CITY OF ALBUQUERQUE

June 2, 2015

Graeme Means, PE
High Mesa Consulting Group, Inc.
6010-B Midway Park Blvd. NE
Albuquerque, NM 87109

RE: **Aztec Elementary School, 2611 Eubank Blvd. NE**
Grading and Drainage Plan
Engineer's Stamp Date 5-01-2015 (File: H20-D033)

Dear Mr. Means:

Based upon the information provided in your submittal received 5-05-15, the above referenced Grading and Drainage Plan cannot be approved for Building Permit until the following comments are addressed:

1) Provide the AHYMO input and output files and inlet capacity calculations.
2) Show roof drain locations on the grading plan.

If you have any questions, you can contact me at 924-3924.

Sincerely,

Jeanne Wolfenbarger, P.E.
Senior Engineer, Planning Dept.
Development Review Services

Orig: Drainage file
c.pdf Addressee via Email

PO Box 1293
Albuquerque
New Mexico 87103
www.cabq.gov

City of Albuquerque

Planning Department

Development & Building Services Division

DRAINAGE AND TRANSPORTATION INFORMATION SHEET

(REV 02/2013)

Project Title: AZTEC SPECIAL EDUCATION FACILITY Building Permit #: _____ City Drainage #: H20D033

DRB#: _____ EPC#: _____ Work Order#: _____

Legal Description: TRACTS E AND F, LANDS OF BOARD OF EDUCATION, AZTEC ELEM. SCHOOL

City Address: 2611 EUBANK BLVD NE, ALBUQUERQUE NM 87110

Engineering Firm: High Mesa Consulting Group Contact: Graeme Means #13676

Address: 6010-B Midway Park Blvd NE, Albuquerque NM 87109

Phone#: 505-345-4250 Fax#: 505-345-4254 E-mail: gmeans@highmesacg.com

Owner: Albuquerque Public Schools Contact: Annelle Darby

Address: 915 Oak Street SE, 87106

Phone#: 848-8829 Fax#: _____ E-mail: annelle.darby@aps.edu

Architect: Westwork Architects Contact: Mark DePree

Address: PO Box 10921, Albuquerque NM 87184

Phone#: 884-5252 Fax#: 884-5255 E-mail: westwork@swcp.com

Surveyor: High Mesa Consulting Group Contact: Chuck Cala NMPS#11184

Address: 6010-B Midway Park Blvd NE, Albuquerque NM 87109

Phone#: 505-345-4250 Fax#: 505-345-4254 E-mail: ccala@highmesacg.com

Contractor: To be selected Contact: _____

Address: _____

Phone#: _____ Fax#: _____ E-mail: _____

TYPE OF SUBMITTAL:

- ____ DRAINAGE REPORT (DMP)
- X DRAINAGE PLAN 1st SUBMITTAL
- ____ DRAINAGE PLAN RESUBMITTAL
- ____ CONCEPTUAL G & D PLAN
- X GRADING PLAN
- ____ EROSION & SEDIMENT CONTROL PLAN (ESC)
- ____ ENGINEER'S CERT (HYDROLOGY)
- ____ CLOMR/LOMR
- ____ TRAFFIC CIRCULATION LAYOUT (TCL)
- ____ ENGINEER'S CERT (TCL)
- ____ ENGINEER'S CERT (DRB SITE PLAN)
- ____ ENGINEER'S CERT (ESC)
- ____ SO-19
- ____ OTHER (SPECIFY)

CHECK TYPE OF APPROVAL/ACCEPTANCE SOUGHT:

- ____ SIA/FINANCIAL GUARANTEE RELEASE
- ____ PRELIMINARY PLAT APPROVAL
- ____ S. DEV. PLAN FOR SUB'D APPROVAL
- ____ S. DEV. FOR BLDG. PERMIT APPROVAL
- ____ SECTOR PLAN APPROVAL
- ____ FINAL PLAT APPROVAL
- ____ CERTIFICATE OF OCCUPANCY (PERM)
- ____ CERTIFICATE OF OCCUPANCY (TCL TEMP)
- ____ FOUNDATION PERMIT APPROVAL
- X BUILDING PERMIT APPROVAL
- X GRADING PERMIT APPROVAL
- X PAVING PERMIT APPROVAL
- ____ WORK ORDER APPROVAL
- ____ GRADING CERTIFICATION
- ____ SO-19 APPROVAL
- ____ ESC PERMIT APPROVAL
- ____ ESC CERT. ACCEPTANCE
- ____ OTHER (DMP)

AO verified

RECEIVED

MAY 05 2015

DRAINAGE DEVELOPMENT SECTION

Paid $50.00 ✓

WAS A PRE-DESIGN CONFERENCE ATTENDED: ____ Yes X No ____ Copy Provided

DATE SUBMITTED: 05-05-2015 By: Justin Schara

Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a drainage submittal. The particular nature, location, and scope to the proposed development defines the degree of drainage detail. One or more of the following levels of submittal may be required based on the followin

1. **Conceptual Grading and Drainage Plan:** Required for approval of Site Development Plans greater than five (5) acres and Sector Plans
2. **Drainage Plans:** Required for building permits, grading permits, paving permits and site plans less than five (5) acres
3. **Drainage Report:** Required for subdivision containing more than ten (10) lots or constituting five (5) acres or more
4. **Erosion and Sediment Control Plan:** Required for any new development and redevelopment site with 1-acre or more of land disturbing area, including project less than 1-acre than are part of a larger common plan of development

DRAINAGE PLAN ⚠1

I. INTRODUCTION AND EXECUTIVE SUMMARY

THIS PROJECT, LOCATED IN THE NORTHEAST HEIGHTS OF THE ALBUQUERQUE METROPOLITAN AREA, REPRESENTS A MODIFICATION TO AN EXISTING APS SITE WITHIN AN INFILL AREA. THE PROPOSED DEVELOPMENT IS COMPRISED OF THE CONSTRUCTION OF A NEW PERMANENT BUILDING TO REPLACE EXISTING PORTABLE CLASSROOM BUILDINGS AND OLDER PERMANENT BUILDINGS. THE PROPOSED DEVELOPMENT WILL BE LOCATED AT THE CENTRAL AND SOUTHWEST PORTIONS OF THE CAMPUS. THE PROPOSED IMPROVEMENTS REFERENCED HEREIN ARE THE FIRST PHASE OF A MULTI–PHASE PROJECT THAT WILL ULTIMATELY ELIMINATE THE EXISTING PERMANENT BUILDINGS AND INCLUDE A NEW PAVED PARKING LOT; THE REMOVAL OF THE EXISTING PERMANENT BUILDINGS AND NEW PARKING LOT CONSTRUCTION WILL BE BY SEPARATE SUBMITTAL.

THE DRAINAGE CONCEPT FOR THIS PROJECT WILL BE THE CONTINUED DISCHARGE OF DEVELOPED RUNOFF TO AN EXISTING ONSITE DETENTION POND AT THE SOUTHWEST CORNER OF THE COMPLEX SITE, WITH CONTROLLED DISCHARGE TO AN EXISTING DOWNSTREAM DRAINAGE EASEMENT.

II. PROJECT DESCRIPTION

AS SHOWN BY THE VICINITY MAP, THE AZTEC COMPLEX IS LOCATED AT THE SOUTHWEST CORNER OF THE INTERSECTION OF EUBANK BLVD NE AND LEXINGTON AVENUE NE. THE CURRENT LEGAL DESCRIPTION IS TRACTS E , AZTEC ELEMENTARY SCHOOL. AS SHOWN BY PANEL 356 OF 825 OF THE NATIONAL FLOOD INSURANCE PROGRAM FLOOD INSURANCE RATE MAPS PUBLISHED BY FEMA FOR BERNALILLO COUNTY, NEW MEXICO, THIS SITE DOES NOT LIE WITHIN A DESIGNATED FLOOD HAZARD ZONE. PRIOR SITE DEVELOPMENT HAS ESTABLISHED A PRECEDENT FOR ONSITE DETENTION PONDING. INASMUCH AS THIS IS A REDEVELOPMENT PROJECT OF AN EXISTING FULLY DEVELOPED SITE, THE EXISTING DRAINAGE PATTERNS AND CONCEPTS SHALL BE MAINTAINED.

III. BACKGROUND DOCUMENTS

THE PREPARATION OF THIS PLAN RELIED UPON THE FOLLOWING DOCUMENTS:

- DRAINAGE SUBMITTAL FOR SIERRA ALTERNATIVE SCHOOL PREPARED BY WILSON & COMPANY, NMPE 11955, DATED 8–12–98. THIS REFERENCED PLAN ESTABLISHED THE CONCEPT FOR THE ONSITE DETENTION OF STORM WATER RUNOFF WITH CONTROLLED DISCHARGE VIA 18" STORM DRAIN OUTLET OF 7.67 CFS TO A DOWNSTREAM DRAINAGE EASEMENT.
- GRADING AND DRAINAGE PLAN FOR DIAGNOSTICIAN CONSOLIDATION AT AZTEC COMPLEX, PREPARED BY HIGH MESA CONSULTING GROUP, NMPE 8547, DATED 08–28–2010 AND CERTIFIED 12–30–2010. THIS 2010 SUBMITTAL INCLUDED THE RENOVATION OF AN EXISTING PORTABLE CLASSROOM PARK TO SERVE DIAGNOSTICIANS; THESE PORTABLE CLASSROOMS WILL BE REMOVED AND REPLACED WITH PERMANENT BUILDING AS PART OF THE DEVELOPMENT OF THE SITE.
- TOPOGRAPHIC SURVEY PREPARED BY HIGH MESA CONSULTING GROUP, NMPS 11184, DATED 11–18–2013. THIS REFERENCED SURVY PROVIDES THE BASIS FOR THE EXISTING CONDITIONS OF THE PROJECT SITE.

IV. EXISTING CONDITIONS

THE PROJECT SITE PRESENTLY CONSISTS OF SEVERAL EXISTING PERMANENT AND PORTABLE CLASSROOM BUILDINGS. THE SITE IS SERVED BY TWO ASPHALT PAVED PARKING LOTS AT THE NORTHWEST AND NORTHEAST CORNERS OF THE SITE. THE DRAINAGE PATTERN FOR THE SITE ESTABLISHED BY PREVIOUS SUBMITTALS IS SHEETFLOW FROM NORTHEAST TO SOUTHWEST WITH DISCHARGE TO AN EXISTING DETENTION POND LOCATED AT THE SOUTHWEST CORNER OF THE SITE, AND CONTROLLED DISCHARGE FROM THE POND TO AN EXISTING DOWNSTREAM DRAINAGE EASEMENT.

