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RECORD DRAWING

DRAINAGE PLAN

I. INTRODUCTION AND EXECUTIVE SUMMARY

THIS PROJECT, LOCATED WITHIN THE NORTHWEST MESA OF THE ALBUQUERQUE METROPOLITAN AREA, REPRESENTS A MODIFICATION TO AN EXISTING APS SCHOOL SITE WITHIN AN INFILL AREA. THE PROPOSED DEVELOPMENT IS COMPRISED OF TWO PHASES. THE FIRST PHASE IS THE CONSTRUCTION OF A NEW CLASSROOM BUILDING. THE SECOND PHASE IS DEMOLITION OF AN EXISTING CLASSROOM WING FOLLOWED BY CONSTRUCTION OF A SECOND BUILDING AND RECONSTRUCTION OF AN EXISTING COURTYARD. THE CITY HYDROLOGY FILE NO. IS 110-0005. THE DRAINAGE CONCEPT FOR THE PROJECT WILL BE THE CONTINUED FREE DISCHARGE OF DEVELOPED RUNOFF TO EXISTING PUBLIC AND PRIVATE STORM DRAINS THAT OUTFALL TO THE EXISTING WEST MESA DETENTION BASIN AND TO THE WEST MESA DIVERSION STORM DRAIN (CPN 538103).

THIS SUBMITTAL IS BEING MADE FOR BUILDING PERMIT APPROVAL.

II. PROJECT DESCRIPTION

THE SCHOOL SITE IS LOCATED AT THE NORTHWEST CORNER OF THE INTERSECTION OF FORTUNA ROAD NW AND 64TH STREET NW, BOTH FULLY DEVELOPED CITY STREETS. THE SITE IS DEVELOPED AS AN ALBUQUERQUE PUBLIC SCHOOLS HIGH SCHOOL. GLENRIO ROAD NW, A PARTIALLY DEVELOPED CITY STREET LIES TO THE NORTH. IT LACKS CURB AND GUTTER ALONG THE SCHOOL FRONTAGE. THE CITY OF ALBUQUERQUE WEST MESA AQUATIC CENTER LIES TO THE WEST OF THE SCHOOL SITE. THE SURROUNDING AREA IS DEVELOPED MAINLY SINGLE FAMILY RESIDENTIAL, MAKING THIS A MODIFICATION TO AN EXISTING SITE WITHIN AN INFILL AREA. THE SITE CONSISTS OF PREVIOUSLY PLATTED LOTS AND FORMER CITY STREETS THAT MAY HAVE BEEN VACATED. AS SHOWN BY PANEL 35001C0327H OF THE NATIONAL FLOOD INSURANCE PROGRAM FLOOD INSURANCE RATE MAPS PUBLISHED BY FEMA FOR BERNILLO COUNTY, NEW MEXICO, REVISED AUGUST 16, 2012, THIS SITE DOES NOT LIE WITHIN A DESIGNATED FLOOD HAZARD ZONE.

III. BACKGROUND DOCUMENTS

THE PREPARATION OF THIS PLAN RELIED UPON THE FOLLOWING DOCUMENTS AND ACTIVITIES:

