

# CITY OF ALBUQUERQUE

Planning Department  
David Campbell, Director



Mayor Timothy M. Keller

June 20, 2019

David Soule, P.E.  
Rio Grande Engineering  
PO Box 93924  
Albuquerque, NM 87199

RE: **10101 Norman NE**  
**Grading and Drainage Plan**  
**Engineer's Stamp Date: 6/13/19**  
**Drainage File: E10D063**

Dear Mr. Soule:

Based on the submittal received on 6/14/19, the grading and drainage plan is approved for Grading Permit and Building Permit. As a reminder, if the project total area of disturbance (including the staging area and any work within the adjacent Right-of-Way) is 1 acre or more, then an Erosion and Sediment Control (ESC) Plan and Owner's certified Notice of Intent (NOI) is required to be submitted to the Stormwater Quality Engineer (Curtis Cherne, PE, ccherne@cabq.gov, 924-3420) 14 days prior to any earth disturbance.

Prior to Certificate of Occupancy (For Information):

1. Engineer's Certification, per the DPM Chapter 22.7: *Engineer's Certification Checklist For Non-Subdivision* is required.
2. A Bernalillo County Recorded [Private Facility Drainage Covenant](#) is required for the stormwater quality ponds. The original notarized form, exhibit A (legible on 8.5x11 paper), and recording fee (\$25, payable to Bernalillo County) must be turned into DRC (4th, Plaza del Sol) for routing. Please contact Charlotte LaBadie (clabadie@cabq.gov, 924-3996) or Madeline Carruthers (mtafoya@cabq.gov, 924-3997) regarding the routing and recording process for covenants. The routing and recording process for covenants can take a month or longer; Hydrology recommends beginning this process as soon as possible as to not delay approval for certificate of occupancy.

If you have any questions, please contact me at 924-3695 or dpeterson@cabq.gov.

Sincerely,

Dana Peterson, P.E.  
Senior Engineer, Planning Dept.  
Development Review Services



# City of Albuquerque

Planning Department  
Development & Building Services Division

## DRAINAGE AND TRANSPORTATION INFORMATION SHEET (REV 6/2018)

**Project Title:** 10101 Norman **Building Permit #:** \_\_\_\_\_ **Hydrology File #:** \_\_\_\_\_

**DRB#:** \_\_\_\_\_ **EPC#:** \_\_\_\_\_ **Work Order#:** \_\_\_\_\_

**Legal Description:** LOT 44 BLOCK 125 SNOW HEIGHTS

**City Address:** 10101 Norman

**Applicant:** BRIAN LEE **Contact:** \_\_\_\_\_

**Address:** \_\_\_\_\_

**Phone#:** \_\_\_\_\_ **Fax#:** \_\_\_\_\_ **E-mail:** \_\_\_\_\_

**Other Contact:** RIO GRANDE ENGINEERING **Contact:** DAVID SOULE

**Address:** PO BOX 93924 ALB NM 87199

**Phone#:** 505.321.9099 **Fax#:** 505.872.0999 **E-mail:** david@riograndeengineering.com

**TYPE OF DEVELOPMENT:** \_\_\_\_\_ PLAT \_\_\_\_\_ RESIDENCE \_\_\_\_\_ DRB SITE ☒ ADMIN SITE

Check all that Apply:

### DEPARTMENT:

☒ HYDROLOGY/ DRAINAGE  
\_\_\_\_\_ TRAFFIC/ TRANSPORTATION

### TYPE OF SUBMITTAL:

\_\_\_\_\_ ENGINEER/ARCHITECT CERTIFICATION  
\_\_\_\_\_ PAD CERTIFICATION  
\_\_\_\_\_ CONCEPTUAL G & D PLAN  
☒ GRADING PLAN  
\_\_\_\_\_ DRAINAGE REPORT  
\_\_\_\_\_ DRAINAGE MASTER PLAN  
\_\_\_\_\_ FLOODPLAIN DEVELOPMENT PERMIT APPLIC  
\_\_\_\_\_ ELEVATION CERTIFICATE  
\_\_\_\_\_ CLOMR/LOMR  
\_\_\_\_\_ TRAFFIC CIRCULATION LAYOUT (TCL)  
\_\_\_\_\_ TRAFFIC IMPACT STUDY (TIS)  
\_\_\_\_\_ STREET LIGHT LAYOUT  
\_\_\_\_\_ OTHER (SPECIFY) \_\_\_\_\_  
\_\_\_\_\_ PRE-DESIGN MEETING?

IS THIS A RESUBMITTAL?: \_\_\_\_\_ Yes ☒ No

### TYPE OF APPROVAL/ACCEPTANCE SOUGHT:

☒ BUILDING PERMIT APPROVAL  
\_\_\_\_\_ CERTIFICATE OF OCCUPANCY  
\_\_\_\_\_ PRELIMINARY PLAT APPROVAL  
\_\_\_\_\_ SITE PLAN FOR SUB'D APPROVAL  
\_\_\_\_\_ SITE PLAN FOR BLDG. PERMIT APPROVAL  
\_\_\_\_\_ FINAL PLAT APPROVAL  
\_\_\_\_\_ SIA/ RELEASE OF FINANCIAL GUARANTEE  
\_\_\_\_\_ FOUNDATION PERMIT APPROVAL  
☒ GRADING PERMIT APPROVAL  
\_\_\_\_\_ SO-19 APPROVAL  
\_\_\_\_\_ PAVING PERMIT APPROVAL  
\_\_\_\_\_ GRADING/ PAD CERTIFICATION  
\_\_\_\_\_ WORK ORDER APPROVAL  
\_\_\_\_\_ CLOMR/LOMR  
\_\_\_\_\_ FLOODPLAIN DEVELOPMENT PERMIT  
\_\_\_\_\_ OTHER (SPECIFY) \_\_\_\_\_

**DATE SUBMITTED:** \_\_\_\_\_ **By:** \_\_\_\_\_

COA STAFF:

ELECTRONIC SUBMITTAL RECEIVED: \_\_\_\_\_

FEE PAID: \_\_\_\_\_

DRAINAGE REPORT

For

**Norman Apartment**

**Albuquerque, New Mexico**

Prepared by

Rio Grande Engineering  
PO Box 93924  
Albuquerque, New Mexico 87199

June 2019



David Soule P.E. No. 14522

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### **Appendix**

Site Hydrology/ AHYMO MODEL .....	A
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### **Map Pocket**

Site Grading and Drainage Plan

## **PURPOSE**

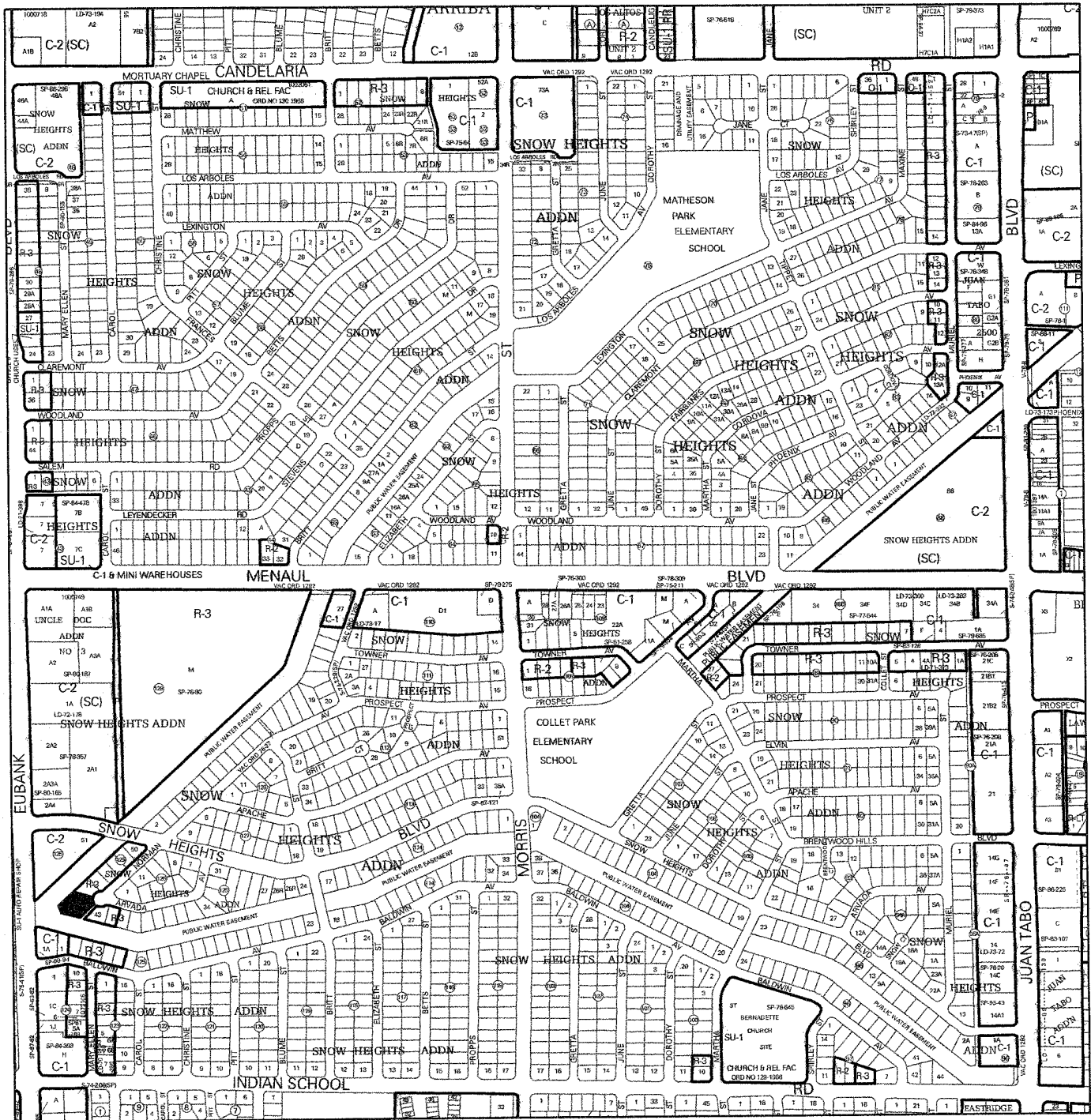
The purpose of this report is to provide the Drainage Management Plan for the infill development of an existing lot between Snow Heights and Indian School east of Eubank. . This plan was prepared in accordance with the City of Albuquerque design regulations, utilizing the City of Albuquerque's Development Process Manual drainage guidelines. This report will demonstrate that the grading does not adversely affect the surrounding properties, nor the upstream or downstream facilities.