THERE ARE NO APPARENT OFFSITE FLOWS IMPACTING THIS SITE. LEXINGTON AVE NE AND EUBANK BLVD NE, FULLY DEVELOPED CITY STREETS, LIE TO THE NORTH AND EAST WITH FLOWS APPARANTLY CONFINED TO THE CONSTRUCTED STREETS. EXISTING RESIDENTIAL LOTS TO THE SOUTH AND WEST ARE TOPOGRAPHICALLY LOWER AND APPEAR TO DISCHARGE DEVELOPED RUNOFF TO THE FRONTING CITY STREETS.

V. DEVELOPED CONDITIONS

THE PROPOSED CONSTRUCTION CONSISTS OF A NEW PERMANENT BUILDING AND PAVED ACCESS AND PARKING IMPROVEMENTS TO REPLACE EXISTING PERMANENT BUILDINGS AND PORTABLE CLASSROOM BUILDINGS. THE DEVELOPED CONDITIONS DEPICTED HEREIN ARE PART OF A MULTI–PHASE PROJECT, WHEREIN THE PORTABLE CLASSROOM BUILDINGS WILL BE REMOVED PRIOR TO CONSTRUCTION OF THE NEW BUILDING, AND REMOVAL OF THE EXISTING PERMANENT BUILDINGS WILL OCCUR IN A FUTURE PHASE BY SEPARATE SUBMITTAL. RUNOFF FROM THE SITE WILL CONTINUE TO GENERALLY SHEETFLOW FROM EAST TO WEST AND NEW PRIVATE STORM DRAIN IMPROVEMENTS WILL BE CONSTRUCTED TO COLLECT AND CONVEY RUNOFF DIRECTLY TO THE ONSITE PRIVATE DETENTION POND AT THE SOUTHWEST CORNER OF THE SITE. THE PROPOSED IMPROVEMENTS WILL RESULT IN A MINOR DECREASE IN PEAK DISCHARGE AND VOLUME OF RUNOFF GENERATED BY THE SITE.

THE EXISTING RETENTION POND WILL BE REGRADED TO INCREASE PONDING CAPACITY. IN ADDITION, THE BOTTOM OF THE POND WILL BE LOWERED BELOW THE EXISTING STORM DRAIN OUTLET, RESULTING IN A RETENTION CAPACITY OF 29,460 CF. AS A RESULT, THE 2 YEAR, 24 HOUR DEVELOPED RUNOFF (20,870 CF) GENERATED BY THE SITE WILL BE RETAINED ONSITE, WHICH WILL ALSO MEET THE CONDITION OF MANAGING AND CONTROLLING THE FIRST FLUSH OF DEVELOPED RUNOFF DUE TO THE NEW IMPERVIOUS AREAS, AS WELL AS LEED REQUIREMENTS FOR STORMWATER QUANTITY AND QUALITY. THE EXISTING 18" STORM DRAIN OUTLET PIPE WILL NOT BE MODIFIED, AND WILL CONTINUE TO CONTROL DISCHARGE TO THE DOWNSTREAM DRAINAGE EASEMENT. AS DETERMINED BY AHYMO RESERVOIR ROUTING, THE 100–YEAR RELEASE RATE WILL BE 2.2 CFS WHICH IS MUCH LESS THAN THE PREVIOUSLY APPROVED 7.67 CFS RATE ESTABLISHED IN THE 1998 DRAINAGE PLAN REFERENCED ABOVE.

THERE WILL CONTINUE TO BE NO APPARENT OFFSITE FLOWS IMPACTING THE SITE AS A RESULT OF THESE DEVELOPED CONDITIONS.

VI. GRADING PLAN

THE GRADING PLAN SHOWS 1.) EXISTING AND PROPOSED GRADES INDICATED BY SPOT ELEVATIONS AND CONTOURS AT 1'–0" INTERVALS, 2.) THE LIMIT AND CHARACTER OF THE EXISTING AND PROPOSED IMPROVEMENTS, AND 3.) CONTINUITY BETWEEN EXISTING AND PROPOSED GRADES. AS SHOWN BY THIS PLAN, THE ROPOSED GRADING WILL DIRECT DEVELOPED RUNOFF TO THE EXISTING ONSITE PRIVATE DETENTION POND.

VII. CALCULATIONS

THE CALCULATIONS CONTAINED HEREON ANALYZE THE EXISTING AND DEVELOPED CONDITIONS FOR THE 100–YEAR, 6–HOUR AND THE 2–YEAR, 24–HOUR RAINFALL EVENTS. THE PROCEDURE FOR 40 ACRE AND SMALLER BASINS, AS SET FORTH IN THE REVISION OF SECTION 22.2, HYDROLOGY OF THE DEVELOPMENT PROCESS MANUAL, VOLUME 2, DESIGN CRITERIA, DATED JANUARY 1993, HAS BEEN USED TO QUANTIFY THE PEAK RATE OF DISCHARGE AND VOLUME OF RUNOFF GENERATED. AS DEMONSTRATED BY THESE CALCULATIONS, THE PROPOSED DEVELOPMENT WILL RESULT IN A MINOR DECREASE IN THE DEVELOPED RUNOFF GENERATED BY THE AZTEC COMPLEX SITE. IN ADDITION THE AVERAGE END AREA METHOD WAS USED TO QUANTIFY THE CAPACITY OF THE REGRADED PRIVATE DETENTION POND, AND MANNING'S EQUATION WAS USED TO CALCULATE THE CAPACITY OF THE PROPOSED PRIVATE STORM DRAIN SYSTEM. AHYMO CALCULATIONS ROUTING THE SITE RUNOFF THROUGH THE DETENTION POND WERE RUN TO DEMONSTRATE THAT THE A PEAK DISCHARGE OF 2.2 CFS WILL DISCHARGE THROUGH THE EXISTING 18" STORM DRAIN OUTLET DURING A 100–YEAR STORM EVENT, WELL BELOW THE 7.67 CFS ALLOWABLE DISCHARGE RATE ESTABLISHED BY PRIOR SUBMITTAL.

VIII. CONCLUSIONS

THE FOLLOWING CONCLUSIONS HAVE BEEN ESTABLISHED AS A RESULT OF THE EVAULATIONS CONTAINED HEREIN:

1. THE PROPOSED IMPROVEMENTS ARE CONSISTENT WITH THE CONCEPT FOR DETENTION PONDING ESTABLISHED BY PRIOR SUBMITTALS
2. THE PROPOSED IMPROVEMENTS WILL RESULT IN A MINOR DECREASE IN DEVELOPED PEAK DISCHARGE AND VOLUME OF RUNOFF GENERATED BY THE SITE
3. THE REGRADED POND WILL RETAIN THE 2–YEAR, 24 HOUR RAINFALL EVENT, THEREFORE THE CONDITION OF MANAGING AND CONTROLLING THE FIRST FLUSH OF DEVELOPED RUNOFF FROM NEW IMPERVIOUS AREAS WILL BE MET
4. THE EXISTING 18" STORM DRAIN OUTLET FOR THE SITE WILL NOT BE MODIFIED BY THE SITE DEVELOPMENT, MAINTAINING THE CONTROLLED DISCHARGE FROM THE SITE TO THE DOWNSTREAM DRAINAGE EASEMENT.

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STORM DRAIN HYDRAULICS					
PIPE ID	Mannings Coefficient	Slope (ft/ft)	Diameter (in)	Maximum Discharge (cfs)	Discharge Full (cfs)
A	0.013	0.0234	24	37.2	34.6
B	0.013	0.0200	24	34.4	32.0
C	0.013	0.0105	24	24.9	23.2
D	0.013	0.0112	30	46.7	43.4
E	0.013	0.0054	30	32.4	30.1
F	0.013	0.0142	30	52.6	48.9
G	0.013	0.0142	12	4.6	4.3
H	0.013	0.0085	30	40.7	37.8
I	0.013	0.0111	30	46.5	43.2

⚠1

POND VOLUME by ELEVATION				
ELEV FT	AREA SF	VOL CF	∑ VOL CF	∑ VOL AC-FT
81.20	12070	0	0	0.00
82.00	21490	13420	13420	0.31
82.70	24350	16040	29460	0.68
83.00	25870	7530	36990	0.85
84.00	30500	23180	60170	1.38
85.00	37300	33900	94070	2.16

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File Name: 140371_C–001–R1.DWG | Plot Time: 3:19 pm

