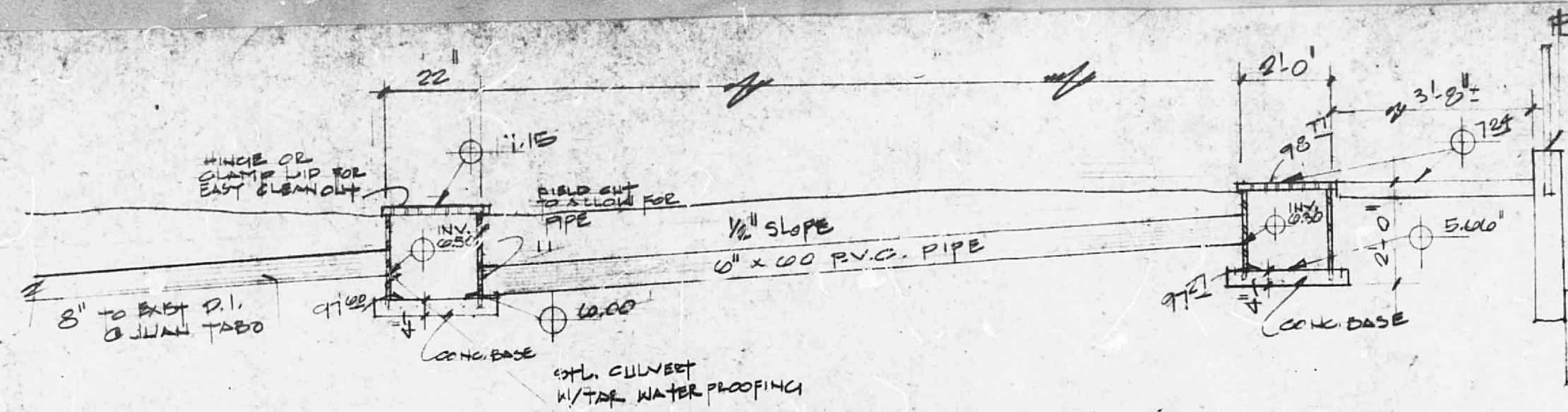


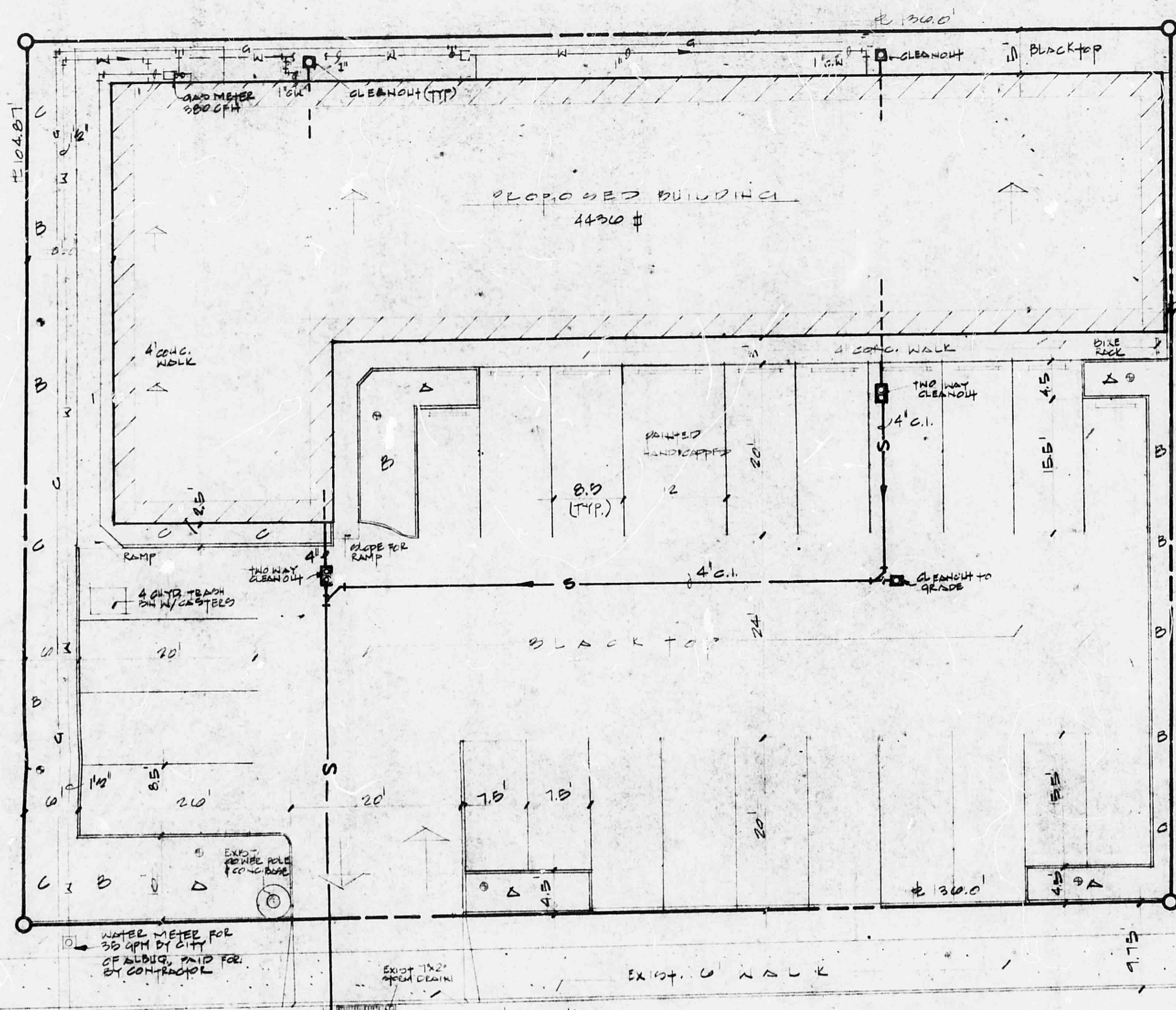
NOTE
OCCUPANCY QPP
BUILDING TYPE
ZONING
ROOF LIVE LOAD
FL. LIVE LOAD
WIND LOADING
CONC. STRENGTH
SOIL PRESSURE
FIRE CODE
RFR PARKING
OPEN AREA