## **INTRODUCTION**

The subject of this report, as shown on the Exhibit A, is a 0.39-acre parcel of land located at 10101 Norman NE in north east Albuquerque. The legal description of this site is lot 44 Block 125 Snow heights Addition. As shown on FIRM map35001C0356HG, the entire property is located within Flood Zone X. This site is surrounded by fully developed parcels. The site is bound by two city maintained drainage channels. This site is an existing developed site within fully developed areas. Based on the site location and the adjacent drainage infrastructure this development must maintain existing drainage patterns and match existing conditions as closely as possible.

## **EXISTING CONDITIONS**

The site is currently developed as a single family residential lot. The site is not in native condition. Due to curb and gutter and adjacent drainage infrastructure, this lot is not impacted by upland flow. The site currently discharges 1.07 cfs to thru the rear wall to the adjacent channels. The discharge leaves the site thru a series of turned blocks within the perimeter wall.



For more current information and more details visit: <http://www.cabq.gov/gis>

Zone Atlas Page:

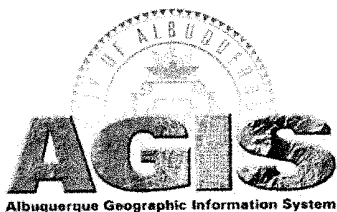
**H-21-Z**

Selected Symbols

- |                      |                        |
|----------------------|------------------------|
| SECTOR PLANS         | Escarpment             |
| Design Overlay Zones | 2 Mile Airport Zone    |
| City Historic Zones  | Airport Noise Contours |
| H-1 Buffer Zone      | Wall Overlay Zone      |
| Petroglyph Mon.      |                        |

Note: Grey Shading  
Represents Area Outside  
of the City Limits

0 750 1,500 Feet



Map amended through: 2/4/2010

## **PROPOSED CONDITIONS**

The proposed improvements consist of a two unit townhouse building and associated parking area. The site will be graded to accommodate the new building while maintaining the existing drainage patterns. As shown on the grading plan, the site will be graded to contain two basins. The site was modeled utilizing NOAA atlas rainfall data and AHMYO, located appendix B. The northern basin contains the northern half of the new units and a portion of the existing building. This basin generates 0.50 cfs. The flow drains through a water quality/detention basin at the northwest corner of the site. The pond discharges to the existing drainage rights of way via the existing turned blocks in the wall. The required first flush volume of 44 cf is retained onsite. The southern basin contains the remaining portion of the lot. This basin generates 0.85 cfs. The basin discharges to a new 6" pipe connected to an existing concrete channel. The parking area serves as a detention basin and the peak routed flow leaving this basin 0.43 cfs. An emergency overflow for this basin is provided by a turned block. The combined proposed peak flow leaving the site will be 0.82 cfs. The required first flush volume of 142 cf is retained onsite.

## **SUMMARY AND RECOMMENDATIONS**

This project is an infill project within a completely developed area of North Albuquerque. The site is currently undeveloped. The site currently discharges 1.07 cfs to the adjacent drainage right of way. The site is not impacted by upland flows. The proposed drainage plan will maintain the existing drainage patterns and reduce the peak flow to 0.82 cfs to pass through the site. The first flush volumes are retained onsite. since this site work area encompasses less than 1 acre, a NPDES permit and Erosion and Sediment Control Plan may not be required prior to any construction activity.

## **APPENDIX A**

### **SITE HYDROLOGY / AHYMO MODEL**





# NOAA's National Weather Service Hydrometeorological Design Studies Center Precipitation Frequency Data Server (PFDS)

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## NOAA ATLAS 14 POINT PRECIPITATION FREQUENCY ESTIMATES: NM

### Data description

 Data type:  Precipitation depth  Units:  English  Time series type:  Partial duration 

### Select location

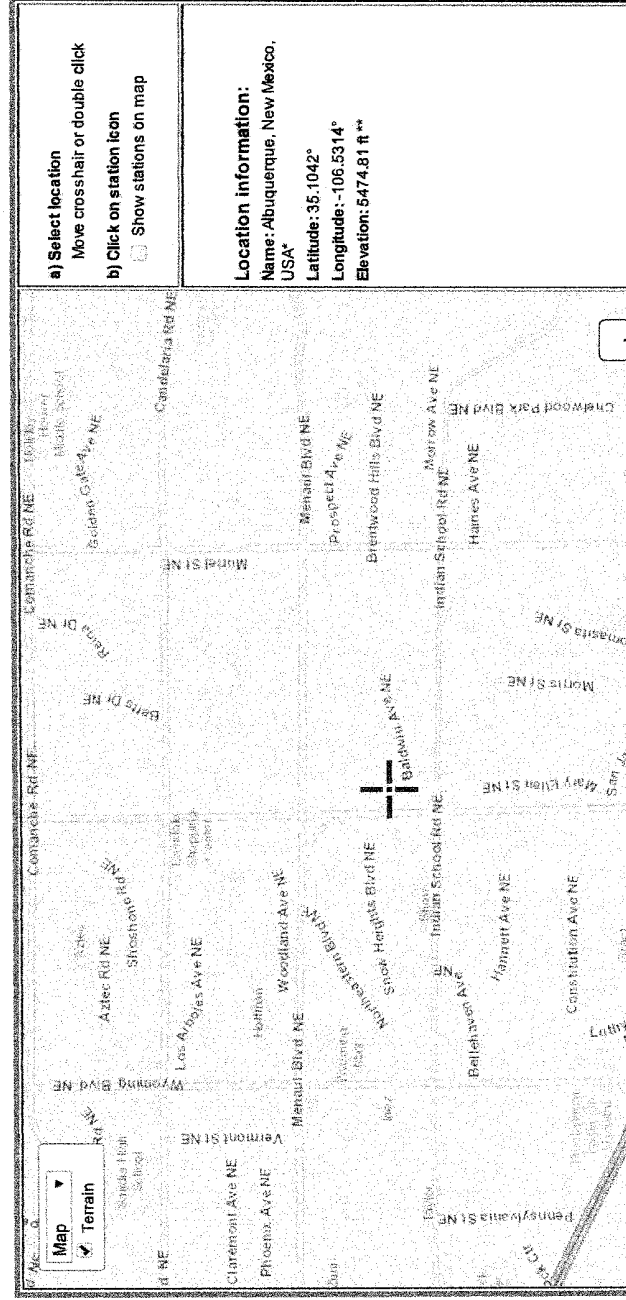
#### 1) Manually:

 a) By location (decimal degrees, use "." for S and W): Latitude:  Longitude: 

Submit

 b) By station (list of NM stations): 

 c) By address  Search 

 2) Use map (If ESRI interactive map is not loading, try adding the host: <https://js.arcgis.com/> to the firewall, or contact us at [hdsc.questions@noaa.gov](mailto:hdsc.questions@noaa.gov)):


General Information

Homepage

Progress Reports

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Glossary

Precipitation

Frequency

Data Server

GIS Data

Maps

Type Series

Terminology

Documentation

Probable Maximum

Precipitation

Documents

Miscellaneous

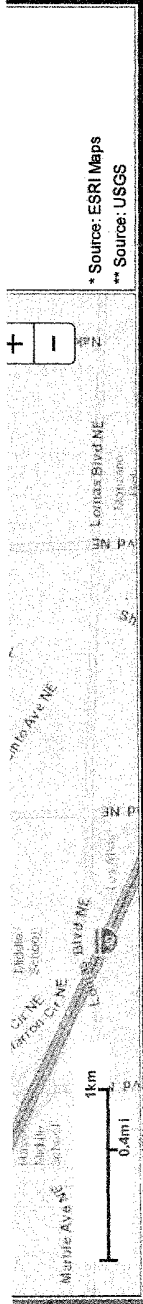
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Severe Precipitation

