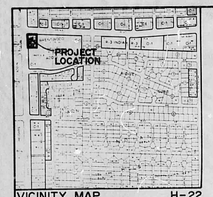
PUBLIC WORKS/HYDROLOGY

H22/D1B



VICINITY MAP

SCALE : 1" = 800'

H-22

PROJECT BENCHMARK - TBM

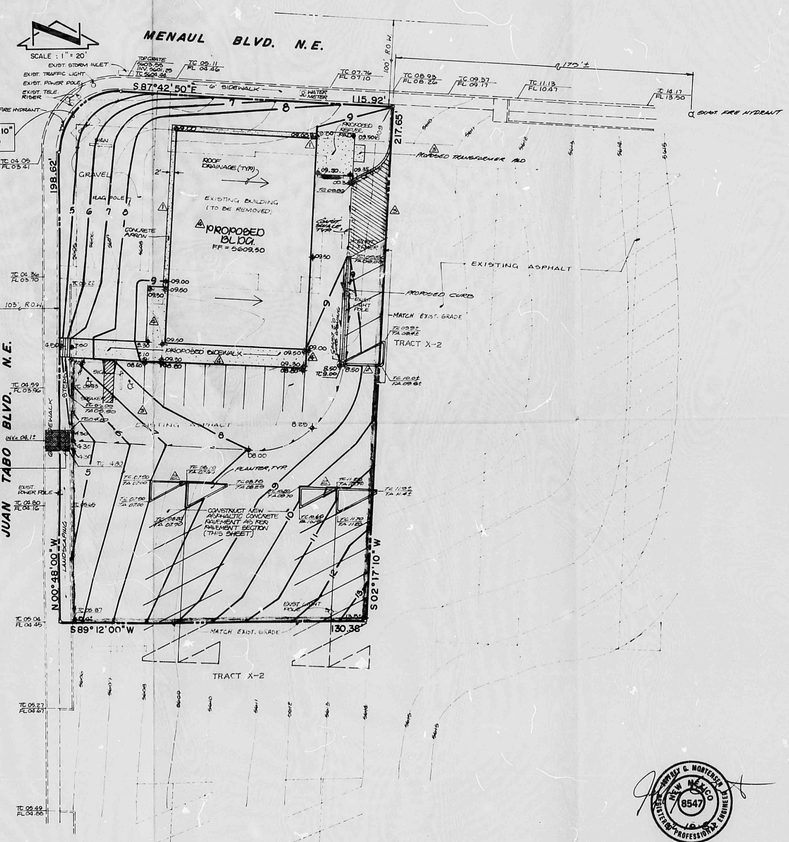
STATION IS A STANDARD ACS BRASS TABLET STAMPED "Z-H21-A", SET FLUSH WITH THE CURB. STATION IS LOCATED IN THE NOSE OF THE SOUTH ISLAND IN THE INTERSECTION OF MENAUL BLVD. NE & JUAN TABO BLVD. N.E.
ELEVATION = 5603.083 FEET (MSL.D)

LEGAL DESCRIPTION

TRACT "X-3", BREITWOOD HILLS SUBDIVISION

LEGAL DESCRIPTION

5609	EXISTING CONTOUR
09	PROPOSED CONTOUR
	PROPOSED ASPHALT
	PROPOSED CONCRETE
TC	TOP OF CURB
FL	FLOW LINE



DRAINAGE PLAN

The following items concerning the Checker Auto Parts No 852 Drainage Plan are contained hereon:

