

# ANALYSIS OF UNSER & LADERA INTERSECTION FOR PAINTED SKY SUBDIVISION



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**SEPTEMBER 2000**



**PREPARED FOR:**

**WESTLAND DEVELOPMENT COMPANY, INC.  
401 COORS BLVD, NW  
ALBUQUERQUE, NM 87121**

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## I. INTRODUCTION

Westland Development Company, Inc proposes to build a subdivision called Painted Sky between Interstate 40 and Ladera Drive and west of Unser Blvd. Access to this site will be through the Unser and Ladera intersection. See Figure 1 for the vicinity map.

### A. Study Purpose

This study will identify the traffic impacts of the development on the Unser and Ladera Intersection and any required street improvements at that intersection.

### B. Study Procedure

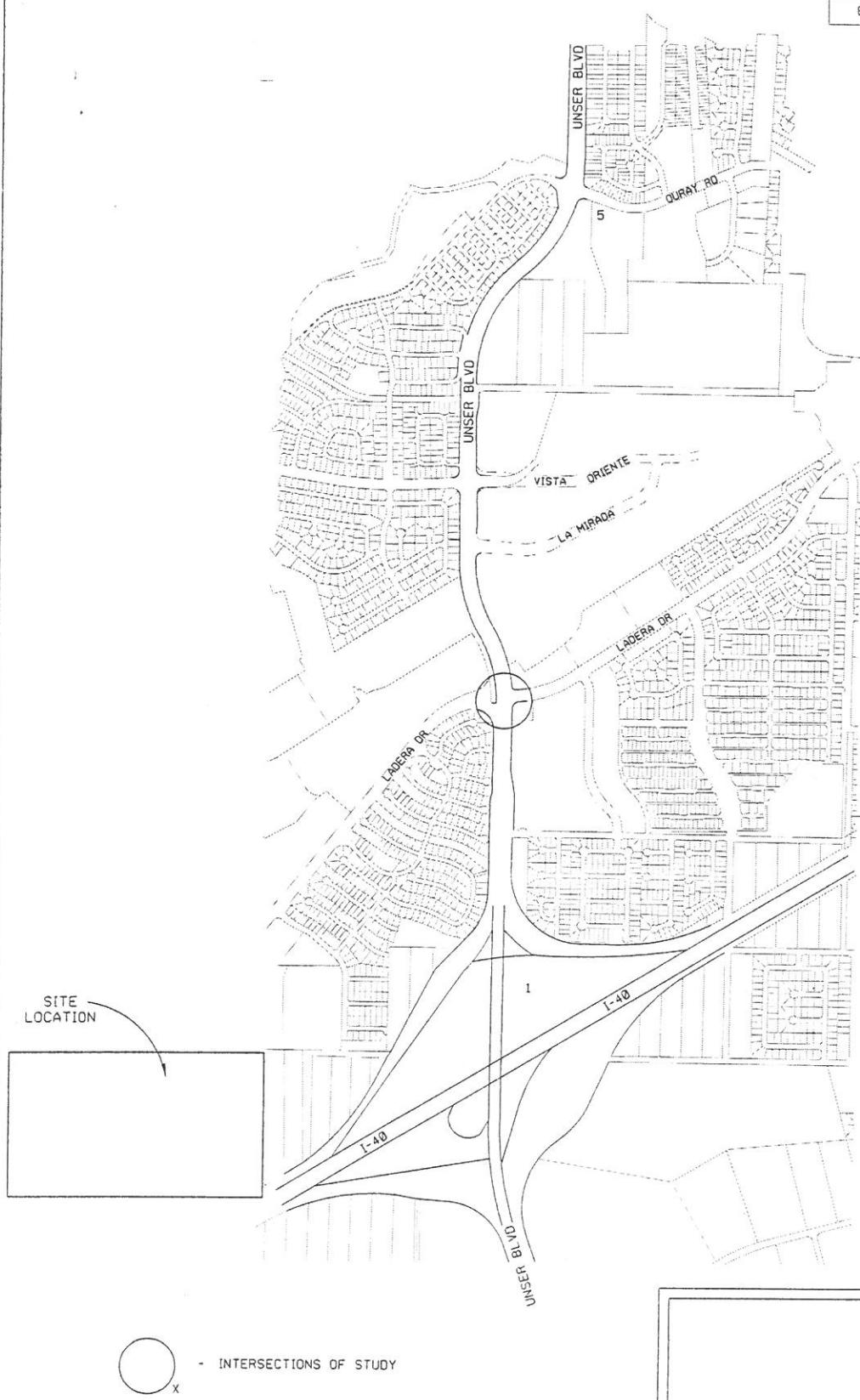
The Painted Sky Subdivision is proposed to be completely built out by 2005. This will be considered the build year. The following analysis included Existing, No Build in Year 2005 and Build in Year 2005. A scoping meeting was held with the City of Albuquerque. The Scoping letter is shown in Appendix A.

A previous traffic study was prepared in 1998 for the Westside Suntran Facility & Ladera Industrial Park. This site, which was located north and west of the Unser and Ladera Intersection, is no longer going to be constructed. In the industrial park study the intersections of Unser / I-40, Unser / Ladera, Unser / Vista Oriente, and Unser / Ouray were analyzed. All of these intersections had a level of service of A or B with full build out of the industrial park except for Unser / Ladera. Since the industrial park generated more trips than the Painted Sky Subdivision only the Unser / Ladera Intersection will be analyzed in this study.





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**FIGURE 1**  
VICINITY MAP

## II. EXISTING AREA CHARACTERISTICS

### A. Existing Geometry

The existing Unser and Ladera Intersection has 2 through lanes and a left turn lane on the north approach. The south approach has 2 through lanes, a left and a right turn lane. The west approach has 2 through lanes and 2 left turn lanes, one of which is striped out. The east approach has 1 through lane and 2 left turn lanes.

A third southbound through lane picks up south of the intersection and drops at the westbound I-40 on-ramp. The Unser overpass over I-40 has two through lanes in each direction.

### B. Existing Traffic Volumes

The City of Albuquerque Transportation Development Department provided existing traffic counts for the Unser/Ladera Intersection. The traffic count was performed in May 2000. Figure 2 on page 4 summarizes the a.m. and p.m. peak hour counts. The complete traffic count can be found in Appendix B.

### C. Existing Levels of Service

The existing intersection traffic volumes were analyzed using the signalized intersection methodology from the Highway Capacity Manual (HCM). Teapac software was used to perform the analysis. The individual runs for the a.m. and p.m. peak hours are included in Appendix C. The results are summarized in the following table.

**TABLE 1 – YEAR 2000 EXISTING CAPACITY ANALYSIS RESULTS**

INTERSECTION	2000 AM PEAK HOUR		2000 PM PEAK HOUR	
	DELAY	LOS	DELAY	LOS
Unser/Ladera	43.8	D	18.2	B

### III. 2005 BACKGROUND TRAFFIC PROJECTIONS

#### A. 2005 No-Build Traffic Projections

The Westside Suntran Facility & Ladera Industrial Park Traffic Impact Study analyzed the 1992 through 1996 Middle Rio Grande Council of Governments (MRGCOG) traffic flow maps to estimate the background traffic growth rate at 11%. A growth rate of 11% was not considered unreasonable given the rapid development of the area. The same growth rate was used for this analysis. The No-Build background traffic volumes are summarized in Figure 3, page 5.

#### B. 2005 No Build Intersection Capacity Analysis

The intersections within the study area were analyzed using the Highway Capacity Software (HCS). The following table is a summary of the results. The HCS output is included in Appendix D.

INTERSECTION	2005 AM PEAK HOUR		2005 PM PEAK HOUR	
	DELAY	LOS	DELAY	LOS
Unser/Ladera	111.1	F	56.5	E
Scenario 1	47.2	D	37.8	D
Scenario 2	47.9	D	32.7	C
Scenario 3	43.1	D	47.8	D

With the existing geometry the intersection operates at a level of service F in the a.m. peak hour and a level of service of E in the p.m. peak hour.

Scenario 1 in order to achieve a level of service of D for the intersection is to add a southbound through lane, restripe one of the eastbound through lanes to be a right turn only and add a northbound left turn lane. The dual northbound left turn lanes need to operate as permissive lefts in order for this alternative to work.

Scenario 2 using protected left turn movements only requires two additional southbound through lanes. One of the eastbound through lanes would still be redesignated as a right-turn only. This configuration gives a LOS of D in the AM Peak Hour and C in the PM Peak Hour.

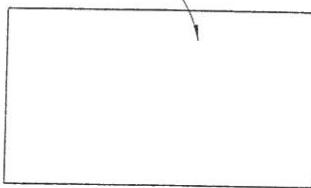
Scenario 3 is to add one southbound through lane, one westbound left turn lane, to redesignate one of the eastbound through lanes as a right-turn only and to add an eastbound right turn bay. This gives a LOS of D in the AM Peak Hour and D in the PM Peak Hour.

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SITE  
LOCATION

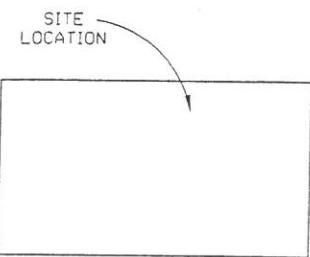


TURNING MOVEMENT COUNTS  
PEAK HOUR  
AM(PM)

PAINTED SKY SUBDIVISION  
2000 TRAFFIC VOLUMES  
FIGURE 2



NOT TO SCALE



TURNING MOVEMENT COUNTS  
PEAK HOUR  
AM(PM)

PAINTED SKY SUBDIVISION  
2005 NO BUILD  
FIGURE 3

## IV. PROPOSED SITE CHARACTERISTICS

### A. Trip Generation

The proposed Painted Sky Subdivision consists of 290 homes located south of Ladera and west of Unser Blvd. Projected trips for the proposed development were calculated from data in the Institute of Transportation Engineers (ITE) Trip Generation, 6<sup>th</sup> Edition, 1997. Trips were generated for 290 dwelling units of Single Family Detached Housing. The trip generation, which is summarized in the following table, can be found in Appendix E.

TABLE 3 – TRIP GENERATION						
Land use	Size	24 hr Volume	AM Peak Hour		PM Peak Hour	
		2 way volume	Enter	Exit	Enter	Exit
Single Family Homes	290	2775	55	162	189	104

### B. Trip Distribution and Assignment

The trip distribution calculated for the Westside Suntran Facility & Ladera Industrial Park Traffic Impact Study was used for the Painted Sky Subdivision. The trip percentages going to the industrial park were redistributed to the subdivision. The trip distribution for the Painted Sky Subdivision is shown in Figure 4 on page 8. The trip assignment volumes are shown in Figure 5 on page 9.

## V. 2005 BUILD TRAFFIC ANALYSIS

### A. 2005 Build Traffic Volumes

The trip assignments for the Painted Sky Subdivision were added to the 2005 No-Build Traffic Volumes to determine the 2005 Build Traffic Volumes. The Traffic Volumes are shown in Figure 6, page 10.

### B. 2005 Build Capacity Analysis

Using the HCM signalized intersection methodology, HCS and Teapac a capacity analysis was run for the Unser and Ladera intersection. The proposed geometry scenarios given for the No-Build were used as the starting point. The results are shown in Table 4 below.

INTERSECTION	2005 AM PEAK HOUR		2005 PM PEAK HOUR	
	DELAY	LOS	DELAY	LOS
Unser/Ladera				
Scenario 1	48.8	D	38.1	D
Scenario 2	47.7	D	35.3	D
Scenario 3	49.2	D	28.2	C

Scenario 1 - One alternative for the ~~No-Build~~ was to add one southbound through lane, a northbound left turn lane, and redesignate one of the eastbound through lanes as a right turn only. In the Build condition an additional EB left turn is needed also. If the left turn movements are allowed to operate permissively then a level of service of D is achieved.

Scenario 2 - Another ~~No-Build~~ alternative was to add two southbound through lanes and to redesignate one of the two eastbound through lanes as a right-turn only. In the Build condition an additional NB through lane is also needed in order to achieve a LOS of D. One northbound through lane could be picked up where the I-40 westbound off-ramp merges with Unser and carried through the intersection. One southbound through lane could be carried through the intersection and tied to the existing three southbound lanes south of the intersection. One of these lanes drops at the I-40 westbound on-ramp. A fourth southbound through lane seems impractical given the existing geometry. In order for the lane to carry any traffic, the I-40 Unser Overpass would need to be widened to accommodate three lanes southbound.

Scenario 3 - A third ~~No-Build~~ alternative is to add one southbound through lane, one westbound left turn lane and to redesignate one of the eastbound through lanes as a right-turn

CLARIFY

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OR

TABLE

only. In the Build condition an additional eastbound right turn lane is needed in order to achieve a LOS of D.

The capacity analysis can be found in Appendix F.

### C. Queuing Analysis

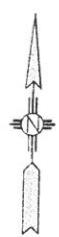
A queuing analysis of the Unser and Ladera Intersection was conducted. This analysis utilized a Poisson distribution with a 95% service rate to determine the length of queue required at each intersection. The queue length recommendations contained in the following table should be included in the final design of the intersection. The worst case condition (a.m. or p.m. peak hour) was evaluated to determine the necessary length of turning bay. The full analysis can be found in Appendix G.

