



City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

December 8, 1995

Mark Goodwin
Mark Goodwin & Associates
P.O. Box 90606
Albuquerque, NM 87199

RE: ENGINEER CERTIFICATION FOR AN ADDITION TO DUKE CITY STUDIOS
(H17-D70) CERTIFICATION STATEMENT DATED 11/28/95.

Dear Mr. Goodwin:

Based on the information provided on your November 28, 1995 submittal, Engineer Certification for the above referenced site is acceptable.

Please be advised that only a 30 day temporary Certificate of Occupancy will be issued until the work order items are completed and accepted by the City.

If I can be of further assistance, please feel free to contact me at 768-2667.

Sincerely,

Bernie J. Montoya, CE
Engineering Associate

BJM/dl

c: Andrew Garcia
Duke City Studios
Lou Sadler
File



City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

June 12, 1995

Roger Martinez
Mark Goodwin & Associates
P.O. Box 90606
Albuquerque, NM 87199

RE: GRADING/PAVING PLAN FOR DUKE CITY STUDIO OFF-SITE
IMPROVEMENTS (H17-D70) ENGINEER'S STAMP DATED 5/22/95.

Dear Mr. Martinez:

Based on the information provided on your May 10, 1995 submittal, the above referenced site is approved for Grading/Paving and Work Order.

If I can be of further assistance, please feel free to contact me at 768-2667.

Sincerely,

Bernie J. Montoya, CE
Engineering Associate

BJM/dl

c: Andrew Garcia
File



D. Mark Goodwin & Associates, P.A.
Consulting Engineers and Surveyors

PROJECT CUTLER AVE.
SUBJECT DRAINAGE
BY RM DATE 1 May 95
CHECKED _____ DATE _____
SHEET 1 OF 2

AREA OF BASIN

$$A = 18.0349 \text{ ACRES} = 0.02825 \text{ MILES}$$

PROPOSED & EXISTING SITE DISCHARGE DIRECTLY INTO CHANNEL. A 4' EARTHEN BERM HAS BEEN CONSTRUCTED SURROUNDING A PORTION OF THE BASIN. BOTH OF THESE AREAS ARE EXCLUDED FROM THE AREA FIGURE (SEE MAPS PROVIDED)

LAND TREATMENTS

$$\text{TYPE "A"} = 83.25\%$$

$$\text{TYPE "B"} = 5.58\%$$

$$\text{TYPE "C"} = 5.58\%$$

$$\text{TYPE "D"} = 5.59\%$$

ZONE 2 AHYMO DESIGN CRITERIA

$$P_{60} = 2.01 \quad P_{360} = 2.35 \quad P_{1440} = 2.75$$

AHYMO RESULTS

$$Q = 33.14 \text{ cfs} @ 1.533425$$

$$V_{PK} = 0.9870 \text{ ACRES-FEET}$$

CHANNEL & INLET SIZING

-INLET OPENING

$$Q = CLH^{3/2} \quad H = 0.5' \quad C = 2.9 \quad Q = 33.14 \text{ cfs}$$

$$L = \frac{Q}{C H^{3/2}}$$

$$L = 32.32'$$

∴ MINIMUM INLET OPENING 33'

CONTINUED ON NEXT PAGE



D. Mark Goodwin & Associates, P.A.
Consulting Engineers and Surveyors

PROJECT CUTLER AVE
SUBJECT DRAINAGE
BY RM DATE 2 May 95
CHECKED _____ DATE _____
SHEET 2 OF 2

CHANNEL & INLET SIZING (CONTINUED)

- CHANNEL - DEPTH OF FLOW WILL NOT EXCEED $\frac{1}{2}$ WALL HEIGHT

8" WALL HEIGHT SLOPE = 0.50%

$$Q = \frac{K}{n} A R^{\frac{2}{3}} S^{\frac{1}{2}}$$

$$Q = \frac{1.49}{0.016} (32 * 0.333) \left(\frac{32 * 0.333}{32.67} \right)^{\frac{2}{3}} (0.005)^{\frac{1}{2}}$$

$$Q = 33.31 \text{ cfs}$$

$\therefore$ CHANNEL WIDTH 32'

