CITY OF ALBUQUERQUE



Richard J. Berry, Mayor

July 18, 2017

Anna Caffrey, PE Bohannan Huston, Inc. 7500 Jefferson St NE Albuquerque, NM 87109

RE: Lobo LL Field Expansion at Balduini Park Grading Plan Stamp Date: 7/13/17 Hydrology File: H16D149

Dear Ms. Caffrey:

PO Box 1293

Based upon the information provided in your submittal received 7/14/17, the Grading Plan **is not** approved for Grading Permit. The following comments need to be addressed for approval of the above referenced project:

1. Since the work to be done lays partially on AMAFA property and the work to be

done affects an existing AMAFA pond for Embudo Channel, approval by AMAFCA will be need prior to Hydrology approval. Please contact Bradley Bingham, PE at (505) 884-2215 or <u>bbingham@amafca.org</u>. Please provide an

Albuquerque

New Mexico 87103

www.cabq.gov

- 2. Please provide the floodplain note on the Grading Plan.
- 3. Please label and show the delineation of all floodplain zones within the Grading Plan. Please also include the 100 year flood information for Zone "AH".

If you have any questions, please contact me at 924-3995 or rbrissette@cabq.gov.

approval letter from AMAFCA in your resubmittal to Hydrology.

Sincerely,

Renée C. Brissette

Reneé C. Brissette, P.E. Senior Engineer, Hydrology Planning Department



City of Albuquerque

Planning Department Development & Building Services Division DRAINAGE AND TRANSPORTATION INFORMATION SHEET (REV 10/2015)

Project Title: Lobo LL Field Expansion at Balduini	ParkBuilding Peri	mit #: Hydrology File #:
DRB#:	EPC#:	Work Order#:
Legal Description: Balduini Park		
City Address: 2101 Bryn Mawr Dr NE		
Applicant: Bohannan Huston Inc		Contact: Anna Caffrey
Address:7500 Jefferson St NE, Courtyard II, Albu	querque, NM, 87109	
Phone#:505-823-1000	Fax#:505-7	798-7988 E-mail: acaffrey@bhinc.com
Other Contact:City of Albuquerque		Contact: Richard Zita
Address: One Civic Plaza, Room 7057, Albuquerqu	ue, NM, 87103	
Phone#: 505-768-3856	Fax#: <u>505-768-</u>	2310 E-mail: rzita@cabq.gov
Check all that Apply:		
DEPARTMENT: X HYDROLOGY/ DRAINAGE TRAFFIC/ TRANSPORTATION MS4/ EROSION & SEDIMENT CONTE TYPE OF SUBMITTAL: ENGINEER/ARCHITECT CERTIFICAT CONCEPTUAL G & D PLAN X GRADING PLAN DRAINAGE MASTER PLAN DRAINAGE REPORT CLOMR/LOMR TRAFFIC CIRCULATION LAYOUT (TO TRAFFIC IMPACT STUDY (TIS) EROSION & SEDIMENT CONTROL PI	ROL ION CL) LAN (ESC)	TYPE OF APPROVAL/ACCEPTANCE SOUGHT: BUILDING PERMIT APPROVAL CERTIFICATE OF OCCUPANCY PRELIMINARY PLAT APPROVAL SITE PLAN FOR SUB'D APPROVAL SITE PLAN FOR BLDG. PERMIT APPROVAL FINAL PLAT APPROVAL SIA/ RELEASE OF FINANCIAL GUARANTEE FOUNDATION PERMIT APPROVAL X GRADING PERMIT APPROVAL APPROVAL MORK ORDER APPROVAL CLOMR/LOMR
		PRE-DESIGN MEETING?
IS THIS A RESUBMITTAL?: Yes	No	OTHER (SPECIFY)
DATE SUBMITTED:	Ву:	Jmn. Ceffrey

COA STAFF: ELECTRONIC SUBMITTAL RECEIVED: ____

Bohannan 🛦 Huston

Engineering Spatial Data Advanced Technologies

Courtyard I 7500 Jefferson St. NE Albuquerque, NM 87109-4335

MEMORANDUM

DATE: July 13, 2017

TO: Richard Zita, City of Albuquerque

FROM: Anna Caffrey, PE



www.bhinc.com voice: 505.823.1000 facsimile: 505.798.7988 toll free: 800.877.5332