**TABLE 5 – QUEUING DISTANCES SCENARIO 1**

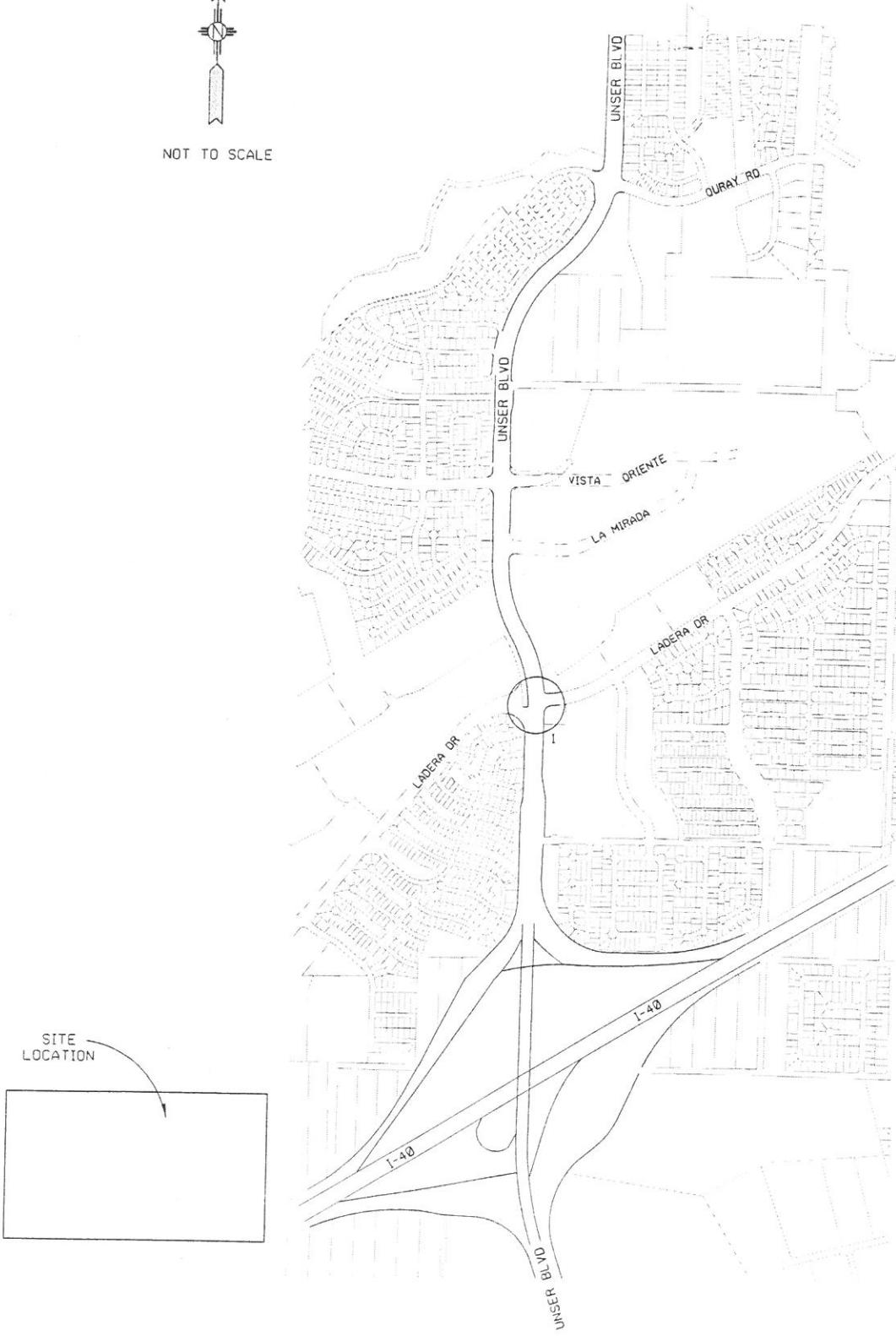
INTERSECTION	2005-BUILD (ft)
Unser/Ladera	
Dual EB LT	50
SB LT	100
Dual WB LT	250
Dual NB LT	138

**TABLE 6 – QUEUING DISTANCES SCENARIO 3**

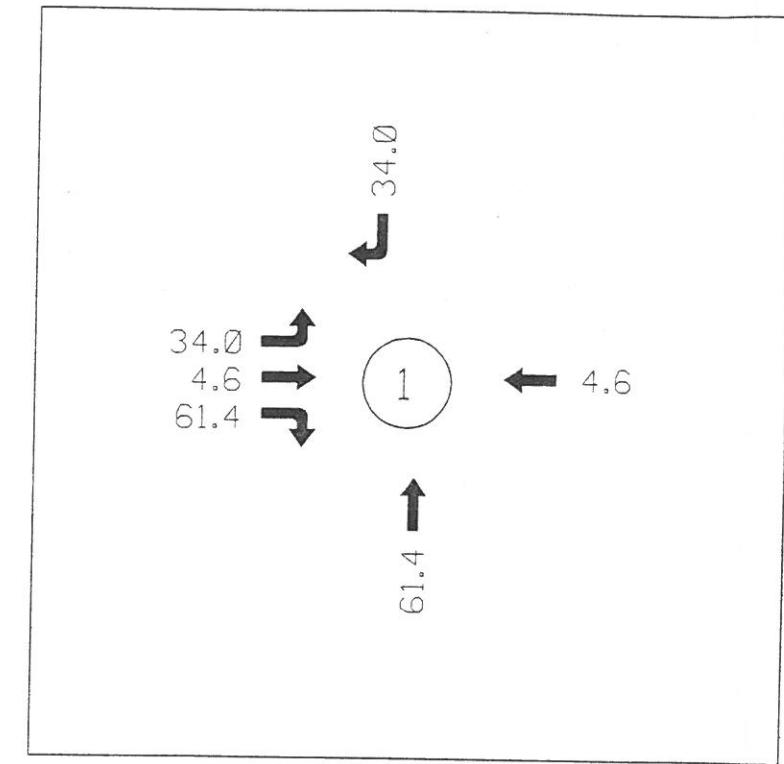
INTERSECTION	2005-BUILD (ft)
Unser/Ladera	
EB LT	100
SB LT	100
Triple WB LT	260
NB LT	150



NOT TO SCALE

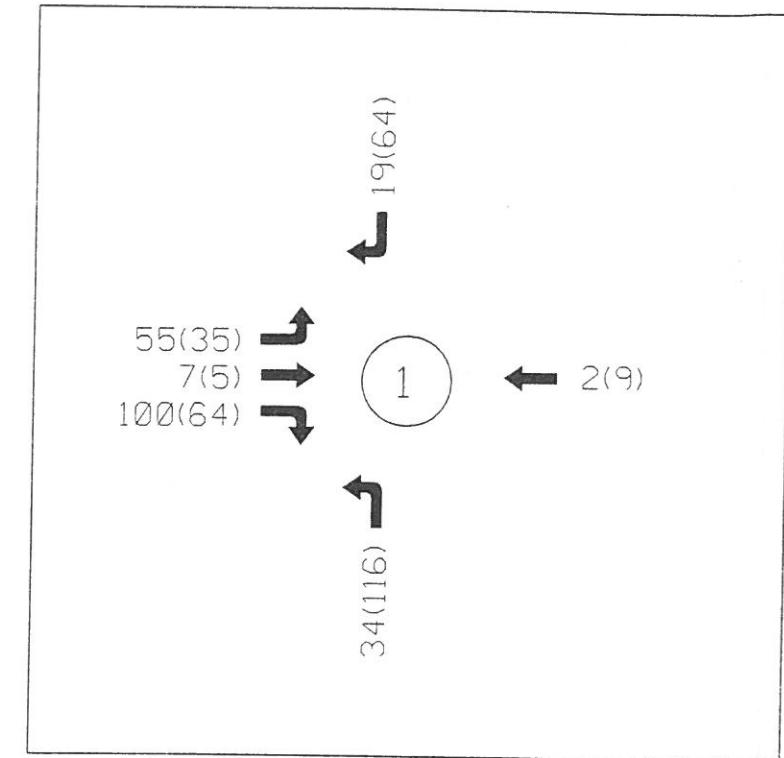
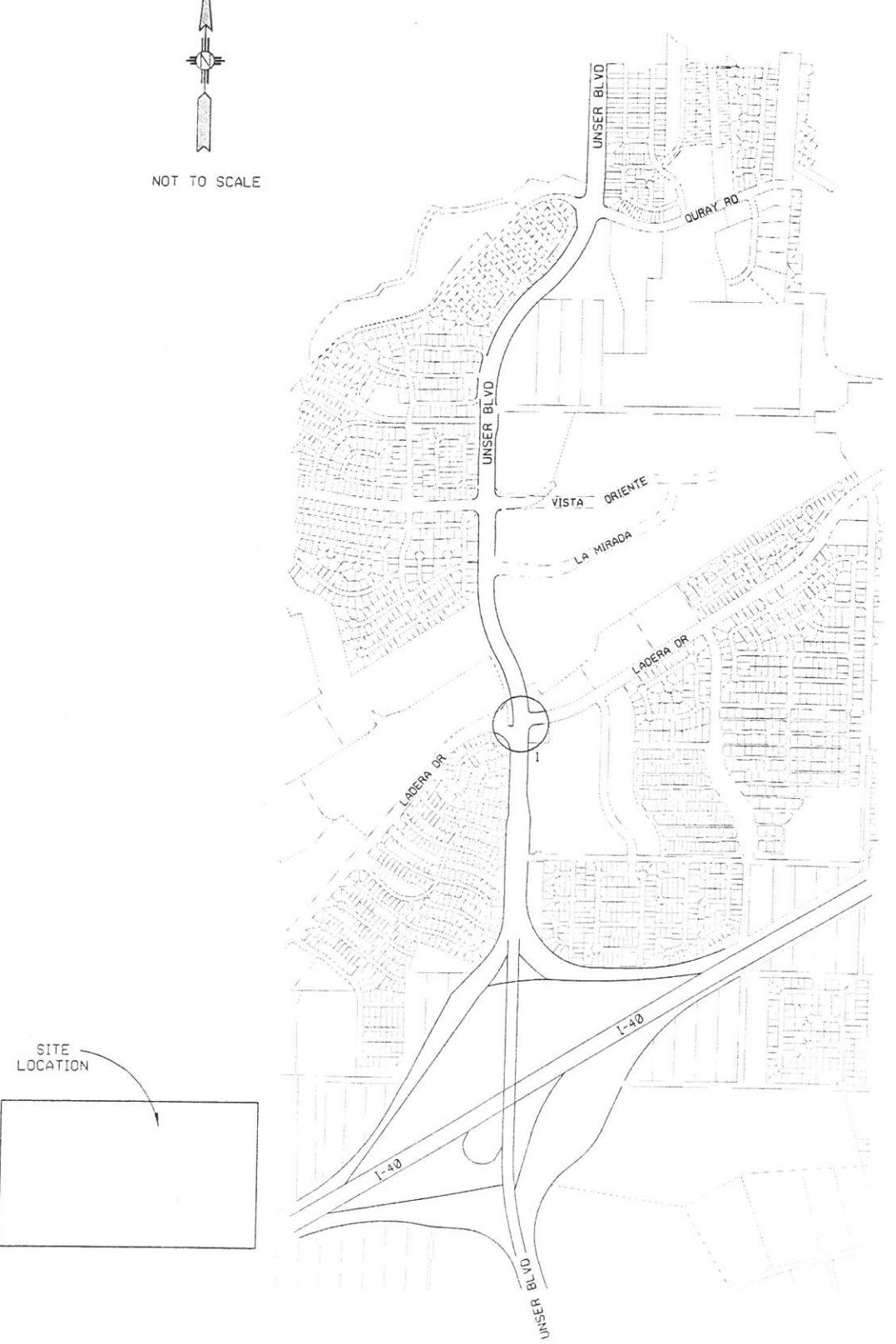


TRAFFIC JAW TRANSITION



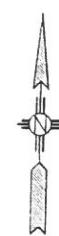
PAINTED SKY SUBDIVISION  
TRIP DISTRIBUTION PERCENTAGES  
FIGURE 4

NOT TO SCALE

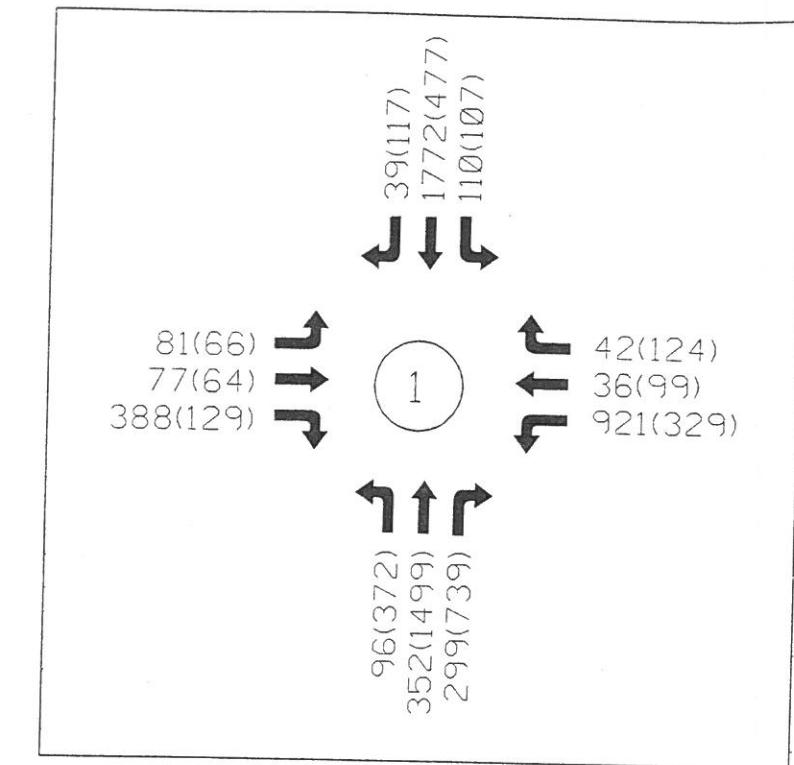


TURNING MOVEMENT COUNTS  
PEAK HOUR  
AM(PM)

PAINTED SKY SUBDIVISION  
TRIP ASSIGNMENT VOLUMES  
FIGURE 5



NOT TO SCALE



TURNING MOVEMENT COUNTS  
PEAK HOUR  
AM(PM)

PAINTED SKY SUBDIVISION  
2005 BUILD  
FIGURE 6

## VI. CONCLUSIONS AND RECOMMENDATIONS

In the existing condition the intersection of Unser and Ladera operates at a D in the a.m. peak hour and B in the p.m. peak hour.

The results of the 2005 No-Build capacity analysis indicate that the intersection will fail. The traffic volume exceeds the capacity available with the existing laneage. Three scenarios were developed for additional laneage at the intersections.

In Scenario 1 an additional southbound through lane and northbound left turn lane are required, as well as, signing one of the eastbound through lanes as a right turn only lane. If the left turn movements are allowed to operate permissively then a level of service of D is achieved.

In Scenario 2 two additional southbound through lanes are needed, as well as, signing one of the eastbound through lanes as a right turn only lane.

In Scenario 3 one additional southbound through lane and one additional westbound left turn lane are needed, as well as, signing one of the eastbound through lanes as a right turn only lane.

