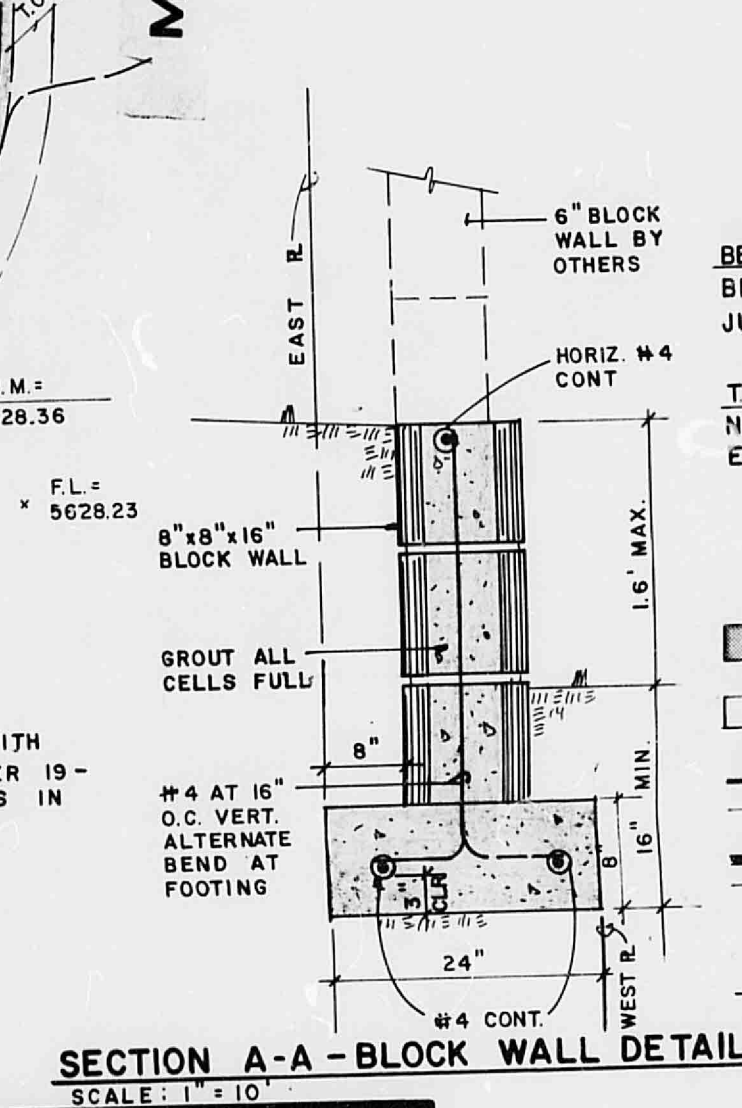