SUBJECT: Final Drainage Analysis Results for the Lobo Little League Field Expansion in Balduini Park

Background

This memorandum provides information on the drainage design of the Lobo Little League Field Expansion in Balduini Park. The project is located on the northeast side of the City of Albuquerque, just south of Menaul Blvd and east of the Embudo Channel and North Diversion Channel convergence. The existing ponding area was built in 1969 as part of the North Diversion Channel Phase III project and receives flow from the Lower Menaul Basin. The ponding area originally discharged to the North Diversion Channel via a 24-inch gated outlet pipe. In 2001, the outlet pipe was retrofitted with a ported riser.

This Field Expansion project will require the baseball field fence, AMAFCA maintenance road, and roadside side swale to be relocated into the ponding area as shown on Figure 1. The existing embankment for the Embudo Channel will not be disturbed in order to avoid the USACE requirement for modifications to a levee. The 8-foot existing maintenance road will remain adjacent to the embankment slope, and a new 15-foot wide maintenance road to the north will be constructed.

Background Review

Several previous studies and as-builts were reviewed and are summarized below:

- Storm Drainage Report Princeton & Menaul Drainage Areas at the North Diversion Channel, William Matotan & Associates Engineers, June 1965 – The configuration recommended in this report was different from what was ultimately constructed. Two ponding areas were recommended: one north of Menaul and one south of Menaul connected by an equalizer pipe. The principal spillway was a lift station which pumped flow from the north pond to the North Diversion Channel; however, the report does provide flow rates and volumes reaching the Ponding Area of 175 cfs and 13.5 ac-ft for the 100-year event and 195 cfs and 32 ac-ft for the larger, project storm.
- North Diversion Channel Phase III As-builts, USACE, 1969 A 24-inch gated outlet pipe was constructed through the North Diversion Channel embankment, and some of the Menaul Ponding area was regraded so that the area would drain to the outlet pipe.
- Albuquerque Master Drainage Study, Bohannan Huston, 1987 Provides peak 100-year flow rates on Menaul at Carlisle Blvd of 111 cfs and on Bryn Mawr Dr at the Embudo Channel of 98 cfs.
- Construction Plans for Principal Spillway Inlet Modifications at Menaul Ponding Area, Boyle Engineering Cooperation, December 2001 The existing 24-inch gated outlet pipe was modified to include a ported riser inlet structure with an inlet grate on top.

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• *FEMA Flood Insurance Rate Map*, August 2012 – The map shows an approximate 100-year elevation of 5,089 feet, but no backup data is provided in the Flood Insurance Study to determine how it was calculated.

Ponding Area Stage Storage Analysis

A topographic survey of the existing site was completed for the Field Expansion project; however, the survey did not cover the entire ponding area, so 2010 city-wide contours were used to supplement the survey data. Using the merged contours, a stage-storage curve was computed using the Average End Area Method at a 1-foot contour interval. This computation is included as Table 1.

NAVD88	CONTOUR	CONTOUR	AVG.			TOTAL			
ELEV.	AREA	AREA	AREA	REIGHT	VOLUIVIE	VOLUME			
(FT)	(SQ-FT)	(AC)	(AC)	(FT)	(AC-FT)	(AC-FT)			
5,088	0	0.00	-	-	-	0.00			
5,089	68,608	1.58	0.79	1	0.79	0.79			
5,090	84,210	1.93	1.75	1	1.75	2.54			
5,091	93,603	2.15	2.04	1	2.04	4.58			
5,092	103,212	2.37	2.26	1	2.26	6.84			
5,093	118,372	2.72	2.54	1	2.54	9.39			
5,094	134,211	3.08	2.90	1	2.90	12.28			
5,095	174,029	4.00	3.54	1	3.54	15.82			
5,096	222,004	5.10	4.55	1	4.55	20.37			
5,097	255,115	5.86	5.48	1	5.48	25.84			
5,098	306,721	7.04	6.45	1	6.45	32.29			
5,099	366,459	8.41	7.73	1	1 7.73				
5,100	407,649	9.36	8.89	1	8.89	48.91			

 Table 1 – Existing Menaul Ponding Area Stage Storage Calculations

 (Average End Area Method)

Grading for the new maintenance road was done in Civil3D. The proposed contours were merged with existing contours, and a stage-storage curve was computed using the Average End Area Method at a 1-foot contour interval, see Table 2. A comparison of Table 1 and Table 2 shows a slight decrease in storage. The existing and proposed WSEL are compared at various design criteria in Table 3.

