

Sity of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

June 2, 1983

Billy J. Goolsby, PE Espey, Huston & Associates, Inc. 4801 Indian School Road NE Suite 204 Albuquerque, NM 87110

REF: El Jardin Office Building (H22-D29)

Dear Mr. Goolsby:

Consider this an approval of the revised drainage plan referenced above.

Please advise your client that a drainage inspection and a signed-off copy of the "Private Storm Drain Facilities Within City Right-of Way" will be required for a green tag from this office.

If you have any questions, please call me at 766-7644.

Sincerely

Fred J. @guirre, PE Civil Engineer/Hydrology

FJA:mrk

TRANSMITTAL LETTER

ESPEY, HUSTON & ASSOCIATES, INC.
Engineering & Environmental Consultants
4801 INDIAN SCHOOL ROAD R.E. SUITE 204
ALBUQUERQUE, NEW MEXICO 87110 Ph. (508) 255-1625

PROJE	CT: El	Jardin	office Building	
(marrie,	Ma	di find	Drainage Plan.	PROJECT NO: 3234-03
	,		Oralnage Plan.	
	г			DATE: 5/10/83
TO:	Fred	Aguir	re, P.E.	
	LIVITEN	MINCER	/ Hydrolegy	If enclosures are not as noted, ple inform us immediately.
	P.D. Box	129:	. 2	If checked below, please:
ATTN:	Albuane	Lrout.	N.M. 87103 _	() Acknowledge receipt of enclosur
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	(X) herewith		under separate cover via	
FOR YO	(X) in accord	lance with		1
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-	() use		()	
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	() Change C		Ox Drawing Reproducibles) Product Literature
			El Jardin Office	Building Orainage Plan
COPIES	DATE	REV NO.		
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-		-	Drainage Plan	
			3	
ACTION A	Action indicated	t on item to		
CODE B	. No action requi	red	D. For signati	ure and forwarding as noted below under REMARKS
DEMARKS	For signature an	d return to	his office E. See REMAI	CKS Delow
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ESPEY, HUSTON/ ASSOCIATES, INC.

Engineering & Environmental Consultants 4801 INDIAN SCHOOL TOAD, N.E., SUITE 204 ALBUQUERQUE, N.M. 87110 (505) 255-1625

> CORPORATE OFFICE P.O. BOX 519 AUSTIN, TEXAS 78767

April 22, 1983

Mr. Fred Aguirre, P.E. Civil Engineer/Hydrology City of Albuquerque P.O. Box 1293 Albuquerque, New Mexico 8/103

Subject:

Drainage Correction for El Jardin Office Building

Dear Fred:

Submitted to you herewith is the Construction Drawing for the installation of the drain pipes for the El Jardin Office Building.

As per our telephone conversation the size of the pipes were changed from 6" PVC to 4" PVC as proposed in my letter to you dated 4/12/83. Due to this change in pipe sizes, the total discharge will now be 3.57 cfs instead of 3.84 previously stated in that letter.

The attached plan is being submitted for your review and approval. Should you have any comments or questions concerning this plan please let me know.

Sincerely,

Staff Engineer

Enclosure

ESPEY, HUSTON & ASSOCIATES, INC.

Engineering & Environmental Consultants 4801 INDIAN SCHOOL TOAD, N.E., SUITE 204 ALBUQUERQUE, N.M. 87110 (505) 255-1625

April 12, 1983

CORPORATE OFFICE P.O. BOX 519 AUSTIN, TEXAS 78767

Mr. Fred Aguire, P.E. Civil Engineer/Hydrology City of Albuquerque P.O. Box 1293 Albuquerque, New Mexico 87103

SUBJECT:

Drainage Correction for El Jardin Office Building

Dear Fred:

As per our telephone conversation on Friday, April 8, 1983, the corrected drainage plan for the El Jardin Office Building now consists of discharging a maximum of 3.88 cfs to Menaul Blvd. and eliminating the roof ponding.