- BOUNDARY, TOPOGRAPHIC, AND UTILITY SURVEY PREPARED BY HIGH MESA CONSULTING GROUP DATED 3/28/2014 (NMPS 11184). THIS SURVEY PROVIDES THE BASIS FOR THE EXISTING CONDITIONS OF THE PROJECT SITE.
- PREDICTION CONFERENCE RECAPS DATED 02/13/2014 AND 5/10/2014 CONDUCTED WITH HIGH MESA CONSULTING GROUP. THE RECAPS CONFIRMED THAT THE CONTINUED FREE DISCHARGE OF DEVELOPED RUNOFF TO THE ADJACENT PUBLIC STORM DRAIN SYSTEMS VIA PRIVATE STORM DRAIN CONNECTIONS IS APPROPRIATE, AND THAT IT IS PERMISSIBLE TO DIVERT RUNOFF FROM EXISTING BASINS 104 AND 105 TO THE FORTUNA STORM DRAIN.
- DRAINAGE PLAN FOR WEST MESA HIGH SCHOOL CLASSROOM WING "M" REPLACEMENT & COURTYARD IMPROVEMENTS PREPARED BY HIGH MESA CONSULTING GROUP DATED 12/21/2014 (NMPE 13676). THIS SUBMITTAL SUPPORTED WORK ORDER APPROVAL FOR A NEW STORM DRAIN CONNECTION TO FORTUNA THAT WILL SERVE THIS NEW BUILDING. THIS SUBMITTAL INCLUDED A COMPREHENSIVE ANALYSIS OF EXISTING CONDITIONS AND SUPPORTED THE DIVERSION OF APPROXIMATELY 2.2 ACRES OF THE HIGH SCHOOL SITE TO THE EXISTING 72 INCH PUBLIC STORM DRAIN IN FORTUNA RD VIA THE NEW STORM DRAIN CONNECTION. AS IDENTIFIED IN THIS REFERENCE DOCUMENT, HISTORIC BUILDOUT IN THIS WATERSHED HAS RESULTED IN A NET REDUCTION OF 4.7 ACRES OF DEVELOPED PROPERTY DRAINING TO FORTUNA AS COMPARED TO THE BASINS ESTABLISHED FOR THE PUBLIC STORM DRAINS, 4.0 ACRES FROM THE WEST MESA AQUATIC CENTER, AND A NET OF 0.7 ACRES WITHIN THE HIGH SCHOOL, DUE TO ON-SITE PRIVATE STORM DRAIN CONSTRUCTION UNDER REFERENCE 4. THIS DECREASE IN AREA DRAINING TO FORTUNA MEANS THERE IS CORRESPONDING EXCESS CAPACITY. SWITCHING 2.2 ACRES AS PROPOSED WILL STILL LEAVE A NET DECREASE OF 2.5 ACRES DRAINING TO FORTUNA AFTER IMPLEMENTATION OF THIS NEW PROPOSED CONNECTION AND DIVERSION.
- WORK ORDER CONSTRUCTION PLANS FOR WEST MESA HIGH SCHOOL PUBLIC AND PRIVATE WATER, SANITARY SEWER, AND STORM DRAIN LINE EXTENSIONS AND RELOCATIONS PREPARED BY HIGH MESA CONSULTING GROUP, CPN 749982, DATED 01/14/2015 (NMPE 13676). THIS WORK ORDER PLAN SET INCLUDES CONSTRUCTION OF A NEW 24 INCH CONNECTION TO THE EXISTING 72 INCH PUBLIC STORM DRAIN IN FORTUNA RD NW. THIS CONNECTION WILL SERVE THE PROPOSED BUILDING AND COURTYARD IMPROVEMENTS.

- WORK ORDER PLAN SET INCLUDES CONSTRUCTION OF A NEW 24 INCH CONNECTION TO THE EXISTING 72 INCH PUBLIC STORM DRAIN IN FORTUNA RD NW. THIS CONNECTION WILL SERVE THE PROPOSED BUILDING AND COURTYARD IMPROVEMENTS.

THIS SITE IS DEVELOPED AS A HIGH SCHOOL OPERATED AND MAINTAINED BY THE ALBUQUERQUE PUBLIC SCHOOLS (APS). THE SCHOOL SITE CONSISTS OF PERMANENT AND PORTABLE CLASSROOM BUILDINGS, PAVED PARKING AREAS AND WALKWAYS, LANDSCAPED ATHLETIC FIELDS, AND OTHER SITE IMPROVEMENTS APPLICABLE TO A HIGH SCHOOL SITE. AS EXPLAINED AND DEMONSTRATED BY THE AFOREMENTIONED DRAINAGE PLAN (REF. 4), THE OVERALL SITE IS CHARACTERIZED BY THIRTEEN (13) DRAINAGE BASINS, OF WHICH FOUR WILL BE AFFECTED BY THIS PROJECT.