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## POINT PRECIPITATION FREQUENCY (PF) ESTIMATES

WITH 90% CONFIDENCE INTERVALS AND SUPPLEMENTARY INFORMATION  
NOAA Atlas 14, Volume 1, Version 5

PF tabular

PF gridview

Supplementary information

Print page

### PDS-based precipitation frequency estimates with 90% confidence intervals (in inches)<sup>1</sup>

Duration	Average recurrence interval (years)									
	1	2	5	10	25	50	100	200	500	1000
5-min	0.179 (0.152-0.211)	0.232 (0.198-0.274)	0.310 (0.262-0.367)	0.371 (0.313-0.438)	0.455 (0.382-0.538)	0.520 (0.434-0.612)	0.589 (0.489-0.692)	0.661 (0.544-0.776)	0.758 (0.619-0.892)	0.836 (0.679-0.983)
10-min	0.272 (0.231-0.321)	0.352 (0.288-0.418)	0.473 (0.395-0.558)	0.565 (0.478-0.666)	0.692 (0.581-0.815)	0.791 (0.661-0.932)	0.898 (0.743-1.05)	1.00 (0.829-1.18)	1.15 (0.942-1.36)	1.27 (1.03-1.50)
15-min	0.338 (0.288-0.388)	0.437 (0.369-0.516)	0.586 (0.495-0.692)	0.701 (0.590-0.825)	0.858 (0.720-1.01)	0.981 (0.820-1.15)	1.11 (0.921-1.31)	1.25 (1.03-1.47)	1.43 (1.17-1.68)	1.58 (1.28-1.86)
30-min	0.455 (0.385-0.536)	0.588 (0.497-0.685)	0.789 (0.667-0.932)	0.944 (0.794-1.11)	1.16 (0.970-1.38)	1.32 (1.10-1.56)	1.50 (1.24-1.76)	1.68 (1.38-1.97)	1.93 (1.57-2.27)	2.12 (1.72-2.50)
60-min	0.563 (0.477-0.663)	0.727 (0.615-0.860)	0.976 (0.825-1.15)	1.17 (0.983-1.38)	1.43 (1.20-1.68)	1.64 (1.37-1.93)	1.85 (1.54-2.18)	2.08 (1.71-2.44)	2.38 (1.95-2.80)	2.63 (2.13-3.09)
2-hr	0.668 (0.557-0.817)	0.857 (0.714-1.05)	1.13 (0.942-1.38)	1.35 (1.12-1.64)	1.66 (1.36-2.01)	1.91 (1.59-2.31)	2.17 (1.76-2.62)	2.44 (1.97-2.94)	2.82 (2.25-3.40)	3.13 (2.48-3.78)
3-hr	0.713 (0.588-0.866)	0.907 (0.758-1.10)	1.19 (0.984-1.44)	1.41 (1.18-1.70)	1.72 (1.42-2.08)	1.97 (1.62-2.38)	2.24 (1.83-2.66)	2.52 (2.04-3.03)	2.91 (2.34-3.49)	3.23 (2.57-3.88)
6-hr	0.831 (0.703-1.00)	1.05 (0.885-1.27)	1.35 (1.14-1.63)	1.59 (1.34-1.91)	1.92 (1.60-2.30)	2.17 (1.81-2.60)	2.44 (2.02-2.92)	2.72 (2.24-3.25)	3.10 (2.54-3.70)	3.42 (2.77-4.08)
12-hr	0.926 (0.795-1.08)	1.17 (1.00-1.37)	1.48 (1.26-1.73)	1.73 (1.47-2.02)	2.06 (1.75-2.40)	2.32 (1.96-2.70)	2.59 (2.19-3.01)	2.87 (2.39-3.34)	3.24 (2.68-3.78)	3.55 (2.91-4.14)
24-hr	1.07 (0.932-1.24)	1.34 (1.17-1.55)	1.68 (1.46-1.94)	1.95 (1.70-2.25)	2.32 (2.01-2.68)	2.60 (2.25-3.00)	2.90 (2.49-3.34)	3.20 (2.74-3.68)	3.61 (3.07-4.15)	3.93 (3.32-4.52)
2-day	1.14 (0.993-1.30)	1.43 (1.25-1.63)	1.80 (1.57-2.05)	2.08 (1.82-2.38)	2.48 (2.15-2.83)	2.78 (2.41-3.19)	3.10 (2.67-3.54)	3.43 (2.94-3.92)	3.86 (3.29-4.43)	4.21 (3.57-4.83)
3-day	1.25 (1.12-1.40)	1.56 (1.40-1.74)	1.94 (1.74-2.17)	2.24 (2.00-2.50)	2.65 (2.36-2.96)	2.96 (2.63-3.31)	3.28 (2.90-3.66)	3.61 (3.18-4.03)	4.04 (3.54-4.53)	4.38 (3.82-4.91)
4-day	1.36 (1.25-1.49)	1.68 (1.55-1.85)	2.09 (1.91-2.28)	2.40 (2.19-2.62)	2.82 (2.56-3.08)	3.14 (2.85-3.43)	3.46 (3.13-3.78)	3.79 (3.42-4.14)	4.22 (3.79-4.62)	4.56 (4.07-5.00)
7-day	1.57 (1.44-1.71)	1.95 (1.79-2.12)	2.38 (2.19-2.60)	2.72 (2.49-2.97)	3.18 (2.90-3.45)	3.51 (3.21-3.82)	3.85 (3.51-4.19)	4.19 (3.80-4.55)	4.62 (4.18-5.04)	4.95 (4.46-5.40)
10-day	1.74 (1.61-1.90)	2.17 (1.99-2.36)	2.67 (2.45-2.90)	3.06 (2.81-3.32)	3.58 (3.28-3.88)	3.97 (3.63-4.30)	4.37 (3.98-4.73)	4.76 (4.32-5.16)	5.28 (4.77-5.73)	5.67 (5.10-6.16)
20-day	2.23 (2.05-2.43)	2.76 (2.55-3.03)	3.39 (3.11-3.68)	3.84 (3.53-4.18)	4.43 (4.06-4.82)	4.86 (4.45-5.28)	5.28 (4.82-5.74)	5.68 (5.17-6.17)	6.19 (5.62-6.73)	6.56 (5.94-7.15)

30-day	2.69 (2.47-2.92)	3.35 (3.07-3.63)	4.04 (3.71-4.38)	4.56 (4.18-4.94)	5.22 (4.78-5.64)	5.89 (5.20-6.14)	6.14 (5.60-5.63)	6.57 (5.98-7.10)	7.09 (6.44-7.67)	7.47 (6.77-8.09)
45-day	3.29 (3.04-3.56)	4.08 (3.77-4.42)	4.88 (4.50-5.28)	5.46 (5.03-5.90)	6.17 (5.67-6.67)	6.66 (6.12-7.20)	7.12 (6.54-7.70)	7.54 (6.91-8.15)	8.04 (7.35-8.70)	8.37 (7.65-9.07)
60-day	3.79 (3.50-4.11)	4.70 (4.34-5.09)	5.62 (5.19-6.09)	6.29 (5.80-6.81)	7.09 (6.54-7.68)	7.65 (7.05-8.26)	8.17 (7.52-8.85)	8.64 (7.95-9.38)	9.21 (8.45-10.00)	9.58 (8.80-10.4)

1 Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS). Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values. Please refer to NOAA Atlas 14 document for more information.

Estimates from the table in CSV format: [Precipitation frequency estimates](#)

Main Link Categories:  
[Home](#) | [OWP](#)

US Department of Commerce  
National Oceanic and Atmospheric Administration  
National Weather Service  
Office of Water Prediction (OWP)  
1325 East West Highway  
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Page last modified: April 21, 2017

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**FIRST FLUSH VOLUMES**

**NORTH BASIN**

**REQUIRED=1550X.034/12=44 CU. FT**

**PROVIDED=60 CU.FT**

**SOUTH BASIN= 5024 SF x0.34/12=142 CU FT**

**PROVIDED=144 CU.FT**

# VOLUME CALCULATIONS

pond b

OUTLET@73

outlet at 74

ACTUAL ELEV.	DEPTH (FT)	AREA SF	VOLUME PER UNIT	VOLUME CUMULATIVE	VOLUME AC-FT	Q (CFS)
73.00	0.00	30.00	0	0	0.000	0.00
74.00	0.00	1106.00	568.00	568	0.001	0.00
74.50	0.83	1520.00	656.50	1224.5	0.028	0.86
75.00	1.33	2400.00	980.00	2204.5	0.051	1.09

Orifice Equation

$$Q = CA \sqrt{2gH}$$

C =

0.6

Diameter (in)

6

Area (ft^2)=

0.196349541

g =

32.2

H (Ft) =

Depth of water above center of orifice

Q (CFS)=

Flow

\*S AHYMO - DETENTION-NORMAN  
\*S POND ROUTING

START TIME=0.0 PUNCH CODE=0

RAINFALL TYPE=2  
QUARTER=0.0 ONE= 1.85 IN  
SIX=2.44 IN DAY= 2.90 IN DT = 0.05 HR

\*Basin EXISTING  
COMPUTE NM HYD ID=1 HYD NO=101 DA= .000606348 SQ MI  
PER A=00 PER B=41 PER C=42 PER D=17  
TP=-.170 MASSRAIN=-1