At the time the project was designed, the City policy for drainage was to allow no more than the existing condition runoff. The approved Drainage Plan Sheet 4 of 25 (P-41) Record No. 79-3-26A indicates the existing condition 100-year frequency discharge to be 3.88 cfs. Therefore, the discharge now will be 3.88 cfs or less.

The new discharge rate is to be accomplished by adding 1 - 6 inch outlet pipe from the northwest corner of Pond B to Menaul with a maximum discharge of 1.16 cfs; adding 1 - 4 inch outlet pipe from the northwest corner of Pond C to Menaul with a maximum discharge of 0.5 cfs; plugging the outlet between Ponds C and D and adding 1 - 6 inch outlet pipe to the existing 2 - 4 inch pipe from Pond D for a total maximum discharge of 2.18 cfs.

The total maximum discharge from the three proposed outlets will be 3.84 cfs which is at the allowable limit.

Enclosed herewith, are the associated computations, hydrographs and plan sheet for your use and approval.

Should you have any questions concerning the amended drainage plan or need additional information, do not hesitate to contact me.

Sincerely,

Billy J. Goolsb, P Staff Engineer

/cs

enclosures

BURK and BURK ARCHITECTS, A.I.A.

BB

August 31, 1981

#1662

City of Albuquerque Municipal Development Department Engineering Division - Hydrology City Hall 403 Albuquerque, NM 87102

Attention: Fred Aguirre, PE

RE: 11930 Menaul Blvd., NE

1133,037

Dear Sir:

This is to certify that, to the best of our knowledge, the above project is in substantial conformance with the drainage design approved May 21, 1980.

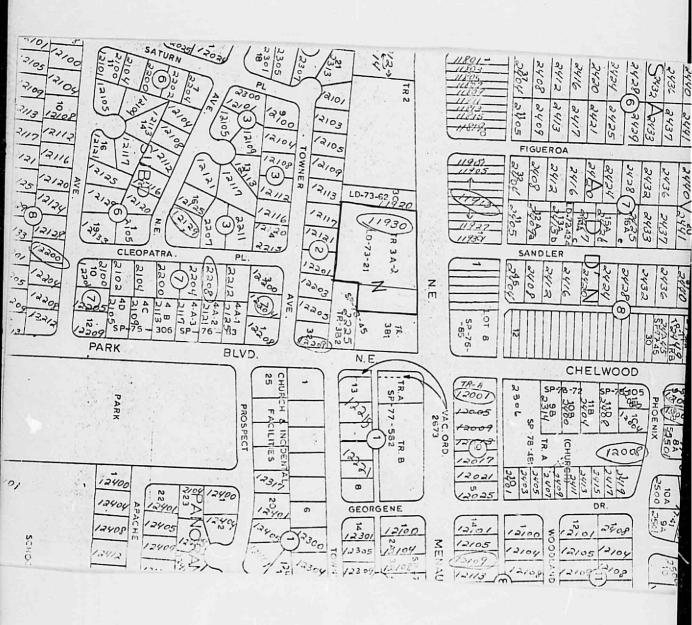
The floor elevations are very close to those indicated on the drawings.

William E. Buch III

ohn W. Bettis, PE **Enchantment Engineering**



Wm E Burk, Jr. Wm E Burk, III 11930 Menaul Blvd. NE, No 7 P.O. Box 11606 Albuquerque, New Mexico 87192 Telephone (505) 292 6566



TRANSMITTAL LETTER



ESPEY, HUSTON & ASSOCIATES, INC.

