# CITY OF ALBUQUERQUE

*Planning Department* Brennon Williams, Interim Director



Mayor Timothy M. Keller

August 28, 2019

Jeremy Shell Respec 5971 Jefferson St. NE Albuquerque, NM 87109

RE: Carlisle Marketplace 2100 Carlisle NE Conceptual Grading Plan Stamp Date: 8/21/19 Drainage Report Stamp Date: (8/28/19) Hydrology File: H17D097

Dear Mr. Shell:

PO Box 1293 Based on the submittal received on 8/21/19, the Conceptual Grading and Drainage Plan cannot be approved until the following are corrected:

Prior to Site Plan:

Albuquerque

1. Show the grading and site work on the grading plan per the DPM Ch.22.7, *Grading Plan Checklist*; the following items are missing/deficient:

NM 87103

- a. Existing contoursb. Proposed contours
- c. Spot elevations adequately showing conditions on-site.
- d. Contours and spot elevations extending a minimum of 25' beyond property line.

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- f. Elevations along property lines
- g. Phasing (if phased)

e. Ponds

#### Prior to Building Permit (For Information):

- 2. Required SWQV needs to be calculated for the impervious area only and using the redevelopment amount (0.26/12"). It looks like you multiplied .34/12" by the impervious area.
- 3. Please provide the stormwater quality volume (SWQV) calculations for each basin draining to each pond. The stormwater quality ponds need to be sized for the areas draining to them.
- 4. Payment in Lieu of onsite management of the SWQV must be made prior to Building Permit (Amount T.B.D).

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- 5. Please show and label the pond(s) and include a label on each with the SWQV and elevation, the 100-year volume and elevation, the peak 100 year inflow and outflow, the spillway crest elevation, and the spillway flow depth.
- 6. Additional comments should be expected, based on the outcome of the above remarks and level of detail shown on plans.

If you have any questions, please contact me at 924-3695 or dpeterson@cabq.gov.

Sincerely,

Dana Peterson, P.E. Senior Engineer, Planning Dept. Development Review Services

PO Box 1293

Albuquerque

NM 87103

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# DRAINAGE REPORT FOR CARLISLE MARKETPLACE

**PREPARED FOR** City of Albuquerque, Planning Department Development Review Services, Hydrology Section

PREPARED BY RESPEC, Inc. 5971 Jefferson St. NE, Suite 101 Albuquerque, NM 87109 505.253.9718

**AUGUST 2019** 





I, Sheldon Greer, do hereby certify that this report was duly prepared by me or under my direction and that I am a duly registered Professional Engineer under the laws of the State of New Mexico.



Sheldon Greer, P.E. NMPE No. 17154

8/28/2019

Date

i



### **TABLE OF CONTENTS**

1.0	INTR	ODUCTION	3			
	1.1	Purpose	3			
	1.2	Location and Description	3			
		Figure 1.2.1 – Project Location	3			
2.0	MET	HODOLOGY	4			
3.0	HYD	ROLOGY	4			
	3.1	Existing Conditions	4			
		Table 3.1.1 – Hydrologic Data - Existing	5			
	3.2	Proposed Conditions	5			
		Table 3.2.1 – Hydrologic Data - Proposed	6			
4.0	CON	CLUSION	6			
APPE	NDIX A	EXISTING SUB-BASINS	7			
APPE	NDIX B	PROPOSED SUB-BASINS	8			
APPE	PPENDIX C HYDROLOGY CALCULATIONS					

ii



### **1.0 INTRODUCTION**

#### 1.1 PURPOSE

The purpose of this drainage report is to demonstrate that the proposed re-development of Tracts A and B of Carlisle and Indian School Subdivision safely conveys the peak 100-year storm runoff. The drainage intent for proposed conditions is to match current existing conditions for the site.

#### **1.2 LOCATION AND DESCRIPTION**

Tracts A and B are located at the northeast corner of the Carlisle Boulevard and Indian School Road intersection and contain approximately 10.7 acres. See Figure 1.2.1 below. The existing site includes a Burger King restaurant located on Tract B and an old K-Mart building and parking lot on Tract A that is currently vacant. The existing conditions are described in more detail in Section 3.1 and the proposed conditions are described in Section 3.2.