202
V-H
C-1
40 PSI
80 PSI
25 PSI
3000 PSI
2500 PSI
5
25
2404 SQ. FT.



NOTE
ELEVATIONS REFER. TO STATION 2+H21=8002.521
ADD 5000' TO ELEVATIONS ON PLAN
FOR CITY DATA

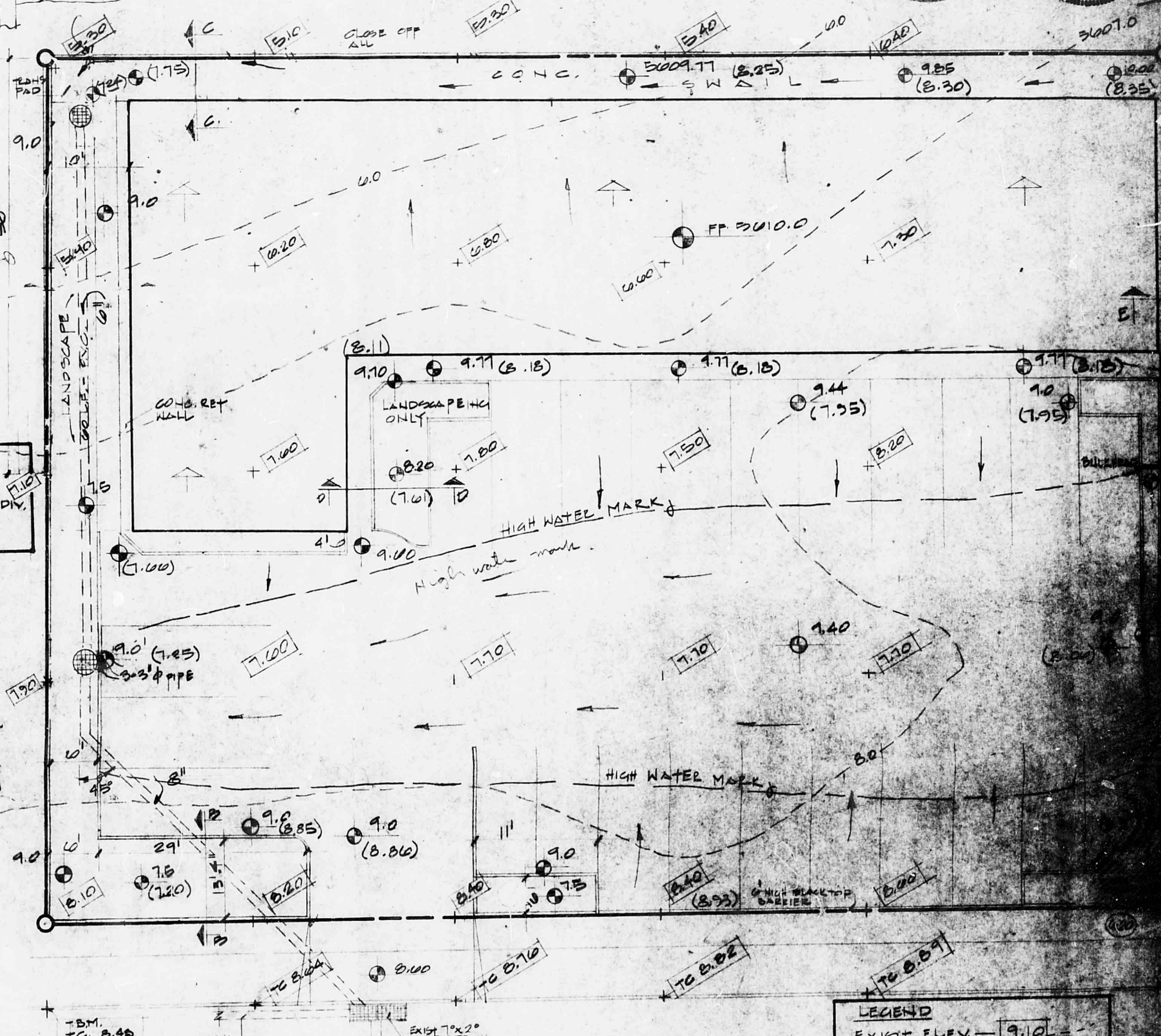
NOTE
1. FILL MATERIAL TO BE ENGINEERED
TO HAVE A P.I. NO GREATER
THAN 8
2. MATERIAL SHALL BE APPROVED
BY ARCHITECT



