



May 17, 2016

Graeme Means, P.E. High Mesa Consulting Group 6010-B Midway Park Blvd NE Albuquerque, New Mexico 87109

RE: West Mesa High School New Classroom Building 6701 Fortuna Rd NW Grading and Drainage Plan Engineers Stamp Date 5/12/16 (J10D005)

Dear Mr. Means,

Based upon the information provided in your submittal received 5/12/15, this plan is approved for Grading Permit and Paving Permit. This will now be the plan used for building permit and CO.

- PO Box 1293 Please have the owner/contractor attach a copy of this approved plan dated 5/12/16 to the construction sets in the permitting process prior to sign-off by Hydrology. If this plan is not in the construction sets at permitting the project will be rejected.
- Albuquerque Once the project is complete, provide our department with a copy of the as-build survey for our records.

New Mexico 87103 If you have any questions, please contact me at 924-3986 or Rudy Rael at 924-3977.

www.cabq.gov

Sincerely

Abiel Carrillo, P.E. Principal Engineer, Hydrology Planning Department

RR/AC C: File



City of Albuquerque

Planning Department Development & Building Services Division DRAINAGE AND TRANSPORTATION INFORMATION SHEET (REV 09/2015)

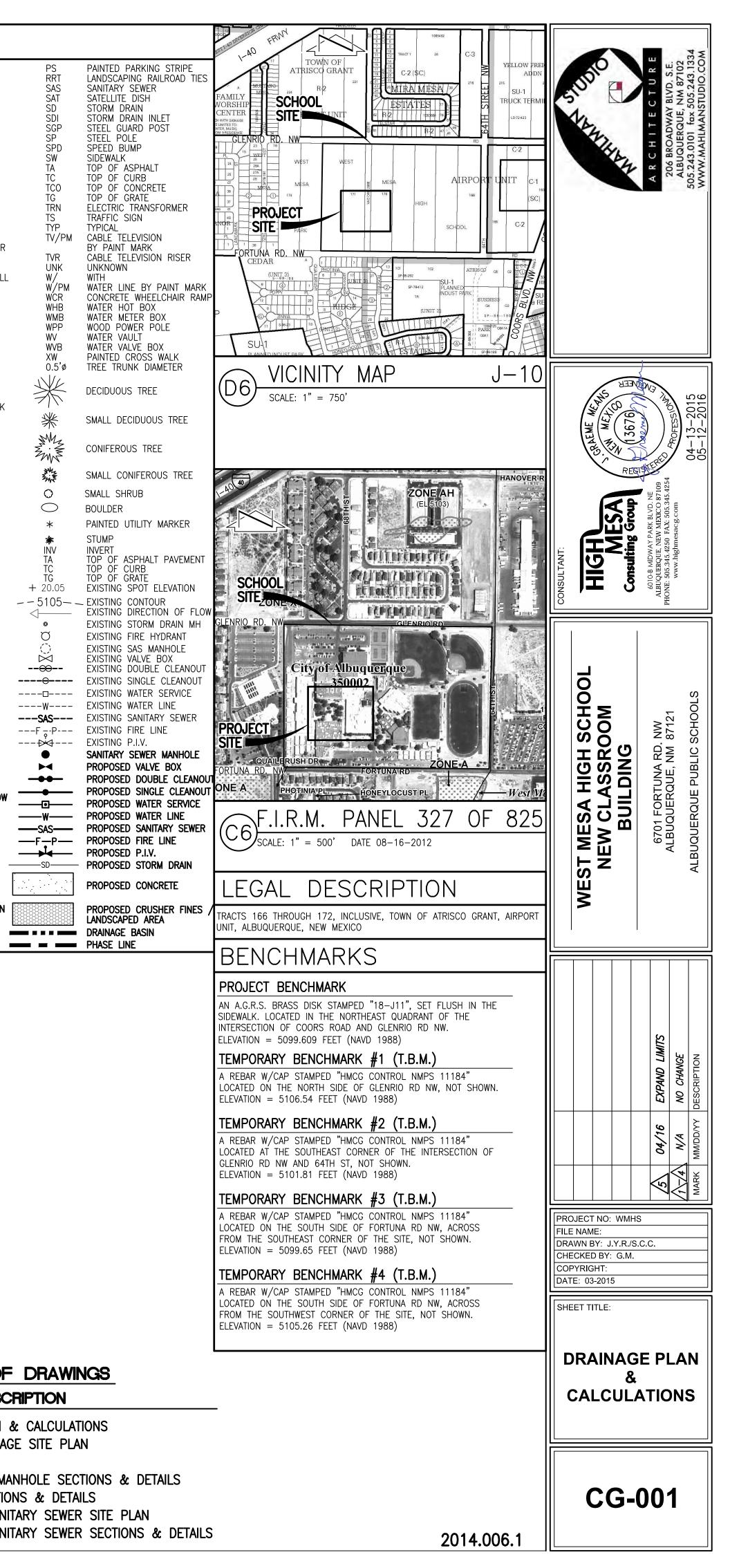
Project Title:	Building Permit #:	City Drainage #:		
DRB#: EPC#:		Work Order#:		
Legal Description:				
City Address:				
Engineering Firm:		Contact:		
Address:				
Phone#: Fax#:		_ E-mail:		
Owner:		Contact:		
Address:				
Phone#: Fax#:		_ E-mail:		
Architect:		Contact:		
Address:				
Phone#: Fax#:		_ E-mail:		
Other Contact:		Contact:		
Address:				
Phone#: Fax#:		_ E-mail:		
TRAFFIC/ TRANSPORTATION MS4/ EROSION & SEDIMENT CONTROL TYPE OF SUBMITTAL: ENGINEER/ ARCHITECT CERTIFICATION CONCEPTUAL G & D PLAN GRADING PLAN DRAINAGE MASTER PLAN DRAINAGE REPORT CLOMR/LOMR TRAFFIC CIRCULATION LAYOUT (TCL) TRAFFIC IMPACT STUDY (TIS)	CERTIFICA PRELIMINA SITE PLAN SITE PLAN FINAL PLA SIA/ RELEA FOUNDATIO GRADING F SO-19 APPR PAVING PE	RMIT APPROVAL PAD CERTIFICATION		
EROSION & SEDIMENT CONTROL PLAN (ESC)	CLOMR/LOI	CLOMR/LOMR		
OTHER (SPECIFY) IS THIS A RESUBMITTAL?:YesNo	PRE-DESIGN OTHER (SP	MEETING ECIFY)		
DATE SUBMITTED:				