1. Vicinity Map
2. Grading Plan
3. Calculations

As shown by the Vicinity Map, the site is located on the southeast quadrant of the intersection of Juan Tabo Boulevard N.E. and Menaul Boulevard N.E. At present, the site is developed, making this a modification to an existing site. Both Juan Tabo Boulevard and Menaul Boulevard are currently developed commercially, thereby making this an infill site. As shown on Plate H-22 of the Albuquerque Master Drainage Study, this site does not lie within a designated Flood Hazard Zone. However, the site does lie adjacent to a designated flood Hazard Zone. Because of this, the finished floor of the proposed building has been established at least 1' above the corresponding flow line elevation on the adjacent Hazard Zone. At present, runoff generated developed runoff from this site will have a negligible effect on the existing conditions. At present, runoff generated by this site flows from east to west on Menaul Boulevard N.E. At this point, runoff flows in a northerly direction along the east side of Juan Tabo Boulevard N.E. to existing storm drain inlets located at the intersection of Menaul and Juan Tabo. Unfortunately, this storm drain system does not have ample capacity, hence street flooding occurs. No offsite flows enter the site along the north and west property lines since the existing roads route runoff away from the project site. No offsite flows enter the site along the south property line since the existing site is graded in a manner which will route runoff away from the south property line. Offsite flows enter the existing site in a manner which will route runoff to the east property line. Runoff is then collected and conveyed through the project site. Based upon the fact that site along the east property line which will receive runoff is currently developed and conveyed through the proposed improvements, the flow discharge of runoff from this site is appropriate. The proposed development will also not adversely impact the existing flooding problem.

The Grading Plan shows 1) existing grades indicated by spot elevations and contours at 1'0" intervals, 2) continuity between existing and proposed grades, and 3) the nature and character of the proposed improvements. As shown by the plan, the proposed improvements consist of the removal of an existing building along with adjacent paving and the construction of a proposed building along with adjacent paving and landscaping. Flows generated by this site will be routed from east to west onto Juan Tabo Boulevard N.E. From that point, runoff flows north along the east edge of Juan Tabo Boulevard N.E. to aforementioned storm drain inlets. Some of the runoff generated by this site and discharged to the street may be captured by the existing downstream storm inlets. This pattern is consistent with existing site drainage.

The Calculations which appear hereon analyze both the existing and developed conditions for the 100-year, 6-hour rainfall event. The Rational Method has been used to quantify the peak rate of discharge and the SCS Method has been used to quantify the volume of runoff. Both Methods have been used in accordance with the City of Albuquerque Development Process Manual, Volume II, and the Mayor's Emergency Rule adopted January 14, 1986. As shown by these Calculations, the proposed improvements will decrease the discharge by approximately 0.7 cfs.

CALCULATIONS

Ground Cover Information

From SCS Bernalillo County Soil Survey,
Plate 12 (12) - Bernalillo Tierras Complex
Hydrologic Soil Group: C
Existing Pervious CN = 71 (DPM Plate 22.2 C-2)
Pasture or Range Land fair condition)
Developed Pervious CN = 61 (DPM Plate 22.2 C-2)

Time of Concentration/Time to Peak

$T_C = 0.0078\ L^{0.77}/S^{0.385}$ (Kirpich Equation)

$T_P = T_C + 10$ min.

Point Rainfall

$P_6 = 2.5$ in. (DPM Plate 22.2 D-1)

Rational Method

Discharge: $Q = CIA$

where C varies
$I = P_6\ (6.84)\ T_C^{-0.51} = 5.28$ in/hr
$P_6 = 2.5$ in (DPM Plate 22.2D-1)
$T_C = 10$ min (minimum)
A = area, acres

SCS Method

Volume: $V = 3630\,(DRO)\,A$

Where DRO = Direct runoff in inches
A = area, acres

Existing Condition

Acres = 30.610 mf = 0.70 Ac
Roof area = 3,170 mf (0.11)
Paved area = 27,140 mf (0.89)
C = 0.95 (Weighted average per Emergency Rule, 1/14/86)
$Q_{100} = CIA = (0.95)(5.28)(0.70) = 3.5$ cfs
$V_{100} = 3630\ (DRO)\ A = 3630\ (0.95)\ (0.70) = 2,413$ cf
Composite CN = 80 (DPM Plate 22.2 C-3)
DRO = 2.3 in (DPM Plate 22.2 C-4)
$V_{100} = 3630\ (DRO)\ A = 5,845$ cf