PRINT HYD ID=1 CODE=3

\*Basin NORTH PROPOSED  
COMPUTE NM HYD ID=2 HYD NO=102 DA= .000240796 SQ MI  
PER A=00 PER B=23 PER C=39 PER D=38  
TP=-.170 MASSRAIN=-1

PRINT HYD ID=2 CODE=3

\*Basin SOUTH PROPOSED  
COMPUTE NM HYD ID=3 HYD NO=103 DA= .000365552 SQ MI  
PER A=00 PER B=10 PER C=25 PER D=65  
TP=-.170 MASSRAIN=-1

PRINT HYD ID=3 CODE=3

\* ROUTE THE TOTAL FLOW THROUGH THE PROPOSED RESERVOIR  
ROUTE RESERVOIR ID=4 HYD NO=104 INFLOW=3 CODE=3  
OUTFLOW(CFS) STORAGE(AC-FT) ELEV(FT)  
0.00 0.001 74.00  
0.86 0.028 74.50  
1.09 0.051 75.00

\*TOTAL PROPOSED

ADD HYD ID=5 HYD NO=105 ID I=2 ID II=4

PRINT HYD ID=5 CODE=3

FINISH

AHYMO.OUT

AHYMO PROGRAM (AHYMO-S4)

- Version: S4.01a - Rel:

01a

RUN DATE (MON/DAY/YR) = 06/13/2019

START TIME (HR:MIN:SEC) = 17:00:38

USER NO.=

RioGrandeSingleA41963517

INPUT FILE = C:\Documents and Settings\Owner\Desktop\2019

JOBS\1976-norman\pondrout061219.txt

\*S AHYMO - DETENTION-NORMAN

\*S POND ROUTING

START TIME=0.0 PUNCH CODE=0

RAINFALL TYPE=2

QUARTER=0.0 ONE= 1.85 IN

SIX=2.44 IN DAY= 2.90 IN DT = 0.05 HR

24-HOUR RAINFALL DIST. - BASED ON NOAA ATLAS 14 FOR

CONVECTIVE AREAS (NM & AZ) - D1

DT =	0.050000	HOURS	END TIME =	24.000002	HOURS
0.0000	0.0039	0.0080	0.0123	0.0170	0.0218
0.0350	0.0473	0.0605	0.0742	0.0891	0.1041
0.1355	0.1517	0.1696	0.1883	0.2083	0.2329
0.2955	0.3361	0.3863	0.4532	0.5285	0.6585
1.2070	1.4505	1.6425	1.7389	1.8235	1.8843
1.9747	2.0056	2.0339	2.0573	2.0777	2.0966
2.1297	2.1439	2.1574	2.1706	2.1833	2.1938
2.2054	2.2110	2.2163	2.2216	2.2267	2.2317
2.2414	2.2460	2.2506	2.2551	2.2595	2.2637
2.2720	2.2761	2.2801	2.2839	2.2877	2.2915
					2.2952

AHYMO.OUT							
2.2988	2.3024	2.3060	2.3095	2.3130	2.3164	2.3198	
2.3232	2.3265	2.3298	2.3330	2.3362	2.3394	2.3425	
2.3456	2.3487	2.3518	2.3548	2.3577	2.3607	2.3636	
2.3665	2.3694	2.3722	2.3750	2.3778	2.3805	2.3833	
2.3860	2.3886	2.3913	2.3939	2.3965	2.3991	2.4017	
2.4042	2.4068	2.4093	2.4117	2.4142	2.4166	2.4191	
2.4215	2.4238	2.4262	2.4285	2.4309	2.4332	2.4355	
2.4377	2.4400	2.4423	2.4445	2.4468	2.4490	2.4512	
2.4535	2.4557	2.4579	2.4601	2.4623	2.4645	2.4667	
2.4689	2.4711	2.4733	2.4755	2.4776	2.4798	2.4820	
2.4841	2.4863	2.4884	2.4906	2.4927	2.4948	2.4969	
2.4991	2.5012	2.5033	2.5054	2.5075	2.5096	2.5117	
2.5137	2.5158	2.5179	2.5199	2.5220	2.5241	2.5261	
2.5281	2.5302	2.5322	2.5342	2.5363	2.5383	2.5403	
2.5423	2.5443	2.5463	2.5483	2.5503	2.5522	2.5542	
2.5562	2.5581	2.5601	2.5621	2.5640	2.5659	2.5679	
2.5698	2.5717	2.5736	2.5756	2.5775	2.5794	2.5813	
2.5832	2.5850	2.5869	2.5888	2.5907	2.5925	2.5944	
2.5962	2.5981	2.5999	2.6018	2.6036	2.6054	2.6073	
2.6091	2.6109	2.6127	2.6145	2.6163	2.6181	2.6198	
2.6216	2.6234	2.6252	2.6269	2.6287	2.6304	2.6322	
2.6339	2.6356	2.6374	2.6391	2.6408	2.6425	2.6442	
2.6459	2.6476	2.6493	2.6510	2.6527	2.6543	2.6560	
2.6577	2.6593	2.6610	2.6626	2.6643	2.6659	2.6675	
2.6692	2.6708	2.6724	2.6740	2.6756	2.6772	2.6788	
2.6804	2.6820	2.6835	2.6851	2.6867	2.6882	2.6898	
2.6913	2.6929	2.6944	2.6959	2.6975	2.6990	2.7005	
2.7020	2.7035	2.7050	2.7065	2.7080	2.7095	2.7109	
2.7124	2.7139	2.7153	2.7168	2.7182	2.7197	2.7211	
2.7225	2.7240	2.7254	2.7268	2.7282	2.7296	2.7310	
2.7324	2.7338	2.7352	2.7366	2.7379	2.7393	2.7407	
2.7420	2.7434	2.7447	2.7461	2.7474	2.7487	2.7500	
2.7513	2.7527	2.7540	2.7553	2.7566	2.7578	2.7591	
2.7604	2.7617	2.7629	2.7642	2.7655	2.7667	2.7680	
2.7692	2.7704	2.7717	2.7729	2.7741	2.7753	2.7765	
2.7777	2.7789	2.7801	2.7813	2.7825	2.7836	2.7848	
2.7860	2.7871	2.7883	2.7894	2.7906	2.7917	2.7928	
2.7939	2.7951	2.7962	2.7973	2.7984	2.7995	2.8006	
2.8017	2.8027	2.8038	2.8049	2.8059	2.8070	2.8081	
2.8091	2.8101	2.8112	2.8122	2.8132	2.8142	2.8153	
2.8163	2.8173	2.8183	2.8193	2.8202	2.8212	2.8222	
2.8232	2.8241	2.8251	2.8260	2.8270	2.8279	2.8289	
2.8298	2.8307	2.8316	2.8325	2.8334	2.8344	2.8352	
2.8361	2.8370	2.8379	2.8388	2.8396	2.8405	2.8414	
2.8422	2.8431	2.8439	2.8447	2.8456	2.8464	2.8472	
2.8480	2.8488	2.8496	2.8504	2.8512	2.8520	2.8528	
2.8536	2.8543	2.8551	2.8559	2.8566	2.8574	2.8581	
2.8588	2.8596	2.8603	2.8610	2.8617	2.8624	2.8631	
2.8638	2.8645	2.8652	2.8659	2.8666	2.8672	2.8679	
2.8686	2.8692	2.8699	2.8705	2.8711	2.8718	2.8724	
2.8730	2.8736	2.8742	2.8748	2.8754	2.8760	2.8766	
2.8772	2.8778	2.8783	2.8789	2.8795	2.8800	2.8806	
2.8811	2.8816	2.8822	2.8827	2.8832	2.8837	2.8842	
2.8847	2.8852	2.8857	2.8862	2.8867	2.8872	2.8876	
2.8881	2.8886	2.8890	2.8895	2.8899	2.8903	2.8908	
2.8912	2.8916	2.8920	2.8924	2.8928	2.8932	2.8936	
2.8940	2.8944	2.8948	2.8951	2.8955	2.8958	2.8962	
2.8965	2.8969	2.8972	2.8976	2.8979	2.8982	2.8985	
2.8988	2.8991	2.8994	2.8997	2.9000			

\*Basin EXISTING

COMPUTE NM HYD ID=1 HYD NO=101 DA= .000606348 SQ MI



AHYMO.OUT  
 PER A=00 PER B=41 PER C=42 PER D=17

TP=-.170 MASSRAIN=-1

K = 0.092650HR TP = 0.170000HR K/TP RATIO = 0.545000

SHAPE CONSTANT, N = 7.106428

UNIT PEAK = 0.31911 CFS UNIT VOLUME = 0.9655 B =

526.28 P60 = 1.8500

AREA = 0.000103 SQ MI IA = 0.10000 INCHES INF = 0.04000

INCHES PER HOUR

RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT

= 0.050000

K = 0.150918HR TP = 0.170000HR K/TP RATIO = 0.887754

SHAPE CONSTANT, N = 3.995652

UNIT PEAK = 1.0506 CFS UNIT VOLUME = 0.9888 B =

354.88 P60 = 1.8500

AREA = 0.000503 SQ MI IA = 0.42410 INCHES INF = 1.03747

INCHES PER HOUR

RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT

= 0.050000

PRINT HYD

ID=1 CODE=3

PARTIAL HYDROGRAPH 101.00

TIME	FLOW	TIME	FLOW	TIME	FLOW
TIME	FLOW	TIME	FLOW	TIME	FLOW
HRS	CFS	HRS	CFS	HRS	CFS
0.000	0.0	4.050	0.0	8.100	0.0
12.150	0.0	4.200	0.0	8.250	0.0
0.150	0.0	4.350	0.0	8.400	0.0
12.300	0.0	4.500	0.0	8.550	0.0
0.300	0.0	4.650	0.0	8.700	0.0
12.450	0.0				
0.450	0.0				
12.600	0.0				
0.600	0.0				

