# DRAINAGE

#### EXISTING CONDITIONS:

The site is presently totally paved with asphalt pavement and slopes generally from southeast to northwest. The adjoining lot to the east is also paved with asphalt and has some buildings and drains into the site across the east property line. Property to the south also partially drains into the site across the south property line. There is an existing drive pad which will continue to be used. Wellesley Street is paved with curb and gutter and slopes to the north.

#### PROPOSED CONDITIONS:

It is proposed, initially, to construct one 60' X 40' maintenance building on the northwest corner of the site. Some removal and replacement of asphalt pavement will be required to achieve accepable drainage. Landscaping will be added along all sides of the site. Along the east and south sides of the site, where off-site flow must be accepted, landscaping will be provided in square planters with asphalt pavement between them so that the flow will pass through. Future buildings are planned along the north and south sides of the site. Grading and finish floor elevations are shown for future reference but for the first phase only the landscaping and the one building will

#### SOIL INFORMATION:

(Refer to "Soil Survey of Bernalillo County", June, 1977). Soil is WeB, Wink-Embudo complex, hydrologic soil group "B".

#### TIME OF CONCENTRATION:

(Use ten (10) minutes, minimum time of concentration).

#### RAINFALL, 100-YEAR, 6-HOUR:

(Refer to D.P.M., Plate 22.2 D-1).  $R_6 = 2.25$  inches.

### RAINFALL INTENSITY:

 $I = R_6 \times 6.84 \times Tc^{-0.51} = 2.25 \times 6.84 \times 10^{-0.51} = 4.76$  inches per hour.

# SITE IMPERVIOUSNESS:

"C"	"CN"	RUNOFF	AREA (SQ.FT.)	AREA (SQ.FT.)
0.90	98	2.30		2400
0.95	98	2.30	33650	-27473
0.25	61	0.25		3777
			33650	33650
	0.90	0.90 98 0.95 98	"C"         "CN"         RUNOFF           0.90         98         2.30           0.95         98         2.30	"C" "CN" RUNOFF AREA (SQ.FT.)  0.90 98 2.30  0.95 98 2.30 33650  0.25 61 0.25

Site Area in Acres = 0.7725 ac. WEIGHTED "C" VALUES:

Existing Conditions: C = 0.95

Developed Conditions:  $C_{1} = \frac{(0.90 \text{ X } 2400 + 0.95 \text{ X } 27473 + 0.25 \text{ X } 3777)}{23650} = 0.87$ 

# PEAK DISCHARGE:

Existing Conditions:  $Q_{100} = 0.95 \text{ X } 4.76 \text{ X } 0.7725 = 3.50 \text{ cfs}$ 

 $Q_{10} = 0.657 \text{ X } 3.50 = 2.30 \text{ cfs}$ 

# Developed Conditions:

Q100 = CIA = 0.87 X 4.76 X 0.7725 = 3.20 cfs; Q10 = 0.657 X 3.20 = 2.10 cfs

# CALCULATIONS

VOLUME, 100-YEAR AND 10-YEAR, 6-HOUR:

Existing Conditions:  $V_{100} = 33650(2.3 / 12) = 6450 \text{ cf}$ 

 $V_{10} = 0.657 \text{ X } 6450 = 4238 \text{ cf}$ 

 $V_{100} = (29873 \text{ X } 2.3 + 3777 \text{ X } 0.25)/12 = 5804 \text{ cf}$ 

 $V_{10} = 0.657 \text{ X } 5804 = 3813 \text{ cf}$ 

#### OFF-SITE FLOW:

Off-site flow is generated by the parcel south of the site (Off-site Area 1), and the parcel directly east of the site (Off-site Area 2).

Off-site Area 1 290 X 200 -  $(40 \times 210 + 40 \times 40) = 48000 \text{ sf}$ 

 $Q_{100} = 0.95 \text{ X} 4.76 \text{ X} (48000 / 43560) = 4.98 \text{ cfs}$ . This flow quantity enters across the south boundary of the site.