Developed Condition

Acres = 30.610 mf = 0.70 Ac
Roof area = 9,400 mf (0.18)
Paved area = 16,230 mf (0.54)
Landscaped area = 4,980 mf (0.28)
C = 0.84 (Weighted average per Emergency Rule, 1/14/86)
$Q_{100} = CIA = (0.79)(5.28)(0.70) = 2.8$ cfs
Aimp = 21,630 mf % Impervious = 72
Composite CN = 87.5 (DPM Plate 22.2 C-3)
DRO = 1.35 in (DPM Plate 22.2 C-4)
$V_{100} = 3630\ (DRO)\ A = 3,430$ cf

Flows Entering Sidewalk Culvert (Includes On Site & Offsite Flows)

Point Rainfall (10-Year, 6-Hour Rainfall Event)
$P_6 = (0.857)(2.5\ in) = 1.64$ in
$I = 3.43$ (in/hr)
Acres = 30.610 mf = 0.83 Ac
Impervious = 26,400 mf (0.31)
Paved area = 26,770 mf (0.79)
C = 0.84 (Weighted average per Emergency Rule, 1/14/86)
$Q_{100} = CIA = (0.84)(3.43)(2.83) = 9.2$ cfs

Sidewalk Culvert Capacity (2'-0" Curb Opening) Weir Equation

$Q = CLH^{3/2} = 2.2$ cfs
Where C = 3.08
L = 2'-0"
H = 0.3'
Therefore, construct 4 - 2'-0" and 1 - 1'-0" sidewalk culverts for a total capacity of 9.9 cfs

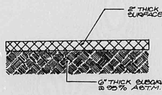
Comparing
$Q_{100} = 3.5 - 2.8 = 0.7$ cfs (decrease)
$V_{100} = 5845 - 3430 = 2415$ cf (decrease)

CONSTRUCTION NOTES:

1. TWO (2) WORKING DAYS PRIOR TO ANY EXCAVATION, CONTRACTOR MUST CONTACT LINE LOCATING SERVICE, 766-1234, FOR LOCATION OF EXISTING UTILITIES.
2. PRIOR TO CONSTRUCTION, THE CONTRACTOR SHALL EXCAVATE AND VERIFY THE HORIZONTAL AND VERTICAL LOCATION OF ALL UTILITIES AT THE CONNECTION POINTS. SHOULD A CONFLICT EXIST, THE CONTRACTOR SHALL NOTIFY THE ENGINEER SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM AMOUNT OF DELAY.
3. ALL WORK ON THIS PROJECT SHALL BE PERFORMED IN ACCORDANCE WITH APPLICABLE FEDERAL, STATE AND LOCAL LAWS, RULES AND REGULATIONS CONCERNING CONSTRUCTION SAFETY AND HEALTH.
4. ALL CONSTRUCTION WITHIN PUBLIC RIGHT-OF-WAY SHALL BE PERFORMED IN ACCORDANCE WITH APPLICABLE CITY OF ALBUQUERQUE STANDARDS AND PROCEDURES.
5. IF ANY UTILITY LINES, PIPELINES, OR UNDERGROUND UTILITY LINES ARE SHOWN ON THESE DRAWINGS, THE CONTRACTOR SHALL ASSUME THAT ALL UTILITY LINES AND EQUIPMENT MAY EXIST WHERE NONE ARE SHOWN. IF ANY SUCH EXISTING LINES ARE SHOWN, THE LOCATION IS BASED UPON INFORMATION PROVIDED BY THE OWNER OF SAID UTILITY, AND THE INFORMATION MAY BE INCOMPLETE OR INACCURATE. PRIOR TO CONSTRUCTION COMMENCE, THE ENGINEER HAS UNDERTAKEN NO FIELD VERIFICATION OF THE LOCATION, DEPTH, SIZE, OR TYPE OF EXISTING UTILITY LINES, PIPELINES, OR UNDERGROUND UTILITY LINES. MAKES NO REPRESENTATION PERTAINING THERETO, AND ASSUMES NO LIABILITY THEREFOR. THE CONTRACTOR SHALL INFORM ITSELF OF THE LOCATION OF ANY UTILITY LINE, PIPELINE, OR UNDERGROUND UTILITY LINE IN OR NEAR THE AREA OF THE WORK IN ADVANCE OF AND DURING CONSTRUCTION. THE CONTRACTOR IS ALSO RESPONSIBLE FOR ANY AND ALL DAMAGE CAUSED BY ITS FAILURE TO LOCATE, IDENTIFY AND PRESERVE ANY AND ALL EXISTING UTILITIES, PIPELINES, AND UNDERGROUND UTILITY LINES. IN PLANNING AND CONDUCTING EXCAVATION, THE CONTRACTOR SHALL COMPLY WITH STATE AND LOCAL MUNICIPAL LAWS, RULES AND REGULATIONS, IF ANY, PERTAINING TO THE LOCATION OF THESE LINES AND FACILITIES.
6. AN EXCAVATION/CONSTRUCTION PERMIT WILL BE REQUIRED FOR CONSTRUCTION ANY WORK WITHIN PUBLIC RIGHT-OF-WAY. AN APPROVED COPY OF THESE PLANS MUST BE SUBMITTED AT THE TIME OF APPLICATION FOR THIS PERMIT.
7. BACKFILL COMPACTION SHALL BE ACCORDING TO ASTERIA _____ STREET USE.
8. MAINTENANCE OF THESE FACILITIES SHALL BE THE RESPONSIBILITY OF THE OWNER OF THE PROPERTY SERVED.