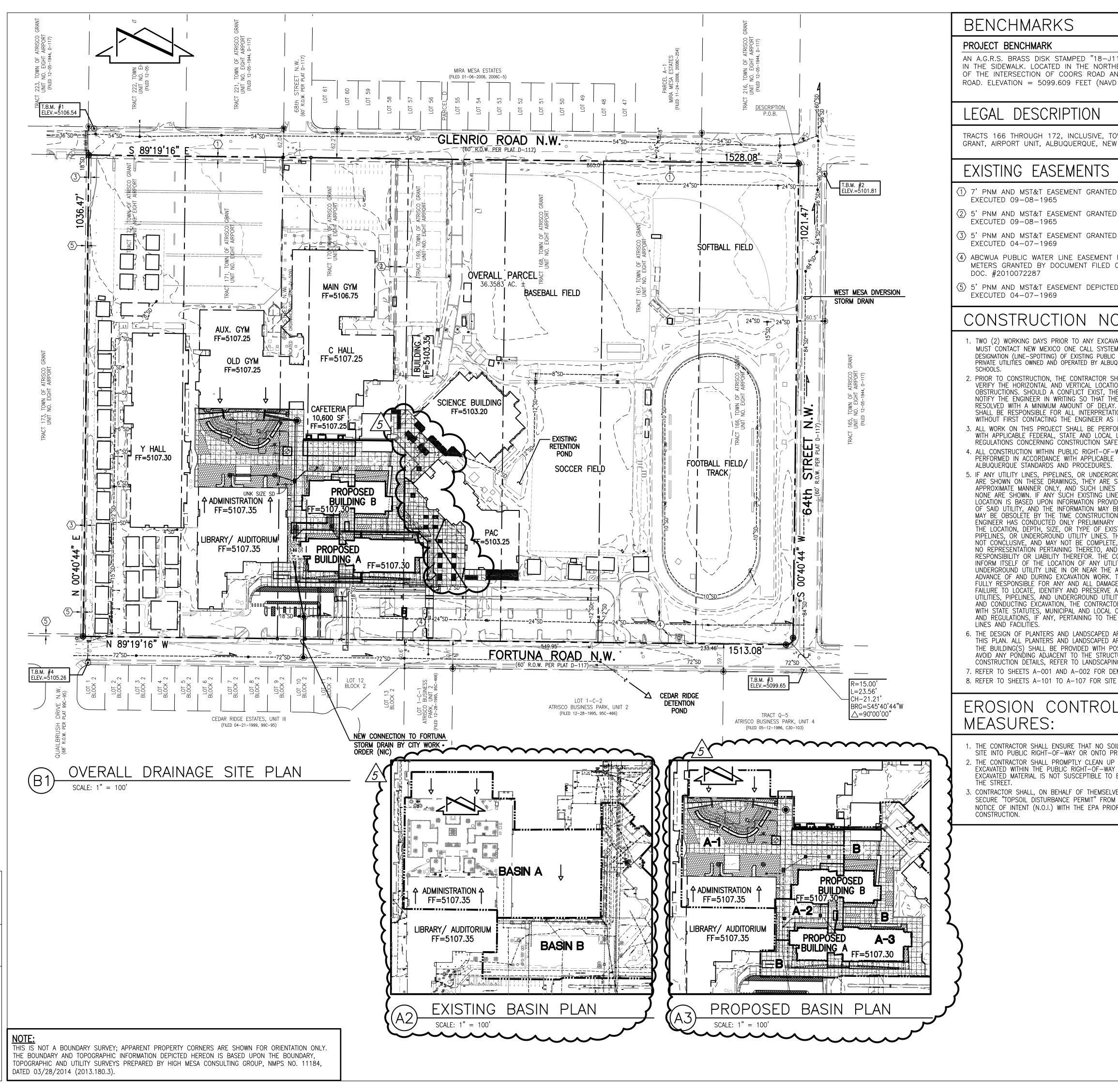
COA STAFF: ELECTRONIC SUBMITTAL RECEIVED: ____