LEGAL DESCRIPTION
TRACT 42-25
JUAN TABO 2500 SUBDIV.
ALBUQUERQUE, N.M.
ZONE ATLAS H-21

- PLANT LEGEND
- ① PONDROSA PINE
 - ② JUNIPER
 - ③ YUCCA

GROUND COVER SHALL
BE GRASS OR GRAVEL
PLANT SIZES TO MEET
ZONING STAMP SIZES
TREE: 1.5 CAL
SHRUBS: 3 CAL



LEGEND
EXIST. ELEV. — 9.10
FIN. ELEV. — 9.10
SO BUILT GRADES — (1.73)
NEW PROPOSED GRADES — (2.2)

DRAINAGE PLAN

RUNOFF INTO DROP INLET NO. 1 - $C = 0.9$ & 0.40 , $I = 2.40$
ROOF + BACK SIDEWALK - ROOF AREA = 4430 S.F.
VOL. OF RUNOFF FROM ROOF = $4430 \times 2.4 = 887 \text{ FT}^3$
VOL. OF RUNOFF FROM SIDE WALK = $5 \times 130 \times \frac{2.4}{12} = 130 \text{ FT}^3$
TOTAL RUNOFF INTO DI NO. 1 = 1017 FT^3

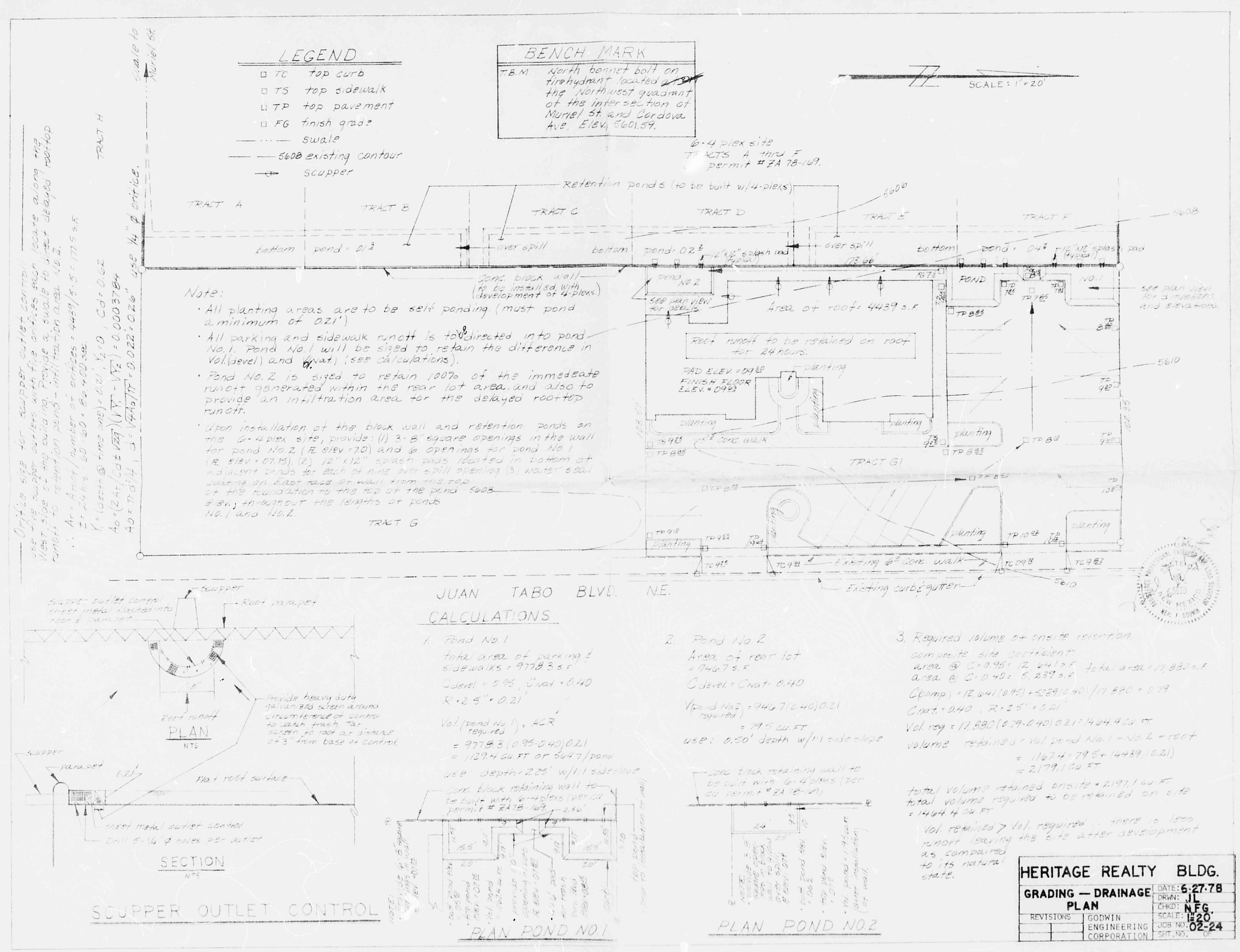
RUNOFF INTO DROP INLET
AREA OF PARKING LOT = $7034 \text{ SF} \times 2.4 \times 0.9 = 2600 \text{ FT}^3$
TOTAL RUNOFF INTO DI, ON JUAN TABO = 2283 FT^3

APPROVED FOR DRAINAGE
DATE 10/21/83
SIGNATURE [Signature]
TITLE
ADVISE DRAINAGE INSPECTOR
WHEN GRADING EXECUTED

SANTA ANNA PLAZA
ALBUQ., N. M.
for MR. and MRS. GIUSEPPE MATONTI



UNITY MAP
H-21-7



LEGEND

- E existing flow line elev.
- TC existing top curb elev.
- TP proposed top pavement elev.
- TS proposed top sidewalk elev.
- FG proposed finished grade elev.
- 5600 existing contour
- swale
- drainage area boundary

TYPICAL LOT AREAS OF USE	
USE	AREA
total	7620 SF
garage	3642 SF
parking	860 SF
sidewalks	508 SF
open landscaped	960 SF