EROSION CONTROL MEASURES

1. THE CONTRACTOR SHALL ENSURE THAT NO SOIL ERODES FROM THE SITE INTO PUBLIC RIGHT-OF-WAY. THIS MAY BE ACHIEVED BY CONSTRUCTING CURB AS ACHIEVED BY CONSTRUCTING TEMPORARY BERMS AT THE PROPERTY LINES AND WETTING THE SOIL TO KEEP IT FROM BLOWING.
2. THE CONTRACTOR SHALL PROMPTLY CLEAN UP ANY MATERIAL EXCAVATED WITHIN THE PUBLIC RIGHT-OF-WAY SO THAT THE EXCAVATED MATERIAL IS NOT SUSCEPTIBLE TO BEING WASHED DOWN THE STREET.
3. THE CONTRACTOR SHALL SECURE "TOPSOIL DISTURBANCE PERMIT" PRIOR TO BEGINNING CONSTRUCTION.



⚠ PAVEMENT SECTION
SCALE : 1" = 1'-0"

APPROVALS	NAME	DATE
A.C.E./DESIGN		1/14/88
INSPECTOR		5-15-87
A.C.E./FIELD		6-25-87

RECEIVED FEB 11 1988 HYDROLOGY SECTION

H22/D1B

01-27-87
01-05-88
01-27-88
02-11-88

BOHANNAN-HUSTON INC. ENGINEERS
8 DALLAS NE • ALBUQUERQUE • NEW MEXICO

GRADING AND DRAINAGE PLAN

CHECKER AUTO PARTS #852 – JUAN TABO & MENAUL

DESIGN BY	L.P.U.
DRAWN BY	S.G.H.
APPROVED BY	T.T.M.

No.	DATE	BY	REVISION
1	12/87	L.P.U.	SHOW CONCRETE AROUND NEW BUILDING
2	01-1988	L.P.U.	CONCRETE REPLACE PAD
3	01-1988	L.P.U.	SHOW PROPOSED SIDEWALK CULVERTS
4	02-88	J.G.M.	SHOW PATTERN OF CURB, REMOVE UNLOCK, REMOVE N.E. BARRIER AND REMOVE PAD
5			LOWER F.F. TO 5609.50

JOB NO. 870761
DATE 11-87
SHEET 1 OF 1

R# 26 DATE 3-24-88 OP 18P.