INTRODUCTION AND EXECUTIVE SUMMARY THIS PROJECT, LOCATED WITHIN THE NORTHWEST MESA OF THE ALBUQUERQUE METROPOLITAN AREA, REPRESENTS A MODIFICATION TO AN	I. <u>SITE CHARACTERISTICS</u> A. PRECIPITATION ZONE =	1
EXISTING APS SCHOOL SITE WITHIN AN INFILL AREA. THE PROPOSED DEVELOPMENT IS COMPRISED OF TWO PHASES. THE FIRST PHASE IS THE CONSTRUCTION OF A NEW CLASSROOM BUILDING. THE SECOND PHASE IS DEMOLITION OF AN EXISTING CLASSROOM WING FOLLOWED BY CONSTRUCTION OF A SECOND BUILDING AND RECONSTRUCTION OF AN EXISTING COURTYARD. THE CITY HYDROLOGY FILE NO. IS J10-D005. THE CONSTRUCTION OF AN EXISTING COURTYARD. THE CITY HYDROLOGY FILE NO. IS J10-D005. THE CONSTRUCTION OF AN EXISTING COURTYARD. THE CITY HYDROLOGY FILE NO. IS J10-D005. THE CONSTRUCTION OF AN EXISTING COURTYARD. THE CITY HYDROLOGY FILE NO. IS J10-D005. THE CONSTRUCTION OF AN EXISTING COURTYARD. THE CITY HYDROLOGY FILE NO. IS J10-D005. THE CITY HYDROLOGY FILE NO. IS J10-D005.	5 B. P _{6,100} = P ₃₆₀ =	2.20
DRAINAGE CONCEPT FOR THE PROJECT WILL BE THE CONTINUED FREE DISCHARGE OF DEVELOPED RUNOFF TO EXISTING PUBLIC AND PRIVATE STORM DRAINS THAT OUTFALL TO THE EXISTING WEST MESA DETENTION BASIN AND TO THE WEST MESA DIVERSION STORM DRAIN (CPN 538103). THIS SUBMITTAL IS BEING MADE FOR BUILDING PERMIT APPROVAL	C. TOTAL PROJECT AREA (A_{T} = 181,880 4.18 4
PROJECT DESCRIPTION	D. LAND TREATMENTS	\sim
E SCHOOL SITE IS LOCATED AT THE NORTHWEST CORNER OF THE INTERSECTION OF FORTUNA ROAD NW AND 64 TH STREET NW, BOTH FULLY VELOPED CITY STREETS. THE SITE IS DEVELOPED AS AN ALBUQUERQUE PUBLIC SCHOOLS HIGH SCHOOL. GLENRIO ROAD NW, A PARTIALLY VELOPED CITY STREET LIES TO THE NORTH. IT LACKS CURB AND GUTTER ALONG THE SCHOOL FRONTAGE. THE CITY OF ALBUQUERQUE WEST	1. BASIN A	
SA AQUATIC CENTER LIES TO THE WEST OF THE SCHOOL SITE. THE SURROUNDING AREA IS DEVELOPED, MAINLY SINGLE FAMILY RESIDENTIAL, KING THIS A MODIFICATION TO AN EXISTING SITE WITHIN AN INFILL AREA. THE SITE CONSISTS OF PREVIOUSLY PLATTED LOTS AND FORMER Y STREETS THAT MAY HAVE BEEN VACATED. AS SHOWN BY PANEL 35001C0327H OF THE NATIONAL FLOOD INSURANCE PROGRAM FLOOD	TOTAL BASIN AREA	(A _T)= 101,310 \$
URANCE RATE MAPS PUBLISHED BY FEMA FOR BERNALILLO COUNTY, NEW MEXICO, REVISED AUGUST 16, 2012, THIS SITE DOES NOT LIE WITHIN ESIGNATED FLOOD HAZARD ZONE.	EXISTING LAND TREA	ATMENT
ACKGROUND DOCUMENTS PREPARATION OF THIS PLAN RELIED UPON THE FOLLOWING DOCUMENTS AND ACTIVITIES:	TREATMENT	AREA (SF/AC
. BOUNDARY, TOPOGRAPHIC, AND UTILITY SURVEY PREPARED BY HIGH MESA CONSULTING GROUP DATED 3/28/2014 (NMPS 11184). THIS SURVEY PROVIDES THE BASIS FOR THE EXISTING CONDITIONS OF THE PROJECT SITE.	A B	9,900 / 0
PREDESIGN CONFERENCE RECAPS DATED 02-13-2014 AND 9-10-2014 CONDUCTED WITH HIGH MESA CONSULTING GROUP. THE RECAPS CONFIRMED THAT THE CONTINUED FREE DISCHARGE OF DEVELOPED RUNOFF TO THE ADJACENT PUBLIC STORM DRAIN SYSTEMS VIA PRIVATE STORM DRAIN CONNECTIONS IS APPROPRIATE, AND THAT IT IS PERMISSIBLE TO DIVERT RUNOFF FROM EXISTING BASINS 104		7,000 / 0
AND 105 TO THE FORTUNA STORM DRAIN. DRAINAGE PLAN FOR WEST MESA HIGH SCHOOL CLASSROOM WING "M" REPLACEMENT & COURTYARD IMPROVEMENTS PREPARED BY HIGH MESA CONSULTING GROUP DATED 12/31/2014 (NMPE 13676). THIS SUBMITTAL SUPPORTED WORK ORDER APPROVAL FOR A NEW	2. BASIN B	
STORM DRAIN CONNECTION TO FORTUNA THAT WILL SERVE THIS NEW BUILDING CONSTRUCTION. THIS SUBMITTAL INCLUDED A COMPREHENSIVE ANALYSIS OF EXISTING CONDITIONS AND SUPPORTED THE DIVERSION OF APPROXIMATELY 2.2 ACRES OF THE HIGH SCHOOL SITE TO THE EXISTING 72 INCH PUBLIC STORM DRAIN IN FORTUNA RD VIA THE NEW STORM DRAIN CONNECTION. AS IDENTIFIED		\sim
IN THIS REFERENCE DOCUMENT, HISTORIC BUILDOUT IN THIS WATERSHED HAS RESULTED IN A NET REDUCTION OF 4.7 ACRES OF DEVELOPED PROPERTY DRAINING TO FORTUNA AS COMPARED TO THE BASINS ESTABLISHED FOR THE PUBLIC STORM DRAINS, 4.0 ACRES FROM THE WEST MESA AQUATIC CENTER, AND A NET OF 0.7 ACRES WITHIN THE HIGH SCHOOL DUE TO ON-SITE PRIVATE STORM DRAIN	TOTAL BASIN AREA	
CONSTRUCTION UNDER REFERENCE 4. THIS DECREASE IN AREA DRAINING TO FORTUNA MEANS THERE IS CORRESPONDING EXCESS CAPACITY. SWITCHING 2.2 ACRES AS PROPOSED WILL STILL LEAVE A NET DECREASE OF 2.5 ACRES DRAINING TO FORTUNA AFTER IMPLEMENTATION OF THIS NEW PROPOSED CONNECTION AND DIVERSION.	EXISTING LAND TREA	