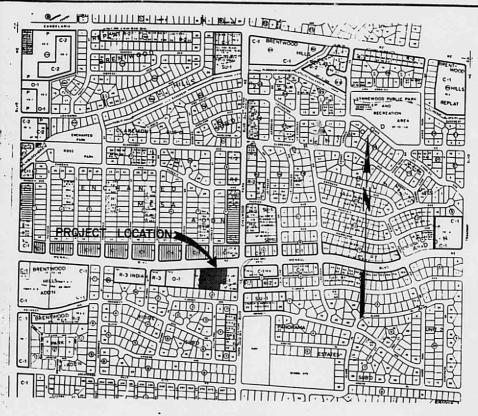
Engineering & Environmental Consultants

4801 INDIAN SCHOOL ROAD N.E. SUITE 204
ALBUQUERQUE, NEW MEXICO 87110 Ph. (505) 250-1825

address) Ja	rdin	Office Building	PROJECT NO: 328	4-03
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(X) herewith	()	under separate cover via		
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DATE	REV. NO.	DESCRIP	PTION	ACTION
5/27/83		Reproducible Modified	Drainage Plan	R
5/13/83		Copy of Approved L	Drainzge Str. Deteil	s B
B. No action requi	red	f San Pfile	ature and forwarding as noted below und ARKS below	er REMARKS
		(with enclosures)		
	Fred Civil English Civil Engli	Fred Aguit Civil Engineary City of Albug Ro. Box 1293 Albuguergue, NSMIT: (X) herewith () () in accordance with 10 DUR: (X) approval () review & comment () use LLOWING: (X) Drawings () Change Order DATE REV. NO. 5/27/83 5/13/83 A Action indicated on item tra B. No action required C. for signature and return to	Frad Aguirra Civil Enginear/Hydrology City of Albuguarqua Rio. Rox 1293 Albuguarqua, NM 87103 Lansamit: (X) herewith () under separate cover via () in accordance with your request DUR: (X) approval () distribution to parties () info () review & comment (X) record () use () use () Shop Drawing Prints () Specifications (X) Shop Drawing Reproducibles () Change Order () DATE REV. NO. DESCRI 5/27/83 Reproducible Modified 5/13/83 Copy of Approved L A Action indicated on item transmitted B. No action required C. For signature and return to this office	DATE: 5/27/8 Frad Aguirra

CITY OF ALBUQUERQUE

DRAINAGE FACILITIES WITHIN CITY RIGHT-OF-WAY

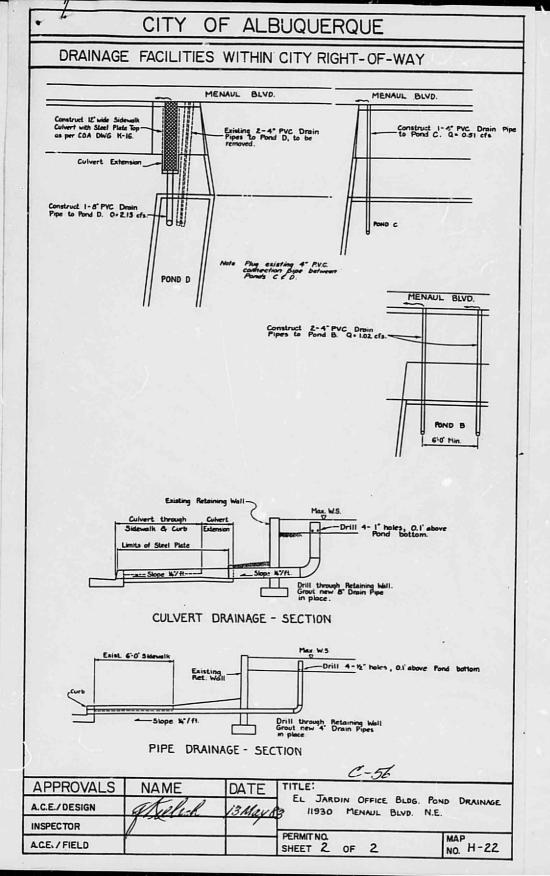


VICINITY MAP

NOTICE TO CONTRACTOR

- An excavation/construction permit will be required before beginning any work within City right-of-way. An approved copy of these plans must be submitted at the time of application for this permit.
 - All work detailed on these plans to be performed, except as otherwise stated or provided hereon, shall be constructed in accordance with "Contract Documents for City-Wide Utilities and Cash Paving No. 3/ "
 - Two working days prior to any excavation, contractor must contact Line Locating Service, 765-1234, for location of existing utilities.
 - Prior to construction, the contractor shall excavate and verify the horizontal and vertical locations of all obstructions. Should a conflict exist, the contractor shall notify the engineer so that the conflict can be resolved with a minimum amount of delay.
 - 5. Backfill compaction shall be according to <u>Arterial</u> street use.