START TIME=0.0

***** HYDROGRAPH FOR DUKE CITY STUDIOS OFFSITE IMPROVEMENTS
***** CUTLER AVENUE FRONTING SITE TO EMBUDO CHANNEL INLET

RAINFALL TYPE=1 RAIN QUARTER=0.0 IN
RAIN ONE=2.01 IN RAIN SIX=2.35 IN
RAIN DAY=2.75 IN DT=0.03333 HR

*HYDROGRAPH FOR BASIN CONTRIBUTING

*TO EMBUDO CHANNEL INLET

COMPUTE NM HYD ID=1 HYD NO=101.1 AREA=0.0282 SQ MI
PER A=83.25 PER B=5.58 PER C=5.58 PER D=5.59
TP=0.1333 HR MASS RAINFALL=-1

PRINT HYD ID=1 CODE=1

FINISH

AHYMD PROGRAM (AHYMD194) - AMAFCA Hydrologic Model - January, 1994
 RUN DATE (MON/DAY/YR) = 05/02/1995
 START TIME (HR:MIN:SEC) = 07:55:24 USER NO.= M_GOODWN.101
 INPUT FILE = DUKE.DAT

START TIME=0.0

***** HYDROGRAPH FOR DUKE CITY STUDIOS OFFSITE IMPROVEMENTS

***** CUTLER AVENUE FRONTING SITE TO EMBUDO CHANNEL INLET

RAINFALL TYPE=1 RAIN QUARTER=0.0 IN
 RAIN ONE=2.01 IN RAIN SIX=2.35 IN
 RAIN DAY=2.75 IN DT=0.03333 HR

COMPUTED 6-HOUR RAINFALL DISTRIBUTION BASED ON NOAA ATLAS 2
 - PEAK AT 1.40 HR.

DT = .033330 HOURS			END TIME = 5.999400 HOURS			
.0000	.0016	.0033	.0049	.0066	.0084	.0102
.0120	.0139	.0158	.0178	.0199	.0219	.0241
.0263	.0286	.0309	.0333	.0358	.0384	.0411
.0439	.0467	.0497	.0529	.0561	.0596	.0631
.0669	.0709	.0751	.0807	.0866	.0930	.1066
.1371	.1640	.2514	.3434	.4644	.6186	.8106
1.0449	1.2624	1.3533	1.4300	1.4982	1.5602	1.6174
1.6704	1.7200	1.7664	1.8102	1.8514	1.8904	1.9273
1.9622	1.9953	2.0268	2.0566	2.0850	2.0915	2.0976
2.1033	2.1088	2.1140	2.1191	2.1239	2.1285	2.1329
2.1373	2.1414	2.1454	2.1494	2.1531	2.1568	2.1604
2.1639	2.1673	2.1706	2.1739	2.1771	2.1802	2.1832
2.1862	2.1891	2.1919	2.1947	2.1975	2.2002	2.2028
2.2054	2.2080	2.2105	2.2130	2.2154	2.2178	2.2202
2.2225	2.2248	2.2270	2.2293	2.2315	2.2336	2.2358
2.2379	2.2399	2.2420	2.2440	2.2460	2.2480	2.2500
2.2519	2.2538	2.2557	2.2576	2.2594	2.2612	2.2631
2.2648	2.2666	2.2684	2.2701	2.2718	2.2735	2.2752
2.2769	2.2785	2.2802	2.2818	2.2834	2.2850	2.2866
2.2881	2.2897	2.2912	2.2928	2.2943	2.2958	2.2973
2.2987	2.3002	2.3017	2.3031	2.3045	2.3060	2.3074
2.3088	2.3102	2.3115	2.3129	2.3143	2.3156	2.3169
2.3183	2.3196	2.3209	2.3222	2.3235	2.3248	2.3261
2.3273	2.3286	2.3298	2.3311	2.3323	2.3335	2.3348
2.3360	2.3372	2.3384	2.3396	2.3408	2.3419	2.3431
2.3443	2.3454	2.3466	2.3477	2.3488	2.3500	