Off-site Area 2  $540 \times 450 = 243000 \text{ sf} = 5.58 \text{ ac.}$ 

 $Q_{100} = 0.95 \text{ X} + 4.76 \text{ X} + 5.58 = 25.23 \text{ cfs}$ . This flow quantity enter across the east boundary of the site.

## SIDEWALK CULVERT FOR OFF-SITE AREA 1:

Design flow includes on-site and off-site flow. On-site flow; Building, 3400 sf; Landscaping, 157.5 sf; Pavement, 1842.5 sf  $C = (3400 \times 0.90 + 1842.5 \times 0.95 + 157.5 \times 0.25) / 5400 = 0.90$ 

 $Q_{100} = 0.90 \text{ X } 4.76 \text{ X } (5400 / 43560) = 0.53 \text{ cfs}; \text{ Off-site Area } 1 = 4.98 \text{ cfs}$ Total design flow = 0.53 + 4.98 = 5.51 cfs. Use Weir Equation, Q = CLH<sup>3/2</sup>  $C = 2.65 \text{ H} = 0.65 \text{ L} = Q / (C X H^{3/2}) = 5.5 / (2.65 X 0.65^{3/2}) = 3.97'$ 

# Use 2 each 2'-0" sidewalk culverts?

SIDEWALK CULVERT AT N.W. CORNER OF SITE: Landscaping 10 X 200 + 15 X 60 = 2900 sf; Roof 3400 sf.

 $C = (3400 \times 0.90 + 2900 \times 0.25) \neq 6300 = 0.60$ 

 $Q_{100} = CIA = 0.60 \text{ X } 4.76 \text{ X } (6300 / 43560) = 0.41 \text{ cfs};$ 

Use Wier Equation  $Q = CLH^{3/2}$  C = 2.65 H = 0.65

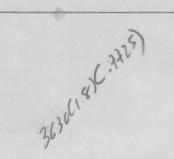
 $L = Q / (CH^{3/2}) = 0.41 / (2.65 \times 0.65^{3/2}) = 0.30'$  Use a 1'0" Sidewalk Culvert

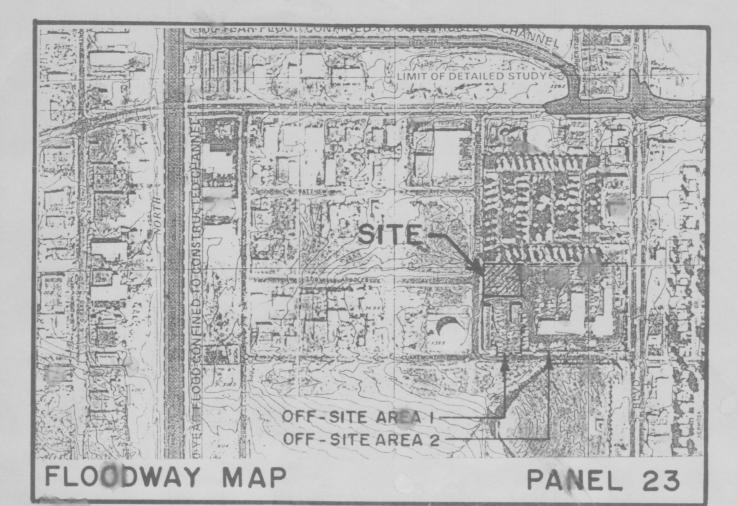
### DISCHARGE THROUGH DRIVEWAY:

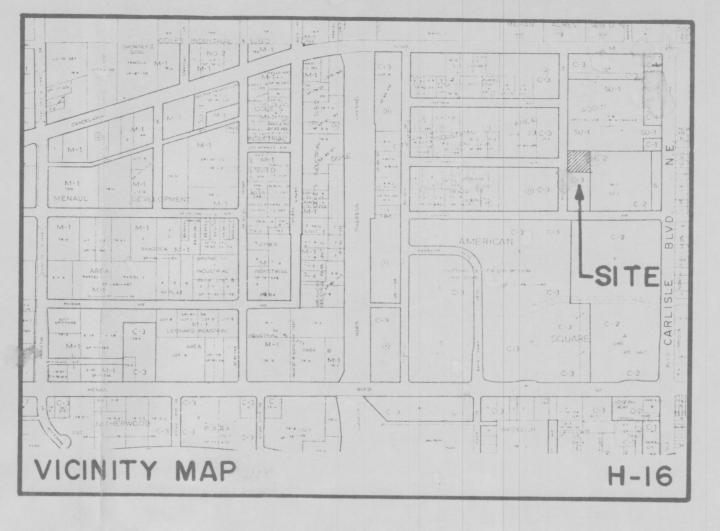
Off-site Area 2 discharges through the driveway.  $Q_{100} = 25.23$  cfs