In the Build condition the additional capacity needed in order to operate at a LOS of D is as follows:

In Scenario 1 an additional eastbound left turn is needed. The pavement for an additional left turn is already in place. Only striping and removal of the existing markings would be required. This left turn movement, as well as, the northbound left turn would need to operate permissively in order to achieve a LOS of D. There is at least one other case of permissive and protected double left turns in Albuquerque at the intersection of Wyoming and Academy which seems to work fine. Since the median on Unser is wide, it would be possible to move the northbound left turn bay over in order to improve the sight distance where the opposite through movements are heavy.

In Scenario 2 an additional northbound through lane is needed. This scenario is not recommended because although the northbound through lane could be easily constructed, only one southbound through lane makes sense with the existing road network.

In Scenario 3 an additional eastbound right turn bay is needed. This scenario involves installing a triple left westbound. While there is no existing triple left turn in Albuquerque, the New Mexico State Highway & Transportation Department is in the process of designing one at the intersection of Paseo del Norte and Pan American East.

CLARIFY

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TABLE

**APPENDIX A**

**SCOPING LETTER**

**STANDARD LETTER**  
**SCOPE OF TRAFFIC IMPACT STUDY (TIS)**

**TO:** Jeanette Walther  
Bohannan-Huston, Inc.  
Courtyard One  
7500 Jefferson St. NE  
Albuquerque, New Mexico 87109

**MEETING DATE:** August 8, 2000

**ATTENDEES:** Jeanette Walther & Kevin Patton, Bohannan-Huston; Tony Loyd, Transportation Development, COA.

**PROJECT:** Painted Sky Development

**REQUESTED CITY ACTION:** Zone Change Site Development Plan

Subdivision Building Permit Sector Plan Sector Plan Amendment  
Curb Cut Permit Conditional Use Annexation Site Plan Amendment

**ASSOCIATED APPLICATION:** 290 single family du development.

The Traffic Impact Study should follow the standard report format which is outlined in the DPM. The following supplemental information is provided for the preparation of this specific study. As each item identified in the scoping letter is completed, check the appropriate  (box).

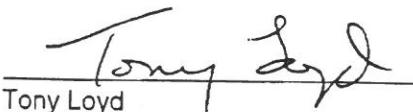
- 1. Trip Generation - Use Trip Generation Manual, 6th Edition.  
Consultant to provide.
- 2. Appropriate study area:  
Signalized Intersections: Ladera/Unser;  
  
Unsignalized Intersections: n/a;  
  
Driveway Intersections: n/a.
- 3. Intersection turning movement counts.  
Intersections provided: signalized intersections above;  
  
Intersections which need to be counted by developer: unsignalized.
- 4. Existing traffic signal timing and synchronization.  
Intersections provided: signalized intersections above.
- 5. Type of intersection progression and factors to be used.  
Type III arrival type (see HCM Special Report 209) unless otherwise justified, peak hour factors and % heavy commercial should be taken directly from the MRGCOG turning movement data provided and the lost time factor should be 5.0 seconds for all phases (a possible reduction may be taken for five to eight phase operations).
- 6. Boundaries of area to be used for trip distribution.  
City Wide - residential.

Painted Sky Development

Page 3 of 3

14. Items to be included in the study:
- a. Intersection analysis.
  - b. Recommended street, intersection and signal improvements.
  - c. Site design features such as turning lanes, median cuts, queuing requirements and site circulation, including driveway signalization and visibility.
  - d. Transportation system impacts.
  - e. Other mitigating measures.
15. Number of copies of report required 1  
Executive Summary Required yes x no  
(12 copies if required)

The Traffic Impact Study for this development proposal, Painted Sky Development, shall be performed in accordance with the above criteria. If there are any questions regarding the above items, please contact me at 924-3994.

  
Tony Loyd  
Tony Loyd  
Transportation Development Section

August 16, 2000  
Date

cc: TIS Task Force attendees  
file

**APPENDIX B**

**EXISTING TRAFFIC COUNT**

MIDDLE RIO GRANDE COUNCIL OF GOVRNMTS  
INTERSECTION TURNING MOVEMENT COUNT

MIDDLE RIO GRANDE COG

Two Vehicle Analysis with Right on Red

Page: 1

Date: 5/19/200

Location: 025650

Starts : 05/15/00 at 06:45:0

Notes : LADERA DR - UNSER BD

Ends : 05/15/00 at 18:00:0

Study ID: 00

Interval : 15 min Intervals: 45

Operator: CM 168

S/N : 168 Type: C,Tr,Ped-rt/red

Weather : BERNALILLO

Correction: 1.00

\*\*\*\*\*

Begins		From North				From South				From East				From West				Interval
		RtRed	Left	Thru	Right	RtRed	Left	Thru	Right	RtRed	Left	Thru	Right	RtRed	Left	Thru	Right	
Mon 5/15/2000																		
6:45	Auto	0	8	182	2	30	7	41	16	1	87	2	1	18	1	5	12	413 <
	Truck	0	1	2	0	0	2	2	0	0	2	0	0	0	0	0	0	9
7:00	Auto	0	17	235	3	13	9	42	18	4	163	4	1	16	2	8	23	558 <
	Truck	0	0	2	1	1	1	1	2	0	0	2	1	1	0	0	1	13
7:15	Auto	1	20	306	2	31	9	65	21	2	136	6	5	24	5	13	31	677 <
	Truck	0	0	0	0	0	0	1	0	0	2	1	0	0	0	0	0	4
7:30	Auto	0	17	328	2	10	9	57	37	2	152	3	3	11	5	12	46	694 <
	Truck	0	1	2	0	0	1	6	0	0	4	0	0	0	0	0	0	14
7:45	Auto	0	16	269	3	15	11	54	41	4	134	6	5	12	5	10	20	605 <
	Truck	0	0	1	1	0	0	1	4	0	3	0	0	0	0	2	1	13
8:00	Auto	0	17	214	7	14	17	61	21	2	109	1	2	19	8	5	26	523 <
	Truck	0	1	0	0	3	1	4	3	0	2	1	0	0	0	0	0	15
8:15	Auto	0	16	128	5	19	27	50	34	2	71	10	4	7	3	6	1	383 <
	Truck	0	0	4	0	2	2	4	3	0	1	1	0	0	0	0	0	17
8:30	Auto	1	9	88	9	28	41	55	26	5	70	9	6	16	5	2	13	383 <
	Truck	0	0	2	3	3	8	2	0	0	0	3	0	4	0	1	2	28
8:45	Auto	7	5	57	6	18	27	48	5	1	57	4	4	4	0	8	4	255 <
	Truck	0	1	0	1	2	0	3	4	0	1	0	0	0	0	0	1	13
9:00	Auto	1	9	75	2	16	18	43	9	5	38	5	1	2	6	11	9	250 <
	Truck	0	0	1	0	1	0	1	1	0	0	1	0	0	0	2	0	7
9:15	Auto	1	8	45	4	12	12	32	4	3	28	12	4	3	0	3	0	171 <
	Truck	0	0	3	0	1	0	2	0	0	1	1	0	0	0	1	0	9
9:30	Auto	0	9	50	0	16	10	35	6	7	43	8	2	8	4	4	1	203 <
	Truck	0	0	1	0	1	0	1	1	0	0	1	0	0	0	0	0	5
9:45	Auto	0	0	5	0	0	0	0	0	0	0	0	0	0	0	0	0	0 <
	Truck	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:00	Auto	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0 <
	Truck	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:15	Auto	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0 <
	Truck	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:30	Auto	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0 <
	Truck	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:45	Auto	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0 <
	Truck	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:00	Auto	0	10	38	0	22	17	45	16	5	28	4	3	4	4	5	2	203 <
	Truck	0	0	1	0	2	0	1	0	0	3	0	0	0	0	0	0	7
11:15	Auto	1	9	42	2	19	10	49	12	5	33	5	1	6	2	7	3	206 <
	Truck	0	0	3	0	1	3	1	3	0	3	0	0	0	0	0	0	14
11:30	Auto	2	10	53	5	20	16	52	24	6	33	13	3	4	3	4	2	250 <
	Truck	0	1	0	1	1	1	3	2	0	0	1	0	0	0	0	1	11

MIDDLE RIO GRANDE COG  
Two Vehicle Analysis with Right on Red

Page: 2  
Date: 5/19/2000

\*\*\*\*\*  
Location: 025650  
Notes : LADERA DR - UNSER BD  
Study ID: 00  
Operator: CM 168  
Weather : BERNALILLO

Starts : 05/15/00 at 06:45:0  
Ends : 05/15/00 at 18:00:0  
Interval : 15 min Intervals: 45  
S/N : 168 Type: C,Tr,Ped-rt/red  
Correction: 1.00

Begins		From North			From South			From East			From West			Interval				
		RtRed	Left	Thru Right	RtRed	Left	Thru Right	RtRed	Left	Thru Right	RtRed	Left	Thru Right					
<b>Mon 5/15/2000</b>																		
11:45	Auto	1	11	45	1	24	11	49	20	5	50	9	5	7	4	10	7	259 <
	Truck	0	0	2	0	4	0	0	4	0	1	1	0	0	1	1	0	14
12:00	Auto	0	11	51	2	27	22	52	31	4	46	7	1	10	3	7	2	276 <
	Truck	0	1	1	0	1	0	2	3	0	5	1	0	0	0	0	1	15
12:15	Auto	0	10	40	3	21	22	55	17	5	33	9	7	6	2	6	3	239 <
	Truck	0	0	0	0	2	0	2	3	0	3	2	0	0	1	0	0	13
12:30	Auto	0	10	39	4	25	20	57	26	4	47	6	3	9	7	3	8	268 <
	Truck	0	0	2	2	1	1	2	2	1	1	0	0	0	2	1	1	16
12:45	Auto	0	10	59	6	20	12	57	23	3	46	10	4	7	2	6	3	268 <
	Truck	0	0	3	0	0	0	0	2	0	3	1	1	0	0	0	0	10
13:00	Auto	0	14	51	1	28	14	61	16	3	41	4	1	11	4	5	5	259 <
	Truck	0	0	1	1	4	0	3	0	0	1	1	0	1	0	0	0	12
13:15	Auto	0	7	39	1	18	15	57	20	6	35	6	2	7	4	6	3	226 <
	Truck	0	1	0	0	0	0	1	1	0	3	2	0	1	0	1	0	10
13:30	Auto	1	7	45	3	24	9	57	27	5	40	5	4	11	0	7	2	247 <
	Truck	0	0	0	1	3	0	4	2	1	1	0	0	1	0	0	0	13
13:45	Auto	0	15	35	4	29	13	78	30	2	38	2	4	9	3	1	2	265 <
	Truck	0	0	2	0	0	0	1	1	0	1	0	1	0	0	0	1	7
14:00	Auto	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0 <
	Truck	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
14:15	Auto	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0 <
	Truck	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
14:30	Auto	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0 <
	Truck	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
14:45	Auto	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0 <
	Truck	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
15:00	Auto	1	11	55	3	22	31	83	51	9	35	8	0	9	3	4	2	327 <
	Truck	0	0	4	0	0	1	1	0	0	2	0	0	0	0	0	0	8
15:15	Auto	1	18	63	11	49	40	132	56	6	46	12	2	13	4	3	0	456 <
	Truck	1	0	2	0	0	3	1	3	2	2	2	0	1	1	0	0	18
15:30	Auto	0	6	51	8	28	35	156	52	12	58	8	2	34	22	13	8	493 <
	Truck	0	2	3	0	0	5	2	3	0	0	1	0	3	1	0	2	22
15:45	Auto	0	13	60	4	36	28	131	42	17	84	6	1	11	7	17	24	481 <
	Truck	0	0	4	0	0	0	1	1	0	5	2	0	3	1	2	1	20
16:00	Auto	0	15	80	3	36	31	130	61	14	48	5	2	11	2	6	0	444 <
	Truck	0	0	4	0	1	0	2	4	0	1	1	0	2	0	0	0	15
16:15	Auto	0	8	56	0	33	24	178	68	10	45	16	7	5	3	6	3	462 <
	Truck	0	0	2	0	0	0	2	0	0	0	0	0	0	0	0	0	4
16:30	Auto	0	14	84	3	47	35	184	74	7	53	11	8	15	7	8	3	553 <
	Truck	0	0	0	0	1	0	1	1	0	1	0	0	0	0	0	0	4
16:45	Auto	1	13	76	7	51	37	199	59	7	67	11	3	9	8	4	1	553 <
	Truck	0	0	0	0	0	0	4	2	0	2	0	0	0	1	0	0	9

## MIDDLE RIO GRANDE COG

Two Vehicle Analysis with Right on Red

Page: 3

Date: 5/19/2000

Location: 025650

Starts : 05/15/00 at 06:45:00

Notes : LADERA DR - UNSER BD

Ends : 05/15/00 at 18:00:00

Study ID: 00

Interval : 15 min Intervals: 45

Operator: CM 168

S/N : 168 Type: C,Tr,Ped-rt/red

Weather : BERNALILLO

Correction: 1.00

\*\*\*\*\*

Begins		From North				From South				From East				From West				Interval
		RtRed	Left	Thru	Right	RtRed	Left	Thru	Right	RtRed	Left	Thru	Right	RtRed	Left	Thru	Right	
<b>Mon 5/15/2000</b>																		
17:00	Auto	0	16	78	9	43	37	221	82	10	42	16	7	18	5	9	0	593 <
	Truck	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	1
17:15	Auto	1	15	79	5	44	38	297	54	8	52	9	12	2	7	13	3	639 <
	Truck	0	0	2	0	1	0	2	0	0	2	0	0	0	0	0	0	7
17:30	Auto	0	23	86	13	48	48	205	80	10	72	13	9	9	5	7	1	629 <
	Truck	1	0	1	0	1	0	1	0	0	0	0	0	0	0	0	0	4
17:45	Auto	1	15	62	4	49	41	241	74	17	43	20	6	8	3	9	1	594 <
	Truck	0	0	0	0	0	1	0	1	0	1	0	0	0	0	0	0	3

## MIDDLE RIO GRANDE COG

## Two Vehicle Analysis with Right on Red

Page: 4

Date: 5/19/200

\*\*\*\*\*  
 Location: 025650 Starts : 05/15/00 at 06:45:0  
 Notes : LADERA DR - UNSER BD Ends : 05/15/00 at 18:00:0  
 Study ID: 00 Interval : 15 min Intervals: 45  
 Operator: CM 168 S/N : 168 Type: C,Tr,Ped-rt/red  
 Weather : BERNALILLO Correction: 1.00  
 \*\*\*\*\*