FOR THE PURPOSES OF THIS SPECIFIC PROJECT, THE IMPACTED PORTION OF THE SITE HAS BEEN DIVIDED INTO TWO DRAINAGE BASINS WITH BASIN A BEING THE PORTION OF THE SITE THAT DISCHARGES DIRECTLY TO AN EXISTING PRIVATE STORM DRAIN SYSTEM THAT CURRENTLY DRAINS TO THE EAST TO THE EXISTING PUBLIC WEST MESA DIVERSION STORM DRAIN, AND BASIN B BEING THE PORTION THAT DRAINS OVERLAND TO THE EXISTING INTERNAL PRIVATE STORM DRAIN SYSTEM. THESE BASINS ARE SHOWN ON SHEET CG-101. THERE ARE NO OFFSITE FLOWS DISCHARGING ONTO THE PROJECT SITE AS THE PROJECT LIMITS LIE WELL WITHIN THE OVERALL SCHOOL SITE AND NOT ADJACENT TO NEIGHBORING PROPERTIES.

THE OVERALL PROJECT CONSISTS OF CONSTRUCTING TWO NEW BUILDINGS TO REPLACE THE EXISTING "M" HALL AND RE-CONSTRUCTING THE EXISTING COURTYARD. THE PROJECT WILL BE CONSTRUCTED IN PHASES, WITH THE FIRST PHASE BEING CONSTRUCTION OF BUILDING "A" WITHIN AN EXISTING PARKING LOT. AS SHOWN BY THE GRADING PLAN ON SHEET CG-102, THE ROOF DRAINS AND A PORTION OF SITE WORK WILL DRAIN DIRECTLY TO THE NEW STORM DRAIN CONNECTION TO THE EXISTING 72 INCH PUBLIC STORM DRAIN IN FORTUNA TO BE CONSTRUCTED UNDER CITY WORK ORDER IN ACCORDANCE WITH THE CONCEPT ESTABLISHED BY THE AFOREMENTIONED PLAN (REF. 3) AND THE PREDICTION RECAPS (REF. 2). THIS FIRST PHASE IS INTENDED TO STAND ALONE WITH THE INTENT OF OBTAINING A CERTIFICATE OF OCCUPANCY FOR BUILDING "A" IN ADVANCE OF PHASE 2 CONSTRUCTION. THIS IS ACCEPTABLE BECAUSE PHASE 1 CONSTRUCTION DOES NOT RELY UPON PHASE 2 IMPROVEMENTS. PHASE 1 CONSTRUCTION LIES WITHIN AN EXISTING PAVED PARKING LOT, HENCE THE NEW CONSTRUCTION WILL NOT RESULT IN AN INCREASE IN RUNOFF DURING THE INTERIM CONDITION BETWEEN PHASE 1 AND PHASE 2.

PHASE 2 CONSTRUCTION WILL INCLUDE THE DEMOLITION OF THE EXISTING "M" HALL, CONSTRUCTION OF BUILDING "B", AND RECONSTRUCTION OF THE EXISTING COURTYARD. THE PRIVATE STORM DRAIN THAT WILL DISCHARGE TO THE NEW FORTUNA CONNECTION WILL BE EXTENDED WITH THIS PHASE TO SERVE BUILDING "B" AND THE COURTYARD. AN EXISTING STORM DRAIN CURRENTLY SERVING THE COURTYARD WILL BE REPLACED WITH A NEW STORM DRAIN.

AS SHOWN BY THE PLANS, SITE ROOF DRAINAGE WILL BE DIRECTLY PIPED TO PROPOSED PRIVATE STORM DRAINS. ALL SITE SURFACE RUNOFF WILL BE DIRECTED TO NEW STORM DRAIN INLETS THAT ARE MOSTLY LOCATED IN LANDSCAPED AREAS THAT WILL HAVE A CRUSHER FINES SURFACING. IN MOST CASES, THE CONCEPT OF "DISCONNECTED IMPERVIOUSNESS" IS EMPLOYED WHEREBY RUNOFF FROM HARDCAPED AREAS FLOW ACROSS THE CRUSHER FINES BEFORE REACHING THE INLETS. BECAUSE THIS IS AN INFILL PROJECT AT AN EXISTING SCHOOL CAMPUS, THERE IS LIMITED ABILITY TO INTRODUCE AREAS OF RETENTION IN THE PERVIOUS AREAS BECAUSE THEY ARE INTENDED FOR STUDENTS TO GATHER DURING LUNCH BREAK, AND RETENTION WOULD RESULT IN STANDING WATER IN PEDESTRIAN AREAS. AS SUCH, THE INTENT OF THE "FIRST FLUSH" REQUIREMENTS WILL NOT BE MET BY A SPECIFIC RETENTION OF VOLUME, BUT RATHER THROUGH THE USE OF DISCONNECTED IMPERVIOUSNESS COMBINED WITH THE AFOREMENTIONED TRANSFER OF 2.2 ACRES OF AREA FROM THE CURRENT FREE DISCHARGE CONDITION TO THE WEST MESA DIVERSION STORM DRAIN, TO THE WEST MESA DETENTION POND THAT IS IDENTIFIED AS A WATER QUALITY FEATURE ON THE AMFAC MANIPULATION MAPS.