				AHYMO. OUT			
12.750	0.0		16.800	0.0			
0.750	0.0	0.0	4.800	0.0		8.850	0.0
12.900	0.0		16.950	0.0			
0.900	0.0	0.0	4.950	0.0		9.000	0.0
13.050	0.0		17.100	0.0			
1.050	0.0	0.0	5.100	0.0		9.150	0.0
13.200	0.0		17.250	0.0			
1.200	0.0	0.0	5.250	0.0		9.300	0.0
13.350	0.0		17.400	0.0			
1.350	0.0	0.2	5.400	0.0		9.450	0.0
13.500	0.0		17.550	0.0			
1.500	0.0	1.0	5.550	0.0		9.600	0.0
13.650	0.0		17.700	0.0			
1.650	0.0	0.9	5.700	0.0		9.750	0.0
13.800	0.0		17.850	0.0			
1.800	0.0	0.4	5.850	0.0		9.900	0.0
13.950	0.0		18.000	0.0			
1.950	0.0	0.2	6.000	0.0		10.050	0.0
14.100	0.0		18.150	0.0			
2.100	0.0	0.1	6.150	0.0		10.200	0.0
14.250	0.0		18.300	0.0			
2.250	0.0	0.1	6.300	0.0		10.350	0.0
14.400	0.0		18.450	0.0			
2.400	0.0	0.1	6.450	0.0		10.500	0.0
14.550	0.0		18.600	0.0			
2.550	0.0	0.0	6.600	0.0		10.650	0.0
14.700	0.0		18.750	0.0			
2.700	0.0	0.0	6.750	0.0		10.800	0.0
14.850	0.0		18.900	0.0			
2.850	0.0	0.0	6.900	0.0		10.950	0.0
15.000	0.0		19.050	0.0			
3.000	0.0	0.0	7.050	0.0		11.100	0.0
15.150	0.0		19.200	0.0			
3.150	0.0	0.0	7.200	0.0		11.250	0.0
15.300	0.0		19.350	0.0			
3.300	0.0	0.0	7.350	0.0		11.400	0.0
15.450	0.0		19.500	0.0			
3.450	0.0	0.0	7.500	0.0		11.550	0.0
15.600	0.0		19.650	0.0			
3.600	0.0	0.0	7.650	0.0		11.700	0.0
15.750	0.0		19.800	0.0			
3.750	0.0	0.0	7.800	0.0		11.850	0.0
15.900	0.0		19.950	0.0			
3.900	0.0	0.0	7.950	0.0		12.000	0.0

16.050 0.0

AHYMO.OUT

RUNOFF VOLUME = 1.25558 INCHES = 0.0406 ACRE-FeET  
PEAK DISCHARGE RATE = 1.07 CFS AT 1.550 HOURS BASIN AREA =

0.0006 SQ. MI.

\*Basin NORTH PROPOSED

COMPUTE NM HYD ID=2 HYD NO=102 DA= .000240796 SQ MI

PER A=00 PER B=23 PER C=39 PER D=38

TP=-.170 MASSRAIN=-1

K = 0.092650HR TP = 0.170000HR K/TP RATIO = 0.545000

SHAPE CONSTANT, N = 7.106428

UNIT PEAK = 0.28327 CFS UNIT VOLUME = 0.9587 B =

526.28

P60 = 1.8500

AREA = 0.000092 SQ MI IA = 0.10000 INCHES INF = 0.04000

INCHES PER HOUR

RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT

= 0.050000

K = 0.146886HR TP = 0.170000HR K/TP RATIO = 0.864036

SHAPE CONSTANT, N = 4.113943

UNIT PEAK = 0.31852 CFS UNIT VOLUME = 0.9582 B =

362.70

P60 = 1.8500

AREA = 0.000149 SQ MI IA = 0.40565 INCHES INF = 0.98581

INCHES PER HOUR

RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT

= 0.050000

PRINT HYD

ID=2 CODE=3

PARTIAL HYDROGRAPH 102.00

AHYMO.OUT

TIME	FLOW	TIME	FLOW	TIME	FLOW		
TIME	HRS	CFS	TIME	HRS	CFS	HRS	CFS
	0.000	0.0		3.900	0.0	7.800	0.0
11.700	0.150	0.0	15.600	4.050	0.0	7.950	0.0
11.850	0.300	0.0	15.750	4.200	0.0	8.100	0.0
12.000	0.450	0.0	15.900	4.350	0.0	8.250	0.0
12.150	0.600	0.0	16.050	4.500	0.0	8.400	0.0
12.300	0.750	0.0	16.200	4.650	0.0	8.550	0.0
12.450	0.900	0.0	16.350	4.800	0.0	8.700	0.0
12.600	1.050	0.0	16.500	4.950	0.0	8.850	0.0
12.750	1.200	0.0	16.650	5.100	0.0	9.000	0.0
12.900	1.350	0.0	16.800	5.250	0.0	9.150	0.0
13.050	1.500	0.0	16.950	5.400	0.0	9.300	0.0
13.200	1.650	0.0	17.100	5.550	0.0	9.450	0.0
13.350	1.800	0.0	17.250	5.700	0.0	9.600	0.0
13.500	1.950	0.0	17.400	5.850	0.0	9.750	0.0
13.650	2.100	0.0	17.550	6.000	0.0	9.900	0.0
13.800	2.250	0.0	17.700	6.150	0.0	10.050	0.0
13.950	2.400	0.0	17.850	6.300	0.0	10.200	0.0
14.100	2.550	0.0	18.000	6.450	0.0	10.350	0.0
14.250	2.700	0.0	18.150	6.600	0.0	10.500	0.0
14.400	2.850	0.0	18.300	6.750	0.0	10.650	0.0

		AHYMO. OUT					
14.550	0.0	18.450	0.0				
3.000	0.0	6.900	0.0	10.800	0.0		
14.700	0.0	18.600	0.0				
3.150	0.0	7.050	0.0	10.950	0.0		
14.850	0.0	18.750	0.0				
3.300	0.0	7.200	0.0	11.100	0.0		
15.000	0.0	18.900	0.0				
3.450	0.0	7.350	0.0	11.250	0.0		
15.150	0.0	19.050	0.0				
3.600	0.0	7.500	0.0	11.400	0.0		
15.300	0.0						
3.750	0.0	7.650	0.0	11.550	0.0		
15.450	0.0						

RUNOFF VOLUME = 1.62552 INCHES = 0.0209 ACRE-Feet  
 PEAK DISCHARGE RATE = 0.50 CFS AT 1.550 HOURS BASIN AREA =

0.0002 SQ. MI.

\*Basin SOUTH PROPOSED

COMPUTE NM HYD ID=3 HYD NO=103 DA= .000365552 SQ MI

PER A=00 PER B=10 PER C=25 PER D=65

TP=-.170 MASSRAIN=-1

K = 0.092650HR TP = 0.170000HR K/TP RATIO = 0.545000

SHAPE CONSTANT, N = 7.106428

UNIT PEAK = 0.73557 CFS UNIT VOLUME = 0.9858 B =

526.28

P60 = 1.8500

AREA = 0.000238 SQ MI IA = 0.10000 INCHES INF = 0.04000

INCHES PER HOUR

RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT

= 0.050000

K = 0.144092HR TP = 0.170000HR K/TP RATIO = 0.847598

SHAPE CONSTANT, N = 4.200762

UNIT PEAK = 0.27722 CFS AHYMO.OUT UNIT VOLUME = 0.9548 B =  
 368.34 P60 = 1.8500  
 AREA = 0.000128 SQ MI IA = 0.39286 INCHES INF = 0.95000  
 INCHES PER HOUR  
 RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT  
 = 0.050000  
 PRINT HYD ID=3 CODE=3