FIGURE 1.2.1 - PROJECT LOCATION

3

# 2.0 METHODOLOGY

The hydrologic analysis was performed for the site in accordance with the Albuquerque Development Process Manual (DPM) Section 22.2 using the Rational Method to calculate peak flow rates for the 100year, 24-hour design storm in order to ensure all flow paths are sufficient to carry flows. The required water quality volume was calculated by multiplying the impervious area by the first flush runoff value of 0.34". All hydrologic and hydraulic calculations are included in this report.

### 3.0 HYDROLOGY

#### **3.1 EXISTING CONDITIONS**

Tracts A & B do not receive any offsite flows. The existing site has approximately 93% impervious area and 7% landscaped. The total flow generated by the property under existing conditions is 48.9 cfs. The site appears to have free discharge and does not have any existing ponds. The existing property has been split into six sub-basins. Appendix A shows the existing sub-basin boundaries for the site.

Sub-basin A consists of the northwest corner of the property and is primarily made up of parking area and also the Burger King restaurant. In general, the sub-basin slopes from southeast to northwest at varying slopes between 3%-5%. Runoff exits the property at the northwest corner of the site and is collected in a drop inlet.

Sub-basin B contains the northeast corner of the property and accounts for surface runoff from the northern portion of the existing building and the drive aisle north of the building. This area accumulates to the northeast corner of the site and discharges out of the property into a concrete rundown. From there, runoff is collected in a drop inlet.

Sub-basin C consists of a majority of the existing building and the drive aisle east of the building. This area flows north along the eastern curb. At the northeast corner of the Sub-basin, there is an opening in the wall that allows runoff to discharge into the adjacent property to the east. Flows that bypass this wall opening enter Sub-basin B.

Sub-basin D contains the southwest corner of the existing building and a majority of the existing parking area. This Sub-basin, in general, sheet flows from southeast to northwest at varying slopes between 2%-5%. Runoff then flows north along a curb along the western property boundary and discharges in Carlisle Boulevard through an existing driveway. From there, flows enter storm inlets located along the eastern curb of Carlisle Boulevard.

Sub-basin E consists of a small portion of the parking area at the southwest corner of the property. This area slopes from southeast to northwest and discharges from the site through an existing driveway. The runoff generated by this Sub-basin is then collected in storm inlets located along the eastern curb of Carlisle Boulevard.

Sub-basin F contains a small area west of the existing Burger King restaurant the flows west into Carlisle Boulevard. Runoff from this Sub-basin is collected in the Carlisle Boulevard storm drain system.

RSI-03337

4



The hydrologic data table below depicts in further detail each sub-basin and its characteristics.

HYDROLOGIC DATA - EXISTING						
			LAND USE PERCENTAGES			
SUB-BASIN	AREA (AC)	A	В	С	D	Q100
A	2.20	0%	0%	0%	100%	10.3
В	0.96	0%	0%	0%	100%	4.5
С	2.47	0%	8%	8%	84%	10.8
D	4.35	0%	4%	4%	92%	19.7
E	0.54	0%	0%	0%	100%	2.6
F	0.20	0%	0%	0%	100%	1.0
TOTAL	10.72					48.9

#### TABLE 3.1.1 - HYDROLOGIC DATA - EXISTING

#### **3.2 PROPOSED CONDITIONS**

The proposed site development is to renovate the existing buildings and parking lot and add both commercial and retail pads along the Carlisle Boulevard property frontage. Under the proposed condition, approximately 87% of the site will consist of impervious area and 13% will be landscaped. The total flow generated by the proposed development is 47.6 cfs. Therefore, the discharge from the proposed site is less than the existing condition. The property has been split into five proposed subbasins. Appendix B shows the proposed sub-basin boundaries for the site.

Sub-basin 1 consists of the northwest corner of the property and is made up of parking area, the existing Burger King restaurant, and a new commercial pad. In general, the sub-basin slopes from southeast to northwest. Runoff is collected in a proposed drop inlet in the new parking lot which discharges into a storm drain that will connect to the existing drop inlet, which is the location that this area is currently discharging to. The existing flow that reaches this inlet is 10.3 cfs while the proposed flow is 9.3 cfs.

Sub-basin 2 contains the roof drainage for the eastern half of the larger building and the truck dock area and drive aisle located east of the building. Runoff generated by this Sub-basin flows north to the northeast corner of the site where it enters a concrete channel. This channel will connect to the existing concrete rundown that discharges to the existing drop inlet. The existing flow that reaches the northeast corner of the property is 15.3 cfs while the proposed flow is 15.4 cfs. The additional 0.1 cfs is considered negligible. The flows that currently discharge to the neighboring property to the east will be cut off under the proposed condition.