CALCULATIONS

I. SITE CHARACTERISTICS

A. PRECIPITATION ZONE = **3**

B. $P_{100,\,6\,HR}$ = P_{360} = **2.6 IN**

$P_{2,\,24\,HR}$ = $P_{1440\text{-}2}$ = **1.3 IN**

C. TOTAL PROJECT AREA (A_T) = 442,510 SF / **10.16 AC**

D. LAND TREATMENTS

1. EXISTING LAND TREATMENT

TREATMENT	AREA (SF/AC)	%
A		
B	22,250 SF / **0.51 AC**	5
C	217,890 SF / **5.00 AC**	49
D	202,370 SF / **4.65 AC**	46

2. DEVELOPED LAND TREATMENT

TREATMENT	AREA (SF/AC)	%
A		
B	123,506 SF / **2.84 AC**	28
C	123,506 SF / **2.84 AC**	28
D	195,498 SF / **4.49 AC**	44

II. HYDROLOGY

A. EXISTING CONDITION 100 YEAR

1. **100-YR STORM**

a. VOLUME 100-YR, 6- HR

$E_W = (E_AA_A+E_BA_B+E_CA_C+E_DA_D)/A_T$

E_W = (0.66*0.00) + (0.92*0.51) + (1.29*5.00) + (2.36*4.65)/10.16 = **1.76 IN**

$V_{100,6\,HR} = (E_W/12)A_T$ = (1.76/12)10.16 = 1.4899 AC-FT = **64,900 CF**

b. VOLUME 100- YR, 24- HR

$V_{100,24\,HR} = V_{6HR}+A_D*(P_{24HR}-P_{6HR})/12$ in/ft

= 1.49+4.65*(3.10-2.60)/12 in/ft= 1.6835 AC-FT = **73,330 CF**

c. PEAK DISCHARGE

$Q_P = Q_{PA}A_A + Q_{PB}A_B + Q_{PC}A_C + Q_{PD}A_D$

Q_P = (1.87 * 0.00) + (2.60 * 0.51) + (3.45 * 5.00) + (5.02 * 4.65) = **41.9 CFS**

2. **2-YR STORM**

a. VOLUME

$E_W = (E_AA_A+E_BA_B+E_CA_C+E_DA_D)/A_T$

E_W = (0.00*0.00) + (0.06*0.51) + (0.20*5.00) + (0.89*4.65)/10.16 = **0.51 IN**

$V_{2,6\,HR} = (E_W/12)A_T$ = (0.51/12)10.16 = 0.4317 AC-FT = **18,810 CF**

b. VOLUME 2- YR, 24- HR

$V_{2,24\,HR} = V_{2,\,6HR}+A_D*(P_{2,\,24HR}-P_{2,\,6HR})/12$ in/ft

= 0.43+4.65*(1.35-1.13)/12 in/ft= 0.5158 AC-FT = **22,470 CF**

c. PEAK DISCHARGE

$Q_P = Q_{PA}A_A + Q_{PB}A_B + Q_{PC}A_C + Q_{PD}A_D$

Q_P = (0.00 * 0.00) + (0.21 * 0.51) + (0.78 * 5.00) + (2.04 * 4.65) = **13.5 CFS**

B. DEVELOPED CONDITION

1. **100-YR STORM**

a. VOLUME

$E_W = (E_AA_A+E_BA_B+E_CA_C+E_DA_D)/A_T$

E_W = (0.66*0.00) + (0.92*2.84) + (1.29*2.84) + (2.36*4.49)/10.16 = **1.66 IN**

$V_{100,6\,HR} = (E_W/12)A_T$ = (1.66/12)10.16 = 1.4053 AC-FT = **61,210 CF**

b. VOLUME 100- YR, 24- HR

$V_{100,24\,HR} = V_{6HR}+A_D*(P_{24HR}-P_{6HR})/12$ in/ft

= 1.41+4.49*(3.10-2.60)/12 in/ft= 1.5923 AC-FT = **69,360 CF**

c. PEAK DISCHARGE

$Q_P = Q_{PA}A_A + Q_{PB}A_B + Q_{PC}A_C + Q_{PD}A_D$

Q_P = (1.87 * 0.00) + (2.60 * 2.84) + (3.45 * 2.84) + (5.02 * 4.49) = **39.7 CFS**

2. **2-YR STORM**

a. VOLUME

$E_W = (E_AA_A+E_BA_B+E_CA_C+E_DA_D)/A_T$

E_W = (0.00*0.00) + (0.06*2.84) + (0.20*2.84) + (0.89*4.49)/10.16 = **0.47 IN**

$V_{2,6\,HR} = (E_W/12)A_T$ = (0.47/12)10.16 = 0.3979 AC-FT = **17,330 CF**

b. VOLUME 2- YR, 24- HR

$V_{2,24\,HR} = V_{2,\,6HR}+A_D*(P_{2,\,24HR}-P_{2,\,6HR})/12$ in/ft

= 0.40+4.49*(1.35-1.13)/12 in/ft= 0.4790 AC-FT = **20,870 CF**

c. PEAK DISCHARGE

$Q_P = Q_{PA}A_A + Q_{PB}A_B + Q_{PC}A_C + Q_{PD}A_D$

Q_P = (0.00 * 0.00) + (0.21 * 2.84) + (0.78 * 2.84) + (2.04 * 4.49) = **12.0 CFS**

C. COMPARISON 100 YEAR

1. **100-YR STORM**

a. VOLUME 100-YR, 6-HR

$\Delta V_{100,\,6\,HR}$ = 61210 - 64900 = **-3,690 CF** (DECREASE)

b. VOLUME 100-YR, 24- Hr

$\Delta V_{100,\,24\,HR}$ = 69360 - 73330 = **-3,970 CF** (DECREASE)

c. PEAK DISCHARGE

ΔQ_{100} = 39.7 - 41.9 = **-2.2 CFS** (DECREASE)

2. **2-YR STORM**

a. VOLUME 2-YR, 6-HR

$\Delta V_{2,\,6\,HR}$ = 17330 - 18810 = **-1,480 CF** (DECREASE)

b. VOLUME 2-YR, 24- Hr

$\Delta V_{2,\,24\,HR}$ = 20870 - 22470 = **-1,600 CF** (DECREASE)

c. PEAK DISCHARGE

ΔQ_2 = 12.0 - 13.5 = **-1.5 CFS** (DECREASE)

CONSTRUCTION NOTES:

1. ALL WORK DETAILED ON THESE PLANS TO BE PERFORMED UNDER CONTRACT SHALL, EXCEPT AS OTHERWISE STATED OR PROVIDED FOR HEREON, BE CONSTRUCTED IN ACCORDANCE WITH THE NEW MEXICO STANDARD SPECIFICATIONS FOR PUBLIC WORKS CONSTRUCTION – 1987, PUBLISHED BY THE NEW MEXICO CHAPTER AMERICAN PUBLIC WORKS ASSOCIATION. (REVISED 12/06)
2. TWO (2) WORKING DAYS PRIOR TO ANY EXCAVATION, CONTRACTOR MUST CONTACT NEW MEXICO ONE CALL SYSTEM (NM 811) FOR DESIGNATION (LINE–SPOTTING) OF EXISTING PUBLIC UTILITIES AND EXISTING PRIVATE UTILITIES OWNED AND OPERATED BY ALBUQUERQUE PUBLIC SCHOOLS.
3. PRIOR TO CONSTRUCTION, THE CONTRACTOR SHALL EXCAVATE AND VERIFY THE HORIZONTAL AND VERTICAL LOCATION OF ALL POTENTIAL OBSTRUCTIONS. SHOULD A CONFLICT EXIST, THE CONTRACTOR SHALL NOTIFY THE ENGINEER IN WRITING SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM AMOUNT OF DELAY. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL INTERPRETATIONS IT MAKES WITHOUT FIRST CONTACTING THE ENGINEER AS REQUIRED ABOVE.
4. ALL WORK ON THIS PROJECT SHALL BE PERFORMED IN ACCORDANCE WITH APPLICABLE FEDERAL, STATE AND LOCAL LAWS, RULES AND REGULATIONS CONCERNING CONSTRUCTION SAFETY AND HEALTH.
5. ALL CONSTRUCTION WITHIN PUBLIC RIGHT–OF–WAY SHALL BE PERFORMED IN ACCORDANCE WITH APPLICABLE CITY OF ALBUQUERQUE STANDARDS AND PROCEDURES.
6. IF ANY UTILITY LINES, PIPELINES, OR UNDERGROUND UTILITY LINES ARE SHOWN ON THESE DRAWINGS, THEY ARE SHOWN IN AN APPROXIMATE MANNER ONLY, AND SUCH LINES MAY EXIST WHERE NONE ARE SHOWN. IF ANY SUCH EXISTING LINES ARE SHOWN, THE LOCATION IS BASED UPON INFORMATION PROVIDED BY THE OWNER OF SAID UTILITY, AND THE INFORMATION MAY BE INCOMPLETE, OR MAY BE OBSOLETE BY THE TIME CONSTRUCTION COMMENCES. THE ENGINEER HAS CONDUCTED ONLY PRELIMINARY INVESTIGATION OF THE LOCATION, DEPTH, SIZE, OR TYPE OF EXISTING UTILITY LINES, PIPELINES, OR UNDERGROUND UTILITY LINES. THIS INVESTIGATION IS NOT CONCLUSIVE, AND MAY NOT BE COMPLETE, THEREFORE, MAKES NO REPRESENTATION PERTAINING THERETO, AND ASSUMES NO RESPONSIBILITY OR LIABILITY THEREFOR. THE CONTRACTOR SHALL INFORM ITSELF OF THE LOCATION OF ANY UTILITY LINE, PIPELINE, OR UNDERGROUND UTILITY LINE IN OR NEAR THE AREA OF THE WORK IN ADVANCE OF AND DURING EXCAVATION WORK. THE CONTRACTOR IS FULLY RESPONSIBLE FOR ANY AND ALL DAMAGE CAUSED BY ITS FAILURE TO LOCATE, IDENTIFY AND PRESERVE ANY AND ALL EXISTING UTILITIES, PIPELINES, AND UNDERGROUND UTILITY LINES. IN PLANNING AND CONDUCTING EXCAVATION, THE CONTRACTOR SHALL COMPLY WITH STATE STATUTES, MUNICIPAL AND LOCAL ORDINANCES, RULES AND REGULATIONS, IF ANY, PERTAINING TO THE LOCATION OF THESE LINES AND FACILITIES.
7. THE DESIGN OF PLANTERS AND LANDSCAPED AREAS IS NOT PART OF THIS PLAN. ALL PLANTERS AND LANDSCAPED AREAS ADJACENT TO THE BUILDING(S) SHALL BE PROVIDED WITH POSITIVE DRAINAGE TO AVOID ANY PONDING ADJACENT TO THE STRUCTURE. FOR CONSTRUCTION DETAILS, REFER TO LANDSCAPING PLANS.

EROSION CONTROL MEASURES:

1. THE CONTRACTOR SHALL ENSURE THAT NO SOIL ERODES FROM THE SITE INTO PUBLIC RIGHT–OF–WAY OR ONTO PRIVATE PROPERTY.
2. THE CONTRACTOR SHALL PROMPTLY CLEAN UP ANY MATERIAL EXCAVATED WITHIN THE PUBLIC RIGHT–OF–WAY SO THAT THE EXCAVATED MATERIAL IS NOT SUSCEPTIBLE TO BEING WASHED DOWN THE STREET.
3. CONTRACTOR SHALL SECURE "TOPSOIL DISTURBANCE PERMIT" FROM THE CITY PREPARE A SWPPP, AND FILE A NOTICE OF INTENT (N.O.I.) ON BEHALF OF THEMSELVES AND THE OWNER WITH THE EPA PRIOR TO BEGINNING CONSTRUCTION.