APPROVALS	NAME	DATE	TITLE:
A.C.E./DESIGN	Kelick		EL JARDIN OFFICE BLOG. POND DRAINAGE
INSPECTOR /		100	11000 1101102 02101 1112,
ACE / FIELD		إسمار	SHEET 1 OF 2 MAP NO. H-22



e_h

ESPEY, HUSTON & ASSOCIATES INC. SUBJECT Drainage

EL Jardin

SHEET / OF ... BY ... BY DATE 4/1/83 CK BY

- Pond "A" receives 13.33% of the developed runoff or 1904 ft. 3
- Pond "B" receives 16.01% of the developed runoff or 2285 ft. 3 + 0.4 of the roof vol. or 1102 ft. 3
- Pond "C" receives discharge from 1 roof drain only plus routing from Pond "B" or 275.5 ft. 2 from drain
- Pond 'D" receives 22.69% of the developed runoff or 3240 ft. $\frac{3}{2}$ + 0.5 of the roof vol. or 1378 ft. $\frac{3}{2}$

ESPEY, HUSTON & ASSOCIATES INC. SUBJECT Drainage

Pond "A"

Runoff - A = 10,432 ft. = or 0.24 ac.

C = 1.0

I = 5.4 in/hr.

Q=1.30 cfs

Pond 'B"

Runoff - A=13,512 ft. = or 0.31 ac.

C = 1.0

I = 5.4 in./Lir.

Q = 1.67 cfs + 1.03 for roof drains = 2.70cfs

Pond "D"

Runoff - A = 20,690 ft. 2 or 0.47 ac

c = 1.0

I = 5.4 in./hr.

Q = 2.54 cfs + 1.29 cfs for roof drains = 3.83cfs

HYDROGRAPH COMPUTATION WORKSHEET

DATE 4/11/83
COMPUTED BY CHECK BY

PROJECT El Jardin Office		(t/T _p)	t (min.)	у	Q (cfs)
LOCATION Area A	1	0	0	0	0
	2	.1		.03	104
ANALYSIS POINT #	3	.2	2	.10	113
	4	.3	3	.190	.25
(DR. AREA) $A = 0.24$ ACRES	5	.4	4	.310	140
	6	.5	5	.470	.61
T _C /OMIN	7	.6	6	.660	,66
	8	.7	7	.820	1.07
POINT RAINFALL 2.5 IN. FROM PLATE 22.2 D-1	9	8.	8	.930	1.21
CH	10	1 .9	9	.990	1.29
CN = FROM PLATES 22.2 C-2, 22.2 C-3	11	1 1.0	10	1.00	1.30
NUMBER 1101 1115 D - IN SPON DI ATE 22 2 5 4	12	11.1	1 /1	.990	1.29
RUNOFF VOLUME R = IN. FROM PLATE 22.2 C-4	14	1.2	13	.860	1,12
COMPUTED T - MIN T - T	15	1.4	14	.780	1.01
COMPUTED $T_p = MIN. T_p = T_c$ (Rounded to even minute)	16	11.5	15	.680	.88
(kounded to even minute)	17	11.6	16	1.560	173
$q_p = \frac{45.4A}{I_p} = \frac{\text{CFS./INCH OF RUNOFF}}{\text{CFS./INCH OF RUNOFF}}$	18	11:7	1 17	.460	.60
чр	19	11.8	18	.390	, 51
	20	1 1.9	1 19	.330	,43
$(R \times q_p) = Q_{peak} = /.30$ CFS	21	1 2.0	20	.280	13/0
· · · · · · · · · · · · · · · · · · ·	22	1 2.2	22	1.207	1,29
$t(COLUMN)=(t/T_p)$ $t=T_p(t/T_p)$	23	1 2.4	24	.147	1.19
	24	1 2.6	26	1.107	1.14
	25	1 2.8	28	.077	1.10
$y = Q Q = y(Q_{peak})$	26	3.0	30	.055	1 .07
Qpeak	27	3.2	32	.040	.05
	28	3.4	34	1.029	,04
	29	3.6	36	.021	,03
	30	3.8	38	1.015	,02
	31	1 4.0	1 40	1.011	,01
	32	1 4.5	1 45	.005	.007
	33	5.0	50	.000	0