CALCULATIONS SHOWN HEREON ANALYZE THE EXISTING AND DEVELOPED CONDITIONS FOR THE 100-YEAR, 6-HOUR RAINFALL EVENT. THE PROCEDURE FOR 40 ACRES AND SMALLER BASINS, AS SET FORTH IN THE REVISION OF SECTION 22.2, HYDROLOGY OF THE DEVELOPMENT PROCESS MANUAL, VOLUME 2, DESIGN CRITERIA, DATED JANUARY 1993, WILL BE USED TO QUANTIFY THE PEAK RATE OF DISCHARGE AND VOLUME OF RUNOFF GENERATED. AS SHOWN BY THE CALCULATIONS, THERE WILL BE A MINOR DECREASE IN 100-YEAR VOLUME AND NO CHANGE IN 100-YEAR PEAK DISCHARGE ATTRIBUTABLE TO THIS PROJECT.

THE FOLLOWING CONCLUSIONS HAVE BEEN ESTABLISHED AS A RESULT OF THE ANALYSIS AND EVALUATIONS CONTAINED HEREIN:

- THE PROPOSED IMPROVEMENTS REPRESENT MODIFICATIONS TO AN EXISTING SITE WITHIN AN INFILL AREA.
- THE FREE DISCHARGE OF DEVELOPED RUNOFF TO THE ADJACENT PUBLIC STORM DRAIN IS CONSISTENT WITH THE PREVIOUSLY APPROVED PLANS FOR THE SCHOOL SITE AND WITH MASTER DRAINAGE PLANS FOR THE WATERSHED.
- THE PROPOSED IMPROVEMENTS WILL NOT ADVERSELY IMPACT DOWNSTREAM PROPERTIES OR DOWNSTREAM DRAINAGE CONDITIONS.
- THE PROPOSED DIVERSION OF RUNOFF OF 2.2 ACRES OF THE HIGH SCHOOL WAS ESTABLISHED BY PREVIOUS SUBMITTAL, AND WILL RESULT IN THIS AREA BEING ROUTED THROUGH A PUBLIC DETENTION FACILITY WITH WATER QUALITY BENEFITS.
- THE PROPOSED IMPROVEMENTS WILL NOT AFFECT NOR BLOCK OFFSITE FLOWS.

CALCULATIONS

I. SITE CHARACTERISTICS

- A. PRECIPITATION ZONE = 1
- B. $P_{8,100} = P_{300} = 2.20$
- C. TOTAL PROJECT AREA (A_T) = 142,880 SF
3.28 AC

D. LAND TREATMENTS

1. BASIN A
- TOTAL BASIN AREA (A_T) = 101,310 SF

EXISTING LAND TREATMENT

| TREATMENT | AREA (SF/AC) | % |
|-----------|---------------|------|
| A | 9,900 / 0.23 | 9.8 |
| B | 7,000 / 0.16 | 6.9 |
| C | 84,410 / 1.94 | 83.3 |
| D | | 100 |

2. BASIN B
- TOTAL BASIN AREA (A_T) = 41,570 SF

EXISTING LAND TREATMENT

| TREATMENT | AREA (SF/AC) | % |
|-----------|---------------|------|
| A | 5,120 / 0.12 | 12.3 |
| B | 5,080 / 0.12 | 12.2 |
| C | 31,370 / 0.72 | 75.5 |
| D | | 100 |