PARTIAL HYDROGRAPH 103.00

TIME		FLOW		TIME		FLOW		TIME		FLOW	
TIME	HRS	FLOW	CFS	TIME	HRS	FLOW	CFS	TIME	HRS	FLOW	CFS
HRS	CFS			HRS	CFS						
0.000	0.0			4.800	0.0			9.600	0.0		
14.400	0.0			19.200	0.0			9.750	0.0		
0.150	0.0			4.950	0.0			9.900	0.0		
14.550	0.0			19.350	0.0			10.050	0.0		
0.300	0.0			5.100	0.0			10.200	0.0		
14.700	0.0			19.500	0.0			10.350	0.0		
0.450	0.0			5.250	0.0			10.500	0.0		
14.850	0.0			19.650	0.0			10.650	0.0		
0.600	0.0			5.400	0.0			10.800	0.0		
15.000	0.0			19.800	0.0			10.950	0.0		
0.750	0.0			5.550	0.0			11.100	0.0		
15.150	0.0			19.950	0.0			11.250	0.0		
0.900	0.0			5.700	0.0			11.400	0.0		
15.300	0.0			20.100	0.0			11.550	0.0		
1.050	0.1			5.850	0.0			11.700	0.0		
15.450	0.0			20.250	0.0						
1.200	0.1			6.000	0.0						
15.600	0.0			20.400	0.0						
1.350	0.2			6.150	0.0						
15.750	0.0			20.550	0.0						
1.500	0.8			6.300	0.0						
15.900	0.0			20.700	0.0						
1.650	0.7			6.450	0.0						
16.050	0.0			20.850	0.0						
1.800	0.3			6.600	0.0						
16.200	0.0			21.000	0.0						
1.950	0.2			6.750	0.0						
16.350	0.0			21.150	0.0						
2.100	0.1			6.900	0.0						

		AHYMO.OUT					
16.500	0.0	21.300	0.0				
2.250	0.1	7.050	0.0	11.850	0.0		
16.650	0.0	21.450	0.0				
2.400	0.0	7.200	0.0	12.000	0.0		
16.800	0.0	21.600	0.0				
2.550	0.0	7.350	0.0	12.150	0.0		
16.950	0.0	21.750	0.0				
2.700	0.0	7.500	0.0	12.300	0.0		
17.100	0.0	21.900	0.0				
2.850	0.0	7.650	0.0	12.450	0.0		
17.250	0.0	22.050	0.0				
3.000	0.0	7.800	0.0	12.600	0.0		
17.400	0.0	22.200	0.0				
3.150	0.0	7.950	0.0	12.750	0.0		
17.550	0.0	22.350	0.0				
3.300	0.0	8.100	0.0	12.900	0.0		
17.700	0.0	22.500	0.0				
3.450	0.0	8.250	0.0	13.050	0.0		
17.850	0.0	22.650	0.0				
3.600	0.0	8.400	0.0	13.200	0.0		
18.000	0.0	22.800	0.0				
3.750	0.0	8.550	0.0	13.350	0.0		
18.150	0.0	22.950	0.0				
3.900	0.0	8.700	0.0	13.500	0.0		
18.300	0.0	23.100	0.0				
4.050	0.0	8.850	0.0	13.650	0.0		
18.450	0.0	23.250	0.0				
4.200	0.0	9.000	0.0	13.800	0.0		
18.600	0.0	23.400	0.0				
4.350	0.0	9.150	0.0	13.950	0.0		
18.750	0.0	23.550	0.0				
4.500	0.0	9.300	0.0	14.100	0.0		
18.900	0.0	23.700	0.0				
4.650	0.0	9.450	0.0	14.250	0.0		
19.050	0.0						

RUNOFF VOLUME = 2.07762 INCHES = 0.0405 ACRE-Feet  
 PEAK DISCHARGE RATE = 0.85 CFS AT 1.550 HOURS BASIN AREA =

0.0004 SQ. MI.

HYMO.OUT

\* ROUTE THE TOTAL FLOW THROUGH THE PROPOSED RESERVOIR

ROUTE RESERVOIR ID=4 HYD NO=104 INFLOW=3 CODE=3

OUTFLOW(CFS)	STORAGE(AC-FT)	ELEV(FT)
0.00	0.001	74.00
0.86	0.028	74.50
1.09	0.051	75.00

\* \* \* \* \*

TIME (HRS)	INFLOW (CFS)	ELEV (FEET)	VOLUME (AC-FT)	OUTFLOW (CFS)
0.00	0.00	74.00	0.001	0.00
0.15	0.00	74.00	0.001	0.00
0.30	0.00	74.00	0.001	0.00
0.45	0.00	74.00	0.001	0.00
0.60	0.00	74.00	0.001	0.00
0.75	0.02	74.00	0.001	0.00
0.90	0.04	74.01	0.001	0.01
1.05	0.05	74.01	0.002	0.02
1.20	0.10	74.02	0.002	0.04
1.35	0.23	74.04	0.003	0.08
1.50	0.77	74.12	0.008	0.21
1.65	0.66	74.23	0.014	0.40
1.80	0.32	74.24	0.014	0.42
1.95	0.18	74.21	0.012	0.36
2.10	0.11	74.17	0.010	0.29
2.25	0.08	74.13	0.008	0.22
2.40	0.05	74.10	0.006	0.17
2.55	0.02	74.07	0.005	0.13
2.70	0.01	74.05	0.004	0.09
2.85	0.01	74.04	0.003	0.07
3.00	0.01	74.03	0.002	0.05
3.15	0.01	74.02	0.002	0.03
3.30	0.01	74.02	0.002	0.03
3.45	0.01	74.01	0.002	0.02
3.60	0.01	74.01	0.001	0.02
3.75	0.01	74.01	0.001	0.01
3.90	0.01	74.01	0.001	0.01
4.05	0.01	74.01	0.001	0.01
4.20	0.01	74.00	0.001	0.01
4.35	0.01	74.00	0.001	0.01
4.50	0.01	74.00	0.001	0.01
4.65	0.01	74.00	0.001	0.01
4.80	0.01	74.00	0.001	0.01
4.95	0.01	74.00	0.001	0.01
5.10	0.01	74.00	0.001	0.01
5.25	0.01	74.00	0.001	0.01
5.40	0.01	74.00	0.001	0.01
5.55	0.01	74.00	0.001	0.01
5.70	0.01	74.00	0.001	0.01



			AHYMO.OUT	
5.85	0.01	74.00	0.001	0.01
6.00	0.01	74.00	0.001	0.01
6.15	0.01	74.00	0.001	0.01
6.30	0.01	74.00	0.001	0.01
6.45	0.01	74.00	0.001	0.01
6.60	0.01	74.00	0.001	0.01
6.75	0.01	74.00	0.001	0.01
6.90	0.01	74.00	0.001	0.01
7.05	0.01	74.00	0.001	0.01
7.20	0.01	74.00	0.001	0.01
7.35	0.01	74.00	0.001	0.01
7.50	0.01	74.00	0.001	0.01
7.65	0.01	74.00	0.001	0.01
7.80	0.01	74.00	0.001	0.01
7.95	0.01	74.00	0.001	0.01
8.10	0.01	74.00	0.001	0.01
8.25	0.01	74.00	0.001	0.01

TIME (HRS)	INFLOW (CFS)	ELEV (FEET)	VOLUME (AC-FT)	OUTFLOW (CFS)
8.40	0.01	74.00	0.001	0.01
8.55	0.01	74.00	0.001	0.01
8.70	0.01	74.00	0.001	0.01
8.85	0.01	74.00	0.001	0.01
9.00	0.01	74.00	0.001	0.01
9.15	0.01	74.00	0.001	0.01
9.30	0.01	74.00	0.001	0.01
9.45	0.01	74.00	0.001	0.01
9.60	0.01	74.00	0.001	0.01
9.75	0.01	74.00	0.001	0.01
9.90	0.01	74.00	0.001	0.01
10.05	0.01	74.00	0.001	0.01
10.20	0.01	74.00	0.001	0.01
10.35	0.01	74.00	0.001	0.01
10.50	0.01	74.00	0.001	0.01
10.65	0.01	74.00	0.001	0.01
10.80	0.01	74.00	0.001	0.01
10.95	0.01	74.00	0.001	0.01
11.10	0.01	74.00	0.001	0.01
11.25	0.01	74.00	0.001	0.01
11.40	0.01	74.00	0.001	0.01
11.55	0.01	74.00	0.001	0.01
11.70	0.01	74.00	0.001	0.01
11.85	0.01	74.00	0.001	0.01
12.00	0.01	74.00	0.001	0.01
12.15	0.00	74.00	0.001	0.01
12.30	0.00	74.00	0.001	0.01
12.45	0.00	74.00	0.001	0.00

PEAK DISCHARGE = 0.428 CFS - PEAK OCCURS AT HOUR 1.75  
 MAXIMUM WATER SURFACE ELEVATION = 74.249  
 MAXIMUM STORAGE = 0.0144 AC-FT INCREMENTAL TIME=