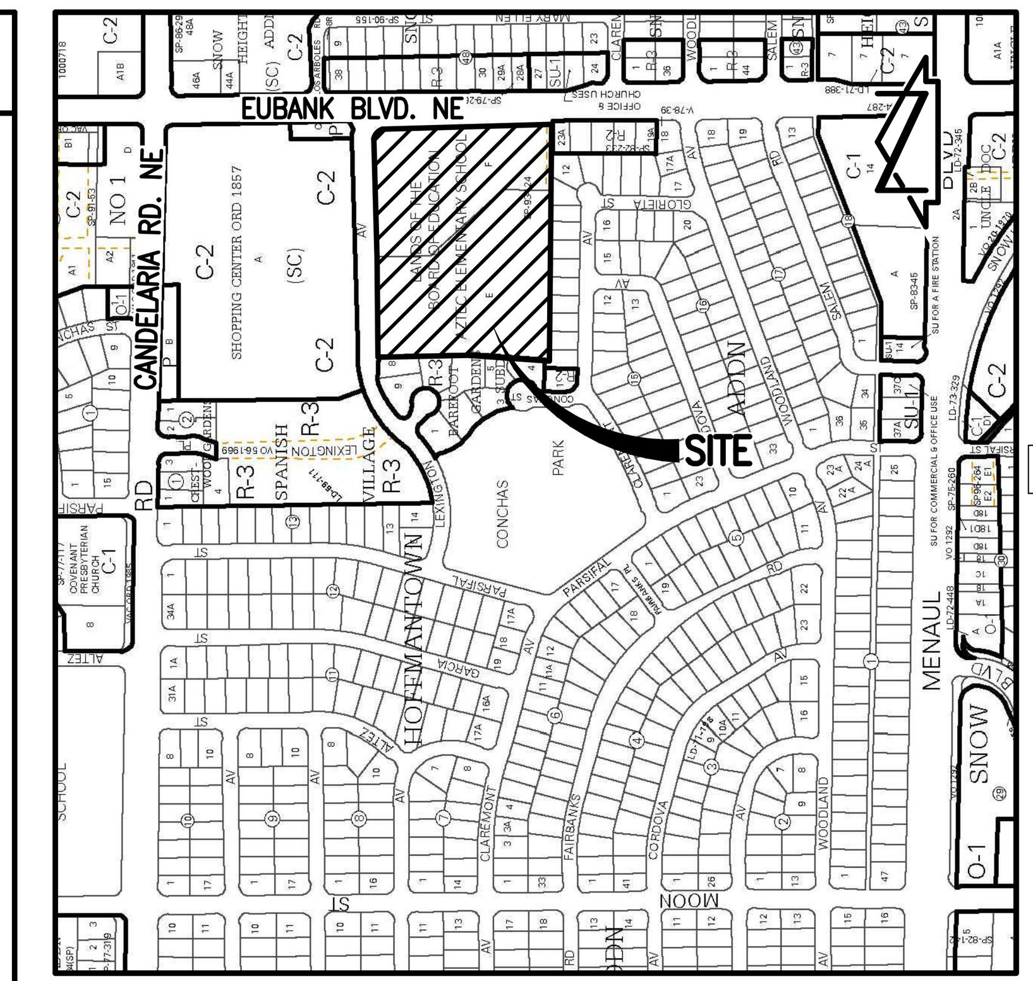
LEGEND

ACD AIR CONDITIONER DRAIN
AIR AIR CONDITIONER
AP ASPHALT PATH
AS ASPHALT SLOPE
ASPH ASPHALT
BBG BASKETBALL GOAL
BOH BUILDING OVERHANG
BOL BOLLARD
BPW BRICK PLANTER WALL
BR BIKE RACK
BW BRICK WALL
C&G CURB AND GUTTER
C/PM COMMUNICATION LINE BY PAINT MARK
CAM SECURITY CAMERA
CAP IRON PIPE, CAPPED
CC CONCRETE CURB
CCAB COMMUNICATION CABINET
CCND COMMUNICATION CONDUIT
CDP CONCRETE DRIVE PAD
CF LANDSCAPING CRUSHER FINES
CI CAST IRON PIPE
CLD CENTERLINE DOOR
CLDD CENTERLINE DOUBLE DOOR
CLF CHAIN LINK FENCE
CMH COMMUNICATION MANHOLE
CMR COMMUNICATION RISER
CMS CONCRETE MOW STRIP
CMU CONCRETE BLOCK WALL
CND ELECTRIC CONDUIT
CO CLEANOUT
CONC CONCRETE
CP CONCRETE PIPE
CPW CONCRETE PLANTER WALL
CR CONCRETE RAMP
CS CONCRETE STEP
CSW CONCRETE SIDEWALK
CTC CONCRETE TRASH CAN
CV COMMUNICATION VAULT
CVC COVERED CONCRETE
CW CONCRETE WALL
CWK COVERED CONCRETE WALKWAY
DCO DOUBLE CLEANOUT
DGT DOUBLE GATE
DPG DOUBLE PIPE GATE
E/PM ELECTRIC LINE BY PAINT MARK
EA EDGE OF ASPHALT
EBB ELECTRIC BREAKER BOX
EM ELECTRIC METER
EP ELECTRIC PANEL
EPB ELECTRIC PULLBOX
EXH BUILDING EXHAUST UNIT
FH FIRE HYDRANT
FL FLOWLINE
FP FLAG POLE
G/PM GAS LINE BY PAINT MARK
GA GUY WIRE ANCHOR
GLR GAS LINE TO ROOF
GM GAS METER
GPR GAS PRESSURE RELIEF VALVE
GRV GRAVEL
GS GAS SERVICE
GT GATE
GTS GATE STOP POST
GVB GAS VALVE BOX
HCP HANDICAPPED PARKING SIGN
HDPE HIGH–DENSITY POLYETHYLENE
HT HEATER
ICB IRRIGATION CONTROL BOX
INV PIPE INVERT
IVB IRRIGATION VALVE BOX
MB METAL BENCH
MBC METAL BUILDING COLUMN
MC METER CAN WITH HOSE BIB
MH MANHOLE
MHR METAL HAND RAIL
MLN METAL LANDING
MLP METAL LIGHT POLE
MPP METAL POWER POLE
MR METAL RAMP
MS METAL STEPS
MSD METAL SHED
MTC METAL TRASH CAN
MTS METAL SIGN
OHC(1) OVERHEAD COMMUNICATION (# OF LINES)
OHE(1) OVERHEAD ELECTRIC (# OF LINES)

OHM OVERHEAD UTILITY MAST
PB CONCRETE WHEEL STOP
PE PLAYGROUND EQUIPMENT
PI PAINTED PARKING LOT ISLAND
PLT PLANTER
PS PAINTED PARKING STRIPE
PT PICNIC TABLE
RD ROOF DRAIN
ROW ROW OF CONCRETE WHEEL STOPS
RRT LANDSCAPING RAILROAD TIES
SAS SANITARY SEWER
SAS/PM SANITARY SEWER LINE BY PAINT MARK
SB SIGN BASE
SD STORM DRAIN
SDI STORM DRAIN INLET
SDP SERVICE DROP POLE
SH ASPHALT SPEED HUMP
STD STANDARD
SW CONCRETE SIDEWALK
SWC SIDEWALK CULVERT
TA TOP OF ASPHALT
TC TOP OF CURB
TCO TOP OF CONCRETE
TG TOP OF GRATE
TRN ELECTRIC TRANSFORMER
TS TRAFFIC SIGN
TW TOP OF WALL
TYP TYPICAL
VCP VITRIFIED CLAY PIPE
VP VENT PIPE
W/PM WATER LINE BY PAINT MARK
WCR CONCRETE WHEELCHAIR RAMP
WDF WOOD FENCE
WDS WOOD STEPS
WF WATER FAUCET
WHB WATER HOT BOX
WL WATER LINE
WLN WOOD LANDING
WLP WOOD LIGHT POLE
WMB WATER METER BOX
WPP WOOD POWER POLE
WPP/SL WOOD POWER POLE WITH STREET LIGHT
WS WOOD SHED
WV WATER VAULT
WVB WATER VALVE BOX
* PAINTED UTILITY MARKER
0.5'ø TREE TRUNK DIAMETER
DECIDUOUS TREE
SMALL DECIDUOUS TREE
CONIFEROUS TREE
SMALL GROUP OF TREES
SHRUB
SMALL SHRUB
HANDICAPPED PARKING SPACE