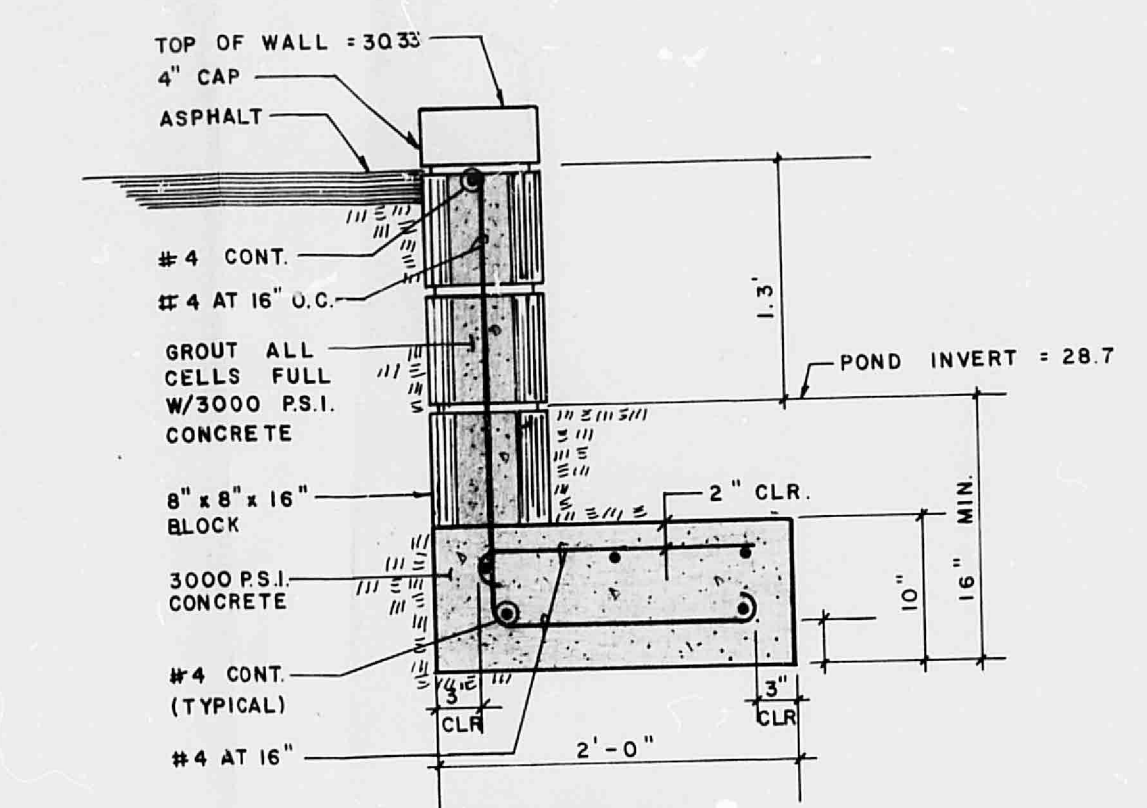
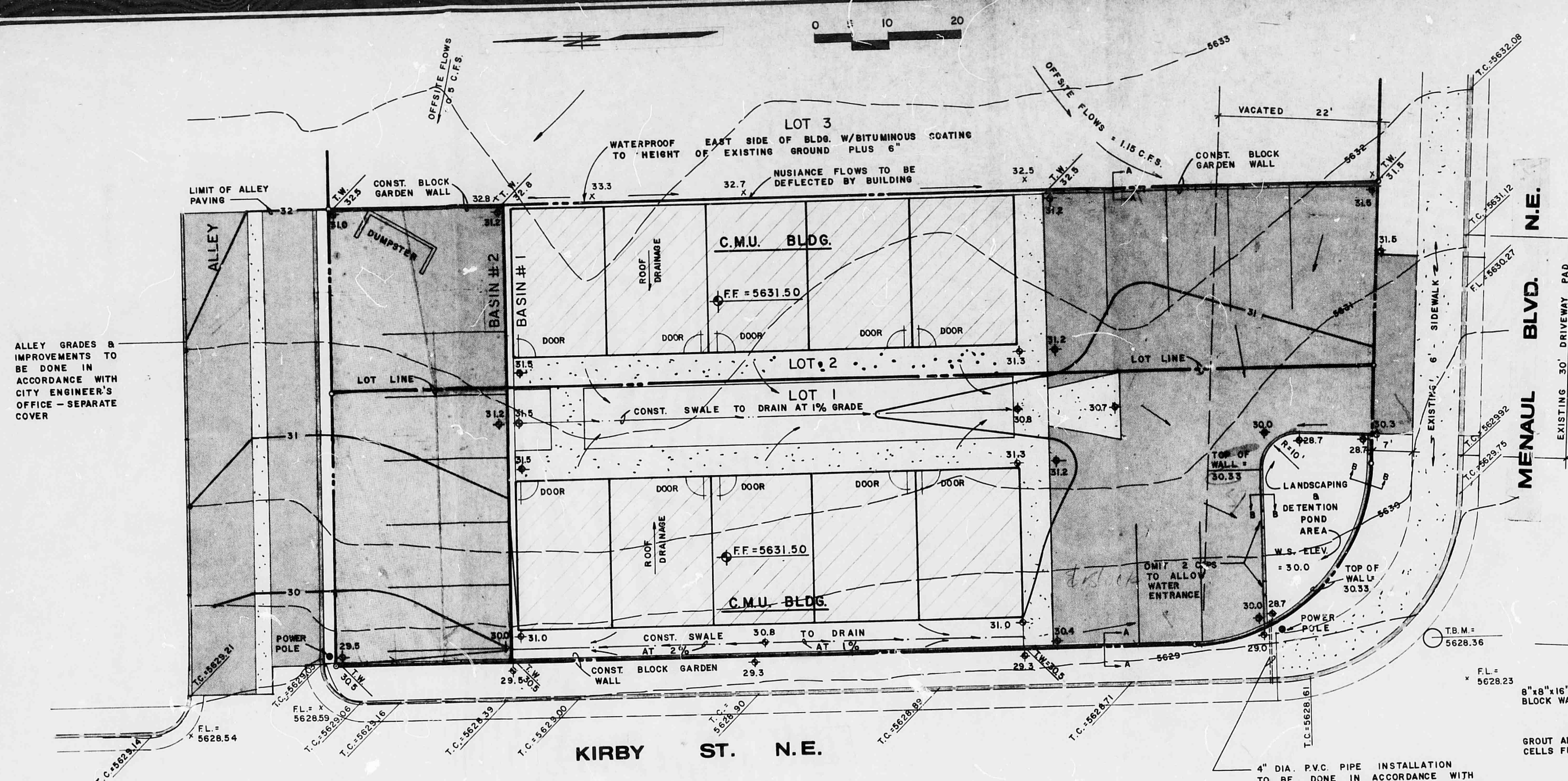