. WORK ORDER CONSTRUCTION PLANS FOR WEST MESA HIGH SCHOOL PUBLIC AND PRIVATE WATER, SANITARY SEWER, AND STORM DRAIN LINE EXTENSIONS AND RELOCATIONS PREPARED BY HIGH MESA CONSULTING GROUP, CPN 749982, DATED 01/14/2015 (NMPE 13676). THIS WORK ORDER PLAN SET INCLUDES CONSTRUCTION OF A NEW 24 INCH CONNECTION TO THE EXISTING 72 INCH PUBLIC STORM DRAIN IN FORTUNA RD NW. THIS CONNECTION WILL SERVE THE PROPOSED BUILDING AND COURTYARD IMPROVEMENTS.	TREATMENT A	AREA (SF/A
WORK ORDER PLAN SET INCLUDES CONSTRUCTION OF A NEW 24 INCH CONNECTION TO THE EXISTING 72 INCH PUBLIC STORM DRAIN IN		7,650 / 9,780 /
FORTUNA RD NW. THIS CONNECTION WILL SERVE THE PROPOSED BUILDING AND COURTYARD IMPROVEMENTS.	3. BASIN A-1	63,140 /
SITE IS DEVELOPED AS A HIGH SCHOOL OPERATED AND MAINTAINED BY THE ALBUQUERQUE PUBLIC SCHOOLS (APS). THE SCHOOL SITE ISTS OF PERMANENT AND PORTABLE CLASSROOM BUILDINGS, PAVED PARKING AREAS AND WALKWAYS, LANDSCAPING, ATHLETIC FIELDS, DTHER SITE IMPROVEMENTS APPLICABLE TO A HIGH SCHOOL SITE. AS EXPLAINED AND DEMONSTRATED BY THE AFOREMENTIONED	TOTAL BASIN AREA	(A _τ)= 72,030
DTHER SITE IMPROVEMENTS APPLICABLE TO A HIGH SCHOOL SITE. AS EXPLAINED AND DEMONSTRATED BY THE AFOREMENTIONED JAGE PLAN (REF. 4), THE OVERALL SITE IS CHARACTERIZED BY THIRTEEN (13) DRAINAGE BASINS, OF WHICH FOUR WILL BE AFFECTED BY PROJECT.	DEVELOPED LAND	
THE PURPOSES OF THIS SPECIFIC PROJECT, THE IMPACTED PORTION OF THE SITE HAS BEEN DIVIDED INTO TWO DRAINAGE BASINS WITH N A BEING THE PORTION OF THE SITE THAT DISCHARGES DIRECTLY TO AN EXISTING PRIVATE STORM DRAIN SYSTEM THAT CURRENTLY NS TO THE EAST TO THE EXISTING PUBLIC WEST MESA DIVERSION STORM DRAIN, AND BASIN B BEING THE PORTION THAT DRAINS OVERLAND	TREATMENT	AREA (SF/AC
HE EXISTING INTERNAL PRIVATE STORM DRAIN SYSTEM. THESE BASINS ARE SHOWN ON SHEET CG-101. THERE ARE NO OFFSITE FLOWS HARGING ONTO THE PROJECT SITE AS THE PROJECT LIMITS LIE WELL WITHIN THE OVERALL SCHOOL SITE AND NOT ADJACENT TO HBORING PROPERTIES.	A	11,770 / 0
VELOPED CONDITIONS		11,140 / (49,120 /
VERALL PROJECT CONSISTS OF CONSTRUCTING TWO NEW BUILDINGS TO REPLACE THE EXISTING "M" HALL AND RE-CONSTRUCTING THE ING COURTYARD. THE PROJECT WILL BE CONSTRUCTED IN PHASES, WITH THE FIRST PHASE BEING CONSTRUCTION OF BUILDING "A" WITHIN ISTING PARKING LOT. AS SHOWN BY THE GRADING PLAN ON SHEET CG-102, THE ROOF DRAINS AND A PORTION OF SITE WORK WILL DRAIN	4. BASIN A-2	
CTLY TO THE NEW STORM DRAIN CONNECTION TO THE EXISTING 72 INCH PUBLIC STORM DRAIN IN FORTUNA TO BE CONSTRUCTED UNDER WORK ORDER IN ACCORDANCE WITH THE CONCEPT ESTABLISHED BY THE AFOREMENTIONED PLAN (REF. 3) AND THE PREDESIGN RECAPS 2). THIS FIRST PHASE IS INTENDED TO STAND ALONE WITH THE INTENT OF OBTAINING A CERTIFICATE OF OCCUPANCY FOR BUILDING "A" IN	TOTAL BASIN AREA	(A _T)= 21,680 \$
NCE OF PHASE 2 CONSTRUCTION. THIS IS ACCEPTABLE BECAUSE PHASE 1 CONSTRUCTION DOES NOT RELY UPON PHASE 2 IMPROVEMENTS. SE 1 CONSTRUCTION LIES WITHIN AN EXISTING PAVED PARKING LOT, HENCE THE NEW CONSTRUCTION WILL NOT RESULT IN AN INCREASE IN DFF DURING THE INTERIM CONDITION BETWEEN PHASE 1 AND PHASE 2.	DEVELOPED LAND	TREATMENT
SE 2 CONSTRUCTION WILL INCLUDE THE DEMOLITION OF THE EXITING "M" HALL, CONSTRUCTION OF BUILDING "B", AND RECONSTRUCTION OF EXISTING COURTYARD. THE PRIVATE STORM DRAIN THAT WILL DISCHARGE TO THE NEW FORTUNA CONNECTION WILL BE EXTENDED WITH	TREATMENT	AREA (SF/AG
PHASE TO SERVE BUILDING "B" AND THE COURTYARD. AN EXISTING STORM DRAIN CURRENTLY SERVING THE COURTYARD WILL BE ACED WITH A NEW STORM DRAIN.	A B C	
IOWN BY THE PLANS, SITE ROOF DRAINAGE WILL BE DIRECTLY PIPED TO PROPOSED PRIVATE STORM DRAINS. ALL SITE SURFACE RUNOFF BE DIRECTED TO NEW STORM DRAIN INLETS THAT ARE MOSTLY LOCATED IN LANDSCAPED AREAS THAT WILL HAVE A CRUSHER FINES ACING. IN MOST CASES, THE CONCEPT OF "DISCONNECTED IMPERVIOUSNESS" IS EMPLOYED WHEREBY RUNOFF FROM HARDSCAPED		580 / 0 21,100 / 0