*HYDROGRAPH FOR BASIN CONTRIBUTING

*TO EMBUDO CHANNEL INLET

PER A=63.25 PER B=5.58 PER C=5.58 PER D=5.59

TP=0.1333 HR MASS RAINFALL=-1

K = .072649HR TP = .133300HR K/TP RATIO = .545000 SHAPE
CONSTANT, N = 7.106420
UNIT PEAK = 6.2236 CFS UNIT VOLUME = .9976 B = 526.28
P60 = 2.0100
AREA = .001576 SQ MI IA = .10000 INCHES INF = .04000 I
NCHES PER HOUR
RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT
= .033330

K = .155380HR TP = .133300HR K/TP RATIO = 1.165644 SHAPE
CONSTANT, N = 3.041822
UNIT PEAK = 56.965 CFS UNIT VOLUME = .9995 B = 285.21
P60 = 2.0100
AREA = .026624 SQ MI IA = .62340 INCHES INF = 1.59553 I
NCHES PER HOUR
RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT
= .033330

PRINT HYD ID=1 CODE=1

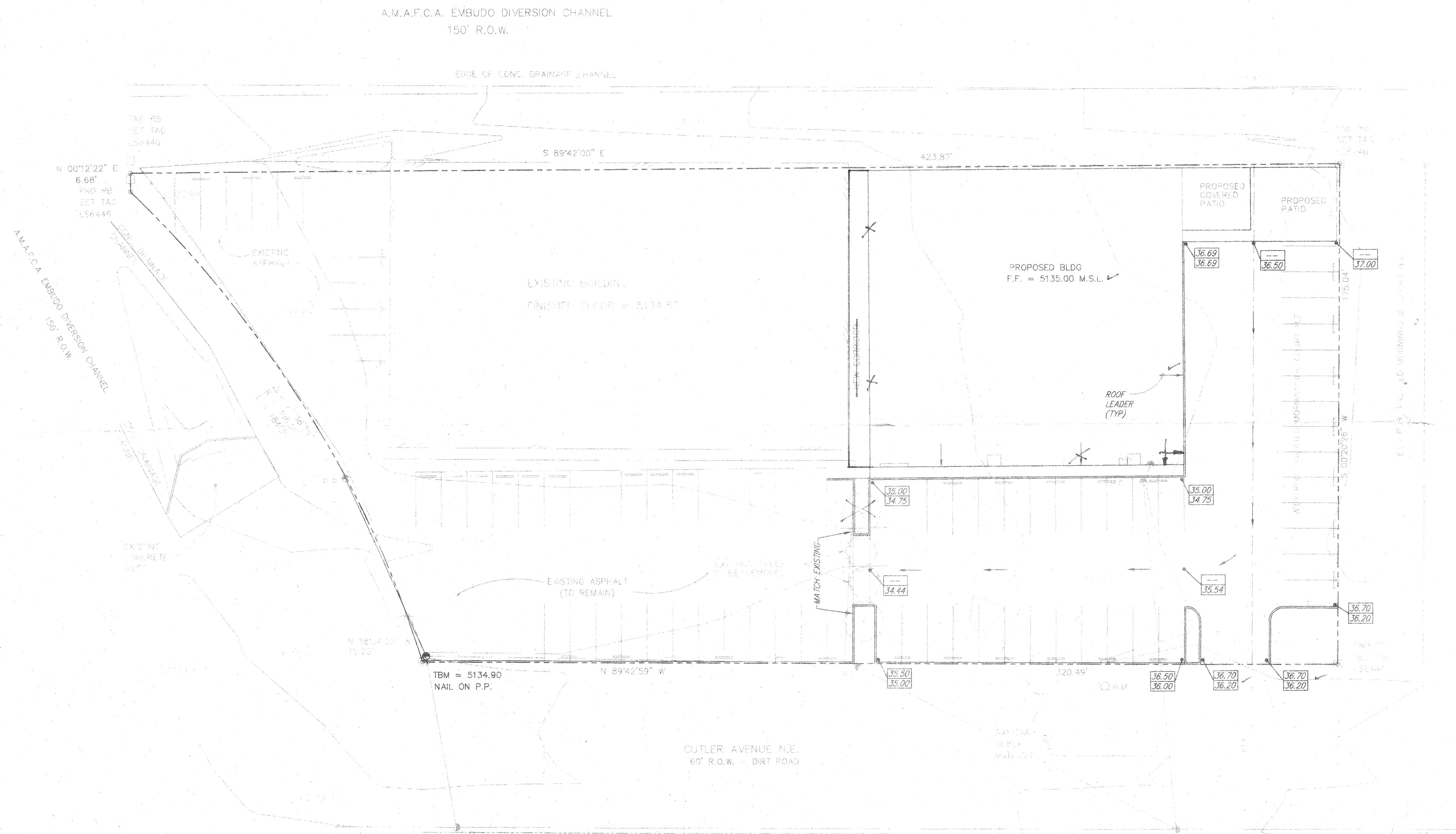
PARTIAL HYDROGRAPH 101.10

RUNOFF VOLUME = .65629 INCHES = .9870 ACRE-FEET
PEAK DISCHARGE RATE = 33.14 CFS AT 1.533 HOURS BASIN AREA =
.0282 SQ. MI.

FINISH

NORMAL PROGRAM FINISH

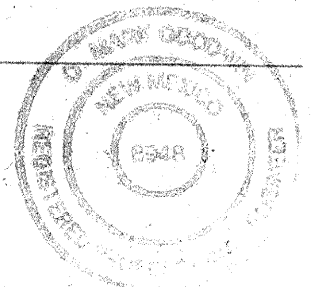
END TIME (HR:MIN:SEC) = 07:55:32



RECORD DRAWING

I hereby certify that the information contained on this drawing has been revised in accordance with information furnished by the Contractor, Sadler Southwest Construction Co., and by the Surveyor, Southwest Surveying, and reflects the construction as actually accomplished. This plan as constructed is in substantial compliance with the Approved Plan.

Mark Goodwin 11/22/95
Mark Goodwin, NMPE 8948



DRAINAGE CALCULATIONS

EXISTING

A = 0.65 Ac.
X0 = 0%
P = 2.35 in.
Tc = 10 Min.
Q = 1.20 cfs
Vol = 1722 Cu. Ft.

