.1964 in2

g = 32.2 FT/52

C = 160

h = 2.55

Q = 1.6 x 3 pipes = 4.9 cfs

APPENDIX B DRAWINGS