CITY OF ALBUQUERQUE

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The photographic process used meets the Basic Microfilm Standards of the National Micrographics Association (MNA-1977)

DIRECTOR

NOTARY PUBLIC

OFFICIAL SEAL
Roberta Duran
NOTARY PUBLIC
STATE OF NEW MEXICO
My Commission Expires 6/19/89

PLAN SHOT TWICE
ONCE ON LIGHT
ONCE ON DARK

CITY OF ALBUQUERQUE
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PLANS/PLATS ON FILE

FILE DESC: H22/D1B

PLANS/PLATS 1

E 30 D 24 C 20 B 16 A 12 CITY OF ALBUQUERQUE C F

PROJECT BENCHMARK = TBM

STATION IS A STANDARD ACS BRASS TABLET STAMPED "Z-H21-A", SET FLUSH WITH THE CURB. STATION IS LOCATED IN THE NOSE OF THE SOUTH ISLAND IN THE INTERSECTION OF MENAUL BLVD NE & JUAN TABO BLVD. N.E

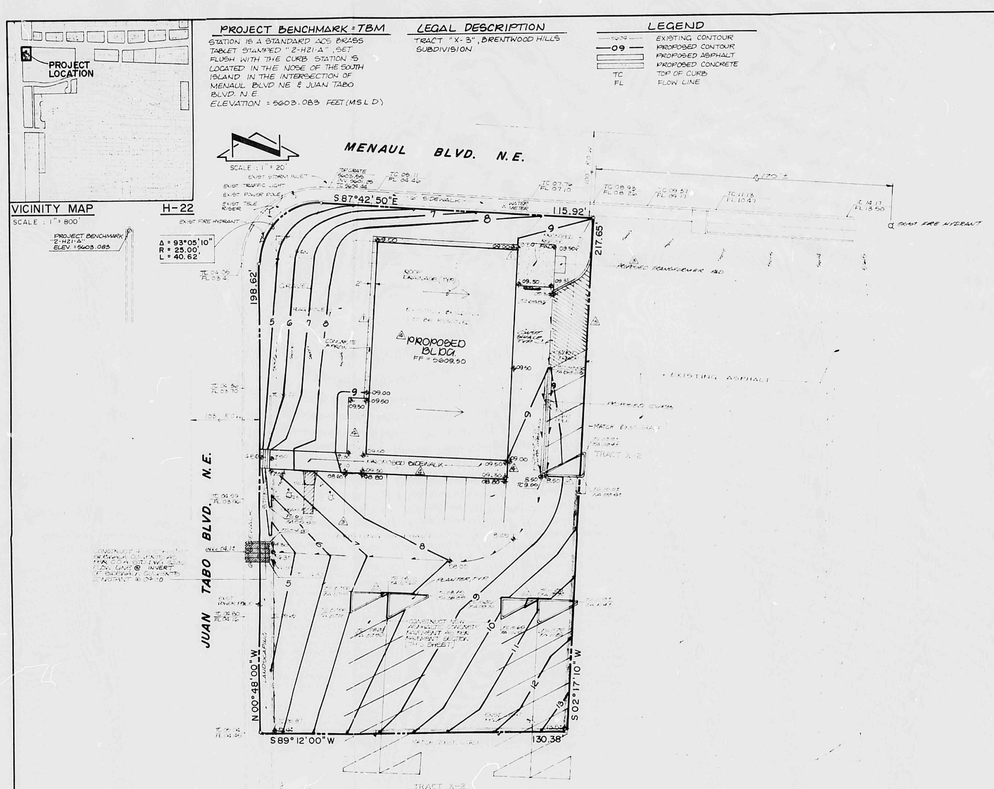
ELEVATION = 5603.083 FEET (MSLD)

LEGAL DESCRIPTION

TRACT "X-B", BRENTWOOD HILLS SUBDIVISION

LEGEND

5629	EXISTING CONTOUR
09	PROPOSED CONTOUR
	PROPOSED ASPHALT
	PROPOSED CONCRETE
TC	TOP OF CURB
FL	FLOW LINE



DRAINAGE PLAN

The following items concerning the Checker Auto Parts No. 852 Drainage Plan are contained hereon:

1. Vicinity Map
2. Grading Plan
3. Calculations

As shown by the Vicinity Map, the site is located on the southeast quadrant of the intersection of Juan Tabo Boulevard N.E. and Menaul Boulevard N.E. At the present, the site is developed, making this a modification to an existing site. Much of the surrounding area is currently developed commercially, thereby making this an infill site. As shown by Plate H-22 of the Albuquerque Master Drainage Study, this site does not lie within a designated Flood Hazard Zone. However, the site does lie adjacent to a designated Flood Hazard Zone. Because of this, the finished floor of the proposed decrease has been established at least 2' above the corresponding flow line elevation of Menaul Boulevard N.E. Runoff generated by this site flows from east to west onto Juan Tabo Boulevard N.E. from that point, runoff flows in a northerly direction along the east side of Juan Tabo Boulevard N.E. to existing storm drain inlets located at the intersection of Juan Tabo Boulevard N.E. and Menaul Boulevard N.E. Unfortunately, this storm drain system does not have ample capacity, hence street flooding occurs. No offsite flows enter the site along the north and west property lines since the existing roads route runoff away from the project site. No offsite flows enter the site along the south property line since the existing site is graded in a manner which will route runoff from the project site. Some offsite flows enter the site along the east property line and are conveyed through the project site. Based upon the fact that site also is an infill site and the view of the net decrease in runoff generated by the proposed improvements, the free discharge of runoff from this site is appropriate. The proposed development will also not adversely impact the existing flooding problem.

The Grading Plan shows 1) existing grades indicated by spot elevations and contours at 1'or' intervals, 2) continuity between existing and proposed grades, 3) the limit and character of the proposed improvements. As shown by the plan, the proposed improvements consist of the removal of an existing building along with adjacent paving and the construction of a new retail building along with adjacent paving and landscaping. Flows generated by this site will be routed from east to west onto Juan Tabo Boulevard N.E. From that point, runoff flows north along the east edge of Juan Tabo Boulevard N.E. to aforementioned storm drain inlets. Some of the runoff generated by this site also flows north along the street but is captured by the existing downstream storm inlets. This pattern is consistent with existing site drainage.

The Calculations which appear hereon analyze both the existing and developed conditions for the 100-year, 6-hour rainfall event. The Rational Method has been used to quantify the peak rate of discharge and the SCS Method has been used to quantify the volume of runoff. Both Methods have been used in accordance with the City of Albuquerque Development Process Manual, Volume II, and the Mayor's Emergency Rule adopted January 14, 1986. As shown by these Calculations, the proposed improvements will decrease the discharge by approximately 0.7 cfs.

CALCULATIONS

Ground Cover Information

From SCS Bernalillo County Soil Survey,
Plate 32: ESC - Embudo Tilerans Complex
Hydrologic Soil Group: B
Existing Pervious CN = 71 (IDM Plate 22.2 C-2)
Pasture or Range Land: fair condition)
Developed Pervious CN = 61 (IDM Plate 22.2 C-2)

Time of Concentration, Time to Peak

$T_c = 0.0078\ L^{0.77}/S^{0.385}$ (Kirpich Equation)

$T_p = T_c = 10$ min.

Point Rainfall

$P_6 = 2.5$ in. (IDM Plate 22.2 D-1)

Rational Method

Discharge: Q = CIA

where C varies
$I = P_6\ (6.84)\ T_p^{-0.51} = 5.28$ in/hr
$P_6 = 2.5$ in (IDM Plate 22.2D-1)
$T_p = 10$ min (minimum)
A = area, acres

SCS Method

Volume: V = 3630(DRO) A

Where DRO = Direct runoff in inches
A = area, acres

Existing Condition

Acres = 30,410 sf = 0.70 Ac
Roof Area = 3,470 sf (0.11)
Paved area = 27,140 sf (0.89)
C = 0.95 (Weighted Average per Emergency Rule, 1/14/86)
Q100 = CIA = (0.95)(5.28)(0.70) = 3.5 cfs
V100 = 3630 (DRO) A = 3,845 cf
Composite CN = 86 (IDM Plate 22.2 C-3)
DRO = 2.3 in (IDM Plate 22.2 C-4)
V100 = 3630 (DRO) A = 5,245 cf