AS FLOW ACROSS THE CRUSHER FINES BEFORE REACHING THE INLETS. BECAUSE THIS IS AN INFILL PROJECT AT AN EXISTING SCHOOL PUS, THERE IS LIMITED ABILITY TO INTRODUCE AREAS OF RETENTION IN THE PERVIOUS AREAS BECAUSE THEY ARE INTENDED FOR DENTS TO GATHER DURING LUNCH BREAK, AND RETENTION WOULD RESULT IN STANDING WATER IN PEDESTRIAN AREAS. AS SUCH, THE	5. BASIN A-3	
NT OF THE "FIRST FLUSH" REQUIREMENTS WILL NOT BE MET BY A SPECIFIC RETENTION OF VOLUME, BUT RATHER THROUGH THE USE OF ONNECTED IMPERVIOUSNESS COMBINED WITH THE AFOREMENTIONED TRANSFER OF 2.2 ACRES OF AREA FROM THE CURRENT FREE HARGE CONDITION TO THE WEST MESA DIVERSION STORM DRAIN. TO THE WEST MESA DETENTION POND THAT IS IDENTIFIED AS A WATER	TOTAL BASIN AREA	(A _T)= 25,950
LITY FEATURE ON THE AMAFCA MAINTENANCE MAPS.	DEVELOPED LAND	REATMENT
GRADING PLAN ON SHEET CG-102 SHOWS 1.) EXISTING AND PROPOSED GRADES INDICATED BY SPOT ELEVATIONS AND CONTOURS AT 1'-0" RVALS, 2.) THE LIMIT AND CHARACTER OF THE EXISTING AND PROPOSED IMPROVEMENTS, 3) PROPOSED PRIVATE STORM DRAIN	TREATMENT A	AREA (SF/AC
ROVEMENTS, AND 4.) CONTINUITY BETWEEN EXISTING AND PROPOSED GRADES. CALCULATIONS		370 / 0
CULATIONS SHOWN HEREON ANALYZE THE EXISTING AND DEVELOPED CONDITIONS FOR THE 100-YEAR, 6-HOUR RAINFALL EVENT. THE CEDURE FOR 40 ACRE AND SMALLER BASINS, AS SET FORTH IN THE REVISION OF SECTION 22.2, HYDROLOGY OF THE DEVELOPMENT		25,580 / 0
DCESS MANUAL, VOLUME 2, DESIGN CRITERIA, DATED JANUARY 1993, WILL BE USED TO QUANTIFY THE PEAK RATE OF DISCHARGE AND VOLUME RUNOFF GENERATED. AS SHOWN BY THE CALCULATIONS, THERE WILL BE A MINOR DECREASE IN 100-YEAR VOLUME AND NO CHANGE IN 100- R PEAK DISCHARGE ATTRIBUTABLE TO THIS PROJECT.	6. BASIN B	~~~~
CONCLUSIONS	TOTAL BASIN AREA	(A _T)= 62,220
E FOLLOWING CONCLUSIONS HAVE BEEN ESTABLISHED AS A RESULT OF THE ANALYSIS AND EVALUATIONS CONTAINED HEREIN: 1. THE PROPOSED IMPROVEMENTS REPRESENT MODIFICATIONS TO AN EXISTING SITE WITHIN AN INFILL AREA 2. THE EREE DISCHARGE OF DEVELOPED BUNDEE TO THE ADJACENT BUBLIC STORM DRAIN IS CONSISTENT WITH THE PREVIOUSLY	DEVELOPED LAND	TREATMENT
 THE FREE DISCHARGE OF DEVELOPED RUNOFF TO THE ADJACENT PUBLIC STORM DRAIN IS CONSISTENT WITH THE PREVIOUSLY APPROVED PLANS FOR THE SCHOOL SITE AND WITH MASTER DRAINAGE PLANS FOR THE WATERSHED. THE PROPOSED IMPROVEMENTS WILL NOT ADVERSELY IMPACT DOWNSTREAM PROPERTIES OR DOWNSTREAM DRAINAGE CONDITIONS. THE PROPOSED DIVERSION OF RUNOFE OF 2.2 ACRES OF THE HIGH SCHOOL WAS ESTABLISHED BY PREVIOUS SUBMITTAL AND WILL 	TREATMENT	AREA (SF/AG
 THE PROPOSED DIVERSION OF RUNOFF OF 2.2 ACRES OF THE HIGH SCHOOL WAS ESTABLISHED BY PREVIOUS SUBMITTAL, AND WILL RESULT IN THIS AREA BEING ROUTED THROUGH A PUBLIC DETENTION FACILITY WITH WATER QUALITY BENEFITS. THE PROPOSED IMPROVEMENTS WILL NOT AFFECT NOR BLOCK OFFSITE FLOWS. 	<u>A</u> <u>B</u>	3,040 / 0
5		5,760 / 0 53,420 / 1
		~~~
MESA HS. THE UPDATE IS IN RESPONSE TO ADDITIONAL SITE WORK BEING ADDED TO PROJECT	II. HYDROLOGY A. EXISTING CONDITIONS	
ALONG THE EAST SIDE OF THE PREVIOUS PROJECT LIMITS. THIS ADDITIONAL AREA WILL BE ADDED TO THE DRAINAGE BASIN B AND RESULT IN A VERY MINOR INCREASE IN THE OVERALL SITE'S DEAK DISCHARGE AND VOLUME OF RUNDEE FOR THE DEVELOPED CONDITION. THIS INCREASE WILL	1. BASIN A	
PEAK DISCHARGE AND VOLUME OF RUNOFF FOR THE DEVELOPED CONDITION. THIS INCREASE WILL NOT ADVERSELY IMPACT THE DOWNSTREAM BASINS LOCATED WITHIN THE SITE AND THE FLOWS THAT EXIT THE ENTIRE SCHOOL SITE TO THE EAST DUE TO THE DIVERSION OF THE DEVELOPED	a. VOLUME	
BASIN A RUNOFF INTO THE FORTUNA RD STORM DRAIN AS DESCRIBED IN THIS DRAINAGE PLAN.	$E_{W} = (E_{A}A_{A} + E_{B}A_{B} + E_{C}A_{A} + E_{B}A_{A} + E_{A}A_{A} + E_{B}A_{A} + E_{B}A_{A} + E_{B}A_{A} + E_{A}A_{A} + E_{B}A_{A} + E_{A}A_{A} + E_{B}A_{A} + E_{A}A_{A} + E_{B}A_{A} + E_{A}A_{A} + E_{A}A_$	_C A _C +E _D A _D )/A _T .44) + (0.23*0.67) +
	$V_{100} = (E_W/12)A_T =$	(1.73/12)1.88
	b. PEAK DISCHARGE	
	$Q_{P} = Q_{PA}A_{A} + Q_{PB}A_{B}$ $Q_{P} = Q_{100} = ((0.00^{*}1))$	
	2. BASIN B	
	a. VOLUME	~~~~
	$E_{W} = (E_A A_A + E_B A_B + E_C E_W)$ $E_W = ((0.00^*0)$	_C A _C +E _D A _D )/A _T .44) + (0.18*0.67) +
	$V_{100} = (E_W/12)A_T =$	(1.73/12)1.85
	b. PEAK DISCHARGE $Q_{P} = Q_{PA}A_{A} + Q_{PB}A_{B}$	
	$Q_{\rm P} = Q_{100} = ((0.00^{*1})^{-1})^{-1}$	신다. 이번 이에 나온다. 이번 것이다.
	~~~~	$\cdots$