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NAVD88	CONTOUR	CONTOUR	AVG.			TOTAL		
ELEV.	AREA	AREA	AREA	HEIGHT	VOLUME	VOLUME		
(FT)	(SQ-FT)	(AC)	(AC)	(FT)	(AC-FT)	(AC-FT)		
5,088	0	0.00	-	-	-	0.00		
5,089	64,711	1.49	0.74	1	0.74	0.74		
5,090	78,940	1.81	1.65	1	1.65	2.39		
5,091	87,594	2.01	1.91	1	1.91	4.30		
5,092	96,159	2.21	2.11	1	2.11	6.41		
5,093	111,400	2.56	2.38	1	2.38	8.79		
5,094	137,177	3.15	2.85	1	2.85	11.65		
5,095	173,088	3.97	3.56	1	3.56	15.21		
5,096	222,011	5.10	4.54	1	4.54	19.74		
5,097	255,115	5.86	5.48	1	5.48	25.22		
5,098	306,721	7.04	6.45	1	6.45	31.67		
5,099	366,459	8.41	7.73	1	7.73	39.40		
5,100	407,649	9.36	8.89	1	8.89	48.28		

Table 2 – Proposed Menaul Ponding Area Stage Storage Calculations(Average End Method)

 Table 3 – Stage Storage Comparison

CRITERIA	EX. WSEL	EX. VOLUME	PROPOSED WSEL	INCREASE IN WSEL
	(FT)	(AC-FT)	(FT)	(FT)
WSEL OF 5089 PER FEMA MAP	5,089	0.79	5,089.03	0.03
100-YR VOLUME PER 1965 REPORT	5,094.34	13.5	5,094.52	0.18
PROJECT STORM VOLUME PER 1965 REPORT	5,097.96	32.0	5,098.04	0.08

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Ditch Analysis

A local drainage basin was delineated for the area reaching the relocated roadside swale. The limits of the basins are Bryn Mawr to the east (because of the existing storm drain) and the top of the channel embankment to the south and west. A 100-year flow rate was calculated using the rational methodology for 40 acre and smaller basins per the COA DPM, Chapter 22, Part A. The basin is mainly Land Treatment Type B for the park but does have 13 percent Land Treatment Type C for the steeper channel embankment slope and 1 percent Land Treatment Type D for the building and dugouts. The project is in Zone 2. A 100-year flow rate of 11.0 cfs was calculated.

The roadside swale was analyzed using Manning's equation in Hydraflow Express. Two sections were used. The upper section is located at the start of the swale and in the middle where the maintenance road to the north and the maintenance road to the west split. The side slopes are 10:1, and the longitudinal slope is 3 percent. The lower section is located where the ditch meets the flatter ponding area. The side slopes are 4:1 on the maintenance road tie slope side and 30:1 on the ponding area side. The longitudinal slope is flat and assumed to be 0.1 percent for this analysis. The upper section has a depth of 0.6 feet and a velocity of 3.2 fps. The lower section has a depth of 0.9 feet and a velocity of 0.8 fps.

Recommendations

In summary, from the analysis performed in this memorandum, the proposed conditions grading of the Lobo Little League Field Expansion at Balduini Park does not adversely impact the function of the Menaul Ponding Area. Although there will be a slight increase to the water surface elevation of the floodplain due to fill from the project, there is plenty of freeboard in the pond and none of the structures will be impacted.