Developed Condition

Acres = 30,410 sf = 0.70 Ac
Roof Area = 5,400 sf (0.18)
Paved area = 16,230 sf (0.54)
Landscaped area = 8,540 sf (0.28)
C = 0.81 (Weighted Average per Emergency Rule, 1/14/86)
Q100 = CIA = (0.81)(5.28)(0.70) = 2.8 cfs
Alpa = 21,630 sf/A (Impervious) = .72 A
Composite CN = 87.5 (IDM Plate 22.2 C-3)
DRO = 1.35 in (IDM Plate 22.2 C-4)
V100 = 3630 (DRO) A = 3,450 cf

Flows Entering Sidewalk Culvert (Includes On Site & Offsite Flows)

Point Rainfall (10-Year, 6-Hour Rainfall Event)
$P_6 = (0.457)(2.5\ in) = 1.44$ in
I = 3.47 in/hr
Acres = .721 (sf = 3.83) Ac
Roof Area = 16,400 sf (0.21)
Paved area = 96,270 sf (0.79)
C = 0.94 (Weighted average per Emergency Rule, 1/14/86)
Q100 = CIA = (0.94)(3.47)(2.83) = 9.2 cfs

Sidewalk Culvert Capacity (2'-0" Curb Opening Weir Equation

$Q = CLH^{3/2} = 2.2$ cfs
Where C = 3.09
L = 2'-0"
H = 0.5'
Therefore, construct a - 2'-0" and 1 - 1'-0" sidewalk culverts for a total capacity of 9.9 cfs

Comparison
Q100 = 9.9 - 2.8 = 0.7 cfs (decrease)
V100 = 5845 - 3430 = 2415 cf (decrease)

CONSTRUCTION NOTES

1. TWO (2) WORKING DAYS PRIOR TO ANY EXCAVATION, CONTRACTOR MUST CONTACT LINE LOCATING SERVICE (65-1234), FOR LOCATION OF EXISTING UTILITIES.
2. PRIOR TO CONSTRUCTION, THE CONTRACTOR SHALL EXCAVATE AND VERIFY THE HORIZONTAL AND VERTICAL LOCATION OF ALL POTENTIAL CONTRACTIONS. SHOULD A CONFLICT EXIST, THE CONTRACTOR SHALL NOTIFY THE ENGINEER SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM AMOUNT OF DELAY.
3. ALL WORK ON THIS PROJECT SHALL BE PERFORMED IN ACCORDANCE WITH ALL FEDERAL, STATE AND LOCAL LAWS, RULES AND REGULATIONS CONCERNING CONSTRUCTION SAFETY AND HEALTH.
4. ALL CONSTRUCTION WITHIN PUBLIC RIGHT-OF-WAY SHALL BE PERFORMED IN ACCORDANCE WITH APPLICABLE CITY OF ALBUQUERQUE STANDARDS AND PROCEDURES.
5. IF ANY UTILITY LINES, PIPELINES, OR UNDERGROUND UTILITY LINES ARE SHOWN ON THESE DRAWINGS, THEY ARE SHOWN IN AN APPROXIMATE WAY ONLY AND ARE NOT NECESSARILY TO BE ACCURATE AND SHOWN. IF ANY SUCH EXISTING LINES ARE SHOWN, THE LOCATION IS BASED UPON INFORMATION PROVIDED BY THE OWNER OF SAID UTILITY, AND THE INFORMATION MAY BE INCORRECT, OR MAY BE INCOMPLETE. THE CONTRACTOR SHALL BE CONSIDERED ADVISED THAT CONSTRUCTION MAY REQUIRE VERIFICATION OF THE LOCATION, DEPTH, SIZE, OR TYPE OF EXISTING UTILITY LINES, PIPELINES, OR UNDERGROUND UTILITY LINES. DUES SO REPRESENTATION CONTAINING, THEREIN, ANY MAXIMUM, THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONTRACTOR SHALL INFORM itself OF THE LOCATION OF ANY UTILITY LINE, PIPELINE, OR UNDERGROUND UTILITY LINE IN OR NEAR THE AREA OF THE WORK IN ADVANCE OF AND DURING CONSTRUCTION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY AND ALL DAMAGE CAUSED BY ITS FAILURE TO LOCATE, IDENTIFY AND PRESERVE ANY AND ALL EXISTING UTILITIES, PIPELINES, AND UNDERGROUND UTILITY LINES. IN PLANNING AND CONDUCTING EXCAVATION, THE CONTRACTOR SHALL COMPLY WITH STATE STATUTES, MUNICIPAL ORDINANCES, AND THE RULES AND REGULATIONS, IF ANY, PERTAINING TO THE LOCATION OF THESE LINES AND FACILITIES.
6. AN EXCAVATION/CONSTRUCTION PERMIT WILL BE REQUIRED BEFORE BEGINNING ANY WORK WITHIN CITY RIGHT-OF-WAY. EXCAVATION COST OF APPLICATION FOR THIS PERMIT SHALL BE SUBMITTED AT THE TIME OF APPLICATION FOR THIS PERMIT.
7. BACKFILL COMPACTION SHALL BE ACCORDING TO ASTM STANDARDS.
8. MAINTENANCE OF THESE FACILITIES SHALL BE THE RESPONSIBILITY OF THE OWNER OF THE PROPERTY SERVED.