²ath: P:\DATA\2014.006.1\ENG\R5\ Plot Date: 05-12-2016 Vame:140061_CG-001-R5.DWG Plot Time: 09:20 am

				LEGEN	D	
	B. DEVELOPED CONDITIONS			AR AS	ITOMATIC DOOR OPENER PHALT RAMP	
	1. BASIN A-1 a. VOLUME			BOH BU BW BA	JILDING JILDING OVERHANG \RBED WIRE	
SF	$E_W = (E_A A_A + E_B A_B + E_C A_C + E_D A_D)/A_T$	· (0.26*0.99) + (0.68*1.97))/1.21	= 1.47 IN	US 2&C C CC	JRB & GUTTER DMMUNICATIONS LINE DMMUNICATIONS LINE	
AC	$V_{100} = (E_W/12)A_T = (1.47/12)1.21$		6,460 CF	CAM CA	MERA MCRETE BUILDING COLUMN	
	b. PEAK DISCHARGE $Q_P = Q_{PA}A_A + Q_{PB}A_B + Q_{PC}A_C + Q_{PD}A_D$			CC CC CDP CC	OMMUNICATIONS CONDUIT	
SF	$Q_{P} = Q_{100} = ((0.00^{*}1.29) + (0.27^{*}2.03) + (0$	(0.26*2.87) + (0.68*4.37)) =	4.3 CFS	CMH CC CHC CC	DNCRETE GUARD POST DMMUNICATIONS MANHOLE DNCRETE HEADER CURB	
	2. BASIN A-2			CLDD CE CLM CC	INTERLINE DOOR INTERLINE DOUBLE DOOR IMMUNICATIONS LINE MARKER	
/AC) %	a. VOLUME E _W = (E _A A _A +E _B A _B +E _C A _C +E _D A _D)/A _T			CL CC CMP CC	ONCRETE LANDING OMMUNICATIONS PANEL ONCRETE MASONRY UNIT WALL	
/ 0.23 9.8 / 0.16 6.9	$E_{W} = ((0.00^{*}0.44) + (0.00^{*}0.67) + V_{100} = (E_{W}/12)A_{T} = (1.95/12)0.49$	(0.01*0.99) + (0.48*1.97))/0.49 = 0.0796 AC-FT =	0 = 1.95 IN 3,470 CF	CO SA CONC CO	NITARY SEWER CLEANOUT DNCRETE DMMUNICATIONS PULLBOX	
<u>/ 1.94</u> <u>83.3</u> 100	b. PEAK DISCHARGE $Q_P = Q_{PA}A_A + Q_{PB}A_B + Q_{PC}A_C + Q_{PD}A_D$			CR CC CRD CC	DNCRETE RAMP DNCRETE RUNDOWN DNCRETE STEPS	
	$Q_P = Q_{100} = ((0.00*1.29) + (0.00*2.03) + (0.00*2.0$	(0.01*2.87) + (0.48*4.37)) =	2.1 CFS	CSP CC CSW CC	NCRETE SPLASH PAD NCRETE SIDEWALK	
SF	3. BASIN A-3			DBL DC DCO DC	DNCRETE TRASH CAN DUBLE DUBLE SANITARY SEWER	
	a. VOLUME E _W = (E _A A _A +E _B A _B +E _C A _C +E _D A _D)/A _T			E/PM EL EA ED	EANOUT ECTRIC LINE BY PAINT MARK DGE OF ASPHALT	
/AC) %	$E_{W} = ((0.00^{*}0.44) + (0.01^{*}0.67) + V_{100} = (E_{W}/12)A_{T} = (1.95/12)0.60$		0 = 1.95 IN 4,250 CF	EO EL	ECTRIC CONDUIT ECTRIC OUTLET ECTRIC PANEL	
/ 0.18 9.5 / 0.22 12.1 / 1.45 78.4	b. PEAK DISCHARGE $Q_P = Q_{PA}A_A + Q_{PB}A_B + Q_{PC}A_C + Q_{PD}A_D$			EPB EL FH FIF	ECTRIC PULLBOX RE HYDRANT OWLINE	
100	$Q_{\rm P} = Q_{100} = \frac{((0.00^{*}1.29) + (0.01^{*}2.03) $	(0.00*2.87) + (0.59*4.37)) =	2.6 CFS	FLC FIF FNT FC	RE LINE CONNECTION DUNTAIN AG POLE	
SF	4. BASIN B		\sim	FRD FR G/PM GA	NOM RECORD DRAWING NS LINE BY PAINT MARK	
	a. VOLUME $E_W = (E_A A_A + E_B A_B + E_C A_C + E_D A_D)/A_T$)	GRV GF GS GA	RAVEL AS SERVICE	
/AC) %	$E_{W} = \frac{((0.00^{*}0.44) + (0.07^{*}0.67) + (0.07^{*}0$	(0.13*0.99) + (1.23*1.97))/1.43 = 0.2169 AC-FT =	9,450 CF	ICB IRI INV IN	NDICAPPED PARKING SIGN RIGATION CONTROL BOX VERT	
/ 0.27 / 0.26 / 1.13 16.3 15.5 68.2	b. PEAK DISCHARGE			IVB IRI	VERT RIGATION VALVE BOX ETAL BUILDING COLUMN	-
100	$Q_{P} = Q_{PA}A_{A} + Q_{PB}A_{B} + Q_{PC}A_{C} + Q_{PD}A_{D}$ $Q_{P} = Q_{100} = \underline{((0.00^{*}1.29) + (0.07^{*}2.03) + (0.07$	(0.13*2.87) + (1.23*4.37)) =	5.9 CFS	MC/B ME MC/V ME	TER CAN WITH BIB VALVE TER CAN WITH VALVE ANHOLE	
SF	C. COMPARISON	~~~~~	\sim	MHR ME MR MF	ETAL HAND RAIL ETAL RAMP ETAL SIGN	
	1. BASIN A			OHC(1) OV	/ERHEAD COMMUNICATIONS OF LINES) /ERHEAD ELECTRIC	
/AC) %	a. VOLUME △V ₁₀₀ = 14,180 - 11,810 =	2,370 CF	(INCREASE)	PI PA	OF LINES) NINTED PARKING LOT ISLAND	
/ 0.01 / 0.48 2.7 97.3	b. PEAK DISCHARGE	2,370 01	(INCINE AGE)	🕀 14.00 PR	DST INDICATOR VALVE ROPOSED SPOT ELEVATION KISTING FLOWLINE	
100	$\Delta Q_{100} = 9.0 - 7.4 =$	1.6 CFS	(INCREASE)	PR	ROPOSED FLOWLINE ROPOSED CONTOUR	
SF	2. BAŠIN B a. VOLUME	•		RI	ROPOSED DIRECTION OF FLOW GHT OF WAY LINE	
	$\Delta V_{100} = 9,450 - 11,620 =$	-2,170 CF	(DECREASE)	A	JBLIC EASEMENT LINE GH POINT / DIVIDE	
/AC) %	b. PEAK DISCHARGE △Q ₁₀₀ = 5.9 - 7.3 =	-1.4 CFS	(DECREASE)	PF	ROPOSED STORM INLET ROPOSED STORM DRAIN	
/ 0.01 1.4 98.6	3. TOTAL SITE		{	та ря	NHOLE ROPOSED FIRE HYDRANT	ŀ
<u>/ 0.59 98.6</u> 100	a. VOLUME ∆V ₁₀₀ = 23,630 - 23430 =	200 CF	(INCREASE)	>	RE DEPARTMENT CONNECTION GH POINT/WATER BLOCK	
SF .	b. PEAK DISCHARGE					
	$\Delta Q_{100} = 14.9 - 14.7 =$	0.2 CFS	(INCREASE)			
/AC) %						
<u>/ 0.07</u> / 0.13 <u>4.9</u> 9.3						
$\frac{70.13}{1.23}$ $\frac{9.3}{85.9}$ 100						
7) + (0.16*0.99) + (1.49*1.97))/1.88 = 1.73 IN 88 = 0.271 AC-FT = 11,810 C						
A_D 3) + (0.16*2.87) + (1.49*4.37)) = 7.4 C	CFS					
~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~						
7) + (0.22*0.99) + (1.45*1.97))/1.85 = 1.73 IN 85= 0.2667 AC-FT = 11,620 C						
					INDEX OF	
$A_D$ 3) + (0.22*2.87) + (1.45*4.37)) = 7.3 C	CFS			SHEET	DESC	<u></u>
				CG-001 CG-101	DRAINAGE PLAN OVERALL DRAINA	
				CG-102	GRADING PLAN	
				CG-501 CG-502	STORM DRAIN MA	
				CU-101	WATER AND SAN	IT/
				CU-501	WATER AND SAN	