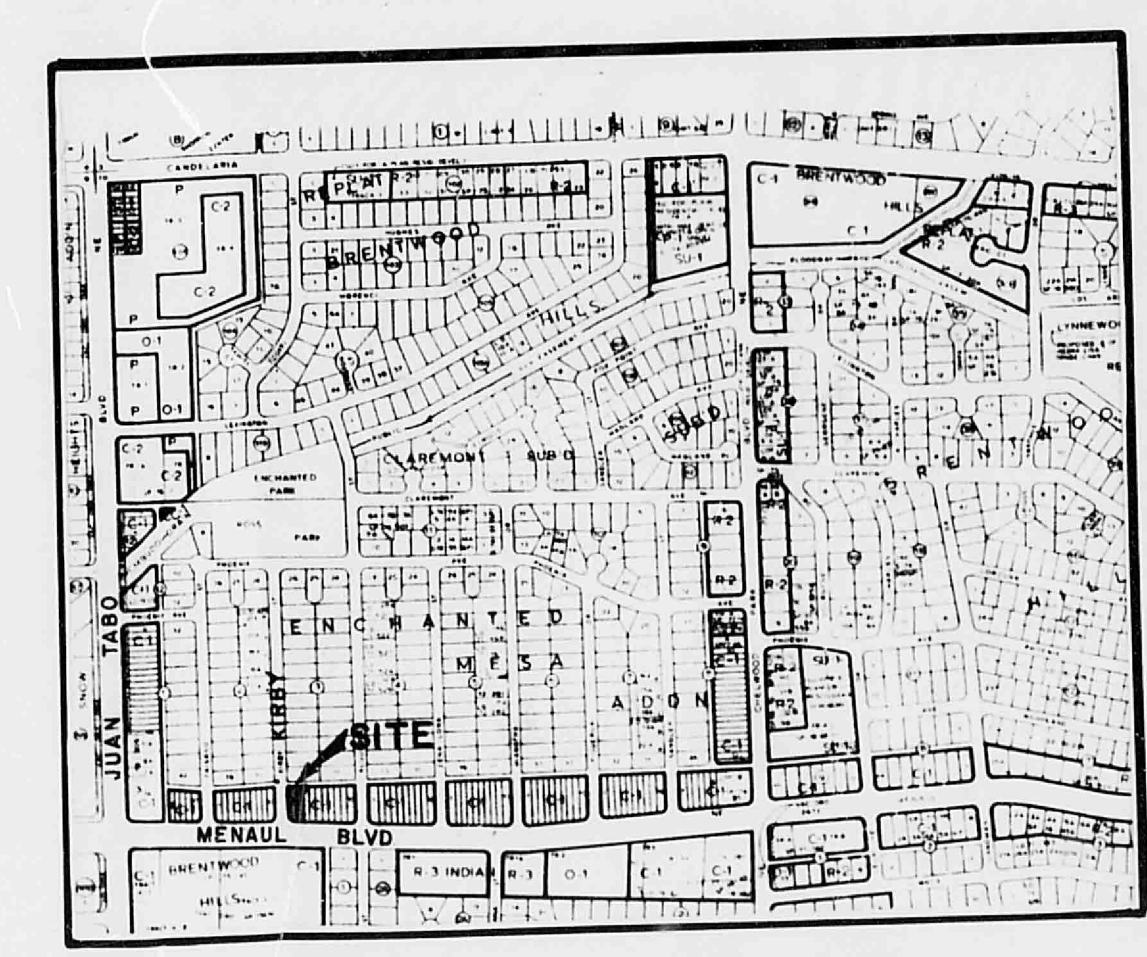