3. BASIN A-1
- TOTAL BASIN AREA (A_T) = 72,030 SF

DEVELOPED LAND TREATMENT

| TREATMENT | AREA (SF/AC) | % |
|-----------|---------------|------|
| A | 11,770 / 0.27 | 16.3 |
| B | 11,140 / 0.26 | 15.5 |
| C | 49,120 / 1.13 | 68.2 |
| D | | 100 |

4. BASIN A-2
- TOTAL BASIN AREA (A_T) = 21,680 SF

DEVELOPED LAND TREATMENT

| TREATMENT | AREA (SF/AC) | % |
|-----------|---------------|------|
| A | | |
| B | | |
| C | 580 / 0.01 | 2.7 |
| D | 21,100 / 0.48 | 97.3 |
| | | 100 |

5. BASIN A-3
- TOTAL BASIN AREA (A_T) = 25,950 SF

DEVELOPED LAND TREATMENT

| TREATMENT | AREA (SF/AC) | % |
|-----------|---------------|------|
| A | | |
| B | 370 / 0.01 | 1.4 |
| C | | |
| D | 25,580 / 0.59 | 98.6 |
| | | 100 |

6. BASIN B
- TOTAL BASIN AREA (A_T) = 23,220 SF

DEVELOPED LAND TREATMENT

| TREATMENT | AREA (SF/AC) | % |
|-----------|---------------|------|
| A | 840 / 0.02 | 3.6 |
| B | 3,560 / 0.08 | 15.3 |
| C | 18,820 / 0.43 | 81.1 |
| D | | 100 |

II. HYDROLOGY

A. EXISTING CONDITIONS

1. BASIN A
- a. VOLUME
- $$E_W = (E_A A_A + E_B A_B + E_C A_C + E_D A_D) / A_T$$
- $$E_W = ((0.00^*0.44) + (0.23^*0.67) + (0.16^*0.99) + (1.49^*1.97)) / 1.88 = 1.73 \text{ IN}$$
- $$V_{100} = (E_W / 12) A_T = (1.73 / 12) 1.88 = 0.271 \text{ AC-FT} = 11,810 \text{ CF}$$

- b. PEAK DISCHARGE
- $$Q_P = Q_{PA} A_A + Q_{PB} A_B + Q_{PC} A_C + Q_{PD} A_D$$
- $$Q_P = Q_{100} = ((0.00^*1.29) + (0.23^*2.03) + (0.16^*2.87) + (1.49^*4.37)) = 7.4 \text{ CFS}$$

2. BASIN B
- a. VOLUME
- $$E_W = (E_A A_A + E_B A_B + E_C A_C + E_D A_D) / A_T$$
- $$E_W = ((0.00^*0.44) + (0.12^*0.67) + (0.12^*0.99) + (0.72^*1.97)) / 0.96 = 1.69 \text{ IN}$$
- $$V_{100} = (E_W / 12) A_T = (1.69 / 12) 0.96 = 0.1352 \text{ AC-FT} = 5,890 \text{ CF}$$

- b. PEAK DISCHARGE
- $$Q_P = Q_{PA} A_A + Q_{PB} A_B + Q_{PC} A_C + Q_{PD} A_D$$
- $$Q_P = Q_{100} = ((0.00^*1.29) + (0.12^*2.03) + (0.12^*2.87) + (0.72^*4.37)) = 3.7 \text{ CFS}$$

B. DEVELOPED CONDITIONS

1. BASIN A-1
- a. VOLUME
- $$E_W = (E_A A_A + E_B A_B + E_C A_C + E_D A_D) / A_T$$
- $$E_W = ((0.00^*0.44) + (0.27^*0.67) + (0.26^*0.99) + (0.68^*1.97)) / 1.21 = 1.47 \text{ IN}$$
- $$V_{100} = (E_W / 12) A_T = (1.47 / 12) 1.21 = 0.1482 \text{ AC-FT} = 6,460 \text{ CF}$$

- b. PEAK DISCHARGE
- $$Q_P = Q_{PA} A_A + Q_{PB} A_B + Q_{PC} A_C + Q_{PD} A_D$$
- $$Q_P = Q_{100} = ((0.00^*1.29) + (0.27^*2.03) + (0.26^*2.87) + (0.68^*4.37)) = 4.3 \text{ CFS}$$