	From North				From South				From East				From West				Total
	RtRed	Left	Thru	Right	RtRed	Left	Thru	Right	RtRed	Left	Thru	Right	RtRed	Left	Thru	Right	
Grand																	
Total Auto	21	442	3349	147	985	803	3409	1253	218	2203	285	135	375	158	253	274	14310
%	0.1	3.0	22.8	1.0	6.7	5.5	23.2	8.5	1.5	15.0	1.9	0.9	2.5	1.1	1.7	1.9	97.3%
Truck	2	9	55	11	37	30	65	56	5	57	26	3	17	8	11	12	404
%	0.0	0.1	0.4	0.1	0.3	0.2	0.4	0.4	0.0	0.4	0.2	0.0	0.1	0.1	0.1	0.1	2.7%
All	23	451	3404	158	1022	833	3474	1309	223	2260	311	138	392	166	264	286	14714
%	0.2	3.1	23.1	1.1	6.9	5.7	23.6	8.9	1.5	15.4	2.1	0.9	2.7	1.1	1.8	1.9	100.0%

## TURNING MOVEMENTS FOR TOTAL INTERSECTION

	From North	
	.	
	Total 8037	
	Approach 4036	Depart 4001
	.	.
	Rt/red Right Thru Left .	
	23 158 3404 451 .	166 3474 361

181		223 Rt/Red
Depart 1325	311	138 Right
	833	311 Thru Approach 2932
		2260 Left
Total 2433 .....		..... Total 5978

From West N W + E S From East

Left	166	451
Approach 1108	Thru 264	264 Depart 3046
Right 286		2331
Rt/Red 392		

678 3404 2260 .	833 3474 1309 1022
.	Left Thru Right Rt/Red
.	
Depart 6342 .	Approach 6638
Total 12980 .	
.	
From South	

## MIDDLE RIO GRANDE COG

## Two Vehicle Analysis with Right on Red

Page: 5

Date: 5/19/200

Location: 025650

Starts : 05/15/00 at 06:45:0

Notes : LADERA DR - UNSER BD

Ends : 05/15/00 at 18:00:0

Study ID: 00

Interval : 15 min Intervals: 45

Operator: CM 168

S/N : 168 Type: C,Tr,Ped-rt/red

Weather : BERNALILLO

Correction: 1.00

## TOTAL INTERSECTION PEAK HOUR ANALYSIS

Total Intersection Peak is: Mon May 15 07:00:00 2000

DIRECTION	VOLUME						PERCENTS					
	Peds	Rt/Red	Left	Thru	Right	Total	Factor	Peak	Rt/Red	Left	Thru	Right
From North	0	1	71	1143	12	1227	0.88	0.1%	5.8%	93.2%	1.0%	100.0%
From South	0	70	40	227	123	460	0.91	15.2%	8.7%	49.3%	26.7%	100.0%
From East	0	12	594	22	15	643	0.92	1.9%	92.4%	3.4%	2.3%	100.0%
From West	0	64	17	45	122	248	0.84	25.8%	6.9%	18.1%	49.2%	100.0%
Totals	0	147	722	1437	272	2578	0.91	5.7%	28.0%	55.7%	10.6%	100.0%

		From North	
		Total 1498	
		Approach 1227	Depart 271
		Rt/red Right Thru Left	
		1 12 1143 71 . 17 227 27	

		12 Rt/Red
		15 Right
Depart 75	22	22 Thru Approach 643
	40	594 Left

Total 323 .....			Total 952
From West			From East

	N	
	W + E	
	S	
Left	71	
Approach 248	45	Depart 309
Right	122	
Rt/Red	64	

	186 1143 594 . 40 227 123 70
	. Left Thru Right Rt/Red
	Depart 1923 . Approach 460
	Total 2383
	From South

## MIDDLE RIO GRANDE COG

Two Vehicle Analysis with Right on Red

Page: 2

Date: 5/19/200

\*\*\*\*\*  
 Location: 025650 Starts : 05/15/00 at 06:45:0  
 Notes : LADERA DR - UNSER BD Ends : 05/15/00 at 18:00:0  
 Study ID: 00 Interval : 15 min Intervals: 45  
 Operator: CM 168 S/N : 168 Type: C,Tr,Ped-rt/red  
 Weather : BERNALILLO Correction: 1.00  
 \*\*\*\*\*

## TOTAL INTERSECTION PEAK HOUR ANALYSIS

Total Intersection Peak is: Mon May 15 12:00:00 2000

DIRECTION	VOLUME						Factor	PERCENTS					
	Peds	Rt/Red	Left	Thru	Right	Total		Rt/Red	Left	Thru	Right	Total	
From North	0	0	42	195	17	254	0.81	0.0%	16.5%	76.8%	6.7%	100.0%	
From South	0	97	77	227	107	508	0.92	19.1%	15.2%	44.7%	21.1%	100.0%	
From East	0	17	184	36	16	253	0.93	6.7%	72.7%	14.2%	6.3%	100.0%	
From West	0	32	17	23	18	90	0.73	35.6%	18.9%	25.6%	20.0%	100.0%	
Totals	0	146	320	481	158	1105	0.95	13.2%	29.0%	43.5%	14.3%	100.0%	

		From North	
		.	
		.	
		Total 531	
		Approach 254	Depart 277
		.	
		Rt/red Right Thru Left .	
		0 17 195 42 .	17 227 33

			17	Rt/Red
			16	Right
			36	Thru Approach 253
			77	184 Left

Total 220 .....	N	.....	Total 522
From West	W	E	From East
	S		

	Left	17		42
Approach 90	Thru	23		23
	Right	18		204
	Rt/Red	32		

	50	195	184	.	77	227	107	97	
				.	Left	Thru	Right	Rt/Red	
				.					
				.	Depart 429	Approach 508			
				.	Total 937				
				.					
				.	From South				

MIDDLE RIO GRANDE COG  
Two Vehicle Analysis with Right on Red

Page: 2  
Date: 5/19/2000

Location: 025650  
Notes : LADERA DR - UNSER BD  
Study ID: 00  
Operator: CM 168  
Weather : BERNALILLO

Starts : 05/15/00 at 06:45:00  
Ends : 05/15/00 at 18:00:00  
Interval : 15 min Intervals: 45  
S/N : 168 Type: C,Tr,Ped-rt/red  
Correction: 1.00

TOTAL INTERSECTION PEAK HOUR ANALYSIS

Total Intersection Peak is: Mon May 15 17:00:00 2000

DIRECTION	VOLUME						Peak Factor	PERCENTS				
	Peds	Rt/Red	Left	Thru	Right	Total		Rt/Red	Left	Thru	Right	Total
From North	0	3	69	308	31	411	0.83	0.7%	16.8%	74.9%	7.5%	100.0%
From South	0	186	165	967	291	1609	0.92	11.6%	10.3%	60.1%	18.1%	100.0%
From East	0	46	212	58	34	350	0.84	13.1%	60.6%	16.6%	9.7%	100.0%
From West	0	37	20	38	5	100	0.78	37.0%	20.0%	38.0%	5.0%	100.0%
Totals	0	272	466	1371	361	2470	0.96	11.0%	18.9%	55.5%	14.6%	100.0%

	From North	
	Total 1478	
	Approach 411	Depart 1067
	Rt/red Right Thru Left	
	3 31 308 69	20 967 80

			46 Rt/Red
	34		34 Right
Depart 257	58		58 Thru Approach 350
	165		212 Left
Total 357			Total 934

From West

Left 20 69

Approach 100 Thru 38 Depart 584

Right 5 477  
Rt/Red 37

42	308	212	165	967	291	186
			Left	Thru	Right	Rt/Red

Depart 562 Approach 1609

Total 2171

From South

**APPENDIX C**

**YEAR 2000 CAPACITY ANALYSIS**

SIGNAL97/TEAPAC [Ver 1.02.10] - Capacity Analysis Summary

Intersection Averages for Int # 1 - .

Degree of Saturation (v/c) 0.81 Vehicle Delay 43.8 Level of Service D+

Sq 12	Phase 1	Phase 2	Phase 3
*/**			
	+++	^	^
	+++	++++	++++
/\\	<++ + +>	<+++++>	<+++++>
	v	++++	^
	^	v	++++
North	<+ + +>	+> ++++>	
	+++	+  ++++	
	+++	+   v	
G/C=0.410   G/C=0.244   G/C=0.131   G= 28.7"   G= 17.1"   G= 9.2"   Y+R= 5.0"   Y+R= 5.0"   Y+R= 5.0"   OFF= 0.0%   OFF=48.1%   OFF=79.7%			

C= 70 sec G= 55.0 sec = 78.6% Y=15.0 sec = 21.4% Ped= 0.0 sec = 0.0%

Lane Group	Width/Lanes	g/C Req'd	Service Rate @C (vph)	Adj @E	HCM Volume	L v/c	90% Delay	Max S	Queue
------------	-------------	-----------	-----------------------	--------	------------	-------	-----------	-------	-------

N Approach

50.8 D

TH+RT	24/2	0.396	0.367	1201	1285	1313	1.022	52.9	D	411 ft
LT	12/1	0.160	0.367	324	405	81	0.200	16.2	B	51 ft

S Approach

13.2 B+

RT	12/1	0.160	0.683	1056	1071	135	0.126	4.1	A	42 ft
TH	24/2	0.121	0.367	1203	1287	249	0.193	15.4	B	78 ft
LT	12/1	0.239	0.367	56	93	44	0.419	28.4	C	28 ft

E Approach

48.3 D

TH+RT	12/1	0.082	0.404	623	701	40	0.057	12.9	B+	25 ft
LT	24/2	0.231	0.201	534	685	646	0.943	50.5	D	255 ft

W Approach

44.7 D+

TH+RT	24/2	0.116	0.089	119	271	199	0.718	45.9	D	90 ft
LT	12/1	0.068	0.089	31	101	20	0.168	32.5	C	25 ft

Unser / Ladera Intersection  
 Painted Sky Subdivision  
 EXISTING PM PEAK HOUR

08/10/00  
 14:25:00

SIGNAL97/TEAPAC[Ver 1.02.10] - Capacity Analysis Summary

Intersection Averages for Int # 1 - .

Degree of Saturation (v/c) 0.50 Vehicle Delay 18.2 Level of Service B

Sq 12	Phase 1	Phase 2	Phase 3
*/\*	+ + +	^	^
/	+ + +	++++	++++
\	<+ + +>	<++++>	<++++>
North	v	++++	^
	^	v	++++
	<+ + +>	+> ++++>	
	+ + +	+  ++++	
	+ + +	+   v	
	G/C=0.541	G/C=0.139	G/C=0.106
	G= 37.9"	G= 9.7"	G= 7.4"
	Y+R= 5.0"	Y+R= 5.0"	Y+R= 5.0"
	OFF= 0.0%	OFF=61.3%	OFF=82.3%

C= 70 sec G= 55.0 sec = 78.6% Y=15.0 sec = 21.4% Ped= 0.0 sec = 0.0%

Lane Group	Width/Lanes	g/C Req'd	Service Rate @C (vph)	Adj @E (vph)	Volume	HCM v/c	L Delay	90% S	Max Queue
------------	-------------	-----------	-----------------------	--------------	--------	---------	---------	-------	-----------

N Approach

11.7 B+

TH+RT	24/2	0.170	0.499	1690	1721	422	0.245	10.4	B+  105 ft
LT	12/1	0.363	0.499	144	195	85	0.434	18.1	B   42 ft

S Approach

11.7 B+

RT	12/1	0.324	0.709	1102	1111	413	0.372	5.0	A   119 ft
TH	24/2	0.329	0.499	1717	1747	1051	0.602	14.1	B+  261 ft
LT	12/1	0.285	0.499	405	465	179	0.385	13.3	B+  89 ft

E Approach

46.5 D

TH+RT	12/1	0.168	0.273	357	464	165	0.356	22.6	C+  119 ft
LT	24/2	0.135	0.096	156	323	290	0.892	60.1	E+  130 ft

W Approach

34.4 C

TH+RT	24/2	0.057	0.063	65	200	52	0.244	33.9	C   25 ft
LT	12/1	0.066	0.063	20	86	22	0.210	35.6	D+  25 ft

**APPENDIX D**

**YEAR 2005 NO-BUILD CAPACITY ANALYSIS**

HCS: Signals Release 3.1a

Inter: UNSER/LADERA  
Analyst: JAW  
Date: 8/10/00  
E/W St: LADERA

City/St: ALBUQUERQUE, NM  
Proj #:  
Period: 2005 AM PEAK HOUR no build  
N/S St: UNSER

## SIGNALIZED INTERSECTION SUMMARY

	Intersection Summary											
	Eastbound			Westbound			Northbound			Southbound		
	L	T	R	L	T	R	L	T	R	L	T	R
No. Lanes	1	2	0	2	1	0	1	2	1	1	2	0
LGConfig	L	TR		L	TR		L	T	R	L	TR	
Volume	26	70	288	921	34	42	62	352	299	110	1772	20
Lane Width	12.0	12.0		12.0	12.0		12.0	12.0	12.0	12.0	12.0	
RTOR Vol		100			10			100				2

Duration 0.25 Area Type: All other areas

## **III. All Other area Signal Operations**

Appr/ Lane Grp	Lane Group Capacity	Intersection Performance Summary					Lane Group	Approach
		Adj Flow Rate (s)	Sat	Ratios		Delay		
				v/c	g/C			
<b>Eastbound</b>								
L	503		0.06	0.368	18.6	B		
TR	382	3122	0.75	0.122	51.0	D	48.0	D
<b>Westbound</b>								
L	823		1.24	0.367	149.1	F		
TR	209	1710	0.35	0.122	40.9	D	141.8	F
<b>Northbound</b>								
L	82	180	0.84	0.456	83.9	F		
T	1597	3505	0.24	0.456	15.4	B	19.0	B
R	1098	1568	0.20	0.700	5.1	A		
<b>Southbound</b>								
L	438	961	0.28	0.456	16.9	B		
TR	1594	3500	1.25	0.456	141.4	F	134.2	F

Intersection Delay = 111.1 (sec/veh)    Intersection LOS = F

HCS: Signals Release 3.1a

Jeanette Walther  
Bohannan-Huston, Inc.