On-site flow = 3.2 cfs - (0.53 + 0.41) = 2.26 cfs. Total  $Q_{100} = 27.49$  cfs Use Weir Equation.  $Q = CLH^{3/2}$  C = 2.65 L = 30' H = 0.5

 $Q = 2.65 \times 30 \times 0.5^{3/2} = 28.1 \text{ cfs} > 27.49 \text{ cfs}$  Adequate.







### SIDEWALK CULVERT PER NEW ASPHALTY C.O.A. STD. DWG. 2236 MAINTENANCE INV. IN = 30.67 BUILDING INV. OUT = 30.55 4400 SQUARE FEET 2400 S.F. F.F. = 5133,50 F.F.=5133.83 ALLEY GUTTER LIMIT OF PHASE PAVEMENT REMOVAL EXTENDED STEM WALL NEW ASPHALT 32.34 INV. END PH. I ALLEYGUTTER NEW ASPHALT - FUTURE ALLEY GUTTER LIMIT OF FUTURE ASPHALT PAVEMENT R&R ACCEPTED BM SEWER PROJECT 5132.06 / EXTEND STEM WALL F. F. = 5134.00 F.F. = 5134.83 FUTURE BUILDING -6800 SQUARE FEET CONSTRUCT 2 EA. 24"-SIDEWALK CULVERTS PER S 88°07'19" C.O. A. STD. DWG. 2236 INV. IN. = 32.53 OFF - SITE FLOW ACCEPTED INV. OUT = 32.41 - R. & R. ASPHALT

LANDSCAPING

S.O. 19 CITY OF ALBUQUERQUE DRAINAGE FACILITIES WITHIN CITY RIGHT-OF-WAY

NOTICE TO CONTRACTOR 1. An excavation/construction permit will be required before beginning any work within City right-of-way. An approved copy of these plans must be submitted at the time of application for this permit.

2. All work detailed on these plans to be performed under contract, except as otherwise stated or provided hereon, shall be constructed in accordance with Standard Specifications for Public Works Construction, 1986. . Two (2) working days prior to any excavation, contractor must contact New Mexico One Call System, Inc. (260-1990), for location of existing

utilities. Prior to construction, the contractor shall excavate and verify the horizontal and vertical locations of all obstructions. Should a conflict exist, the contractor shall notify the engineer or surveyor so that the conflict can be resolved with a minimum amount of delay.

Backfill compaction shall be according to\_\_\_\_ street use.

. Maintenance of these facilities shall be the responsibility of the owner of the property served.

APPROVALS:		
HYDROLOGY		
	(Name)	(Date)
INSPECTOR		
	(Name)	(Date)
CONSTRUCTION		
	(Name)	(Date)

LEGEND EXISTING NEW FUTURE DESCRIPTION (34)-CONTOUR SPOT ELEVATION PROPERTY LINE SWALE SHEET FLOW DOWN SPOUT ROOF DRAINAGE TA TOP OF ASPHALT TC TOP OF CURB FL FLOW LINE

Station 1-G16, located on the south side of Candelaria Blvd. N.E., west of Wellesley St. in front of house # 3310 Candelaria, N.E. A square chiseled on top of concrete curb. Elevation 5119.15 Feet.

# TEMPORARY BENCH MARK (TBM):

A line scored on top of curb opposite the S.W. corner of the Site. Elevation = 5133.07 Feet.

# LEGAL DESCRIPTION:

Lot C-2, Duke City Industrial Area Addition, replat filed October 10, 1991.

GRADING & DRAINAGE PLAN



JAN 3 0 1992

SHEET

JOB NO: DATE: REVISIONS