2. BASIN A-2
- a. VOLUME
- $$E_W = (E_A A_A + E_B A_B + E_C A_C + E_D A_D) / A_T$$
- $$E_W = ((0.00^*0.44) + (0.00^*0.67) + (0.00^*0.99) + (0.48^*1.97)) / 0.49 = 1.95 \text{ IN}$$
- $$V_{100} = (E_W / 12) A_T = (1.95 / 12) 0.49 = 0.0796 \text{ AC-FT} = 3,470 \text{ CF}$$

- b. PEAK DISCHARGE
- $$Q_P = Q_{PA} A_A + Q_{PB} A_B + Q_{PC} A_C + Q_{PD} A_D$$
- $$Q_P = Q_{100} = ((0.00^*1.29) + (0.00^*2.03) + (0.01^*2.87) + (0.48^*4.37)) = 2.1 \text{ CFS}$$

3. BASIN A-3
- a. VOLUME
- $$E_W = (E_A A_A + E_B A_B + E_C A_C + E_D A_D) / A_T$$
- $$E_W = ((0.00^*0.44) + (0.01^*0.67) + (0.00^*0.99) + (0.58^*1.97)) / 0.60 = 1.95 \text{ IN}$$
- $$V_{100} = (E_W / 12) A_T = (1.95 / 12) 0.60 = 0.0975 \text{ AC-FT} = 4,250 \text{ CF}$$

- b. PEAK DISCHARGE
- $$Q_P = Q_{PA} A_A + Q_{PB} A_B + Q_{PC} A_C + Q_{PD} A_D$$
- $$Q_P = Q_{100} = ((0.00^*1.29) + (0.01^*2.03) + (0.00^*2.87) + (0.58^*4.37)) = 2.6 \text{ CFS}$$

4. BASIN B
- a. VOLUME
- $$E_W = (E_A A_A + E_B A_B + E_C A_C + E_D A_D) / A_T$$
- $$E_W = ((0.00^*0.44) + (0.02^*0.67) + (0.08^*0.99) + (0.43^*1.97)) / 0.53 = 1.77 \text{ IN}$$
- $$V_{100} = (E_W / 12) A_T = (1.77 / 12) 0.53 = 0.0782 \text{ AC-FT} = 3,410 \text{ CF}$$

- b. PEAK DISCHARGE
- $$Q_P = Q_{PA} A_A + Q_{PB} A_B + Q_{PC} A_C + Q_{PD} A_D$$
- $$Q_P = Q_{100} = ((0.00^*1.29) + (0.02^*2.03) + (0.08^*2.87) + (0.43^*4.37)) = 2.1 \text{ CFS}$$

5. BASIN C
- a. VOLUME
- $$E_W = (E_A A_A + E_B A_B + E_C A_C + E_D A_D) / A_T$$
- $$E_W = ((0.00^*0.44) + (0.02^*0.67) + (0.08^*0.99) + (0.43^*1.97)) / 0.53 = 1.77 \text{ IN}$$
- $$V_{100} = (E_W / 12) A_T = (1.77 / 12) 0.53 = 0.0782 \text{ AC-FT} = 3,410 \text{ CF}$$

- b. PEAK DISCHARGE
- $$Q_P = Q_{PA} A_A + Q_{PB} A_B + Q_{PC} A_C + Q_{PD} A_D$$
- $$Q_P = Q_{100} = ((0.00^*1.29) + (0.02^*2.03) + (0.08^*2.87) + (0.43^*4.37)) = 2.1 \text{ CFS}$$

6. BASIN D
- a. VOLUME
- $$E_W = (E_A A_A + E_B A_B + E_C A_C + E_D A_D) / A_T$$
- $$E_W = ((0.00^*0.44) + (0.02^*0.67) + (0.08^*0.99) + (0.43^*1.97)) / 0.53 = 1.77 \text{ IN}$$
- $$V_{100} = (E_W / 12) A_T = (1.77 / 12) 0.53 = 0.0782 \text{ AC-FT} = 3,410 \text{ CF}$$