Phone:  
E-Mail:

Fax:

#### OPERATIONAL ANALYSIS

Intersection: UNSER/LADERA  
 City/State: ALBUQUERQUE, NM  
 Analyst: JAW  
 Project No:  
 Time Period Analyzed: 2005 AM PEAK HOUR no build  
 Date: 8/10/00  
 East/West Street Name: LADERA  
 North/South Street Name: UNSER

## HCS: Signals Release 3.1a

Inter: UNSER/LADERA  
 Analyst: JAW  
 Date: 8/10/00  
 E/W St: LADERA

City/St: ALBUQUERQUE, NM  
 Proj #:  
 Period: 2005 PM PEAK HOUR no build  
 N/S St: UNSER

## SIGNALIZED INTERSECTION SUMMARY

	Eastbound			Westbound			Northbound			Southbound		
	L	T	R	L	T	R	L	T	R	L	T	R
No. Lanes	1	2	0	2	1	0	1	2	1	1	2	0
LGConfig	L	TR		L	TR		L	T	R	L	TR	
Volume	31	59	65	329	90	124	256	1499	789	107	477	53
Lane Width	12.0	12.0		12.0	12.0		12.0	12.0	12.0	12.0	12.0	
RTOR Vol		20			30			300			15	

Duration 0.25 Area Type: All other areas  
 Signal Operations

Phase Combination		1	2	3	4	5	6	7	8
EB	Left	P	P		NB	Left	P		
	Thru		P			Thru	P		
	Right		P			Right	P		
	Peds		X			Ped	X		
WB	Left	P	P		SB	Left	P		
	Thru		P			Thru	P		
	Right		P			Right	P		
	Peds		X			Ped	X		
NB	Right	P			EB	Right			
SB	Right				WB	Right			
Green		17.0	14.0				44.0		
Yellow		5.0	5.0				5.0		
All Red		0.0	0.0				0.0		
Cycle Length:	90.0	secs							

Appr/ Lane Grp	Lane Group Capacity	Adj Sat Flow Rate (s)	Intersection Performance Summary		Lane Group	Approach		
			Ratios v/c	Ratios g/C				
<b>Eastbound</b>								
L	394		0.09	0.368	19.3	B		
TR	401	3278	0.29	0.122	37.8	D	33.6	C
<b>Westbound</b>								
L	844		0.43	0.367	21.9	C		
TR	208	1704	0.98	0.122	97.0	F	48.8	D
<b>Northbound</b>								
L	356	782	0.80	0.456	37.8	D		
T	1597	3505	1.04	0.456	59.2	E	45.6	D
R	1098	1568	0.49	0.700	7.8	A		
<b>Southbound</b>								
L	82	180	1.45	0.456	283.0	F		
TR	1579	3466	0.36	0.456	16.6	B	62.5	E
Intersection Delay = 48.6 (sec/veh)				Intersection LOS = D				

HCS: Signals Release 3.1a

Jeanette Walther  
Bohannan-Huston, Inc.

Phone:  
E-Mail:

Fax:

#### OPERATIONAL ANALYSIS

Intersection: UNSER/LADERA  
 City/State: ALBUQUERQUE, NM  
 Analyst: JAW  
 Project No:  
 Time Period Analyzed: 2005 PM PEAK HOUR no build  
 Date: 8/10/00  
 East/West Street Name: LADERA  
 North/South Street Name: UNSER

VOLUME DATA

	Eastbound			Westbound			Northbound			Southbound		
	L	T	R	L	T	R	L	T	R	L	T	R
Volume	31	59	65	329	90	124	256	1499	789	107	477	53
PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
PK 15 Vol	9	16	18	91	25	34	71	416	219	30	133	15
Hi Ln Vol												
% Grade		0			0			0			0	
Ideal Sat	1900	1900		1900	1900		1900	1900	1900	1900	1900	
ParkExist												
NumPark												
% Heavy Veh	3	3	3	3	3	3	3	3	3	3	3	3
No. Lanes	1	2	0	2	1	0	1	2	1	1	2	0
LGConfig	L	TR		L	TR		L	T	R	L	TR	
Lane Width	12.0	12.0		12.0	12.0		12.0	12.0	12.0	12.0	12.0	
RTOR Vol		20			30			300				15
Adj Flow	34	116		366	204		284	1666	543	119	572	
%InSharedLn												
Prop Turns		0.43			0.51							0.07
NumPeds		0			0			0				0
NumBus	0	0		0	0		0	0	0	0	0	

Duration 0.25 Area Type: All other areas

OPERATING PARAMETERS

	Eastbound			Westbound			Northbound			Southbound		
	L	T	R	L	T	R	L	T	R	L	T	R
Init Unmet	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	
Arriv. Type	3	3		3	3		3	3	3	3	3	
Unit Ext.	3.0	3.0		3.0	3.0		3.0	3.0	3.0	3.0	3.0	
I Factor		1.000			1.000			1.000			1.000	
Lost Time	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	
Ext of g	2.1	2.0		2.0	2.0		2.0	2.0	2.0	2.0	2.0	
Ped Min g		0.0			0.0			0.0			0.0	

PHASE DATA

Phase Combination 1			2	3	4			5	6	7	8
EB	Left	P	P				NB	Left	P		
	Thru		P				Thru		P		
	Right		P				Right		P		
	Peds		X				Ped		X		
WB	Left	P	P				SB	Left	P		
	Thru		P				Thru		P		
	Right		P				Right		P		
	Peds		X				Ped		X		
NB	Right	P					EB	Right			
SB	Right						WB	Right			
Green		17.0	14.0					44.0			
Yellow		5.0	5.0					5.0			
All Red		0.0	0.0					0.0			

Cycle Length: 90.0      secs

Intersection Performance Summary							
Appr/ Lane Lane Grp	Lane Group Capcity	Adj Sat Flow Rate (s)	Ratios		Lane Group	Approach	
			v/c	g/C	Delay LOS	Delay LOS	

#### Eastbound

L	209	1315	0.14	0.159	33.2	C	
T	294	1845	0.27	0.159	34.7	C	42.4
R	249	1568	0.66	0.159	47.6	D	

#### Westbound

L	1012		1.01	0.432	61.5	E	
TR	744	1723	0.09	0.432	15.0	B	58.6

#### Northbound

L	721	1866	0.10	0.386	17.5	B	
T	1354	3505	0.29	0.386	19.2	B	15.5
R	1033	1568	0.16	0.659	6.1	A	

#### Southbound

L	371	961	0.33	0.386	21.3	C	
TR	1943	5029	1.02	0.386	53.2	D	51.4

Intersection Delay = 47.2 (sec/veh)    Intersection LOS = D

HCS: Signals Release 3.1a

Jeanette Walther  
Bohannan-Huston, Inc.

Phone:  
E-Mail:

Fax:

#### OPERATIONAL ANALYSIS

Intersection:  
City/State:  
Analyst:  
Project No:  
Time Period Analyzed: AM 2005 NO BUILD  
Date: 8/10/00  
East/West Street Name: LADERA  
North/South Street Name: UNSER

VOLUME DATA

	Eastbound			Westbound			Northbound			Southbound		
	L	T	R	L	T	R	L	T	R	L	T	R
Volume	26	70	288	921	34	42	62	352	299	110	1772	20
PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
PK 15 Vol	7	19	80	256	9	12	17	98	83	31	492	6
Hi Ln Vol												
% Grade		0			0			0			0	
Ideal Sat	1900	1900	1900	1900	1900		1900	1900	1900	1900	1900	
ParkExist												
NumPark												
% Heavy Veh	3	3	3	3	3	3	3	3	3	3	3	3
No. Lanes	1	1	1	2	1	0	2	2	1	1	3	0
LGConfig	L	T	R	L	TR		L	T	R	L	TR	
Lane Width	12.0	12.0	12.0	12.0	12.0		12.0	12.0	12.0	12.0	12.0	
RTOR Vol		140			15			150				5
Adj Flow	29	78	164	1023	68		69	391	166	122	1986	
%InSharedLn												
Prop Turns					0.44							0.01
NumPeds			0		0			0				0
NumBus	0	0	0	0	0		0	0	0	0	0	

Duration

0.25

Area Type: All other areas

OPERATING PARAMETERS

	Eastbound			Westbound			Northbound			Southbound		
	L	T	R	L	T	R	L	T	R	L	T	R
Init Unmet	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	
Arriv. Type	3	3	3	3	3		3	3	3	3	3	
Unit Ext.	3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0	3.0	3.0	
I Factor	1.000			1.000			1.000			1.000		
Lost Time	5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0	5.0	5.0	
Ext of g	2.0	2.0	2.0	2.0	2.0		2.0	2.0	2.0	2.0	2.0	
Ped Min g	0.0			0.0			0.0			0.0		

PHASE DATA

Phase Combination 1			2	3	4				5	6	7	8
EB	Left	P					NB	Left	P			
	Thru	P						Thru	P			
	Right	P						Right	P			
	Peds	X						Ped	X			
WB	Left	P	P				SB	Left	P			
	Thru	P	P					Thru	P			
	Right	P	P					Right	P			
	Peds	X						Ped	X			
NB	Right	P					EB	Right				
SB	Right						WB	Right				
Green	19.0	17.0							37.0			
Yellow	5.0	5.0							5.0			
All Red	0.0	0.0							0.0			

Cycle Length: 88.0      secs



Intersection Performance Summary

Appr/ Lane Lane Grp	Lane Group Capacity	Adj Sat Flow Rate (s)	Ratios		Lane Group	Approach	
			v/c	g/C		Delay LOS	Delay LOS
<b>Eastbound</b>							
L	125	1149	0.27	0.109	50.3	D	
T	201	1845	0.33	0.109	49.6	D	49.3 D
R	171	1568	0.23	0.109	47.9	D	
<b>Westbound</b>							
L	496		0.74	0.245	48.0	D	
TR	416	1696	0.52	0.245	40.5	D	45.2 D
<b>Northbound</b>							
L	527		0.54	0.609	13.5	B	
T	1657	3505	1.01	0.473	52.4	D	39.9 D
R	955	1568	0.57	0.609	15.3	B	
<b>Southbound</b>							
L	178		0.67	0.609	41.4	D	
TR	2348	4967	0.25	0.473	17.6	B	21.6 C

Intersection Delay = 37.8 (sec/veh)      Intersection LOS = D

HCS: Signals Release 3.1a

Jeanette Walther  
Bohannan-Huston, Inc.

Phone:  
E-Mail:

Fax:

OPERATIONAL ANALYSIS

Intersection:  
City/State:  
Analyst:  
Project No:  
Time Period Analyzed: PM 2005 NO BUILD  
Date: 8/10/00  
East/West Street Name: LADERA  
North/South Street Name: UNSER

VOLUME DATA

	Eastbound			Westbound			Northbound			Southbound		
	L	T	R	L	T	R	L	T	R	L	T	R
Volume	31	59	65	329	90	124	256	1499	789	107	477	53
PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
PK 15 Vol	9	16	18	91	25	34	71	416	219	30	133	15
Hi Ln Vol												
% Grade		0			0			0			0	
Ideal Sat	1900	1900	1900	1900	1900		1900	1900	1900	1900	1900	
ParkExist												
NumPark												
% Heavy Veh	3	3	3	3	3	3	3	3	3	3	3	3
No. Lanes	1	1	1	2	1	0	2	2	1	1	3	0
LGConfig	L	T	R	L	TR		L	T	R	L	TR	
Lane Width	12.0	12.0	12.0	12.0	12.0		12.0	12.0	12.0	12.0	12.0	
RTOR Vol		30			20			300				
Adj Flow	34	66	39	366	216		284	1666	543	119	583	5
%InSharedLn												
Prop Turns					0.54							
NumPeds		0			0			0				0.09
NumBus	0	0	0	0	0		0	0	0	0	0	

Duration 0.25 Area Type: All other areas

OPERATING PARAMETERS

	Eastbound			Westbound			Northbound			Southbound		
	L	T	R	L	T	R	L	T	R	L	T	R
Init Unmet	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	
Arriv. Type	3	3	3	3	3		3	3	3	3	3	
Unit Ext.	3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0	3.0	3.0	
I Factor		1.000			1.000			1.000			1.000	
Lost Time	5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0	5.0	5.0	
Ext of g	2.0	2.0	2.0	2.0	2.0		2.0	2.0	2.0	2.0	2.0	
Ped Min g		0.0			0.0			0.0			0.0	

PHASE DATA

Phase Combination 1			2	3	4			5	6	7	8
EB	Left		P				NB	Left	P	P	
	Thru		P					Thru		P	
	Right		P					Right		P	
	Peds		X					Ped		X	
WB	Left	P	P				SB	Left	P	P	
	Thru	P	P					Thru		P	
	Right	P	P					Right		P	
	Peds		X					Ped		X	
NB	Right	P					EB	Right			
SB	Right						WB	Right			
Green		10.0	15.0					10.0	55.0		
Yellow		5.0	5.0					5.0	5.0		
All Red		0.0	0.0					0.0	0.0		

Cycle Length: 110.0    secs

Unser / Ladera Intersection  
 Painted Sky Subdivision  
 YEAR 2005 AM PEAK HOUR - NO-BUILD

09/25/00  
 11:31:28

### SIGNAL97/TEAPAC[Ver 1.02.14] - Capacity Analysis Summary

Intersection Averages for Int # 1 - Unser & Ladera  
 Degree of Saturation (v/c) 0.79 Vehicle Delay 47.9 Level of Service D

Sq 44	Phase 1	Phase 2	Phase 3	Phase 4
**/**	+	* * +		^
.	+	* * +		++++
/ \	+>	<* * +>		<++++
North	<+	<+ + +>	+> ++++>	
	***** +	+ + +	+  *****	
	v +	+ + +	+   v	
	G/C=0.045	G/C=0.338	G/C=0.351	G/C=0.099
	G= 5.4"	G= 40.6"	G= 42.1"	G= 11.9"
	Y+R= 5.0"	Y+R= 5.0"	Y+R= 5.0"	Y+R= 5.0"
	OFF= 0.0%	OFF= 8.6%	OFF=46.6%	OFF=85.9%