CALCULATIONS

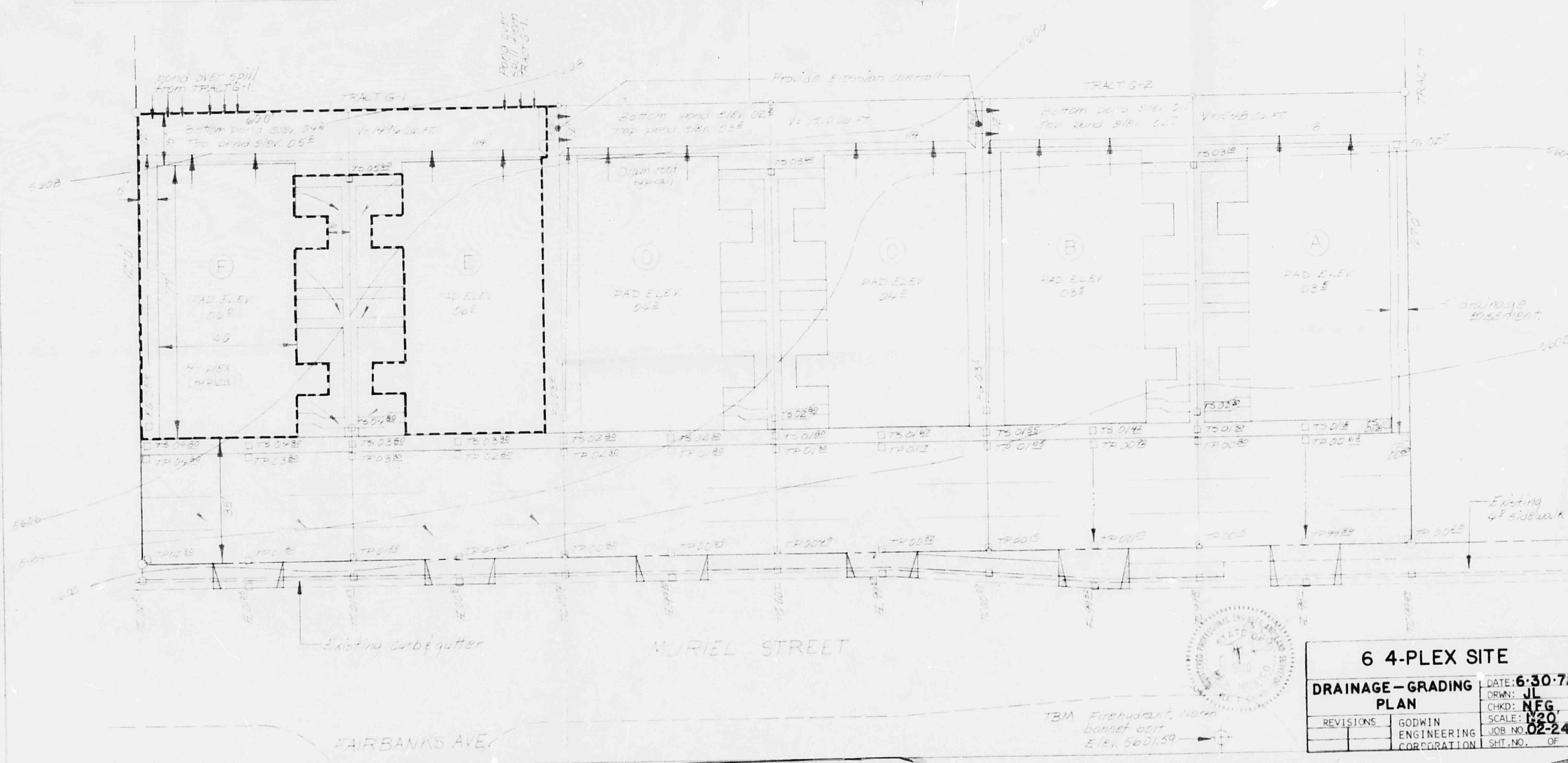
- Composite coefficient
area B $0.25 = 5670$ SF
area C $0.40 = 960$ SF
Comp. $(5670 \times 0.25) + (960 \times 0.40)$
 $= 0.81$
- Required onsite ponding volume per lot
Vol. req. $= 7620 (0.81 - 0.40) 0.20$
using $R = 2.4 \times 0.20$
 $Crat = 0.40$
Vol. req. $= 654$ cu ft.

Using lots 1 & 2 as being typical of the drainage area it is proposed to retain 100% of the runoff within the drainage area. This runoff is to be retained in ponds located at the rear of the site.

- Composite coefficient within the drainage area $= 0.76$
area total 7924 SF
- Required pond volume to retain 100% of runoff within drainage area
 $= 0.76 (7924) 0.20 = 1208$ cu ft.
or 754 per lot if not required on
- Pond configuration
• all side slopes into ponds are 2:1 where not adjacent to retaining wall.
• ponds are intended to hold 10' in depth.
• pond dimensions and volumes are as shown.
• erosion control shall be provided where shown in areas of pond over spill.

NOTE:

- All roofs are to drain to the rear (directly into the retention ponds).
- Provide a swale along the south side of lot A within the drainage easement to allow overflow to Muriel Street.
- Install a retaining wall along the North, East and South property lines.



THIS MICROIMAGE IS THE BEST POSSIBLE REPRODUCTION DUE TO THE POOR QUALITY OF THE ORIGINAL DOCUMENT