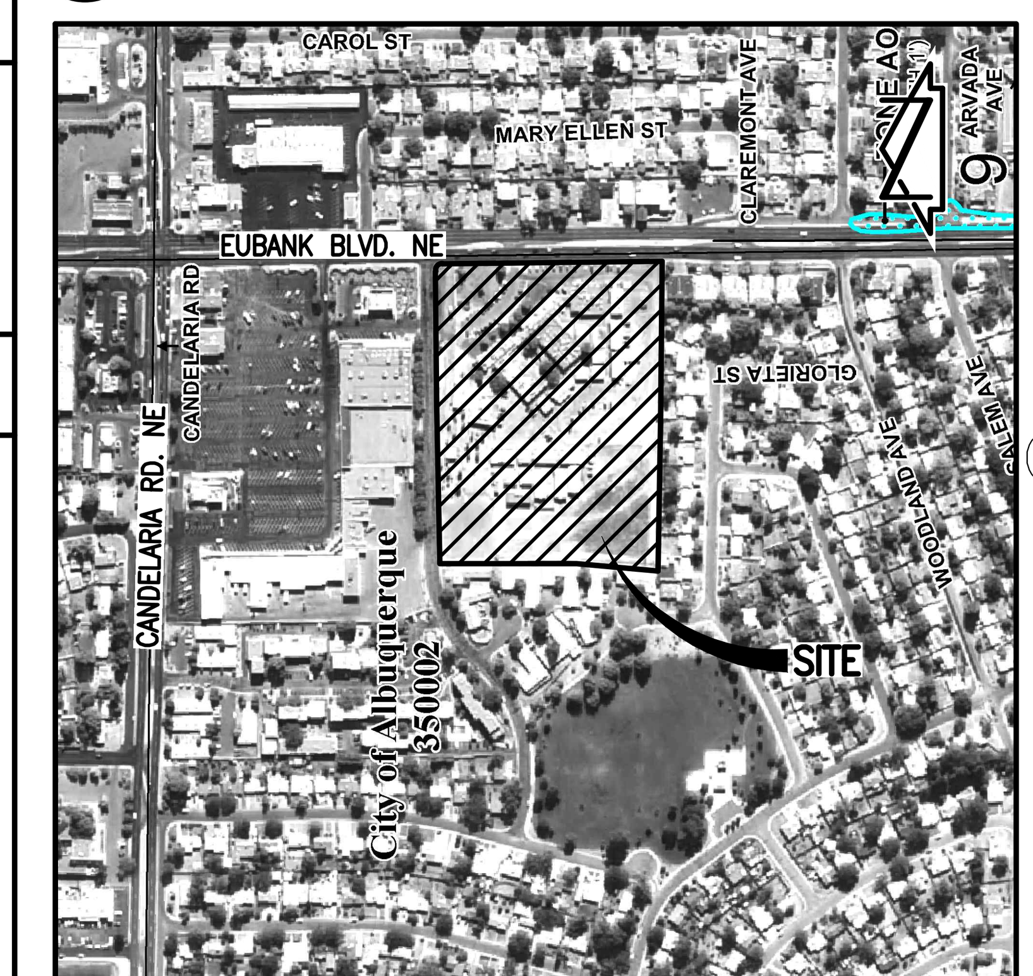
INV INVERT
TA TOP OF ASPHALT PAVEMENT
TC TOP OF CURB
TG TOP OF GRATE
+ 488.70 EXISTING SPOT ELEVATION
85.00 PROPOSED SPOT ELEVATION
EXISTING FLOWLINE
PROPOSED FLOWLINE
5495 EXISTING CONTOUR
95 PROPOSED CONTOUR
EXISTING DIRECTION OF FLOW
PROPOSED DIRECTION OF FLOW
RIGHT OF WAY LINE
PUBLIC EASEMENT LINE
SD PROPOSED STORM DRAIN
PROPOSED INFILTRATION PIT
PROPOSED STORM INLET
PROPOSED STORM DRAIN MANHOLE
EXISTING STORM DRAIN MANHOLE
EXISTING FIRE HYDRANT
PROPOSED FIRE HYDRANT
FIRE DEPARTMENT CONNECTION
EXISTING SAS MANHOLE
PROPOSED SAS MANHOLE
EXISTING VALVE BOX
PROPOSED VALVE BOX
EXISTING DOUBLE CLEANOUT
PROPOSED DOUBLE CLEANOUT
EXISTING SINGLE CLEANOUT
PROPOSED SINGLE CLEANOUT
EXISTING WATER SERVICE
PROPOSED WATER SERVICE
W EXISTING WATER LINE
W PROPOSED WATER LINE
SAS EXISTING SANITARY SEWER LINE
SAS PROPOSED SANITARY SEWER LINE
F P EXISTING FIRE LINE
F P PROPOSED FIRE LINE
EXISTING POST INDICATOR VALVE
PROPOSED POST INDICATOR VALVE
HIGH POINT / DIVIDE
RETAINING WALL
PROPOSED CONCRETE
PROPOSED ASPHALT PAVING
STABILIZED CRUSHER FINES

INDEX OF DRAWINGS



D5 VICINITY MAP H–20

SCALE: 1" = 750'



B5 F.I.R.M. PANEL 356 OF 825

SCALE: 1" = 500' DATE 08–16–2012

LEGAL DESCRIPTION

TRACTS E AND F, LANDS OF THE BOARD OF EDUCATION, AZTEC ELEMENTARY SCHOOL, ALBUQUERQUE, NEW MEXICO

BENCHMARKS

PROJECT BENCHMARK

AGRS 3 1/4" ALUMINUM DISC STAMPED "13–H21 1989", SET FLUSH IN THE TOP OF CURB, AT THE NORTH END OF A TRAFFIC ISLAND IN THE NORTHEAST QUADRANT OF THE INTERSECTION OF EUBANK BOULEVARD N.E. AND CLAREMONT AVENUE N.E.
ELEVATION = 5499.574 FEET (NAVD 1988)

TEMPORARY BENCHMARK #1 (T.B.M.)

A MAG NAIL WITH WASHER SET IN ASPHALT AT THE NORTHEASTERN PORTION OF THE CAMPUS, AS SHOWN ON SHEETS CG–100 AND CU–100.
ELEVATION = 5501.63 FEET (NAVD 1988)

TEMPORARY BENCHMARK #2 (T.B.M.)

A MAG NAIL WITH WASHER SET IN ASPHALT AT THE NORTHWESTERN PORTION OF THE CAMPUS, AS SHOWN ON SHEETS CG–100 AND CU–100.
ELEVATION = 5489.20 FEET (NAVD 1988)

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2014.037.1

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04/14/2015
05/01/2015

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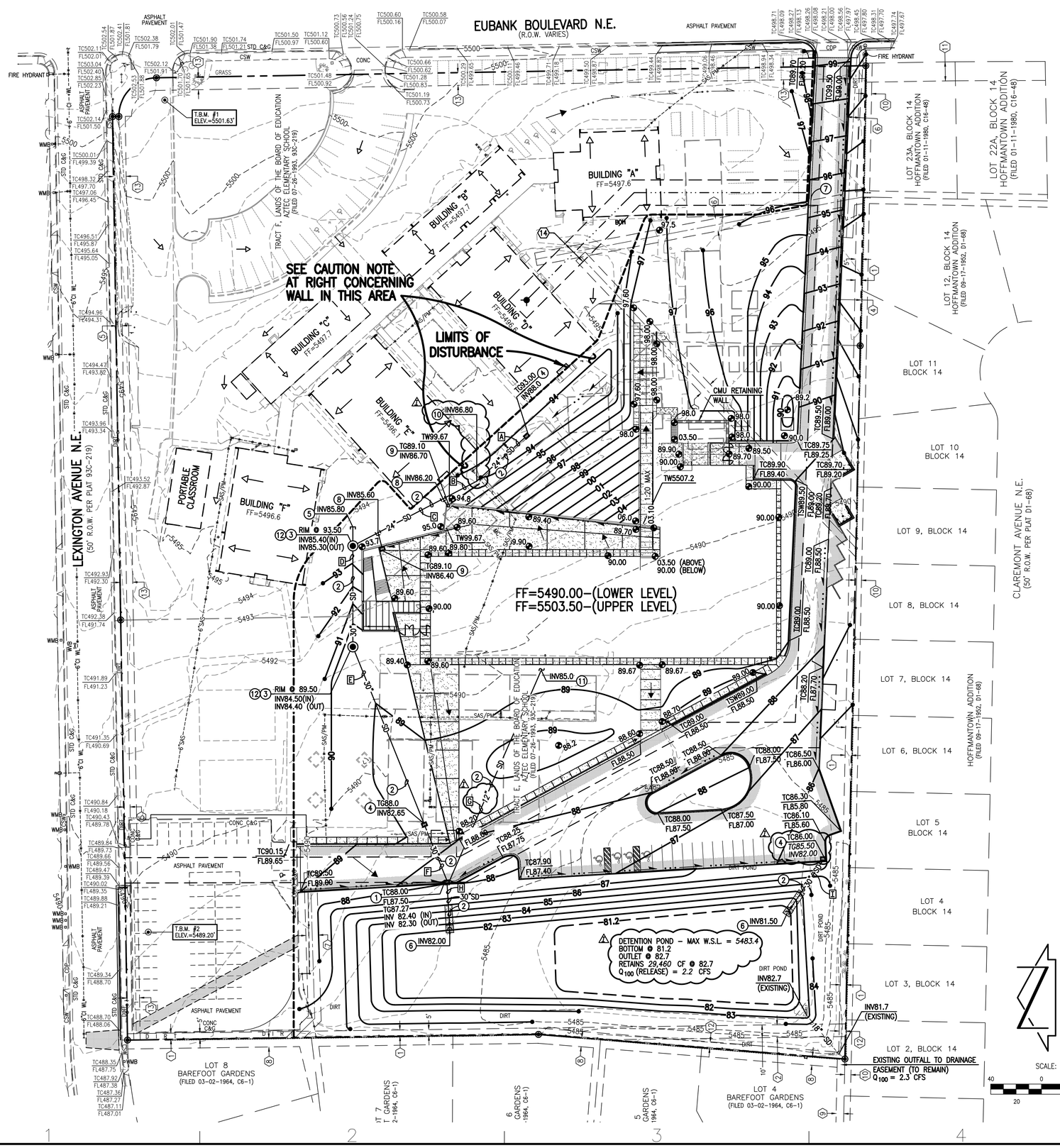
04	4/15/15	100% C.O.A. PERMIT DOCUMENTS
I	5/1/15	ADDENDUM I

PROJECT NO: 2014.037.1
CAD DWG FILE:
DRAWN BY: J.Y.R.
CHK'D BY: G.M.