EROSION CONTROL MEASURES

1. THE CONTRACTOR SHALL ENSURE THAT NO SOIL ERODES FROM THE SITE INTO PUBLIC RIGHT-OF-WAY. THE CONTRACTOR SHALL PROTECT ALL SOIL ON ADJACENT PRIVATE PROPERTY. IN THE TEMPORARY BERMS AT THE PROPERTY LINES AND WETTING THE SOIL TO KEEP IT FROM BLOWING.
2. THE CONTRACTOR SHALL PROMPTLY CLEAN UP ANY SOIL THAT IS EXCAVATED WITHIN THE PUBLIC RIGHT-OF-WAY SO THAT EXCAVATED MATERIAL IS NOT SUSCEPTIBLE TO BEING WASHED DOWN THE STREET.
3. THE CONTRACTOR SHALL SECURE "TOWNSHIP DISTURBANCE PERMIT" PRIOR TO BEGINNING CONSTRUCTION.

PAVEMENT SECTION
SCALE 1" = 1'-0"

APPROVALS	NAME	DATE
A.C.E./DESIGN	Benny Montoya	1/14/88
INSPECTOR		5-15-88
A.C.E./FIELD	Russell Baker	6-23-88

H22/DIB

PLAN SHOT TWICE ONCE ON LIGHT ONCE ON DARK

CITY OF ALBUQUERQUE
THIS MICROIMAGE IS THE BEST POSSIBLE REPRODUCTION DUE TO THE POOR QUALITY OF THE ORIGINAL DOCUMENT.

R1 26 DATE 3-27-90 OF 27

CITY OF ALBUQUERQUE

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The photographic process used meets the Basic Microfilm Standards of the National Micrographics Association (MS115-1977)

DIRECTOR

NOTARY PUBLIC

OFFICIAL SEAL
Roberta Duran
NOTARY PUBLIC
07/02/92

NEW MEXICO 8547 REGISTERED PROFESSIONAL ENGINEER

12-8-87
01-25-88
02-11-88

GRADING AND DRAINAGE PLAN

CHECKER AUTO PARTS #852 – JUAN TABO & MENAUL

DESIGN BY	LPU
DRAWN BY	S.G.H.
APPROVED BY	T.T.M.

No.	DATE	By	Revision
1	12/1/87	L.P.U.	SHOW CONCRETE ASPHALT AND DRIVEWAY
2			CONFORM TO NOTES 1-4
3	01-25-88	L.P.U.	SHOW PROPOSED BUILDING
4			MINOR CHANGES
5	02-11-88	L.P.U.	SHOW PROPOSED ASPHALT, CONC. & DRIVEWAY AT NEW BUILDING AND
6	02-88	J.G.M.	LOWER F.F. TO 5609.50

JOB NO 870761
DATE 11-87
SHEET 1 OF 1

F 30 D 24 C 20 B 16 A 12

CITY OF ALBUQERQUE C D F