PLATE 22.2 F-1

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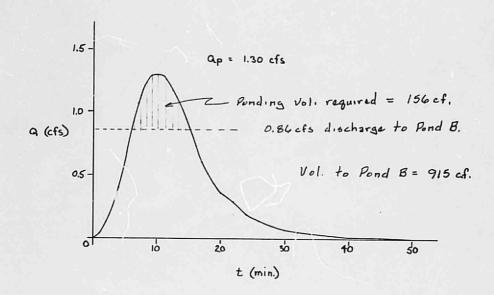
ESPEY, HUSTON & ASSOCIATES INC.
Engineering & Environmental Consultants

SUBJECT HYDROGRAPH

EL JARDIN OFFICE

SHEET / OF BY TOWERS
DATE 4/11/83 CK BY

AREA A

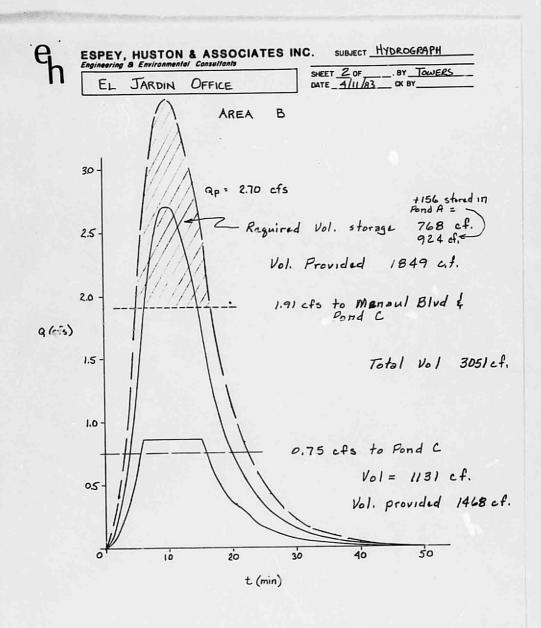


HYDROGRAPH COMPUTATION WORKSHEET

DATE		
COMPUTED	BY	
CHECK BY		ī

PROJECT El Jardin Office		(t/T _p)	t (min.)	у	(cfs)
LOCATION Area B	1	0	0	0	0
	2	.1		.03	.08
ANALYSIS POINT #	3	1 .2	2	.10	.27
	4	.3	3	.190	.51
(DR. AREA) $A = 0.31$ ACRES	5	.4	1 4	.310	.84
	6	.5	5	.470	1,2
T _C /O MIN	7	1 .6	6	.660	1.78
	8	1 .7	17	.820	2,21
POINT RAINFALL 2.5 IN. FROM PLATE 22.2 D-1	9	8.	8	.930	2.51
	10	1 .9	9	.990	2.67
CN = FROM PLATES 22.2 C-2, 22.2 C-3	11	1.0	1	1.00	2.70
	12	1.1	1 11	.990	2.67
RUNOFF VOLUME R = IN. FROM PLATE 22.2 C-4	13	1.2	13	.930	2.32
	14	11.3	14		2.11
COMPUTED $T_p = \frac{MIN.}{(Rounded to even minute)}$	15	11.4		1.780	1.84
	16	1 1.5	15	1.560	1.51
q _p = 45.4A = CFS./INCH OF RUNOFF	18	1 1:9	1 19	1460	1.24
qp = 43.4A Cr3./ Inch or Robot.	19	1 1.8	18	.390	1.05
	20	1 1.9	1 19	.330	.89
$(R \times q_p) = Q_{peak} = 2.70$ CFS	21	2.0	20	.280	.76
the A up - upeak	22	1 2.2	22	.207	1 ,5€
$t(COLUMN)=(t/T_p)$ $t=T_p(t/T_p)$	23	1 2.4	24	.147	,40
c(cocom)-(c/ip/ c-ip(c/ip/	24	1 2.6	26	.107	,29
	25	2.8	28	.077	.21
$y = Q$ $Q = y(Q_{peak})$	26	1 3.0	30	.055	.15
Jane 4 Stapeaks	27	1 3.2	32	.040	.11
speak .	28	3.4	34	.029	.08
	29	3.6	36	.021	.06
	30	3.8	3 B	.015	.04