SHEET TITLE

DRAINAGE PLAN & CALCULATIONS

C-001



EASEMENT KEYED NOTES

EASEMENTS

1. 5' UTILITY EASEMENT GRANTED BY PLAT C5-45 TO REMAIN
2. 10' UTILITY EASEMENT GRANTED BY PLAT C5-45 TO BE VACATED
3. 10' PNM AND MST&T EASEMENT GRANTED BY DOCUMENT EXECUTED 03-01-1957 TO BE VACATED
4. 20' PUBLIC WATER LINE EASEMENT GRANTED BY PLAT 93C-219 TO BE VACATED
5. 5' PUBLIC ROADWAY EASEMENT GRANTED BY PLAT 93C-219 TO BE VACATED
6. 10' PNM AND US WEST COMMUNICATIONS, INC. EASEMENT GRANTED BY DOCUMENT EXECUTED 07-26-1995 TO BE VACATED
7. 10' PNM EASEMENT GRANTED BY DOCUMENT FILED 06-30-2011, DOC. #2011060938 TO BE VACATED

EASEMENTS - OFFSITE

8. 5' UTILITY EASEMENT GRANTED BY PLAT C6-1
9. 6' DRAINAGE AND UTILITY EASEMENT GRANTED BY PLAT C6-1
10. 5' UTILITY EASEMENT GRANTED BY PLAT D1-68
11. 15' UTILITY EASEMENT GRANTED BY PLAT B20-18

NEW EASEMENTS

12. 5' PUBLIC UTILITY EASEMENT TO BE GRANTED BY FORTHCOMING PLATTING ACTION
13. 10' PUBLIC UTILITY EASEMENT TO BE GRANTED BY FORTHCOMING PLATTING ACTION
14. ABCWUA WATER LINE EASEMENT TO BE GRANTED BY FORTHCOMING PLATTING ACTION
15. PNM EASEMENT TO BE GRANTED BY FORTHCOMING PLATTING ACTION

DOCUMENTARY EASEMENT

NON-SPECIFIC EASEMENT FOR RIGHT-OF-WAY FOR COMMUNICATIONS GRANTED BY DOCUMENT FILED 08-05-1937, BOOK 152, PAGE 133 TO BE VACATED

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LEGEND

- PROPOSED CONCRETE
- PROPOSED ASPHALT PAVING
- B — STORM DRAIN IDENTIFICATION (SEE SHEET C-001 FOR HYDRAULICS)

KEYED NOTES

1. CONSTRUCT TYPE "C" INLET PER STD DWG 2205, SHEET CG-501
2. INSTALL HDPE STORM DRAIN (ADS N-12), SIZE AS NOTED.
3. CONSTRUCT 4' DIAMETER STORM DRAIN MANHOLE PER STD DWG 2101, SHEET CG-502
4. CONSTRUCT TYPE "D" INLET PER STD DWG 2206, SHEET CG-501
5. INSTALL 10 LF 24" HDPE STUB TO EAST WITH PLUG
6. CONSTRUCT POND OUTLET WITH 6'x6' COBBLE SPLASH PAD SHEET CG-501
7. CONSTRUCT 2 FT CURB OPENING FOR RUNOFF FROM PAVEMENT TO FLOW TO NEW DRIVE. INV @ FL95.50
8. INSTALL 24"x24"x18" HDPE TEE, EXTEND 18" HDPE TO INLET
9. CONSTRUCT 24"x24"STORM INLET PER TYPICAL SECTION, SHEET CG-501
10. INSTALL HDPE BEND
11. CONNECT TO BUILDING ROOF DRAIN
12. PROVIDE VENTED LID PER STD DWG 2110, SHEET CG-502

SURVEY NOTE

THIS IS NOT A BOUNDARY SURVEY; DATA IS SHOWN FOR ORIENTATION ONLY. THE BOUNDARY INFORMATION DEPICTED BY THIS PLAN IS BASED UPON AN UNRECORDED BOUNDARY SURVEY PREPARED BY HIGH MESA CONSULTING GROUP, NMPS 11184, DATED 11-18-2013 (2013.181.9). THE TOPOGRAPHIC INFORMATION DEPICTED HEREON IS BASED UPON THE TOPOGRAPHIC AND UTILITY SURVEY PREPARED BY HIGH MESA CONSULTING GROUP, NMPS NO. 11184, DATED 11-18-2013 (2013.181.9).

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1	*5/1/15*	*ADDENDUM 1*

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SHEET TITLE

OVERALL GRADING PLAN - BASE BID

CG-100

J. GRAEME MEANS — NEW MEXICO — 13676 — REGISTERED PROFESSIONAL ENGINEER

05/01/2015

HIGH MESA Consulting Group
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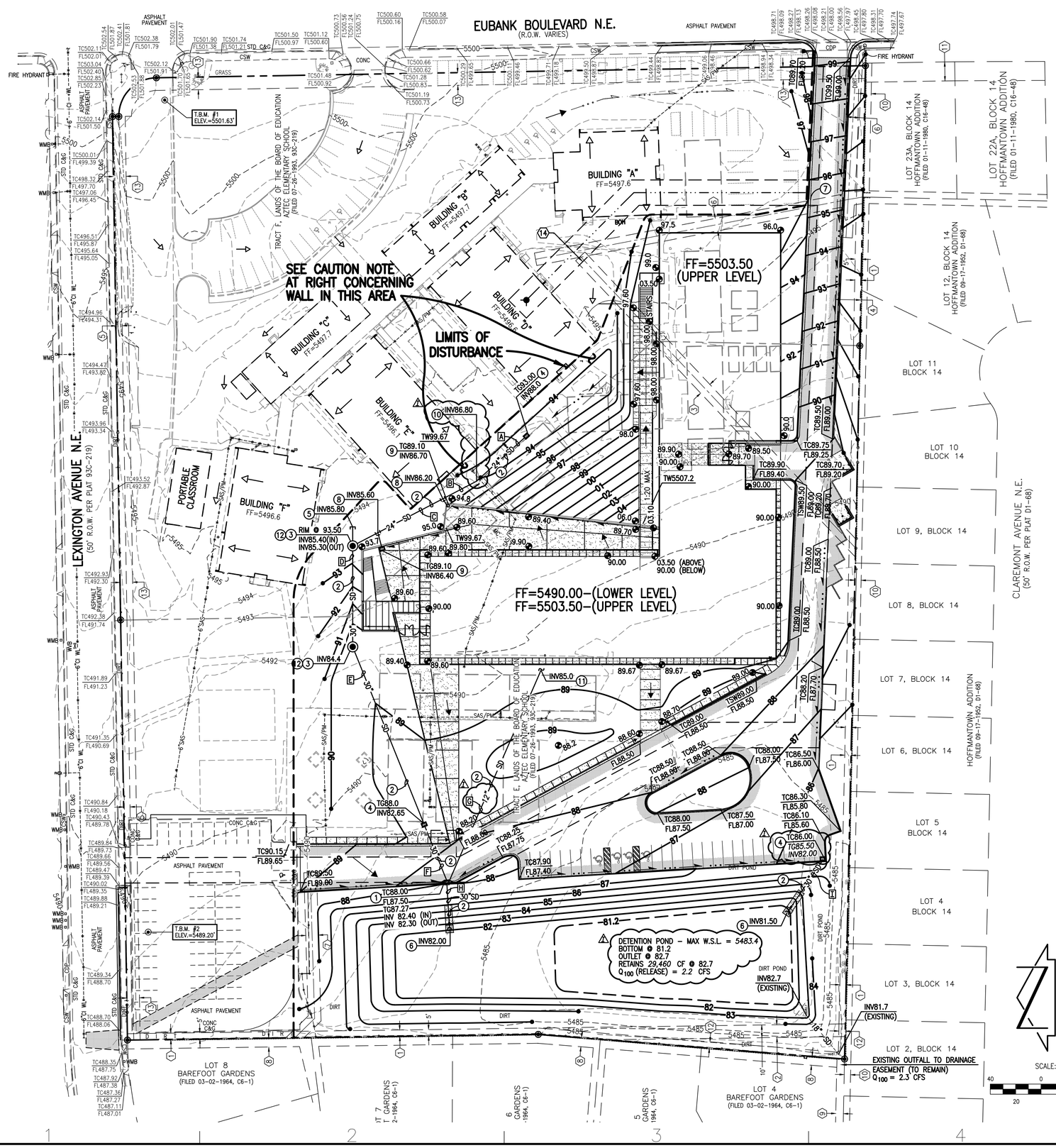

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- PROPOSED ASPHALT PAVING
- B — STORM DRAIN IDENTIFICATION (SEE SHEET C-001 FOR HYDRAULICS)

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10. INSTALL HDPE BEND
11. CONNECT TO BUILDING ROOF DRAIN
12. PROVIDE VENTED LID PER STD DWG 2110, SHEET CG-502

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04	4/15/15	100% C.O.A. PERMIT DOCUMENTS
1	5/1/15	ADDENDUM 1

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CAD DWG FILE:	
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SHEET TITLE

OVERALL GRADING (IF BID LOT 4 IS TAKEN)

CG-101

J. GRAEME MEANS NEW MEXICO 13676 REGISTERED PROFESSIONAL ENGINEER
04/30/2015
05/01/2015

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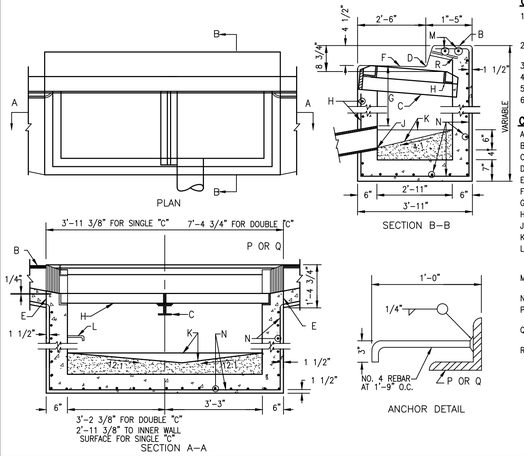


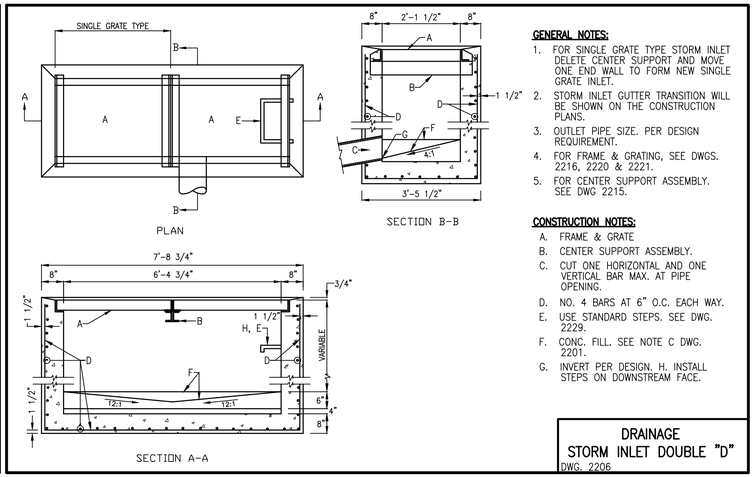
GENERAL NOTES:

1. FOR SINGLE GRATE TYPE STORM INLET DELETE CENTER SUPPORT AND MOVE ONE END WALL TO FORM NEW SINGLE GRATE INLET.
2. FOR STORM INLET GUTTER TRANSITION, SEE DWG 2207.
3. OUTLET PIPE SIZE. PER DESIGN REQUIREMENT.
4. FOR FRAME & GRATING, SEE DWGS. 2216 AND 2220.
5. FOR ANCHOR SEE DETAIL.
6. FOR CENTER SUPPORT ASSEMBLY. SEE DWG 2215.