0.050000HRS

\*TOTAL PROPOSED

ADD HYD ID=5 HYD NO=105 ID I=2 ID II=4

PRINT HYD ID=5 CODE=3

AHYMO.OUT

PARTIAL HYDROGRAPH 105.00

TIME FLOW				TIME FLOW				TIME FLOW	
TIME	HRS	FLOW	CFS	TIME	HRS	FLOW	CFS	HRS	CFS
	HRS	CFS			HRS	CFS			
	0.000		0.0		4.800		0.0	9.600	0.0
14.400	0.150	0.0	0.0	19.200	4.950	0.0	0.0	9.750	0.0
14.550	0.300	0.0	0.0	19.350	5.100	0.0	0.0	9.900	0.0
14.700	0.450	0.0	0.0	19.500	5.250	0.0	0.0	10.050	0.0
14.850	0.600	0.0	0.0	19.650	5.400	0.0	0.0	10.200	0.0
15.000	0.750	0.0	0.0	19.800	5.550	0.0	0.0	10.350	0.0
15.150	0.900	0.0	0.0	19.950	5.700	0.0	0.0	10.500	0.0
15.300	1.050	0.0	0.0	20.100	5.850	0.0	0.0	10.650	0.0
15.450	1.200	0.0	0.1	20.250	6.000	0.0	0.0	10.800	0.0
15.600	1.350	0.0	0.2	20.400	6.150	0.0	0.0	10.950	0.0
15.750	1.500	0.0	0.7	20.550	6.300	0.0	0.0	11.100	0.0
15.900	1.650	0.0	0.8	20.700	6.450	0.0	0.0	11.250	0.0
16.050	1.800	0.0	0.6	20.850	6.600	0.0	0.0	11.400	0.0
16.200	1.950	0.0	0.5	21.000	6.750	0.0	0.0	11.550	0.0
16.350	2.100	0.0	0.3	21.150	6.900	0.0	0.0	11.700	0.0
16.500	2.250	0.0	0.3	21.300	7.050	0.0	0.0	11.850	0.0
16.650	2.400	0.0	0.2	21.450	7.200	0.0	0.0	12.000	0.0
16.800	2.550	0.0	0.1	21.600	7.350	0.0	0.0	12.150	0.0
16.950	2.700	0.0	0.1	21.750	7.500	0.0	0.0	12.300	0.0

				AHYMO. OUT			
17.100	2.850	0.0	0.1	21.900	7.650	0.0	0.0
17.250	3.000	0.0	0.1	22.050	7.800	0.0	0.0
17.400	3.150	0.0	0.0	22.200	7.950	0.0	0.0
17.550	3.300	0.0	0.0	22.350	8.100	0.0	0.0
17.700	3.450	0.0	0.0	22.500	8.250	0.0	0.0
17.850	3.600	0.0	0.0	22.650	8.400	0.0	0.0
18.000	3.750	0.0	0.0	22.800	8.550	0.0	0.0
18.150	3.900	0.0	0.0	22.950	8.700	0.0	0.0
18.300	4.050	0.0	0.0	23.100	8.850	0.0	0.0
18.450	4.200	0.0	0.0	23.250	9.000	0.0	0.0
18.600	4.350	0.0	0.0	23.400	9.150	0.0	0.0
18.750	4.500	0.0	0.0	23.550	9.300	0.0	0.0
18.900	4.650	0.0	0.0	23.700	9.450	0.0	0.0
19.050	0.0			23.850	0.0		

RUNOFF VOLUME = 1.88736 INCHES = 0.0610 ACRE-Feet  
 PEAK DISCHARGE RATE = 0.82 CFS AT 1.600 HOURS BASIN AREA =

0.0006 SQ. MI.

FINISH

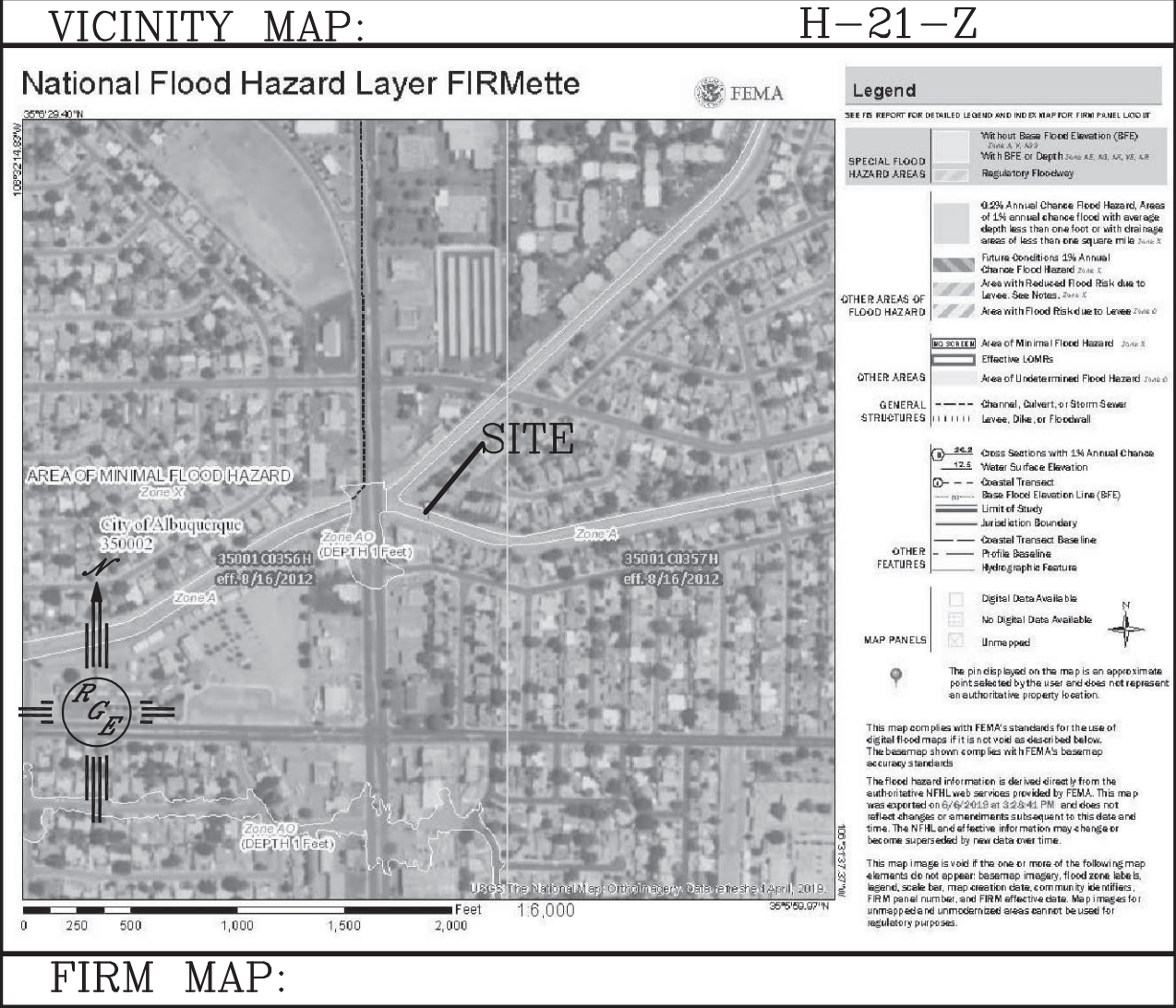
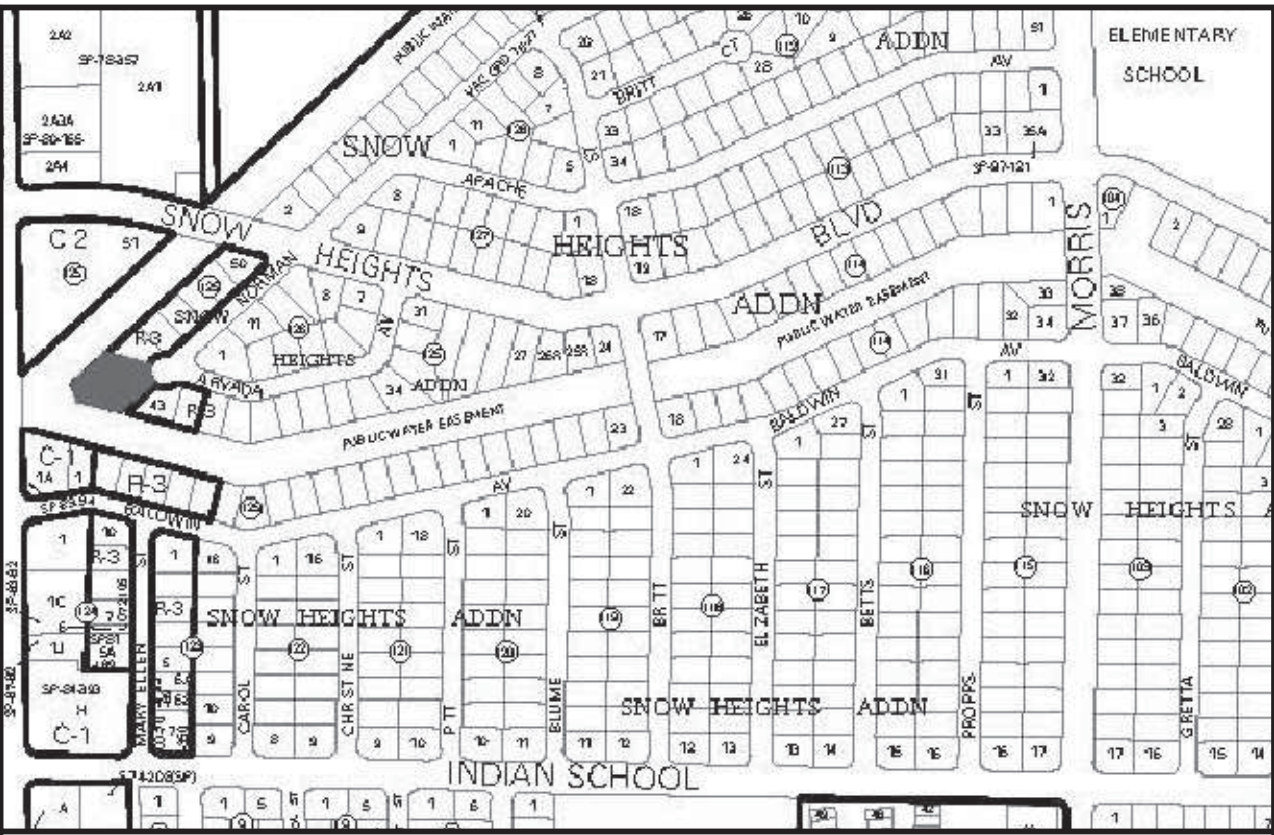
NORMAL PROGRAM FINISH