- b. PEAK DISCHARGE
- $$Q_P = Q_{PA} A_A + Q_{PB} A_B + Q_{PC} A_C + Q_{PD} A_D$$
- $$Q_P = Q_{100} = ((0.00^*1.29) + (0.02^*2.03) + (0.08^*2.87) + (0.43^*4.37)) = 2.1 \text{ CFS}$$

7. BASIN E
- a. VOLUME
- $$E_W = (E_A A_A + E_B A_B + E_C A_C + E_D A_D) / A_T$$
- $$E_W = ((0.00^*0.44) + (0.02^*0.67) + (0.08^*0.99) + (0.43^*1.97)) / 0.53 = 1.77 \text{ IN}$$
- $$V_{100} = (E_W / 12) A_T = (1.77 / 12) 0.53 = 0.0782 \text{ AC-FT} = 3,410 \text{ CF}$$

- b. PEAK DISCHARGE
- $$Q_P = Q_{PA} A_A + Q_{PB} A_B + Q_{PC} A_C + Q_{PD} A_D$$
- $$Q_P = Q_{100} = ((0.00^*1.29) + (0.02^*2.03) + (0.08^*2.87) + (0.43^*4.37)) = 2.1 \text{ CFS}$$

8. BASIN F
- a. VOLUME
- $$E_W = (E_A A_A + E_B A_B + E_C A_C + E_D A_D) / A_T$$
- $$E_W = ((0.00^*0.44) + (0.02^*0.67) + (0.08^*0.99) + (0.43^*1.97)) / 0.53 = 1.77 \text{ IN}$$
- $$V_{100} = (E_W / 12) A_T = (1.77 / 12) 0.53 = 0.0782 \text{ AC-FT} = 3,410 \text{ CF}$$

- b. PEAK DISCHARGE
- $$Q_P = Q_{PA} A_A + Q_{PB} A_B + Q_{PC} A_C + Q_{PD} A_D$$
- $$Q_P = Q_{100} = ((0.00^*1.29) + (0.02^*2.03) + (0.08^*2.87) + (0.43^*4.37)) = 2.1 \text{ CFS}$$

9. BASIN G
- a. VOLUME
- $$E_W = (E_A A_A + E_B A_B + E_C A_C + E_D A_D) / A_T$$
- $$E_W = ((0.00^*0.44) + (0.02^*0.67) + (0.08^*0.99) + (0.43^*1.97)) / 0.53 = 1.77 \text{ IN}$$
- $$V_{100} = (E_W / 12) A_T = (1.77 / 12) 0.53 = 0.0782 \text{ AC-FT} = 3,410 \text{ CF}$$

- b. PEAK DISCHARGE
- $$Q_P = Q_{PA} A_A + Q_{PB} A_B + Q_{PC} A_C + Q_{PD} A_D$$
- $$Q_P = Q_{100} = ((0.00^*1.29) + (0.02^*2.03) + (0.08^*2.87) + (0.43^*4.37)) = 2.1 \text{ CFS}$$

10. BASIN H
- a. VOLUME
- $$E_W = (E_A A_A + E_B A_B + E_C A_C + E_D A_D) / A_T$$
- $$E_W = ((0.00^*0.44) + (0.02^*0.67) + (0.08^*0.99) + (0.43^*1.97)) / 0.53 = 1.77 \text{ IN}$$
- $$V_{100} = (E_W / 12) A_T = (1.77 / 12) 0.53 = 0.0782 \text{ AC-FT} = 3,410 \text{ CF}$$

- b. PEAK DISCHARGE
- $$Q_P = Q_{PA} A_A + Q_{PB} A_B + Q_{PC} A_C + Q_{PD} A_D$$
- $$Q_P = Q_{100} = ((0.00^*1.29) + (0.02^*2.03) + (0.08^*2.87) + (0.43^*4.37)) = 2.1 \text{ CFS}$$

11. BASIN I
- a. VOLUME
- $$E_W = (E_A A_A + E_B A_B + E_C A_C + E_D A_D) / A_T$$
- $$E_W = ((0.00^*0.44) + (0.02^*0.67) + (0.08^*0.99) + (0.43^*1.97)) / 0.53 = 1.77 \text{ IN}$$
- $$V_{100} = (E_W / 12) A_T = (1.77 / 12) 0.53 = 0.0782 \text{ AC-FT} = 3,410 \text{ CF}$$