Sub-basins 3, 4, and 5 consist of the southwest corner of the property, which contains a majority of the parking lot and the roof drainage from the western half of the larger building as well as two new retail pads. Sub-basins 3 and 5 free discharge from the site through two new driveways. Sub-basin 4 is collected in a drop inlet toward the northwest corner of the Sub-basin. These three Sub-basins all discharge to the existing storm drain system in Carlisle Boulevard. The existing flow that reaches the Carlisle storm drain under existing conditions is 23.3 cfs while the proposed flow is 22.9 cfs.

5

The hydrologic data table below depicts in further detail each sub-basin and its characteristics.



HYDROLOGIC DATA - PROPOSED						
			LAND USE PERCENTAGES			
SUB-BASIN	AREA (AC)	A	В	С	D	Q100
1	2.10	0%	6%	6%	87%	9.3
2	3.46	0%	6%	6%	87%	15.4
3	1.54	0%	6%	6%	87%	6.8
4	2.47	0%	6%	6%	87%	11.0
5	1.14	0%	6%	6%	87%	5.1
TOTAL	10.72					47.6

#### TABLE 3.2.1 - HYDROLOGIC DATA - PROPOSED

The total required water quality volume for the site is 11,526 cubic feet. The owner has elected to pay the fee in lieu for any required stormwater quality volume not provided in on-site ponds. More details regarding water quality will be provided at Building Permit review.

## 4.0 CONCLUSION

This drainage report is prepared in support of the new development for Tracts A and B. The existing buildings and parking area will be renovated and new commercial and retail pads will be added. The proposed conditions closely match the current conditions of the existing property. The hydrologic calculations are included in Appendix C.





# **APPENDIX A** EXISTING SUB-BASINS









# **APPENDIX B** PROPOSED SUB-BASINS







# **APPENDIX C** Hydrology calculations



#### Hydrology Calculations

The following calcualtions are based on Albuquerque's Development Process Manual, Seciton 22.2

#### **Existing Conditions**

#### Runoff Rate:

Treatment Type Areas

Subbasin	Area <sub>A</sub> (ac)	Area <sub>B</sub> (ac)	Area <sub>c</sub> (ac)	Area <sub>D</sub> (ac)	Total (ac)
A	0.00	0.00	0.00	2.20	2.20
В	0.00	0.00	0.00	0.96	0.96
С	0.00	0.19	0.19	2.09	2.47
D	0.00	0.19	0.19	3.97	4.35
E	0.00	0.00	0.00	0.54	0.54
F	0.00	0.00	0.00	0.20	0.20
Total	0.00	0.38	0.38	9.96	10.72

Peak Discharge values based on Zone 2 from Table A-9

 $Q_A = 1.56$  cfs/ac

 $Q_B = 2.28$  cfs/ac

 $Q_C = 3.14$  cfs/ac  $Q_D = 4.70$  cfs/ac

Peak Discharge calculation for a 100-yr, 24-hr storm event from equation A-10

Subbasin	Discharge (cfs)
А	10.3
В	4.5
С	10.8
D	19.7
E	2.6
F	1.0
Total	48.9

#### **Proposed Conditions**

#### Runoff Rate:

**Treatment Type Areas** 

Subbasin	Area <sub>A</sub> (ac)	Area <sub>B</sub> (ac)	Area <sub>c</sub> (ac)	Area <sub>D</sub> (ac)	Total (ac)
1	0.00	0.14	0.14	1.83	2.10
2	0.00	0.22	0.22	3.02	3.46
3	0.00	0.10	0.10	1.34	1.54
4	0.00	0.16	0.16	2.15	2.47
5	0.00	0.07	0.07	0.99	1.14
Total	0.00	0.69	0.69	9.34	10.72

Peak Discharge values based on Zone 2 from Table A-9

 $Q_A = 1.56$  cfs/ac

 $Q_B = 2.28 \text{ cfs/ac}$   $Q_C = 3.14 \text{ cfs/ac}$   $Q_D = 4.70 \text{ cfs/ac}$ 

Peak Discharge calculation for a 100-yr, 24-hr storm event from equation A-10

Subbasin	Discharge (cfs)
1	9.3
2	15.4
3	6.8
4	11.0
5	5.1
Total	47.6

#### Water Quality:

Required Water Quality volume for first flush of 0.34"

Subbasin	Volume (cu. ft.)
1	2,260
2	3,725
3	1,658
4	2,656
5	1,227
Total	11,526