ath: P:\DATA\2014.006.1\ENG\R5\ Plot Date: 05-12-2016 lame:140061_CG-101-R5.DWG Plot Time: 09:19 am

	GENERAL NOTES:	OM 42
1", SET FLUSH IEAST QUADRANT ND GLENRIO ) 1988)	<ol> <li>ALL WORK DETAILED ON THESE PLANS TO BE PERFORMED UNDER CONTRACT SHALL, EXCEPT AS OTHERWISE STATED OR PROVIDED FOR HEREON, BE CONSTRUCTED IN ACCORDANCE WITH THE NEW MEXICO STANDARD SPECIFICATIONS FOR PUBLIC WORKS CONSTRUCTION – 1987, PUBLISHED BY THE NEW MEXICO CHAPTER AMERICAN PUBLIC WORKS ASSOCIATION. (REVISED 12/06)</li> </ol>	T E C T U R DWAY BLVD. S.E. GULE, NM 87102 fax 505.243.13 MANSTUDIO.CC
	<ol> <li>TWO (2) WORKING DAYS PRIOR TO ANY EXCAVATION, CONTRACTOR MUST CONTACT NEW MEXICO ONE CALL SYSTEM, 811, FOR DESIGNATION (LINE-SPOTTING) OF EXISTING PUBLIC UTILITIES AND EXISTING UTILITIES OWNED AND OPERATED BY ALBUQUERQUE PUBLIC SCHOOLS.</li> </ol>	R C H I 206 BROAL ALBUQUER 5.243.0101 WW.MAHL
OWN OF ATRISCO V MEXICO.	3. IF ANY UTILITY LINES, PIPELINES, OR UNDERGROUND UTILITY LINES ARE SHOWN ON THESE DRAWINGS, THEY ARE SHOWN IN AN APPROXIMATE MANNER ONLY, AND SUCH LINES MAY EXIST WHERE NONE ARE SHOWN. IF ANY SUCH EXISTING LINES ARE SHOWN, THE LOCATION IS BASED UPON INFORMATION PROVIDED BY THE OWNER OF SAID UTILITY, AND THE INFORMATION MAY BE INCOMPLETE, OR MAY BE OBSOLETE BY THE TIME CONSTRUCTION COMMENCES. THE ENGINEER HAS CONDUCTED ONLY PRELIMINARY INVESTIGATION OF THE	<ul> <li>4</li> <li>63 ≥</li> </ul>
D BY DOCUMENT	LOCATION, DEPTH, SIZE, OR TYPE OF EXISTING UTILITY LINES, PIPELINES, OR UNDERGROUND UTILITY LINES. THIS INVESTIGATION IS NOT CONCLUSIVE, AND MAY NOT BE COMPLETE, THEREFORE, MAKES NO REPRESENTATION PERTAINING THERETO, AND ASSUMES NO RESPONSIBILITY OR LIABILITY THEREFORE. THE	
) BY DOCUMENT	CONTRACTOR SHALL INFORM ITSELF OF THE LOCATION OF ANY UTILITY LINE, PIPELINE, OR UNDERGROUND UTILITY LINE IN OR NEAR THE AREA OF THE WORK IN ADVANCE OF AND DURING EXCAVATION WORK. THE CONTRACTOR IS FULLY RESPONSIBLE FOR ANY AND ALL DAMAGE CAUSED BY ITS FAILURE TO	
) BY DOCUMENT	LOCATE, IDENTIFY AND PRESERVE ANY AND ALL EXISTING UTILITIES, PIPELINES, AND UNDERGROUND UTILITY LINES. IN PLANNING AND CONDUCTING EXCAVATION, THE CONTRACTOR SHALL COMPLY WITH STATE STATUTES,	
FOR WATER 07-20-2010,	MUNICIPAL AND LOCAL ORDINANCES, RULES AND REGULATIONS, IF ANY, PERTAINING TO THE LOCATION OF THESE LINES AND FACILITIES. 4. SHOULD A CONFLICT EXIST BETWEEN THESE PLANS AND ACTUAL FIELD CONDITIONS, THE CONTRACTOR SHALL PROMPTLY NOTIFY THE ENGINEER IN	NO NER
D BY DOCUMENT	WRITING SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM AMOUNT OF DELAY FOR ALL PARTIES. 5. THE CONTRACTOR SHALL MAINTAIN ACCESS TO ADJACENT PROPERTIES DURING	2016 2015 2016
DTES:	CONSTRUCTION. 6. ALL WORK ON THIS PROJECT SHALL BE PERFORMED IN ACCORDANCE WITH APPLICABLE FEDERAL, STATE AND LOCAL LAWS, RULES AND REGULATIONS	S. S. M.
ATION, CONTRACTOR	CONCERNING SAFETY AND HEALTH. 7. THE CONTRACTOR SHALL ENSURE THAT NO SOIL ERODES FROM THE SITE INTO PUBLIC RIGHT-OF-WAY OR ONTO PRIVATE PROPERTY.	REG19111
M (NM 811) FOR ; UTILITIES AND EXISTING QUERQUE PUBLIC	<ul> <li>8. THE CONTRACTOR SHALL PROMPTLY CLEAN UP ANY MATERIAL EXCAVATED WITHIN THE PUBLIC RIGHT-OF-WAY SO THAT THE EXCAVATED MATERIAL IS NOT SUSCEPTIBLE TO BEING WASHED DOWN THE STREET.</li> <li>9. CONTRACTOR SHALL NOTICY THE ENCINEER NOT LESS THAN SEVEN (7) DAYS</li> </ul>	ARK BLVD. M MEXICO &
HALL EXCAVATE AND ON OF ALL POTENTIAL IE CONTRACTOR SHALL	9. CONTRACTOR SHALL NOTIFY THE ENGINEER NOT LESS THAN SEVEN (7) DAYS PRIOR TO STARTING WORK IN ORDER THAT THE ENGINEER MAY TAKE NECESSARY MEASURES TO ENSURE THE PRESERVATION OF SURVEY MONUMENTS. CONTRACTOR SHALL NOT DISTURB PERMANENT SURVEY MONUMENTS WITHOUT	
E CONFLICT CAN BE . THE CONTRACTOR IONS IT MAKES REQUIRED ABOVE.	THE CONSENT OF THE ENGINEER AND SHALL NOTIFY THE ENGINEER AND BEAR THE EXPENSE OF REPLACING ANY THAT MAY BE DISTURBED WITHOUT PERMISSION. REPLACEMENT SHALL BE DONE ONLY BY THE ENGINEER. WHEN A CHANGE IS MADE IN THE FINISHED ELEVATION OF THE PAVEMENT OF ANY	CONSULTANT Consultant 6010-B MIDW ALBUQUERQUE PHONE: 505.345.4
DRMED IN ACCORDANCE LAWS, RULES AND ETY AND HEALTH.	ROADWAY IN WHICH A PERMANENT SURVEY MONUMENT IS LOCATED, CONTRACTOR SHALL, AT HIS OWN EXPENSE, ADJUST THE MONUMENT COVER TO THE NEW GRADE UNLESS OTHERWISE SPECIFIED.	
WAY SHALL BE CITY OF	<ol> <li>ALL PAVEMENT MARKINGS AND TRAFFIC SIGNS SHALL COMPLY WITH THE MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES (MUTCD) PUBLISHED BY THE U.S. DEPARTMENT OF TRANSPORTATION FEDERAL HIGHWAY ADMINISTRATION, LATEST EDITION.</li> </ol>	