AQC/le Attachments





Basin Hydrology Rational Method

Proposed Conditions Basin Data Table

This table is based on the DPM Section 22.2, Zone: 2
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Basin	Area	Area	Lanc	d Treatmer	reatment Percentages Q(100)			
ID	(SQ. FT)	(AC.)	Α	В	B C		(cfs/ac.)	(CFS)
1	196976	4.52	0.0%	85.9%	12.8%	1.3%	2.42	10.95

Channel Report

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Monday, Dec 5 2016

Upper Swale

Triangular		Highlighted	
Side Slopes (z:1)	= 10.00, 10.00	Depth (ft)	= 0.59
Total Depth (ft)	= 1.00	Q (cfs)	= 10.95
		Area (sqft)	= 3.48
Invert Elev (ft)	= 5093.00	Velocity (ft/s)	= 3.15
Slope (%)	= 3.00	Wetted Perim (ft)	= 11.86
N-Value	= 0.035	Crit Depth, Yc (ft)	= 0.60
		Top Width (ft)	= 11.80
Calculations		EGL (ft)	= 0.74
Compute by:	Known Q		
Known Q (cfs)	= 10.95		



Reach (ft)

Channel Report

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Friday, Jul 7 2017

Lower Swale

Triangular		Highlighted	
Side Slopes (z:1)	= 30.00, 4.00	Depth (ft)	= 0.91
Total Depth (ft)	= 1.00	Q (cfs)	= 10.95
		Area (sqft)	= 14.08
Invert Elev (ft)	= 5089.00	Velocity (ft/s)	= 0.78
Slope (%)	= 0.10	Wetted Perim (ft)	= 31.07
N-Value	= 0.035	Crit Depth, Yc (ft)	= 0.49
		Top Width (ft)	= 30.94
Calculations		EGL (ft)	= 0.92
Compute by:	Known Q		
Known Q (cfs)	= 10.95		



Reach (ft)

CONSTRUCTION PLANS FOR LOBO LL FIELD EXPANSION AT BALDUINI PA ALBUQUERQUE, NEW MEXICO

CONSTRUCTION NOTES:

1. THE CONTRACTOR SHALL ABIDE BY ALL LOCAL, STATE, AND FEDERAL LAWS, RULES AND REGULATIONS WHICH APPLY TO THE CONSTRUCTION OF THESE IMPROVEMENTS.

2. PRIOR TO CONSTRUCTION, CONTRACTOR SHALL EXCAVATE AND VERIFY THE HORIZONTAL AND VERTICAL LOCATIONS OF ALL POTENTIAL OBSTRUCTIONS. SHOULD A CONFLICT EXIST, THE CONTRACTOR SHALL NOTIFY THE ENGINEER OR CONSTRUCTION OBSERVER SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM AMOUNT OF DELAY.

3. ALL ELECTRICAL, TELEPHONE, CABLE TV, GAS AND OTHER UTILITY LINES, CABLES AND APPURTENANCES ENCOUNTERED DURING CONSTRUCTION THAT REQUIRE RELOCATION, SHALL BE COORDINATED WITH THAT UTILITY. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATION OF ALL NECESSARY UTILITY ADJUSTMENTS. NO ADDITIONAL COMPENSATION WILL BE ALLOWED FOR DELAYS OR INCONVENIENCES CAUSED BY UTILITY COMPANY WORK CREWS. THE CONTRACTOR MAY BE REQUIRED TO RESCHEDULE HIS ACTIVITIES TO ALLOW UTILITY CREWS TO PERFORM THEIR REQUIRED WORK.

4. DISPOSAL SITE FOR ALL EXCESS EXCAVATION MATERIAL, AND UNSUITABLE MATERIAL SHALL BE OBTAINED BY THE CONTRACTOR IN COMPLIANCE WITH APPLICABLE ENVIRONMENTAL REGULATIONS AND APPROVED BY THE CONSTRUCTION OBSERVER. ALL COSTS INCURRED IN OBTAINING A DISPOSAL SITE AND HAUL THERETO SHALL BE CONSIDERED INCIDENTAL TO THE PROJECT AND NO SEPARATE MEASUREMENT OR PAYMENT SHALL BE MADE.

5. THE CONTRACTOR IS RESPONSIBLE FOR PROTECTING THE EXISTING UTILITY LINES WITHIN THE CONSTRUCTION AREA. ANY DAMAGE TO EXISTING FACILITIES CAUSED BY CONSTRUCTION ACTIVITY SHALL BE REPAIRED OR REPLACED AT THE CONTRACTOR'S EXPENSE AND APPROVED BY THE CONSTRUCTION OBSERVER.