C=120 sec G=100.0 sec = 83.3% Y=20.0 sec = 16.7% Ped= 0.0 sec = 0.0%

Lane Group	Width/ Lanes	g/C Req'd	Service Rate @C (vph)	Adj @E	Volume	HCM v/c	L Delay	90% S	Max Queue
------------	--------------	-----------	-----------------------	--------	--------	---------	---------	-------	-----------

N Approach 49.2 D

TH+RT	48/4	0.381	0.313	1127	2101	1980	0.942	50.2	*D	577 ft
LT	12/1	0.010	0.020	153	307	122	0.382	31.7	C	124 ft

S Approach 27.6 C

RT	12/1	0.319	0.706	1063	1107	216	0.195	6.4	A	108 ft
TH	24/2	0.290	0.313	539	1098	383	0.349	32.6	C	223 ft
LT	12/1	0.052	0.020	28	75	67	0.698	66.9	E+	68 ft

E Approach 53.1 D

TH+RT	24/2	0.246	0.074	1	200	66	0.270	55.2	E+	52 ft
LT	24/2	0.399	0.326	600	1108	1023	0.923	52.9	*D	585 ft

W Approach 65.8 E+

RT	12/1	0.313	0.161	1	219	202	0.802	71.5	*E	288 ft
TH	12/1	0.259	0.074	1	96	75	0.547	68.4	E	118 ft
LT	12/1	0.106	0.326	591	732	28	0.038	17.4	B	25 ft

Add 2 SB Thru - Re-stripe West Approach  
PM Peak - No Build

09/06/00  
16:53:23

SIGNAL97/TEAPAC[Ver 1.02.14] - Capacity Analysis Summary

Intersection Averages for Int # 1 - Unser & Ladera  
Degree of Saturation (v/c) 0.70 Vehicle Delay 32.7 Level of Service C

Sq 48	Phase 1	Phase 2	Phase 3	Phase 4	Phase 5
***/**					
	*	+++	^	^	
	*	+++	++++	++++	
/\\	*>	<++ + +>	<+++++>	<+++++>	
		v	****	^	^
		^	v	++++	****
North	<+	<+ * +>	+> ****>	++++>	
	++++ +	+ * +	+  ++++	++++	
	v +	+ * +	+   v	v	
G/C=0.059   G/C=0.502   G/C=0.166   G/C=0.021   G/C=0.044					
G= 7.1"   G= 60.2"   G= 19.9"   G= 2.5"   G= 5.3"					
Y+R= 5.0"   Y+R= 5.0"   Y+R= 5.0"   Y+R= 5.0"   Y+R= 5.0"					
OFF= 0.0%   OFF=10.1%   OFF=64.4%   OFF=85.1%   OFF=91.4%					

C=120 sec G= 95.0 sec = 79.2% Y=25.0 sec = 20.8% Ped= 0.0 sec = 0.0%

Lane Group	Width/ Lanes	g/C Req'd	Service Rate @C (vph)	Adj @E	Service Rate @C (vph)	HCM v/c	L Delay	90% S Queue	Max

N Approach

22.3 C+

TH+RT	48/4	0.276	0.502	3119	3317	589	0.178	16.5	B	125 ft
LT	12/1	0.089	0.059	81	147	119	0.726	51.3	*D	96 ft

S Approach

30.6 C

RT	12/1	0.557	0.709	1069	1112	766	0.689	13.4	B+	378 ft
TH	24/2	0.520	0.502	1582	1758	1666	0.948	40.6	*D+	705 ft
LT	12/1	0.003	0.059	416	485	284	0.586	17.7	B	192 ft

E Approach

51.0 D

TH+RT	12/1	0.319	0.229	1	365	238	0.618	48.8	D	311 ft
LT	24/2	0.290	0.166	1	545	366	0.649	52.5	*D	259 ft

W Approach

52.7 D

RT	12/1	0.237	0.208	1	301	1	0.003	37.7	D+	25 ft
TH	12/1	0.257	0.107	1	158	66	0.335	54.2	*D	100 ft
LT	12/1	0.184	0.044	1	170	34	0.162	50.4	*D	52 ft

APP | SB THRU  
| EB RIGHT

Unser / Ladera Intersection  
Painted Sky Subdivision NO  
YEAR 2005 AM PEAK HOUR -<sup>A</sup>BUILD

09/20/00  
17:06:54

SIGNAL97/TEAPAC[Ver 1.02.14] - Capacity Analysis Summary

Intersection Averages for Int # 1 - Unser & Ladera  
Degree of Saturation (v/c) 0.77 Vehicle Delay 43.1 Level of Service D+

Sq 35	Phase 1	Phase 2	Phase 3	Phase 4	Phase 5
***/**	* * +	* * +		^	^
/ \	<* * +>			++++	++++
North	v	^	^	<++++	<++++
	<* + +>	<+ + +>	****	****	^
	++++ * + +	+ + +	v	v	++++
	v * + +	+ + +	+	+	+   v
	G/C=0.048	G/C=0.438	G/C=0.048	G/C=0.145	G/C=0.094
	G= 5.3"	G= 48.2"	G= 5.3"	G= 15.9"	G= 10.3"
	Y+R= 5.0"	Y+R= 5.0"	Y+R= 5.0"	Y+R= 5.0"	Y+R= 5.0"
	OFF= 0.0%	OFF= 9.3%	OFF=57.7%	OFF=67.0%	OFF=86.0%

C=110 sec G= 85.0 sec = 77.3% Y=25.0 sec = 22.7% Ped= 0.0 sec = 0.0%

Lane Group	Width/ Lanes	g/C Req'd	Service Rate @C (vph)	Adj @E	Volume	v/c	HCM Delay	L S	90% Queue Max
------------	--------------	-----------	-----------------------	--------	--------	-----	-----------	-----	---------------

N Approach

42.3 D+

TH+RT	36/3	0.438	0.411	1800	2069	1980	0.957	43.4	*D+  604 ft
LT	12/1	0.302	0.411	267	398	122	0.307	23.8	C+  112 ft

S Approach

15.1 B

RT	12/1	0.296	0.788	1225	1235	216	0.175	3.2	A   71 ft
TH	24/2	0.262	0.504	1635	1768	383	0.217	15.4	B   148 ft
LT	12/1	0.039	0.020	43	86	67	0.650	51.2	*D   52 ft

E Approach

59.3 E+

TH+RT	12/1	0.228	0.257	143	438	66	0.149	32.3	C   76 ft
LT	36/3	0.309	0.211	86	1074	1023	0.953	61.0	*E+  419 ft

W Approach

51.9 D

RT	24/2	0.245	0.160	1	425	202	0.457	45.2	D   132 ft
TH	12/1	0.229	0.067	1	88	75	0.610	70.4	*E   109 ft
LT	12/1	0.147	0.020	1	91	28	0.226	50.7	*D   40 ft

**APPENDIX E**  
**TRIP GENERATION**

painted

Summary of Trip Generation Calculation  
For 290 Dwelling Units of Single Family Detached Housing  
August 10, 2000

	Average Rate	Standard Deviation	Adjustment Factor	Driveway Volume
Avg. Weekday 2-Way Volume	9.57	3.69	1.00	2775
7-9 AM Peak Hour Enter	0.19	0.00	1.00	55
7-9 AM Peak Hour Exit	0.56	0.00	1.00	162
7-9 AM Peak Hour Total	0.75	0.90	1.00	218
4-6 PM Peak Hour Enter	0.65	0.00	1.00	189
4-6 PM Peak Hour Exit	0.36	0.00	1.00	104
4-6 PM Peak Hour Total	1.01	1.05	1.00	293
AM Pk Hr, Generator, Enter	0.19	0.00	1.00	55
AM Pk Hr, Generator, Exit	0.58	0.00	1.00	168
AM Pk Hr, Generator, Total	0.77	0.91	1.00	223
PM Pk Hr, Generator, Enter	0.65	0.00	1.00	189
PM Pk Hr, Generator, Exit	0.37	0.00	1.00	107
PM Pk Hr, Generator, Total	1.02	1.05	1.00	296
Saturday 2-Way Volume	10.09	3.67	1.00	2926
Saturday Peak Hour Enter	0.51	0.00	1.00	148
Saturday Peak Hour Exit	0.43	0.00	1.00	125
Saturday Peak Hour Total	0.94	0.99	1.00	273
Sunday 2-Way Volume	8.78	3.33	1.00	2546
Sunday Peak Hour Enter	0.46	0.00	1.00	133
Sunday Peak Hour Exit	0.40	0.00	1.00	116
Sunday Peak Hour Total	0.86	0.95	1.00	249

Note: A zero indicates no data available.  
Source: Institute of Transportation Engineers  
Trip Generation, 6th Edition, 1997.

TRIP GENERATION BY MICROTRANS

**APPENDIX F**

**YEAR 2005 BUILD CAPACITY ANALYSIS**

## SCENARIO 1

HCS: Signals Release 3.1a

Inter:  
 Analyst:  
 Date: 8/10/00  
 E/W St: LADERA

City/St:  
 Proj #:  
 Period: AM 2005 BUILD  
 N/S St: UNSER

## SIGNALIZED INTERSECTION SUMMARY

	Eastbound			Westbound			Northbound			Southbound		
	L	T	R	L	T	R	L	T	R	L	T	R
No. Lanes	1	1	1	2	1	0	2	2	1	1	3	0
LGConfig	L	T	R	L	TR		L	T	R	L	TR	
Volume	81	77	388	921	36	42	96	352	299	110	1772	39
Lane Width	12.0	12.0	12.0	12.0	12.0		12.0	12.0	12.0	12.0	12.0	
RTOR Vol			150			5			60			5

Duration 0.25 Area Type: All other areas

## Signal Operations

Phase Combination 1		2	3	4		5	6	7	8
EB	Left		P		NB	Left	P		
	Thru		P			Thru	P		
	Right		P			Right	P		
	Peds		X			Ped	X		
WB	Left	P	P		SB	Left	P		
	Thru	P	P			Thru	P		
	Right	P	P			Right	P		
	Peds		X			Ped	X		
NB	Right	P			EB	Right			
SB	Right				WB	Right			
Green		21.8	29.0				49.2		
Yellow		5.0	5.0				5.0		
All Red		0.0	0.0				0.0		
Cycle Length:	115.0	secs							

Intersection Performance Summary							
Appr/ Lane Grp	Lane Group Capacity	Adj Sat Flow Rate (s)	Ratios		Lane Group	Approach	
			v/c	g/C		Delay LOS	Delay LOS

#### Eastbound

L	294	1299	0.31	0.226	39.7	D	
T	417	1845	0.21	0.226	37.2	D	48.2
R	355	1568	0.74	0.226	54.6	D	

#### Westbound

L	1017		1.01	0.459	68.3	E	
TR	783	1705	0.10	0.459	17.9	B	64.6

#### Northbound

L	692	1723	0.15	0.402	22.4	C	
T	1408	3505	0.28	0.402	23.7	C	18.7
R	995	1568	0.27	0.635	9.9	A	B

#### Southbound

L	382	951	0.32	0.402	25.8	C	
TR	2018	5022	0.99	0.402	53.1	D	51.5

Intersection Delay = 48.8 (sec/veh)    Intersection LOS = D

HCS: Signals Release 3.1a

Jeanette Walther  
Bohannan-Huston, Inc.

Phone:  
E-Mail:

Fax:

#### OPERATIONAL ANALYSIS

Intersection:

City/State:

Analyst:

Project No:

Time Period Analyzed: AM 2005 BUILD

Date: 8/10/00

East/West Street Name: LADERA

North/South Street Name: UNSER

VOLUME DATA

	Eastbound			Westbound			Northbound			Southbound		
	L	T	R	L	T	R	L	T	R	L	T	R
Volume	81	77	388	921	36	42	96	352	299	110	1772	39
PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
PK 15 Vol	23	21	108	256	10	12	127	98	83	31	492	11
Hi Ln Vol												
% Grade		0			0			0			0	
Ideal Sat	1900	1900	1900	1900	1900		1900	1900	1900	1900	1900	
ParkExist												
NumPark												
% Heavy Veh	3	3	3	3	3	3	3	3	3	3	3	3
No. Lanes	1	1	1	2	1	0	2	2	1	1	3	0
LGConfig	L	T	R	L	TR		L	T	R	L	TR	
Lane Width	12.0	12.0	12.0	12.0	12.0		12.0	12.0	12.0	12.0	12.0	
RTOR Vol		150			5			60			5	
Adj Flow	90	86	264	1023	81		107	391	266	122	2007	
%InSharedLn												
Prop Turns					0.51							0.02
NumPeds			0		0			0			0	
NumBus	0	0	0	0	0		10	0	0	10	0	

Duration 0.25 Area Type: All other areas

OPERATING PARAMETERS

	Eastbound			Westbound			Northbound			Southbound		
	L	T	R	L	T	R	L	T	R	L	T	R
Init Unmet	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Arriv. Type	3	3	3	3	3	3	3	3	3	3	3	3
Unit Ext.	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
I Factor	1.000			1.000			1.000			1.000		
Lost Time	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Ext of g	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Ped Min g	0.0			0.0			0.0			0.0		

PHASE DATA

Phase Combination	1	2	3	4		5	6	7	8
-------------------	---	---	---	---	--	---	---	---	---

EB Left		P				NB Left	P		
Thru		P				Thru	P		
Right		P				Right	P		
Peds		X				Ped	X		

WB Left	P	P				SB Left	P		
Thru	P	P				Thru	P		
Right	P	P				Right	P		
Peds		X				Ped	X		

NB Right	P					EB Right			
----------	---	--	--	--	--	----------	--	--	--

SB Right						WB Right			
----------	--	--	--	--	--	----------	--	--	--

Green	21.8	29.0					49.2		
Yellow	5.0	5.0					5.0		
All Red	0.0	0.0					0.0		

Cycle Length: 115.0    secs

## SCENARIO 1

## HCS: Signalized Intersections Release 3.2

Inter: UNSER/LADERA  
 Analyst: JAW  
 Date: 9/25/00  
 E/W St: LADERA

City/St: ALBUQUERQUE, NM  
 Proj #:  
 Period: 2005 PM PEAK HOUR build  
 N/S St: UNSER

## SIGNALIZED INTERSECTION SUMMARY

	Eastbound			Westbound			Northbound			Southbound		
	L	T	R	L	T	R	L	T	R	L	T	R
No. Lanes	2	1	1	2	1	0	2	2	1	1	3	0
LGConfig	L	T	R	L	TR		L	T	R	L	TR	
Volume	66	64	129	329	99	124	372	1499	739	107	477	117
Lane Width	12.0	12.0	12.0	12.0	12.0		12.0	12.0	12.0	12.0	12.0	
RTOR Vol			40			46			186			3

Duration	0.25	Area Type:	All other areas
		Signal Operations	

Phase Combination 1		2	3	4		5	6	7	8
EB	Left		P		NB	Left	P		P
	Thru		P			Thru			P
	Right		P			Right			P
	Peds					Peds			
WB	Left	P	P		SB	Left	P		P
	Thru	P	P			Thru			P
	Right	P	P			Right			P
	Peds					Peds			
NB	Right	P			EB	Right	P		
SB	Right				WB	Right	P		
Green	10.0	15.0					10.0	55.0	
Yellow	5.0	5.0					5.0	5.0	
All Red	0.0	0.0					0.0	0.0	
Cycle Length:	110.0	secs							

Appr/ Lane Grp	Lane Group	Capacity	Intersection Performance Summary			Lane Group	Approach
			Adj Sat Flow Rate (s)	Ratios	Lane Group		
Eastbound							
L	198	1797	0.39	0.110	51.3	D	
T	201	1845	0.37	0.109	50.7	D	43.5 D
R	428	1568	0.25	0.273	32.5	C	
Westbound							
L	448		0.82	0.245	54.3	D	
TR	423	1722	0.47	0.245	39.0	D	48.9 D
Northbound							
L	507		0.81	0.609	24.1	C	
T	1657	3505	1.01	0.473	52.4	D	40.0 D
R	955	1568	0.64	0.609	17.1	B	
Southbound							
L	178		0.67	0.609	42.2	D	
TR	2312	4890	0.28	0.473	18.0	B	21.7 C

Intersection Delay = 38.1 (sec/veh)    Intersection LOS = D

HCS: Signalized Intersections Release 3.2

Jeanette Walther  
Bohannan-Huston, Inc.