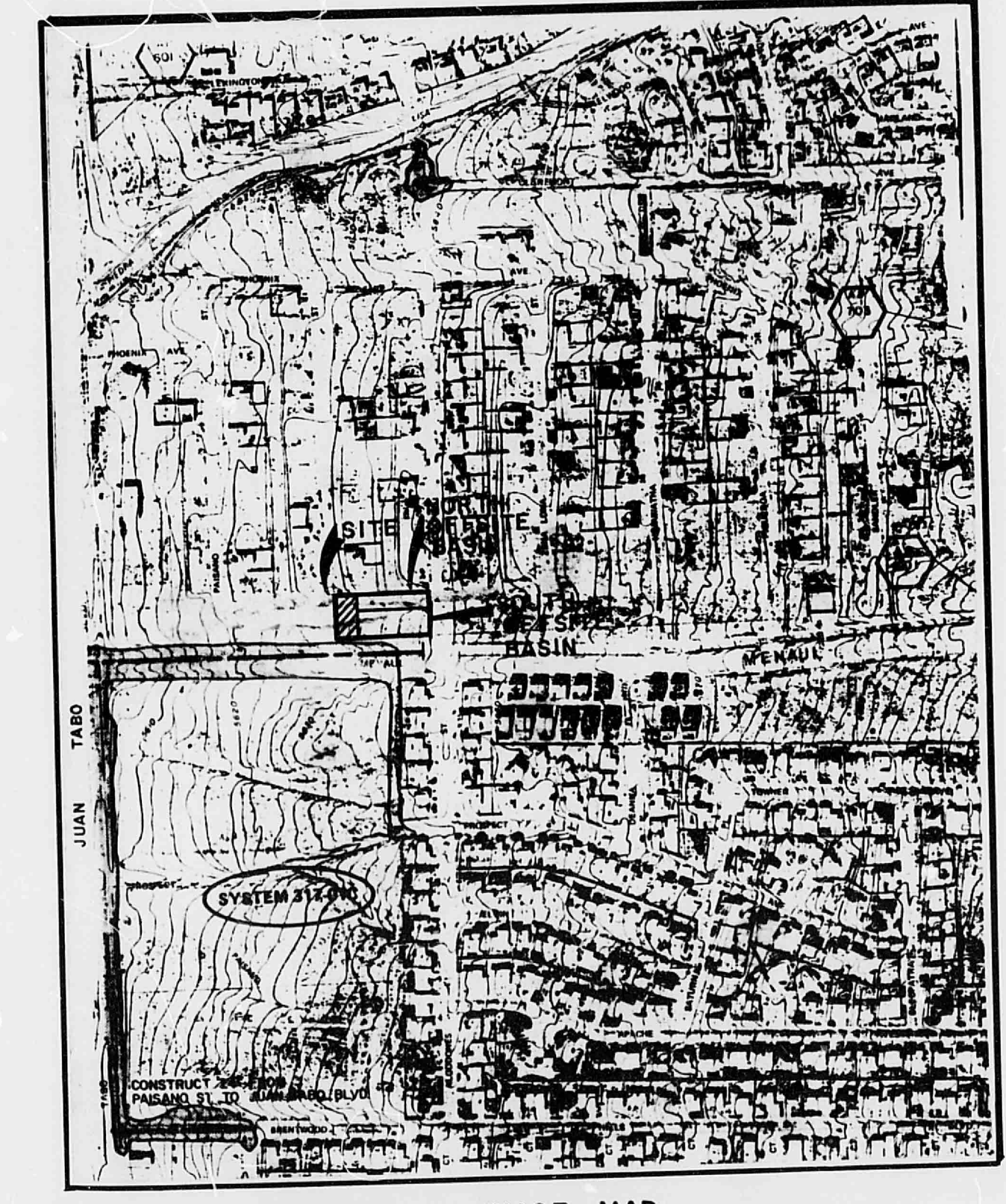