CONSTRUCTION NOTES:

A. GUTTER TRANSITION.
B. TOP OF CURB.
C. CENTER SUPPORT ASSEMBLY.
D. FLOWLINE.
E. CONSTRUCTION JOINT.
F. NORMAL GUTTER LINE.
G. 1'-10" MIN. UNLESS OTHERWISE DIRECTED.
H. FRAME AND GRATE.
J. INVERT OF OUTLET PIPE.
K. CONCRETE FILL. MINIMUM SLOPES AS SHOWN.
L. FOR STORM INLET DEPTHS GREATER THAN 4' INSTALL STD STEPS. SEE DWG. 2229, DOWNSTREAM FACE.
M. EXTEND NO. 4 REBARS 18" INTO CURB ON EACH SIDE OF STORM INLET.
N. NO. 4 BARS AT 6" O.C.
P. 3 1/2" X 3 1/2" X 1/2" X 4'-0" FOR SINGLE GRATE TYPE "C" STORM INLET.
Q. 3 1/2" X 3 1/2" X 1/2" X 7'-6" FOR DOUBLE GRATE TYPE "C" STORM INLET.
R. ANCHOR.

DRAINAGE
STORM INLET TYPE "C"
DWG. 2205



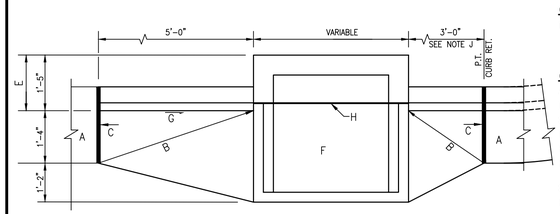
GENERAL NOTES:

1. FOR SINGLE GRATE TYPE STORM INLET DELETE CENTER SUPPORT AND MOVE ONE END WALL TO FORM NEW SINGLE GRATE INLET.
2. STORM INLET GUTTER TRANSITION WILL BE SHOWN ON THE CONSTRUCTION PLANS.
3. OUTLET PIPE SIZE. PER DESIGN REQUIREMENT.
4. FOR FRAME & GRATING, SEE DWGS. 2216, 2220 & 2221.
5. FOR CENTER SUPPORT ASSEMBLY. SEE DWG 2215.

CONSTRUCTION NOTES:

A. FRAME & GRATE
B. CENTER SUPPORT ASSEMBLY.
C. CUT ONE HORIZONTAL AND ONE VERTICAL BAR MAX. AT PIPE OPENING.
D. NO. 4 BARS AT 6" O.C. EACH WAY.
E. USE STANDARD STEPS. SEE DWG. 2229.
F. CONC. FILL. SEE NOTE C DWG. 2201.
G. INVERT PER DESIGN. H. INSTALL STEPS ON DOWNSTREAM FACE.

DRAINAGE
STORM INLET DOUBLE "D"
DWG. 2206



GENERAL NOTES:

1. DETAILS FOR PLACING CATCH BASINS, STANDARD CURB AND GUTTER.

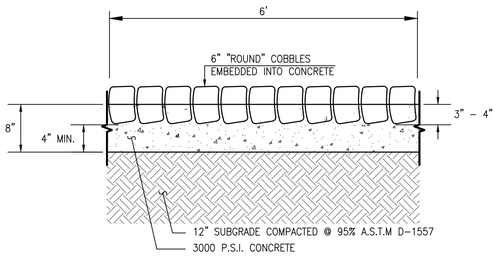
CONSTRUCTION NOTES:

A. STANDARD CURB AND GUTTER
B. STRAIGHT GRADE.
C. EXPANSION JOINT.
D. TOP OF CURB.
E. FLOWLINE.
F. FOR FRAME & GRATE SEE DWGS. 2216 & 2220.
G. DIRECTION OF FLOW.
H. POINT OF MEASUREMENT FOR TOP OF GRATE ELEVATION.
J. PROVIDE 10 FEET TRANSITION EACH SIDE OF CATCH BASIN, WHEN INSTALLING AT SAG POINT IN INSTALLATIONS OTHER THAN AT CURB RETURN.

DRAINAGE
STORM INLET
GUTTER TRANSITION
DWG. 2207

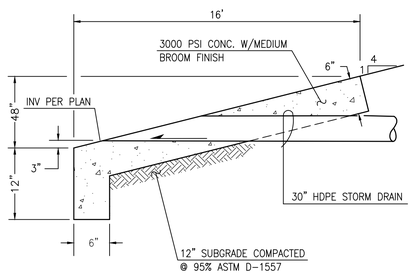
POND INLET / OUTLET PLAN

N.T.S.



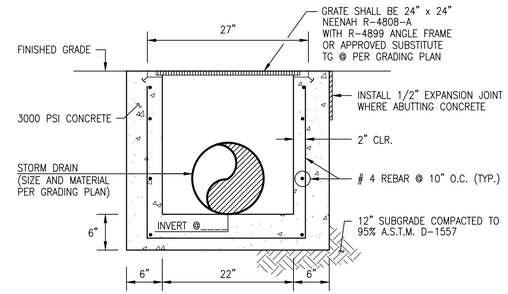
TYPICAL COBBLE SPLASH PAD SECTION

N.T.S.



POND INLET / OUTLET SECTION

N.T.S.



TYPICAL 24"X24" STORM INLET SECTION

SCALE: 1" = 1' - 0"

HDPE PIPE CONSTRUCTION NOTES:

1. HDPE PIPE AND FITTINGS SHALL MEET THE REQUIREMENTS OF AASHTO M 294 TYPE S FOR HDPE STORM DRAIN SYSTEMS.
2. JOINTS SHALL BE WATERTIGHT IN ACCORDANCE WITH THE REQUIREMENTS OF ASTM D3212. THE SPIGOTS SHALL HAVE O-RING GASKETS MEETING THE REQUIREMENTS OF ASTM F 477.
3. THE CONTRACTOR'S PROJECT SUPERINTENDENT AND FOREMAN OF THE PIPE-LAYING CREW SHALL SUBMIT TO THE OWNER A CERTIFICATE INDICATING COMPLETION OF AN ON-LINE TRAINING PROGRAM OFFERED BY ADS (ADS-PIPE.COM) OR OTHER MANUFACTURER AS APPROVED BY THE OWNER.
4. INSTALLATION SHALL BE IN ACCORDANCE WITH THE PIPE MANUFACTURER'S RECOMMENDATIONS.
5. ALL EXCAVATION SHALL BE IN ACCORDANCE WITH THE REQUIREMENTS OF SECTION 701 OF THE NMAPWA STANDARD SPECIFICATIONS.
6. THE PIPE SHALL BE BEDDED IN A FOUNDATION OF COMPACTED GRANULAR MATERIAL THAT IS FREE OF ORGANIC MATTER, CLAY LUMPS, AND OTHER DELETERIOUS MATTER. THIS MATERIAL SHALL EXTEND A MINIMUM OF 6 INCHES BELOW THE OUTERMOST CORRUGATIONS AND BE USED FOR BACKFILL UP TO A MINIMUM OF 1 FOOT ABOVE THE TOP OF PIPE. UNTIL A MINIMUM COVER OF 1 FOOT IS ATTAINED, ONLY HAND OPERATED TAMPING EQUIPMENT MAY BE USED IN THE TRENCH PRISM OVER THE PIPE.
7. CONCRETE STRUCTURE CONNECTIONS FOR HDPE PIPE WILL REQUIRE THE USE OF A WATER STOP THAT MEETS THE PHYSICAL PROPERTIES OF ASTM C923. INSTALLATION SHALL BE PER MANUFACTURER'S SPECIFICATIONS.

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CAD DWG FILE:
DRAWN BY: J.Y.R.
CHK'D BY: G.M.

SHEET TITLE

GRADING AND DRAINAGE SECTIONS AND DETAILS

CG-501

J. GRAEME MEANS NEW MEXICO 13676 REGISTERED PROFESSIONAL ENGINEER
04/14/2015

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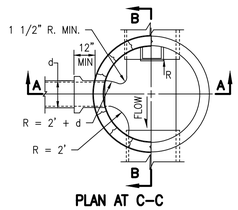

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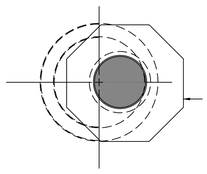

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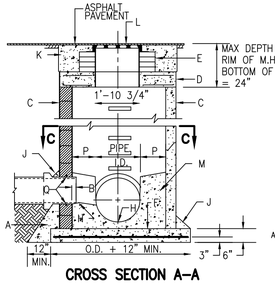
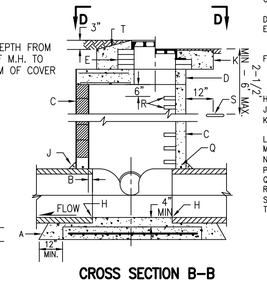


CROSS SECTION A–A



CROSS SECTION B–B

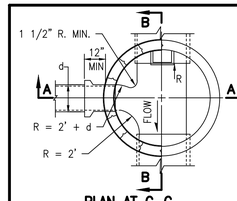
GENERAL NOTES

1. USE TYPE "C" MANHOLE FOR DEPTHS OF LESS THAN 6' MEASURED FROM INVERT TO RIM.
2. CONTRACTOR HAS OPTION TO CONSTRUCT TYPE "C" MANHOLE IN LIEU OF TYPE "E" MANHOLE FOR DEPTHS OF 6' OR MORE.
3. DESIGN APPLIES TO 4' TO 6' I.D. MANHOLES.
4. MANHOLE GREATER THAN 18' IN DEPTH SHALL ONLY BE CONSTRUCTION PRECAST CONCRETE SECTIONS.
5. USE NON–SHRINK GROUT FOR JOINTS, FILLETS AND PENETRATIONS.
6. COMPACT ALL BACKFILL AROUND MANHOLE TO 95%.
7. POSITION MANHOLE OPENING OVER THE UPSTREAM SIDE OF MAIN LINE.