Phone:  
E-Mail:

Fax:

#### OPERATIONAL ANALYSIS

Intersection: UNSER/LADERA  
 City/State: ALBUQUERQUE, NM  
 Analyst: JAW  
 Project No:  
 Time Period Analyzed: 2005 PM PEAK HOUR build  
 Date: 9/25/00  
 East/West Street Name: LADERA  
 North/South Street Name: UNSER

VOLUME DATA

	Eastbound			Westbound			Northbound			Southbound		
	L	T	R	L	T	R	L	T	R	L	T	R
Volume	66	64	129	329	99	124	372	1499	739	107	477	117
PHF	0.85	0.85	0.85	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
PK 15 Vol	19	19	38	91	28	34	103	416	205	30	133	33
Hi Ln Vol												
% Grade	0			0			0			0		
Ideal Sat	1900	1900	1900	1900	1900		1900	1900	1900	1900	1900	
ParkExist												
NumPark												
% Heavy Veh	3	3	3	3	3	3	3	3	3	3	3	3
No. Lanes	2	1	1	2	1	0	2	2	1	1	3	0
LGConfig	L	T	R	L	TR		L	T	R	L	TR	
Lane Width	12.0	12.0	12.0	12.0	12.0		12.0	12.0	12.0	12.0	12.0	
RTOR Vol		40			46			186				3
Adj Flow	78	75	105	366	197		413	1666	614	119	657	
%InSharedLn												
Prop Turns				0.44								0.19
NumPeds		0		0				0				0
NumBus	0	0	0	0	0		0	0	0	0	0	
%RightsInProtPhase	0			0			0					0
Duration	0.25			Area Type: All other areas								

OPERATING PARAMETERS

	Eastbound			Westbound			Northbound			Southbound		
	L	T	R	L	T	R	L	T	R	L	T	R
Init Unmet	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	
Arriv. Type	3	3	3	3	3		3	3	3	3	3	
Unit Ext.	3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0	3.0	3.0	
I Factor		1.000			1.000			1.000			1.000	
Lost Time	5.0	5.0	2.0	5.0	5.0		5.0	5.0	5.0	5.0	5.0	
Ext of g	2.1	2.0	2.0	2.0	2.0		2.0	2.0	2.0	2.0	2.0	
Ped Min g		3.0			3.0			3.0			3.0	

PHASE DATA

Phase Combination	1	2	3	4		5	6	7	8
EB Left		P			NB	Left	P	P	
Thru		P				Thru		P	
Right		P				Right		P	
Peds						Peds			
WB Left	P	P			SB	Left	P	P	
Thru	P	P				Thru		P	
Right	P	P				Right		P	
Peds						Peds			
NB Right		P			EB	Right	P		
SB Right					WB	Right	P		
Green	10.0	15.0				10.0	55.0		
Yellow	5.0	5.0				5.0	5.0		
All Red	0.0	0.0				0.0	0.0		

Cycle Length: 110.0    secs

09/06/00  
16:30:40Add 2 SB Thru and 1 NB Thru - Re-stripe West Approach  
AM Peak - Build

## SIGNAL97/TEAPAC[Ver 1.02.14] - Capacity Analysis Summary

Intersection Averages for Int # 1 - Unser & Ladera  
Degree of Saturation (v/c) 0.78 Vehicle Delay 47.7 Level of Service D

Sq 44	Phase 1	Phase 2	Phase 3	Phase 4
***/**	+	* * +		^
.	+	* * +		++++
/\	+>	<* * +>		<+++++
		v	^ ****	^
		^	++++ v	++++
North	<+	<+ + +>	+> ++++>	
	***** +	+ + +	+  ****	
	v +	+ + +	+   v	
	G/C=0.070	G/C=0.315	G/C=0.322	G/C=0.126
	G= 8.4"	G= 37.8"	G= 38.7"	G= 15.1"
	Y+R= 5.0"	Y+R= 5.0"	Y+R= 5.0"	Y+R= 5.0"
	OFF= 0.0%	OFF=11.2%	OFF=46.8%	OFF=83.2%

C=120 sec G=100.0 sec = 83.3% Y=20.0 sec = 16.7% Ped= 0.0 sec = 0.0%

Lane Group	Width/ Lanes	g/C Rqd	Service Rate @C (vph)	Adj @E	Volume	HCM v/c	L Delay	90% S	Max Queue
------------	--------------	---------	-----------------------	--------	--------	---------	---------	-------	-----------

## N Approach

50.1 D

TH+RT	48/4	0.384	0.315	1148	2108	2012	0.954	51.7	*D	585 ft
LT	12/1	0.011	0.070	274	404	122	0.302	23.4	C+	119 ft

## S Approach

25.1 C+

RT	12/1	0.321	0.679	1009	1064	221	0.208	7.6	A	120 ft
TH	36/3	0.272	0.315	835	1586	391	0.247	30.9	C	151 ft
LT	12/1	0.084	0.070	86	173	107	0.582	39.8	D+	104 ft

## E Approach

54.7 D

TH+RT	12/1	0.266	0.126	1	177	87	0.407	53.9	D	129 ft
LT	24/2	0.399	0.322	576	1095	1023	0.934	54.8	*D	588 ft

## W Approach

54.5 D

RT	12/1	0.360	0.238	5	354	320	0.858	65.5	*E+	414 ft
TH	12/1	0.263	0.126	1	196	86	0.369	52.5	D	127 ft
LT	12/1	0.109	0.322	606	725	90	0.124	16.9	B	78 ft

Scenario 2 - West Approach  
SCENARIO 2

09/06/00  
16:55:09

Add 2 SB Thru - Re-stripe West Approach  
PM Peak - Build

SIGNAL97/TEAPAC[Ver 1.02.14] - Capacity Analysis Summary

Intersection Averages for Int # 1 - Unser & Ladera  
Degree of Saturation (v/c) 0.71 Vehicle Delay 35.3 Level of Service D+

Sq 48	Phase 1	Phase 2	Phase 3	Phase 4	Phase 5
**/**	*	+++	^	^	
.	*	+++	++++	++++	
/ \	*>	<+ + +>	<++++	<++++	
		v	****	^	^
			v	++++	****
North	<+	<+ * +>	+> ****>	++++>	
	++++ +	+ * +	+  ++++	++++	
	v +	+ * +	+   v	v	
	G/C=0.059	G/C=0.500	G/C=0.165	G/C=0.024	G/C=0.044
	G= 7.1"	G= 60.0"	G= 19.8"	G= 2.9"	G= 5.3"
	Y+R= 5.0"				
	OFF= 0.0%	OFF=10.0%	OFF=64.2%	OFF=84.9%	OFF=91.4%

C=120 sec G= 95.0 sec = 79.2% Y=25.0 sec = 20.8% Ped= 0.0 sec = 0.0%

Lane Group	Width/Lanes	g/C Req'd	Service Rate @C (vph)	Adj @E	Volume	HCM v/c	L Delay	S	90% Max Queue
------------	-------------	-----------	-----------------------	--------	--------	---------	---------	---	---------------

N Approach

22.1 C+

TH+RT	48/4	0.282	0.500	3055	3256	660	0.203	16.9	B   140 ft
LT	12/1	0.089	0.059	81	147	119	0.726	51.4	*D   96 ft

S Approach

34.4 C

RT	12/1	0.530	0.707	1064	1108	710	0.641	12.3	B+   353 ft
TH	24/2	0.520	0.500	1573	1751	1666	0.951	41.4	*D+   707 ft
LT	12/1	0.072	0.059	384	452	413	0.914	44.5	D+   280 ft

E Approach

51.3 D

TH+RT	12/1	0.323	0.231	1	371	248	0.636	49.3	D   324 ft
LT	24/2	0.290	0.165	1	543	366	0.651	52.6	*D   259 ft

W Approach

51.3 D

RT	12/1	0.249	0.210	1	304	32	0.097	38.8	D+   43 ft
TH	12/1	0.258	0.110	1	163	71	0.351	54.2	*D   107 ft
LT	12/1	0.193	0.044	1	170	73	0.348	54.0	*D   110 ft

## SCENARIO 3

Unser / Ladera Intersection  
 Painted Sky Subdivision  
 YEAR 2005 PM PEAK HOUR - BUILD

09/20/00  
 15:42:58

## SIGNAL97/TEAPAC [Ver 1.02.14] - Capacity Analysis Summary

Intersection Averages for Int # 1 - Unser & Ladera  
 Degree of Saturation (v/c) 0.64 Vehicle Delay 28.2 Level of Service C

Sq 32	Phase 1	Phase 2	Phase 3	Phase 4
**/**				
.	+ + *		^	^
/ \	+ + *		++++	++++
-	<+ + *>		<++++	<++++
-	v		****	^
-	^	^	v	*****
North	<* + +>	<+ + +>	+> +++++	
-	++++ * + +	+ + +	+  +++++	
-	v * + +	+ + +	+   v	
	G/C=0.205	G/C=0.239	G/C=0.127	G/C=0.144
	G= 14.3"	G= 16.7"	G= 8.9"	G= 10.1"
	Y+R= 5.0"	Y+R= 5.0"	Y+R= 5.0"	Y+R= 5.0"
	OFF= 0.0%	OFF=27.6%	OFF=58.6%	OFF=78.4%

C= 70 sec G= 50.0 sec = 71.4% Y=20.0 sec = 28.6% Ped= 0.0 sec = 0.0%

Lane Group	Width/ Lanes	g/C Reqd	Service Rate @C (vph)	Adj @E	Volume	v/c	HCM Delay	L S	90% Max Queue
<b>N Approach</b>									
TH+RT	36/3	0.170	0.196	792	960	649	0.676	29.9	C 172 ft
LT	12/1	0.245	0.196	80	151	119	0.730	51.2	*D 95 ft

<b>S Approach</b>									
RT	12/1	0.482	0.670	1033	1051	695	0.661	10.1	B+ 227 ft
TH	24/2	0.198	0.472	1613	1654	542	0.328	12.1	B+ 142 ft
LT	12/1	0.181	0.162	368	420	404	0.962	49.2	*D 211 ft

<b>E Approach</b>									
RT	12/1	0.204	0.299	408	509	229	0.450	22.7	C+ 159 ft
TH	36/3	0.113	0.084	218	427	366	0.857	51.1	*D 111 ft

<b>W Approach</b>									
RT	24/2	0.053	0.377	959	1042	31	0.030	13.8	B+ 25 ft
TH	12/1	0.101	0.101	70	174	69	0.369	34.9	C 61 ft
LT	12/1	0.147	0.101	32	97	71	0.617	52.5	*D 63 ft

**APPENDIX G**

**QUEUING ANALYSIS**

**UNSER AND LADERA INTERSECTION**  
SB LT

August 11, 2000

	Variable input	Single Left Turn	
Tot vehicle turns (veh/hr)	110		
Cycle time (seconds)	60.8		
% of time capacity exceeded	5.00%		
Avg vehicle length (feet)	25.00		

Number of average Arriving Vehicles = I =		1.8578 --->	2			
P(x)	Poisson Dist	Sum of Poisson Dist	Prob of at least "x" cars arriving	Are "x" automobiles serviced?	Total Queing Length (feet)	

0	0.156019	0.156019	84.40%	serviced	0	<u>&lt;--- Que Length</u>
1	0.289849	0.445868	55.41%	serviced	25	
2	0.269237	0.715105	28.49%	serviced	50	
3	0.166728	0.881832	11.82%	serviced	75	
4	0.077436	0.959268	4.07%	capacity exceeded	100	
5	0.028772	0.988040	1.20%	capacity exceeded	125	

**UNSER AND LADERA INTERSECTION**

August 11, 2000

WB LT

	Variable Input
Tot vehicle turns (veh/hr)	921
Cycle time (seconds)	54.2
% of time capacity exceeded	5.00%
Avg vehicle length (feet)	25.00