- LEGEND**
- ASPHALT PAVING
 - LANDSCAPED AREA
 - PROPERTY LINE
 - EXISTING CURB & GUTTER W/SIDEWALK
 - EXISTING SPOT ELEVATION
 - EXISTING CONTOUR
 - PROPOSED SPOT ELEVATION
 - PROPOSED FINISHED CONTOUR
 - SURFACE FLOW DIRECTION
 - BUILDING STRUCTURE



LEGAL DESCRIPTION

Lots numbered One (1) and Two (2) in block Three (3) of the Enchanted Mesa Subdivision; together with the Northerly 22 feet of Menaul Boulevard vacated by City Ordinance No. 2615 (B.49-P.948).

OFFSITE FLOWS

The offsite flows are generated by the undeveloped lots east of this site and are bound by Algodones Street, the alley and Menaul Road. The basin is comprised of two basins, the south offsite basin and the north offsite basin. The north basin is 10,500 sq. ft. (0.241 acres). The soil type is B, and is 100% pervious; therefore C = 0.40. The total 100 year flow rate, based upon I = 5.7 inches/hr. is: $Q = CIA = (.4)(5.7)(.241) = 0.5 \text{ c.f.s.}$

The south basin is 22,050 sq. ft. (0.506 acres). The soil type is B and is 100% pervious; therefore C = 0.40. The total 100 year flow rate is: $Q = CIA = (.4)(5.7)(.506) = 1.15 \text{ c.f.s.}$

UNDEVELOPED DRAINAGE CHARACTERISTICS - Onsite

1. Soil Type: Embudo Series, gravelly fine sandy loam, Group B soil.

2. Quantities

100% pervious: C = 0.4

$I = (2.15)(2.6) = 5.7 \text{ inches/hr.}$

$Q_{100} = CIA = (.4)(5.7)(.922/43560) = 0.5 \text{ c.f.s.}$

$Q_{10} = 0.657Q_{100} = 0.657(0.9) = 0.6 \text{ c.f.s.}$

$V_{100} = 1/2 (.9)(50)(60) = 1350 \text{ c.ft.}$

$Q_{\text{allowed}} = \text{Ca } 298 \text{ w/h } = 1.5 \text{ ft.}$

$Q_{\text{allowed}} = (.6)(0.087) 64.4(1.5) = 0.5 \text{ c.f.s.}$

DEVELOPED DRAINAGE CHARACTERISTICS - Onsite

1. OFFSITE FLOWS: The flows will be deflected as shown to the alley or to Menaul. This is a temporary situation until sites are developed and developed flows will be controlled discharge to Menaul or alley.

2. ONSITE FLOWS: The site shall be split into two basins. Basin 1 comprises of the roof areas and the Menaul parking lot. These flows will

be routed through a landscaped pond area and discharge through a 4" dia. pipe to Menaul. Basin 2 shall free-discharge to the alley.

3. Quantities

Basin No. 1 (7,372 s.ft.)

0% pervious: C = 0.95

$I = (2.15)(2.6) = 5.7 \text{ inches/hr.}$

$Q_{100} = CIA = (0.95)(5.7)(7372/43560) = 0.9 \text{ c.f.s.}$

$Q_{10} = 0.657Q_{100} = 0.657(0.9) = 0.6 \text{ c.f.s.}$

$V_{100} = 1/2 (.9)(50)(60) = 1350 \text{ c.ft.}$

$Q_{\text{allowed}} = \text{Ca } 298 \text{ w/h } = 1.5 \text{ ft.}$

$Q_{\text{allowed}} = (.6)(0.087) 64.4(1.5) = 0.5 \text{ c.f.s.}$

DETENTION POND VOLUME REQUIRED (1st trial)

Detention Volume Required = $1/2(22.2)(0.9-0.5)(60) = 266 \text{ c.ft.}$

DETENTION POND VOLUME PROVIDED

Area = 274 s.ft.

$266 = 0.97 \text{ ft. deep say } 1.0 \text{ ft.}$

Try $h = 1.3'$

$Q = .6(.087) 64.4(1.3) = 0.48 \text{ c.f.s.}$

Detention Pond Volume Required = $1/2(50 - .48 - .48) [(.9 - .48)(60)] = 294$

294

$274 = 1.07 \text{ ft. try } h = 1.2 \text{ ft.}$

$Q = .6(.087) 64.4(1.2) = 0.46 \text{ c.f.s.}$

Detention Pond Volume Required = $1/2(50 - .46 - .46) [(.9 - .46)(60)] = 323$

323

$h = 323 = 1.2 \text{ c.f.s. } Q_{\text{allow}} = 0.46 \text{ c.f.s. @ } h = 1.2'$

274

4. Basin No. 2 (1550 sq. ft.)

0% pervious; therefore C = 0.95

$I = (2.15)(2.6) = 5.7 \text{ inches/hr.}$

$Q_{100} = 0.95(5.7)(1550/43560) = 0.19 \text{ c.f.s.}$

Total Developed Discharge = Basin 1 + Basin 2 = $0.46 + 0.19 = 0.65 \text{ c.f.s.}$

Approximately equal to the undeveloped 0.5 c.f.s. discharge.

CONSULTANTS, TERRA SOL INC.

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268-3444

PARK OFFICE - P.O. BOX 142 SANDIA PARK, N.M. 87047

281-1891

Revisions

Date

Designed

Drawn

Checked

7-83

7-83

7-83

7-83

GRADING/DRAINAGE PLAN FOR

BOB SHEPHERD

MENAU & KIRBY N.E.

RECEIVED

7/27/83

H22/D33

SHEET 1 OF 1