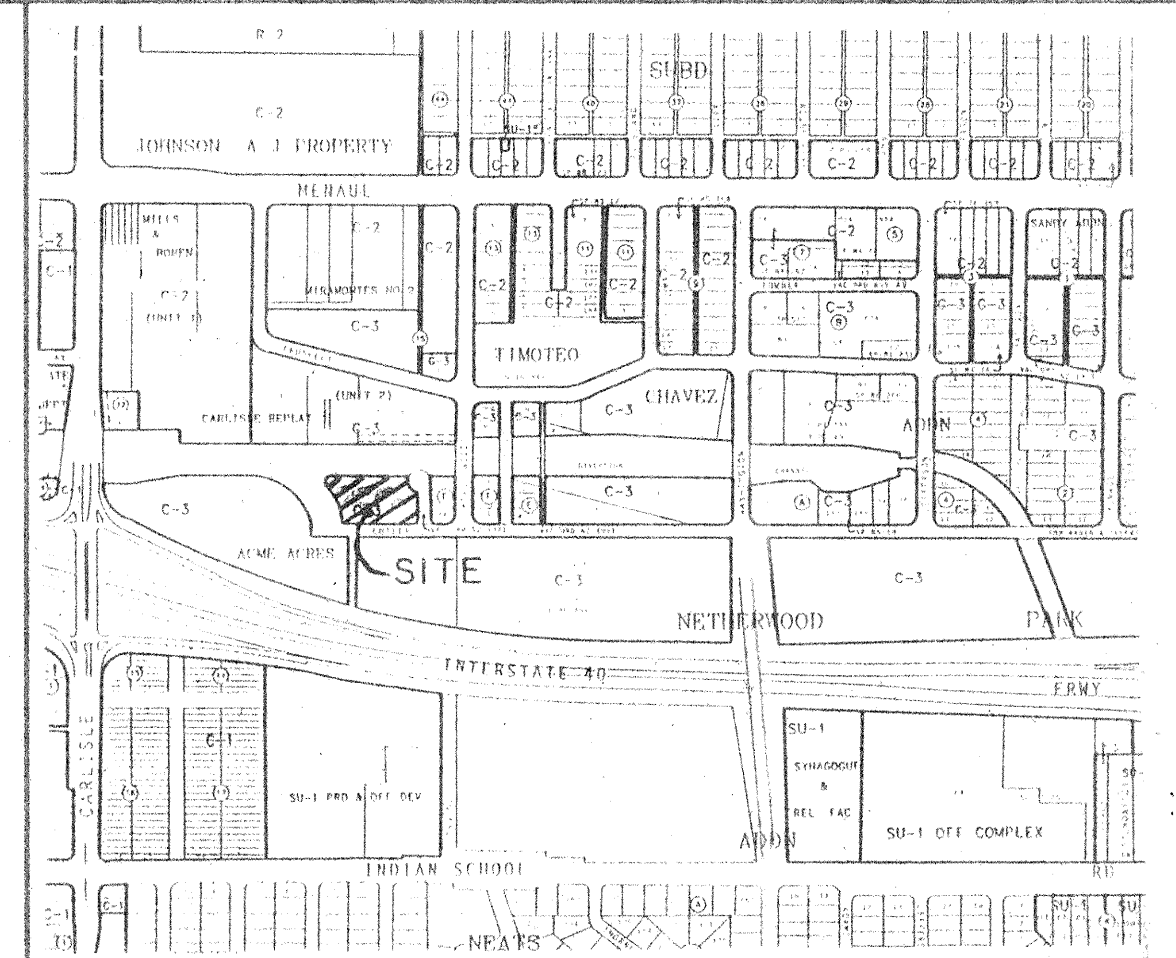
PROPOSED

A = 0.65 Ac.
X0 = 95%
P = 2.35 in.
Tc = 10 Min.
*Q = 3.20 cfs
Vol = 6321 Cu. Ft.

*SINCE FLOW ENTERS EMBUDO DIVERSION,
DIRECT DISCHARGE IS APPROPRIATE.

Mark Goodwin
10/19/94

SCALE: 1" = 20'



ADJACENT MAP ZONE MAP: H-17

TBM (TEMPORARY BENCHMARK)

NAIL ON POWER POLE AT SOUTH EAST CORNER OF PROPERTY.
ELEVATION = 5134.90

A.C.S. BENCHMARK

THE STATION MARK IS A STANDARD A.C.S. BRASS TABLET STAMPED "6-H17,1979" DRILLED INTO THE TOP OF THE CONCRETE CURB.

THE STATION IS LOCATED 3.7 MILES NORTHEAST OF DOWNTOWN ALBUQUERQUE, AT THE INTERSECTION OF CANDELARIA AND GRACELAND DRIVE N.E.

TO REACH THE STATION FROM THE INTERSECTION OF CANDELARIA ROAD AND I-25, GO EAST ON CANDELARIA ROAD 1.5 MILES TO GRACELAND DRIVE. THE STATION IS LOCATED IN THE SOUTHWEST QUADRANT OF THE INTERSECTION.