END TIME (HR:MIN:SEC) = 17:00:38



EROSION CONTROL NOTES:

1. CONTRACTOR IS RESPONSIBLE FOR OBTAINING A TOPSOIL DISTURBANCE PERMIT PRIOR TO BEGINNING WORK.
2. CONTRACTOR IS RESPONSIBLE FOR MAINTAINING RUN-OFF ON SITE DURING CONSTRUCTION.
3. CONTRACTOR IS RESPONSIBLE FOR CLEANING ALL SEDIMENT THAT GETS INTO EXISTING RIGHT-OF-WAY.
4. REPAIR OF DAMAGED FACILITIES AND CLEANUP OF SEDIMENT ACCUMULATIONS ON ADJACENT PROPERTIES AND IN PUBLIC FACILITIES IS THE RESPONSIBILITY OF THE CONTRACTOR.
5. ALL EXPOSED EARTH SURFACES MUST BE PROTECTED FROM WIND AND WATER EROSION PRIOR TO FINAL ACCEPTANCE OF ANY PROJECT.



LEGAL DESCRIPTION:

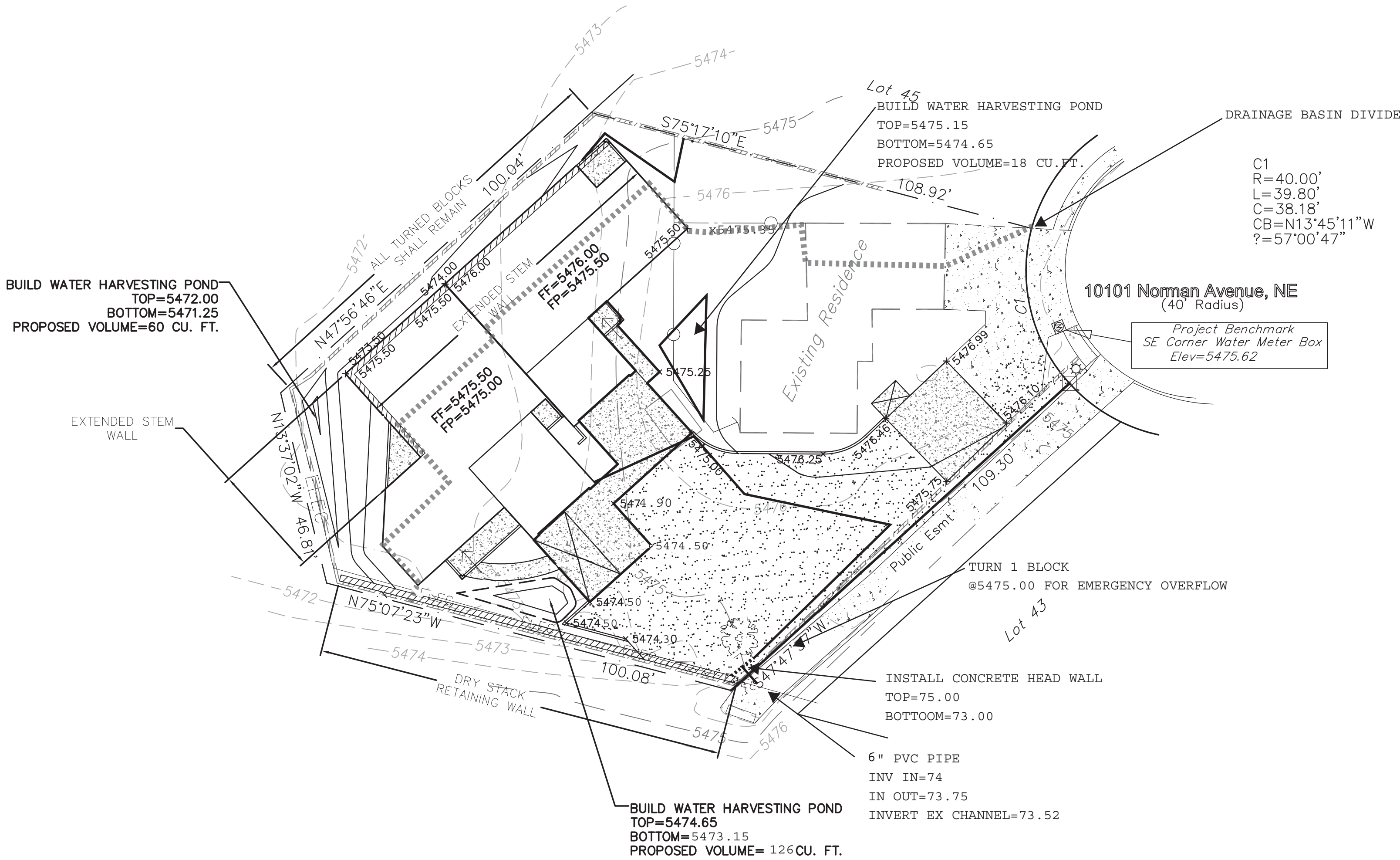
Lot 44, Block 125, Snow Heights Addition

NOTES:

1. ALL SPOT ELEVATIONS REPRESENT FLOWLINE ELEVATION UNLESS OTHERWISE NOTED.
2. ALL CURB AND GUTTER TO 6" HEADER UNLESS OTHERWISE NOTED.
3. ALL RETAINING WALL DESIGN SHALL BE BY OTHERS.
4. ANY CURBS OR PAVEMENT NEGATIVELY IMPACTED BY CONSTRUCTION ACTIVITY SHALL BE REPLACED TO MATCH EXISTING CONDITIONS.
5. ALL SITE WORK SHALL CONFORM TO BERNALILLO COUNTY STANDARDS FOR PUBLIC WORKS CONSTRUCTION EDITION 9

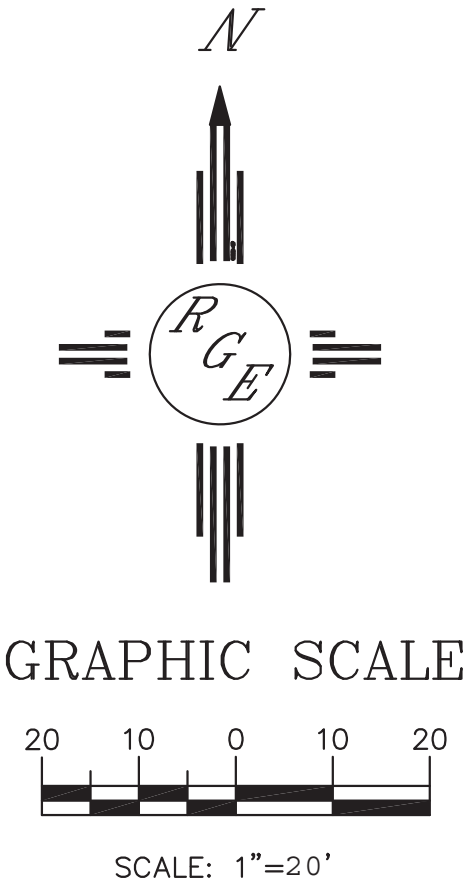
LEGEND

---	5414	---	EXISTING CONTOUR
---	5415	---	EXISTING INDEX CONTOUR
---	5414	---	PROPOSED CONTOUR
---	5415	---	PROPOSED INDEX CONTOUR
---	5414	---	SLOPE TIE
---	5415	---	EXISTING SPOT ELEVATION
---	5414	---	PROPOSED SPOT ELEVATION
---	5415	---	BOUNDARY
---	5414	---	CENTERLINE
---	5415	---	RIGHT-OF-WAY
---	5414	---	PROPOSED CURB
---	5415	---	EXISTING CURB AND GUTTER
---	5414	---	EXISTING SIDEWALK
---	5415	---	PROPOSED RETAINING WALL DESIGN BY OTHERS
---	5414	---	FLOW LINE



CAUTION:

EXISTING UTILITIES ARE NOT SHOWN. IT SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR TO CONDUCT ALL NECESSARY FIELD INVESTIGATIONS PRIOR TO ANY EXCAVATION TO DETERMINE THE ACTUAL LOCATION OF UTILITIES & OTHER IMPROVEMENTS.



ENGINEER'S SEAL	10101 NORMAN	DRAWN BY WCWJ
DAVID SOULE NEW MEXICO 14522 REGISTERED PROFESSIONAL ENGINEER	GRADING AND DRAINAGE PLAN	DATE 6-11-19
6/13/19	Rio Grande Engineering 1606 CENTRAL AVENUE SE SUITE 201 ALBUQUERQUE, NM 87106 (505) 872-0999	2109054-LAYOUT-6-11-19
DAVID SOULE P.E. #14522		SHEET #
		JOB # 2109054