ROUND UTILITY LINES SHOWN IN AN MAY EXIST WHERE ES ARE SHOWN, THE	<ol> <li>IF THE REMOVAL OF EXISTING CURB AND GUTTER, SIDEWALK, AND/OR PAVING IS REQUIRED, THE CONTRACTOR SHALL SAWCUT AND/OR REMOVE TO THE NEAREST JOINT. WHEN ABUTTING NEW PAVEMENT TO EXISTING, THE</li> </ol>	
DED BY THE OWNER BE INCOMPLETE, OR N COMMENCES. THE	CONTRACTOR SHALL CUT BACK THE EXISTING PAVING TO A STRAIGHT LINE IN ORDER TO REMOVE ANY BROKEN OR CRACKED PAVEMENT. CURB AND GUTTER AND/OR PAVEMENT SHOWN AS EXISTING AND NOT TO BE REMOVED UNDER	NW NW 8712 SCI
INVESTIGATION OF STING UTILITY LINES, HIS INVESTIGATION IS , THEREFORE, MAKES	THIS CONTRACT AND WHICH IS DAMAGED OR DISPLACED BY THE CONTRACTOR SHALL BE REMOVED AND REPLACED BY THE CONTRACTOR AT THE CONTRACTOR'S EXPENSE.	HIGH ASSR ASSR ASSR ASSR ASSR ASSR ASSR ASS
D ASSUMES NO CONTRACTOR SHALL ITY LINE, PIPELINE, OR AREA OF THE WORK IN THE CONTRACTOR IS	12. A DISPOSAL SITE FOR ALL EXCESS EXCAVATION MATERIAL (CONTAMINATED OR OTHERWISE), ASPHALTIC PAVING, CONCRETE PAVING, ETC. SHALL BE OBTAINED BY THE CONTRACTOR IN COMPLIANCE WITH APPLICABLE REGULATIONS. ALL COSTS INCURRED IN OBTAINING A DISPOSAL SITE AND IN HAUL THERETO SHALL BE CONSIDERED INCIDENTAL TO CONSTRUCTION, THEREFORE, NO SEPARATE PAYMENT SHALL BE MADE.	CLA CLA SUILI BUILI AUERO ERQUE
E CAUSED BY ITS ANY AND ALL EXISTING TY LINES. IN PLANNING OR SHALL COMPLY ORDINANCES, RULES E LOCATION OF THESE	<ol> <li>A BORROW SITE FOR IMPORT MATERIAL SHALL BE OBTAINED BY THE CONTRACTOR IN COMPLIANCE WITH APPLICABLE REGULATIONS. ALL COSTS INCURRED IN OBTAINING A BORROW SITE AND IN HAUL THERETO SHALL BE CONSIDERED INCIDENTAL TO CONSTRUCTION, THEREFORE, NO SEPARATE PAYMENT SHALL BE MADE.</li> </ol>	
AREAS IS NOT PART OF REAS ADJACENT TO	14. THE CONTRACTOR SHALL BE RESPONSIBLE FOR SAFELY OBTAINING THE REQUIRED COMPACTION. THE CONTRACTOR SHALL SELECT AND USE METHODS WHICH SHALL NOT BE INJURIOUS OR DAMAGING TO THE EXISTING FACILITIES	
DSITIVE DRAINAGE TO TURE. FOR NG PLAN. EMOLITION PLANS.	AND STRUCTURES WHICH SURROUND THE WORK AREAS. 15. THE CONTRACTOR SHALL CONFINE HIS WORK WITHIN THE CONSTRUCTION LIMITS IN ORDER TO PRESERVE THE EXISTING IMPROVEMENTS AND SO AS NOT TO INTERFERE WITH THE OPERATIONS OF THE EXISTING FACILITIES.	
E PLAN AND LAYOUT.	16. THE CONTRACTOR SHALL BE RESPONSIBLE FOR SELECTING APPROPRIATE MEANS AND METHODS TO EXCAVATE AND TRENCH AND/OR INSTALL PIPE SO AS TO NOT EXCEED RIGHT-OF-WAY OR EASEMENT LIMITS, AND SO AS NOT TO INTERFERE WITH OTHER UTILITIES OR IMPROVEMENTS. THIS SHALL BE	
_	CONSIDERED INCIDENTAL TO CONSTRUCTION, THEREFORE, NO SEPARATE PAYMENT WILL BE MADE. 17. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROTECTING, SUPPORTING AND REPLACING, IF DAMAGED, ALL UTILITIES ENCOUNTERED DURING CONSTRUCTION.	DN DN
IL ERODES FROM THE RIVATE PROPERTY.	THIS SHALL BE CONSIDERED INCIDENTAL TO CONSTRUCTION, THEREFORE, NO SEPARATE PAYMENT WILL BE MADE. 18. ALL DIMENSIONS AND RADII OF CURB, CURB RETURNS, AND WALLS ARE SHOWN	24ND CHAM
ANY MATERIAL SO THAT THE BEING WASHED DOWN	TO THE FACE OF CURB AND/OR WALL. 19. THE CONTRACTOR SHALL NOTIFY THE OWNER 48 HOURS PRIOR TO STRIPING SO THAT LAYOUT CAN BE VERIFIED.	
YES AND THE OWNER, I THE CITY AND FILE A DR TO BEGINNING	20. PRIOR TO CONSTRUCTION, THE CONTRACTOR SHALL EXCAVATE AND VERIFY THE HORIZONTAL AND VERTICAL LOCATION OF ALL POTENTIAL OBSTRUCTIONS. SHOULD A CONFLICT EXIST, THE CONTRACTOR SHALL NOTIFY THE ENGINEER IN	04/16 N/A MM/DD/YY
	WRITING SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM AMOUNT OF DELAY. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL INTERPRETATIONS IT MAKES WITHOUT FIRST CONTACTING THE ENGINEER AS REQUIRED ABOVE.	MARK
	21. CONTRACTOR SHALL SECURE, ON BEHALF OF THE OWNER AND OPERATORS, "TOPSOIL DISTURBANCE PERMIT" FROM THE CITY AND FILE A NOTICE OF INTENT (N.O.I.) WITH THE EPA PRIOR TO BEGINNING CONSTRUCTION.	PROJECT NO: WMHS
	22. ALL FILL SHALL BE CLEAN, FREE FROM VEGETATION, DEBRIS, AND OTHER DELETERIOUS MATERIALS, AND SHALL NOT BE CONTAMINATED WITH HYDROCARBONS OR OTHER CHEMICAL CONTAMINANTS.	DRAWN BY: J.Y.R./S.C.C. CHECKED BY: G.M. COPYRIGHT:
	<ul> <li>23. ALL FILL SHALL BE COMPACTED TO A MINIMUM OF 95% ASTM D-1557 UNLESS A GREATER COMPACTION REQUIREMENT IS OTHERWISE SPECIFIED.</li> <li>24. CAUTION: THESE DRAWINGS DO NOT INCLUDE NECESSARY COMPONENTS FOR CONSTRUCTION SAFETY WHICH SHALL REMAIN THE RESPONSIBILITY OF THE</li> </ul>	DATE: 03-2015
	CONTRACTOR. ALL EXCAVATION, TRENCHING AND SHORING ACTIVITIES MUST BE CARRIED—OUT IN ACCORDANCE WITH OSHA 29 CFR 1926, SUBPART P—EXCAVATIONS.	SHEET TILE:
		OVERALL DRAINAGE
		SITE PLAN
	0044 000 4	CG-101
	2014.006.1	