	31	4.0	40	1.011	,03
	32	1 4.5	1 45	.005	.0
	33	1 5.0	50	.000	0

PLATE 22.2 F-1



HYDROGRAPH COMPUTATION WORKSHEET

DATE 4/11/83
COMPUTED BY
CHECK BY

PROJECT El Jardin Office		(t/T _p)	t (min.)	У	Q (cfs)
LOCATION Area C	1	0	0	0	0
ANALYSIS DOTHT 4	2	.1		.03	.1
ANALYSIS POINT #		.2	2	.10	.38
(DR. AREA) A =O.47 ACRES	4	.3	3	.190	.73
(UK. AKEA) A = OI4/ ALKES	5	1 .4	4	.310	1.19
Tc/OMIN	- 6	.5	5	.470	1.80
·c	8	1 .6	6	.820	2.53
POINT RAINFALL 2.5 IN. FROM PLATE 22.2 D-1	9	1 .8	8	.930	3.14
The state of	10	1 .9	3	.990	3.79
CN = FROM PLATES 22.2 C-2, 22.2 C-3	ii	1.0		1.00	3.83
	12	11.1	11	.990	3.79
RUNOFF VOLUME R = IN. FROM PLATE 22.2 C-4	13	1 1.2	12	.930	3.56
	14	11.3	13	.860	3.29
COMPUTED $T_p = \frac{MIN.}{(Rounded to even minute)}$	15	1.4	14	.780	2.99
(Rounded to even minute)	16	1 1.5	15	.680	2.60
	17	1.6	1 16	.560	2.14
$q_p = \frac{45.4A}{I_p} = $ CFS./INCH OF RUNOFF	18	11.7	17	.460	1.76
'p	19 20	1 1.8	18	.390	1.49
$(R \times q_p) = Q_{peak} = 3.83$ CFS	21	1 2.0	19	.330	1.26
(" * 4p) - 4peak Crs		1 2.2	22	.207	1.07
$t(COLUMN)=(t/T_p)$ $t=T_p(t/T_p)$	23	1 2.4	24	.147	.56
- ip(c/ip)		1 2.6	26	.107	.Ai
	25	1 2.8	28	-077	,29
$y = Q Q = y(Q_{peak})$	26	3.0	30	.055	.21
Qpeak	27	3.2	32	.040	.15
True .	28	3.4	34	.029	.11
	29	3.6	36	.021	.08
	30	3.8	38	.015	,06
	31	4.0	40	.011	.04
	32	4.5	1 45	.005	.02
	33	5.0	50	.000	0

ESPEY, HUSTON & ASSOCIATES INC. SUBJECT HYDROGRAPH
Engineering & Environmental Consultants

EL JARDIN OFFICE SHEET 3 OF BY TOWERS
DATE 4/11/83 OX BY

AREA C