Single Left Turn

AM PEAK HOUR

Number of average Arriving Vehicles = I =

13.8662 --->

14

P(x)	Poisson Dist	Sum of Poisson Dist	Prob of at least "x" cars arriving	Are "x" automobiles serviced?	Total Queing Length (feet)	
0	0.000001	0.000001	100.00%	serviced	0	
1	0.000013	0.000014	100.00%	serviced	25	
2	0.000091	0.000106	99.99%	serviced	50	
3	0.000422	0.000528	99.95%	serviced	75	
4	0.001464	0.001992	99.80%	serviced	100	
5	0.004061	0.006053	99.39%	serviced	125	
6	0.009384	0.015437	98.46%	serviced	150	
7	0.018589	0.034027	96.60%	serviced	175	
8	0.032220	0.066247	93.38%	serviced	200	
9	0.049642	0.115889	88.41%	serviced	225	
10	0.068834	0.184722	81.53%	serviced	250	
11	0.086769	0.271491	72.85%	serviced	275	
12	0.100263	0.371754	62.82%	serviced	300	
13	0.106943	0.478698	52.13%	serviced	325	
14	0.105921	0.584619	41.54%	serviced	350	
15	0.097914	0.682533	31.75%	serviced	375	
16	0.084856	0.767389	23.26%	serviced	400	
17	0.069214	0.836603	16.34%	serviced	425	
18	0.053318	0.889921	11.01%	serviced	450	
19	0.038911	0.928832	7.12%	serviced	475	
20	0.026978	0.955810	4.42%	capacity exceeded	500	<u>&lt;--- Que Length</u>
21	0.017813	0.973623	2.64%	capacity exceeded	525	
22	0.011227	0.984850	1.51%	capacity exceeded	550	
23	0.006769	0.991619	0.84%	capacity exceeded	575	

**UNSER AND LADERA INTERSECTION**

August 11, 2000

NB LT

	Variable Input
Tot vehicle turns (veh/hr)	372
Cycle time (seconds)	60.8
% of time capacity exceeded	5.00%
Avg vehicle length (feet)	25.00

Single Left Turn

AM PEAK HOUR

Number of average Arriving Vehicles = I =

6.2827 ---> 7

P(x)	Poisson Dist	Sum of Poisson Dist	Prob of at least "x" cars arriving	Are "x" automobiles serviced?	Total Queuing Length (feet)	
0	0.001868	0.001868	99.81%	serviced	0	
1	0.011739	0.013607	98.64%	serviced	25	
2	0.036875	0.050482	94.95%	serviced	50	
3	0.077224	0.127706	87.23%	serviced	75	
4	0.121293	0.249000	75.10%	serviced	100	
5	0.152409	0.401409	59.86%	serviced	125	
6	0.159589	0.560998	43.90%	serviced	150	
7	0.143235	0.704234	29.58%	serviced	175	
8	0.112487	0.816721	18.33%	serviced	200	
9	0.078525	0.895246	10.48%	serviced	225	
10	0.049334	0.944580	5.54%	serviced	250	
11	0.028177	0.972757	2.72%	capacity exceeded	275	<u>&lt;--- Que Length</u>
12	0.014752	0.987510	1.25%	capacity exceeded	300	
13	0.007130	0.994639	0.54%	capacity exceeded	325	
14	0.003199	0.997839	0.22%	capacity exceeded	350	

**UNSER AND LADERA INTERSECTION**

EB LT

August 11, 2000

	Variable input
Tot vehicle turns (veh/hr)	81
Cycle time (seconds)	81
% of time capacity exceeded	5.00%
Avg vehicle length (feet)	25.00

Single Left Turn

AM PEAK HOUR

Number of average Arriving Vehicles = I =

1.8225 -->

2

P(x)	Poisson Dist	Sum of Poisson Dist	Prob of at least "x" cars arriving	Are "x" automobiles serviced?	Total Queing Length (feet)	
0	0.161621	0.161621	83.84%	serviced	0	
1	0.294555	0.456176	54.38%	serviced	25	
2	0.268413	0.724589	27.54%	serviced	50	
3	0.163061	0.887650	11.24%	serviced	75	
4	0.074295	0.961944	3.81%	capacity exceeded	100	<u>&lt;--- Que Length</u>
5	0.027080	0.989025	1.10%	capacity exceeded	125	

**UNSER AND LADERA INTERSECTION**  
EB LT

September 20, 2000

	Variable input
Tot vehicle turns (veh/hr)	81
Cycle time (seconds)	84.1
% of time capacity exceeded	5.00%
Avg vehicle length (feet)	25.00

Single Left Turn

AM PEAK HOUR

Number of average Arriving Vehicles = I = 1.8923 ---> 2

P(x)	Poisson Dist	Sum of Poisson Dist	Prob of at least "x" cars arriving	Are "x" automobiles serviced?	Total Queing Length (feet)	
0	0.150732	0.150732	84.93%	serviced	0	
1	0.285223	0.435955	56.40%	serviced	25	
2	0.269857	0.705812	29.42%	serviced	50	
3	0.170212	0.876024	12.40%	serviced	75	
4	0.080521	0.956545	4.35%	capacity exceeded	100	<u>&lt;--- Que Length</u>
5	0.030473	0.987018	1.30%	capacity exceeded	125	
6	0.009610	0.996629	0.34%	capacity exceeded	150	
7	0.002598	0.999227	0.08%	capacity exceeded	175	
8	0.000614	0.999841	0.02%	capacity exceeded	200	
9	0.000129	0.999971	0.00%	capacity exceeded	225	
10	0.000024	0.999995	0.00%	capacity exceeded	250	
11	0.000004	0.999999	0.00%	capacity exceeded	275	
12	0.000001	1.000000	0.00%	capacity exceeded	300	
13	0.000000	1.000000	0.00%	capacity exceeded	325	
14	0.000000	1.000000	0.00%	capacity exceeded	350	
15	0.000000	1.000000	0.00%	capacity exceeded	375	
16	0.000000	1.000000	0.00%	capacity exceeded	400	
17	0.000000	1.000000	0.00%	capacity exceeded	425	
18	0.000000	1.000000	0.00%	capacity exceeded	450	
19	0.000000	1.000000	0.00%	capacity exceeded	475	
20	0.000000	1.000000	0.00%	capacity exceeded	500	
21	0.000000	1.000000	0.00%	capacity exceeded	525	
22	0.000000	1.000000	0.00%	capacity exceeded	550	
23	0.000000	1.000000	0.00%	capacity exceeded	575	
24	0.000000	1.000000	0.00%	capacity exceeded	600	
25	0.000000	1.000000	0.00%	capacity exceeded	625	
26	0.000000	1.000000	0.00%	capacity exceeded	650	
27	0.000000	1.000000	0.00%	capacity exceeded	675	
28	0.000000	1.000000	0.00%	capacity exceeded	700	
29	0.000000	1.000000	0.00%	capacity exceeded	725	
30	0.000000	1.000000	0.00%	capacity exceeded	750	
31	0.000000	1.000000	0.00%	capacity exceeded	775	

**UNSER AND LADERA INTERSECTION**  
SB LT

September 20, 2000

	Variable input		
Tot vehicle turns (veh/hr)	110		Single Left Turn
Cycle time (seconds)	57.8		
% of time capacity exceeded	5.00%		
Avg vehicle length (feet)	25.00		AM PEAK HOUR

Number of average Arriving Vehicles = I = 1.7661 ---> 2

P(x)	Poisson Dist	Sum of Poisson Dist	Prob of at least "x" cars arriving	Are "x" automobiles serviced?	Total Queing Length (feet)	
0	0.170997	0.170997	82.90%	serviced	0	
1	0.301999	0.472996	52.70%	serviced	25	
2	0.266682	0.739678	26.03%	serviced	50	
3	0.156997	0.896675	10.33%	serviced	75	
4	0.069318	0.965993	3.40%	capacity exceeded	100	<u>&lt;--- Que Length</u>
5	0.024485	0.990478	0.95%	capacity exceeded	125	
6	0.007207	0.997685	0.23%	capacity exceeded	150	
7	0.001818	0.999503	0.05%	capacity exceeded	175	
8	0.000401	0.999905	0.01%	capacity exceeded	200	
9	0.000079	0.999983	0.00%	capacity exceeded	225	
10	0.000014	0.999997	0.00%	capacity exceeded	250	
11	0.000002	1.000000	0.00%	capacity exceeded	275	
12	0.000000	1.000000	0.00%	capacity exceeded	300	
13	0.000000	1.000000	0.00%	capacity exceeded	325	
14	0.000000	1.000000	0.00%	capacity exceeded	350	
15	0.000000	1.000000	0.00%	capacity exceeded	375	
16	0.000000	1.000000	0.00%	capacity exceeded	400	
17	0.000000	1.000000	0.00%	capacity exceeded	425	
18	0.000000	1.000000	0.00%	capacity exceeded	450	
19	0.000000	1.000000	0.00%	capacity exceeded	475	
20	0.000000	1.000000	0.00%	capacity exceeded	500	
21	0.000000	1.000000	0.00%	capacity exceeded	525	
22	0.000000	1.000000	0.00%	capacity exceeded	550	
23	0.000000	1.000000	0.00%	capacity exceeded	575	
24	0.000000	1.000000	0.00%	capacity exceeded	600	
25	0.000000	1.000000	0.00%	capacity exceeded	625	
26	0.000000	1.000000	0.00%	capacity exceeded	650	
27	0.000000	1.000000	0.00%	capacity exceeded	675	
28	0.000000	1.000000	0.00%	capacity exceeded	700	
29	0.000000	1.000000	0.00%	capacity exceeded	725	
30	0.000000	1.000000	0.00%	capacity exceeded	750	
31	0.000000	1.000000	0.00%	capacity exceeded	775	

**UNSER AND LADERA INTERSECTION**

WB LT

September 20, 2000

	Variable Input
Tot vehicle turns (veh/hr)	921
Cycle time (seconds)	89.5
% of time capacity exceeded	5.00%
Avg vehicle length (feet)	25.00

Single Left Turn

AM PEAK HOUR

Number of average Arriving Vehicles = I =

22.8971 ---> 23

P(x)	Poisson Dist	Sum of Poisson Dist	Prob of at least "x" cars arriving	Are "x" automobiles serviced?	Total Queuing Length (feet)	
0	0.000000	0.000000	100.00%	serviced	0	
1	0.000000	0.000000	100.00%	serviced	25	
2	0.000000	0.000000	100.00%	serviced	50	
3	0.000000	0.000000	100.00%	serviced	75	
4	0.000001	0.000002	100.00%	serviced	100	
5	0.000006	0.000008	100.00%	serviced	125	
6	0.000023	0.000030	100.00%	serviced	150	
7	0.000074	0.000105	99.99%	serviced	175	
8	0.000213	0.000318	99.97%	serviced	200	
9	0.000542	0.000860	99.91%	serviced	225	
10	0.001242	0.002102	99.79%	serviced	250	
11	0.002584	0.004686	99.53%	serviced	275	
12	0.004931	0.009617	99.04%	serviced	300	
13	0.008685	0.018302	98.17%	serviced	325	
14	0.014205	0.032507	96.75%	serviced	350	
15	0.021683	0.054191	94.58%	serviced	375	
16	0.031030	0.085221	91.48%	serviced	400	
17	0.041794	0.127015	87.30%	serviced	425	
18	0.053165	0.180180	81.98%	serviced	450	
19	0.064070	0.244250	75.58%	serviced	475	
20	0.073350	0.317600	68.24%	serviced	500	
21	0.079977	0.397577	60.24%	serviced	525	
22	0.083238	0.480814	51.92%	serviced	550	
23	0.082865	0.563679	43.63%	serviced	575	
24	0.079057	0.642737	35.73%	serviced	600	
25	0.072407	0.715144	28.49%	serviced	625	
26	0.063766	0.778910	22.11%	serviced	650	
27	0.054076	0.832986	16.70%	serviced	675	
28	0.044221	0.877207	12.28%	serviced	700	
29	0.034915	0.912121	8.79%	serviced	725	
30	0.026648	0.938769	6.12%	serviced	750	
31	0.019683	0.958452	4.15%	capacity exceeded	775	<--- Que Length

**UNSER AND LADERA INTERSECTION**  
NB LT

September 20, 2000

	Variable input
Tot vehicle turns (veh/hr)	372
Red Cycle time (seconds)	27.7
% of time capacity exceeded	5.00%
Avg vehicle length (feet)	25.00

Single Left Turn

PM PEAK HOUR

Number of average Arriving Vehicles = I = 2.8623 --> 3

P(x)	Poisson Dist	Sum of Poisson Dist	Prob of at least "x" cars arriving	Are "x" automobiles serviced?	Total Queing Length (feet)	
0	0.057135	0.057135	94.29%	serviced	0	
1	0.163540	0.220676	77.93%	serviced	25	
2	0.234053	0.454729	54.53%	serviced	50	
3	0.223313	0.678042	32.20%	serviced	75	
4	0.159799	0.837841	16.22%	serviced	100	
5	0.091480	0.929320	7.07%	serviced	125	
6	0.043641	0.972961	2.70%	capacity exceeded	150	<u>&lt;--- Que Length</u>
7	0.017845	0.990806	0.92%	capacity exceeded	175	
8	0.006385	0.997191	0.28%	capacity exceeded	200	
9	0.002031	0.999222	0.08%	capacity exceeded	225	
10	0.000581	0.999803	0.02%	capacity exceeded	250	
11	0.000151	0.999954	0.00%	capacity exceeded	275	
12	0.000036	0.999990	0.00%	capacity exceeded	300	
13	0.000008	0.999998	0.00%	capacity exceeded	325	
14	0.000002	1.000000	0.00%	capacity exceeded	350	
15	0.000000	1.000000	0.00%	capacity exceeded	375	
16	0.000000	1.000000	0.00%	capacity exceeded	400	
17	0.000000	1.000000	0.00%	capacity exceeded	425	
18	0.000000	1.000000	0.00%	capacity exceeded	450	
19	0.000000	1.000000	0.00%	capacity exceeded	475	
20	0.000000	1.000000	0.00%	capacity exceeded	500	
21	0.000000	1.000000	0.00%	capacity exceeded	525	
22	0.000000	1.000000	0.00%	capacity exceeded	550	
23	0.000000	1.000000	0.00%	capacity exceeded	575	
24	0.000000	1.000000	0.00%	capacity exceeded	600	
25	0.000000	1.000000	0.00%	capacity exceeded	625	
26	0.000000	1.000000	0.00%	capacity exceeded	650	
27	0.000000	1.000000	0.00%	capacity exceeded	675	
28	0.000000	1.000000	0.00%	capacity exceeded	700	
29	0.000000	1.000000	0.00%	capacity exceeded	725	
30	0.000000	1.000000	0.00%	capacity exceeded	750	
31	0.000000	1.000000	0.00%	capacity exceeded	775	