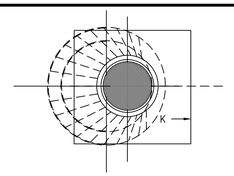
CONSTRUCTION NOTES

A. CONCRETE PIPE SUPPORTS SHALL EXTEND OUTSIDE OF MANHOLE TO BELL OF FIRST JOINT AND SHALL CRADLE PIPE TO SPRING LINE NOT APPLICABLE FLEXIBLE PIPE.
B. PIPE PENETRATION INTO MANHOLE SHALL BE FLUSH TO 2" MAX., MEASURED AT SPRINGLINE OF PIPE.
C. MANHOLE MAY BE CONSTRUCTED OF POURED CONCRETE OR PRECAST REINFORCED CONCRETE (7" MINIMUM WALL THICKNESS).
D. PRECAST CONCRETE COVER, SEE DWG. 2107.
E. USE MAX., 4 COURSES GR. MS BRICK ON UNPAVED STREET FOR FUTURE ADJUSTMENT OF MANHOLE FRAME TO PAVEMENT GRADE PLASTER INSIDE WITH 1/2" MORTAR.
F. CONCRETE BASE TO BE POURED IN PLACE USING NO. 4 BARS AT 6" O.C. EA. WAY FOR MANHOLE DEPTH OF 16' OR GREATER. NO. 4 BARS AT 12" O.C. EA. WAY FOR MANHOLE LESS THAN 16' IN DEPTH.
H. INVERT ELEVATION OF STUB OR LATERAL AS SHOWN ON PLANS.
J. 6" GROUT FILLET ON UPPER HALF OF PIPE AND AROUND BASE.
K. USE A 5' X 5' x 6" THICK OCTAGONAL CONCRETE PAD IN A ALL AREAS.
L. FRAME AND COVER, SEE DWG. 2110.
M. CONCRETE, SEE NMAPWA SPEC. SECTION 101.
N. SLOPE 1" PER FT. FROM PIPE CROWN.
P. SHELF TO BE 9" WIDE MIN.
Q. APPROVED WATERSTOP TO BE COMPATIBLE WITH TYPE OF PIPE.
R. STEPS TO BE INSTALLED AS PER NMAPWA SPEC. SECTION 920.
S. EMD (IN UNPAVED AREAS).
T. IN UNPAVED AREAS SET FRAME TO GRADE AND SLOPE OF PAD.

STORM
MANHOLE TYPE "C"
DWG. 2101



PLAN AT C–C



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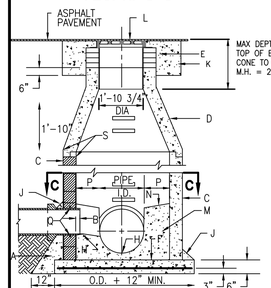
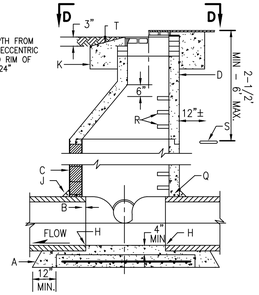


CROSS SECTION A–A



CROSS SECTION B–B

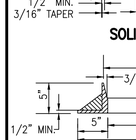
GENERAL NOTES:

1. TYPE "E" MANHOLE NOT TO BE USED FOR DEPTHS LESS THAN 6' MEASURED FROM INVERT TO RIM.
2. MANHOLE GREATER THAN 18' IN DEPTH SHALL BE OF PRECAST CONCRETE SECTIONS ONLY.
3. DESIGN APPLIES TO 4' TO 6' I.D. MANHOLES.
4. USE NON–SHRINK GROUT FOR JOINTS, FILLETS AND PIPE PENETRATIONS.
5. COMPACT ALL BACKFILL AROUND MANHOLE TO 95%.
6. POSITION MANHOLE OPENING OVER THE UPSTREAM SIDE OF MAIN LINE.

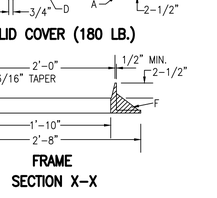
CONSTRUCTION NOTES:

A. CONCRETE PIPE SUPPORTS SHALL EXTEND OUTSIDE OF MANHOLE TO BELL OF FIRST JOINT AND SHALL CRADLE PIPE TO SPRING LINE NOT APPLICABLE FLEXIBLE PIPE.
B. PIPE PENETRATION INTO MANHOLE SHALL BE FLUSH TO 2" MAX., MEASURED AT SPRINGLINE OF PIPE.
C. MANHOLE MAY BE CONSTRUCTED OF POURED CONCRETE OR PRECAST REINFORCED CONCRETE (7" MINIMUM WALL THICKNESS)
D. PRECAST REINFORCED CONCRETE ECCENTRIC CONE. THE CONTRACTOR SHALL PROVIDE SHOP DWGS. FOR APPROVAL.
E. USE MAX., 4 COURSES GR. MS BRICK ON UNPAVED STREET FOR FUTURE ADJ. OF FRAME TO PAVEMENT GRADE PLASTER INSIDE WITH 1/2" MORTAR.
F. BASE TO BE POURED IN PLACE USING NO. 4 BARS AT 6" O.C. EA. WAY FOR MANHOLE DEPTH OF 16" OR GREATER NO. 4 BARS AT 12" O.C. EA. WAY FOR MANHOLE LESS THAN 16' DEEP.
H. INVERT ELEVATION OF STUB OR LATERAL AS SHOWN ON PLANS.
J. 6" GROUT FILLET ON UPPER HALF OF PIPE AND AROUND BASE.
K. USE A 5' X 5' CONCRETE PAD IN ALL AREAS.
L. MANHOLE FRAME AND COVER, SEE DWG. 2110.
M. CONCRETE, SEE NMAPWA SPEC. SECTION 101.
N. SLOPE 1" PER FT. FROM PIPE CROWN.
P. SHELF TO BE 9" WIDE MIN.
Q. APPROVED WATERSTOP TO BE COMPATIBLE WITH TYPE OF PIPE.
R. STEPS TO BE INSTALLED AS PER NMAPWA SPEC. SECTION 920.
S. EMD (IN UNPAVED AREAS).
T. IN UNPAVED AREAS SET FRAME TO GRADE AND SLOPE OF PAD.

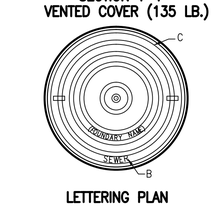
STORM
MANHOLE TYPE "E"
DWG. 2102



4' I.D. MH
SCALE: 1" = 4'



6' I.D. MH
SCALE: 1" = 4'



8' I.D. MH
SCALE: 1" = 4'

GENERAL NOTES

1. ALL MANHOLES 20' DEEP OR DEEPER WILL REQUIRE A INTERMEDIATE LANDING IN THE MANHOLE BARREL. TYPE "C" MANHOLE COVERS SHALL BE USED AS INTERMEDIATE LANDINGS.
2. INTERMEDIATE LANDINGS SHALL BE LOCATED AT THE MID POINT + OR – 2 FEET OF THE HEIGHT OF THE MANHOLE. AT NO TIME SHALL A INTERMEDIATE LANDING OR A SIZE ADJUSTMENT TOP BE INSTALLED CLOSER THAN 8' UP FROM THE INVERT OF THE MANHOLE.

CONSTRUCTION NOTES

A. PRECAST REINFORCED CONCRETE MANHOLE COVER.
B. ALL BARS TO HAVE 1–1/2" MIN. COVER.
C. 1" PIPE SLEEVE VERTICALLY THROUGH COVER.
D. TOP MAT NO. 4 BARS 6" O.C. EA. WAY FOR 4, 6, AND 8 FT. I.D. MANHOLES.
E. NO. 4 BARS.
F. BOTTOM MAT NO. 4 BARS 6" O.C. EA. WAY FOR 4 AND 6 FT. I.D. MANHOLES, NO. 8 BARS 8" O.C. EA. WAY FOR 8 FT. I.D. MANHOLES.
G. NO. 4 BARS FOR 4 AND 6 FT. I.D. MANHOLES.
H. WHEN PRECAST MANHOLE SECTIONS ARE USED, COVER SHALL BE MODIFIED TO SHAPE OF APPROPRIATE SIZE I AND G JOINT.
I. CONCRETE, SEE SECTION 101.

SECTION B–B
SCALE: 1" = 4'
TOP MAT

SECTION C–C
SCALE: 1" = 4'
BOTTOM MAT

SECTION A–A

STORM
CONC. MH COVER TYPE "C"
DWG. 2107

PLAN

PLAN

SOLID COVER (180 LB.)

SECTION Y–Y
VENTED COVER (135 LB.)

FRAME
SECTION X–X

LETTERING PLAN

GENERAL NOTES

1. STANDARD CAST IRON M.H. FRAME AND COVER WEIGHTS: COVER = 180 LBS., FRAME = 145 LBS. TOTAL = 325 LBS. (TOLERANCE = ±5%)
2. REFERENCE SPEC. SECTION 130.

CONSTRUCTION NOTES

A. MACHINED OR GROUND BEARING SURFACES.
B. "SEWER", "WATER", OR "STORM", CAST ON COVER TO IDENTIFY SANITARY SEWER, WATER OR STORM DRAINAGE SYSTEMS RESPECTIVELY.
C. LETTER SIZE TO BE 1" MIN. IN HEIGHT, TYPICAL.
D. VENT HOLE REQUIRED.
E. MONOLITHIC CAST IRON OR STEEL ROD INSERTS AT MANUFACTURER'S OPTION. IF INSERT IS PROVIDED IT MUST HAVE 3/16" MIN. COVER AND 3/4" END EMBEDMENT IN CASTING.
F. GUSSETS OPTIONAL IF REQUIRED BY MANUFACTURER.

MANHOLE FRAME AND COVERS
DWG 2110

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DRAWN BY: J.Y.R.
CHK'D BY: G.M.

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CG-502

J. GRAEME MEANS NEW MEXICO 13676 REGISTERED PROFESSIONAL ENGINEER
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