6. CONSTRUCTION ACTIVITY SHALL BE LIMITED TO THE PROPERTY AND/OR PROJECT LIMITS. ANY DAMAGE TO ADJACENT PROPERTIES RESULTING FROM THE CONSTRUCTION PROCESS IS THE RESPONSIBILITY OF THE CONTRACTOR. ANY COSTS INCURRED FOR REPAIRS SHALL BE THE COST OF THE CONTRACTOR.

7. OVERNIGHT PARKING OF CONSTRUCTION EQUIPMENT SHALL NOT OBSTRUCT DRIVEWAYS OR DESIGNATED TRAFFIC LANES. THE CONTRACTOR SHALL NOT STORE ANY EQUIPMENT OR MATERIAL WITHIN THE PUBLIC RIGHT-OF-WAY.

8. THE CONTRACTOR SHALL OBTAIN ALL THE NECESSARY PERMITS FOR THE PROJECT PRIOR TO COMMENCING CONSTRUCTION (I.E. BARRICADING, SURFACE DISTURBANCE).

9. THE CONTRACTOR SHALL BE RESPONSIBLE TO REPLACE AT HIS EXPENSE ANY AND ALL PROPERTY CORNERS DESTROYED DURING CONSTRUCTION. ALL PROPERTY CORNERS MUST BE RESET BY A REGISTERED LAND SURVEYOR.

10. ALL PERMANENT PAVEMENT MARKING AND TRAFFIC SIGNING SHALL BE FURNISHED AND PLACED BY THE CONTRACTOR PER PLAN.

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11. THE CONTRACTOR SHALL COORDINATE WITH THE CITY OF ALBUQUERQUE, DMD CONSTRUCTION COORDINATION DIVISION, PRIOR TO BEGINNING ANY CONSTRUCTION WORK ON OR ADJACENT TO EXISTING STREETS.

12. ALL BARRICADES AND CONSTRUCTION SIGNING SHALL CONFORM TO APPLICABLE SECTIONS OF THE "MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES" (MUTCD), U.S. DEPARTMENT OF TRANSPORTATION, LATEST EDITION.

13. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING AND MAINTAINING CONSTRUCTION SIGNAGE UNTIL THE PROJECT HAS BEEN ACCEPTED BY THE CITY OF ALBUQUERQUE. THE CONTRACTOR SHALL VERIFY THE PROPER LOCATION OF ALL BARRICADING AT THE END AND BEGINNING OF EACH DAY.

14. ALL SAWCUT PAVEMENT SHALL HAVE A UNIFORM EDGE AND BE SPRAYED WITH TACK.

15. THE CONTRACTOR SHALL ASSUME RESPONSIBILITY FOR ANY DAMAGE TO EXISTING PAVEMENTS, SIGNAGE, PAVEMENT MARKINGS, CURB & GUTTER, DRIVE PADS, WHEELCHAIR RAMPS, AND SIDEWALK DURING CONSTRUCTION, APART FROM THOSE SECTIONS INDICATED FOR REMOVAL ON THE PLANS AND SHALL REPAIR OR REPLACE PER COA STANDARDS, AT HIS OWN EXPENSE.

17. AN SO 19 PERMIT IS REQUIRED TO PLACE ANY MATERIAL ON OR AROUND A STORM DRAIN INLET IN THE CITY RIGHT-OF-WAY THAT WOULD INTERFERE WITH THE INLET RECEIVING STORM WATER PER THE ENGINEER'S DESIGN. CITY PERSONNEL MAY REMOVE THIS MATERIAL AT ANY TIME WITHOUT NOTICE. THE PREFERRED BMP IS TO REMOVE SEDIMENT/POLLUTANTS ON THE PROPERTY WHERE CONSTRUCTION ACTIVITY IS OCCURRING.

18. ALL EXCAVATION, TRENCHING, AND SHORING ACTIVITIES MUST BE CARRIED-OUT IN ACCORDANCE WITH OSHA 29 CFR 1926.650 SUBPART P.



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LE: N.T.S.	Bohannan AF	Juston 800.877.5332			No. Date		Designed Bv: GM	Drawn By: JW	Checked By: GM
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LANDSCAPE	ARCHITECTS
210 La Veta NE Albuquerque, NM 87108	PHONE 505 268 2266 WEB mrwmla.com

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SCALE: 1" = 20' (HORIZ.)

City Project No. 658100

Zone Map No. H-16-Z