X = 396,389.59
Y = 1,497,781.25
Z = 5161.13

LEGEND

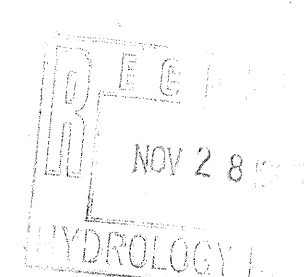
TRACT "G" TIMOTEO CHAVEZ ADDITION

NOTES

1. SITE DOES NOT LIE WITHIN A 100 YEAR FLOOD ZONE.
2. NO OFF-SITE FLOWS ENTER THIS SITE.

LEGEND

- PROPERTY LINE
- EXISTING SPOT ELEVATION
- EXISTING CONTOUR
- EXISTING POWER POLE
- EXISTING WATER METER
- AS-BUILT
- PROPOSED SPOT ELEVATION
- FLOWLINE



DUKE CITY STUDIOS GRADING & DRAINAGE PLAN

dmg D. MARK GOODWIN & ASSOCIATES, P.A.
CONSULTING ENGINEERS & SURVEYORS
P.O. BOX 90606
ALBUQUERQUE, NEW MEXICO 87199
(505) 345-2010

Designed: DMG Drawn: DLD Checked: DMG Sheet 1 of 1
Scale: AS SHOWN Date: 10/94 Job: 94138