- b. PEAK DISCHARGE
- $$Q_P = Q_{PA} A_A + Q_{PB} A_B + Q_{PC} A_C + Q_{PD} A_D$$
- $$Q_P = Q_{100} = ((0.00^*1.29) + (0.02^*2.03) + (0.08^*2.87) + (0.43^*4.37)) = 2.1 \text{ CFS}$$

12. BASIN J
- a. VOLUME
- $$E_W = (E_A A_A + E_B A_B + E_C A_C + E_D A_D) / A_T$$
- $$E_W = ((0.00^*0.44) + (0.02^*0.67) + (0.08^*0.99) + (0.43^*1.97)) / 0.53 = 1.77 \text{ IN}$$
- $$V_{100} = (E_W / 12) A_T = (1.77 / 12) 0.53 = 0.0782 \text{ AC-FT} = 3,410 \text{ CF}$$

- b. PEAK DISCHARGE
- $$Q_P = Q_{PA} A_A + Q_{PB} A_B + Q_{PC} A_C + Q_{PD} A_D$$
- $$Q_P = Q_{100} = ((0.00^*1.29) + (0.02^*2.03) + (0.08^*2.87) + (0.43^*4.37)) = 2.1 \text{ CFS}$$

13. BASIN K
- a. VOLUME
- $$E_W = (E_A A_A + E_B A_B + E_C A_C + E_D A_D) / A_T$$
- $$E_W = ((0.00^*0.44) + (0.02^*0.67) + (0.08^*0.99) + (0.43^*1.97)) / 0.53 = 1.77 \text{ IN}$$
- $$V_{100} = (E_W / 12) A_T = (1.77 / 12) 0.53 = 0.0782 \text{ AC-FT} = 3,410 \text{ CF}$$

- b. PEAK DISCHARGE
- $$Q_P = Q_{PA} A_A + Q_{PB} A_B + Q_{PC} A_C + Q_{PD} A_D$$
- $$Q_P = Q_{100} = ((0.00^*1.29) + (0.02^*2.03) + (0.08^*2.87) + (0.43^*4.37)) = 2.1 \text{ CFS}$$



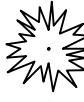
























14. BASIN L
- a. VOLUME
- $$E_W = (E_A A_A + E_B A_B + E_C A_C + E_D A_D) / A_T$$
- $$E_W = ((0.00^*0.44) + (0.02^*0.67) + (0.08^*0.99) + (0.43^*1.97)) / 0.53 = 1.77 \text{ IN}$$
- $$V_{100} = (E_W / 12) A_T = (1.77 / 12) 0.53 = 0.0782 \text{ AC-FT} = 3,410 \text{ CF}$$

- b. PEAK DISCHARGE
- $$Q_P = Q_{PA} A_A + Q_{PB} A_B + Q_{PC} A_C + Q_{PD} A_D$$
- $$Q_P = Q_{100} = ((0.00^*1.29) + (0.02^*2.03) + (0.08^*2.87) + (0.43^*4.37)) = 2.1 \text{ CFS}$$

15. BASIN M
- a. VOLUME
- $$E_W = (E_A A_A + E_B A_B + E_C A_C + E_D A_D) / A_T$$
- $$E_W = ((0.00^*0.44) + (0.02^*0.67) + (0.08^*0.99) + (0.43^*1.97)) / 0.53 = 1.77 \text{ IN}$$
- $$V_{100} = (E_W / 12) A_T = (1.77 / 12) 0.53 = 0.0782 \text{ AC-FT} = 3,410 \text{ CF}$$

- b. PEAK DISCHARGE
- $$Q_P = Q_{PA} A_A + Q_{PB} A_B + Q_{PC} A_C + Q_{PD} A_D$$
- $$Q_P = Q_{100} = ((0.00^*1.29) + (0.02^*2.03) + (0.08^*2.87) + (0.43^*4.37)) = 2.1 \text{ CFS}$$

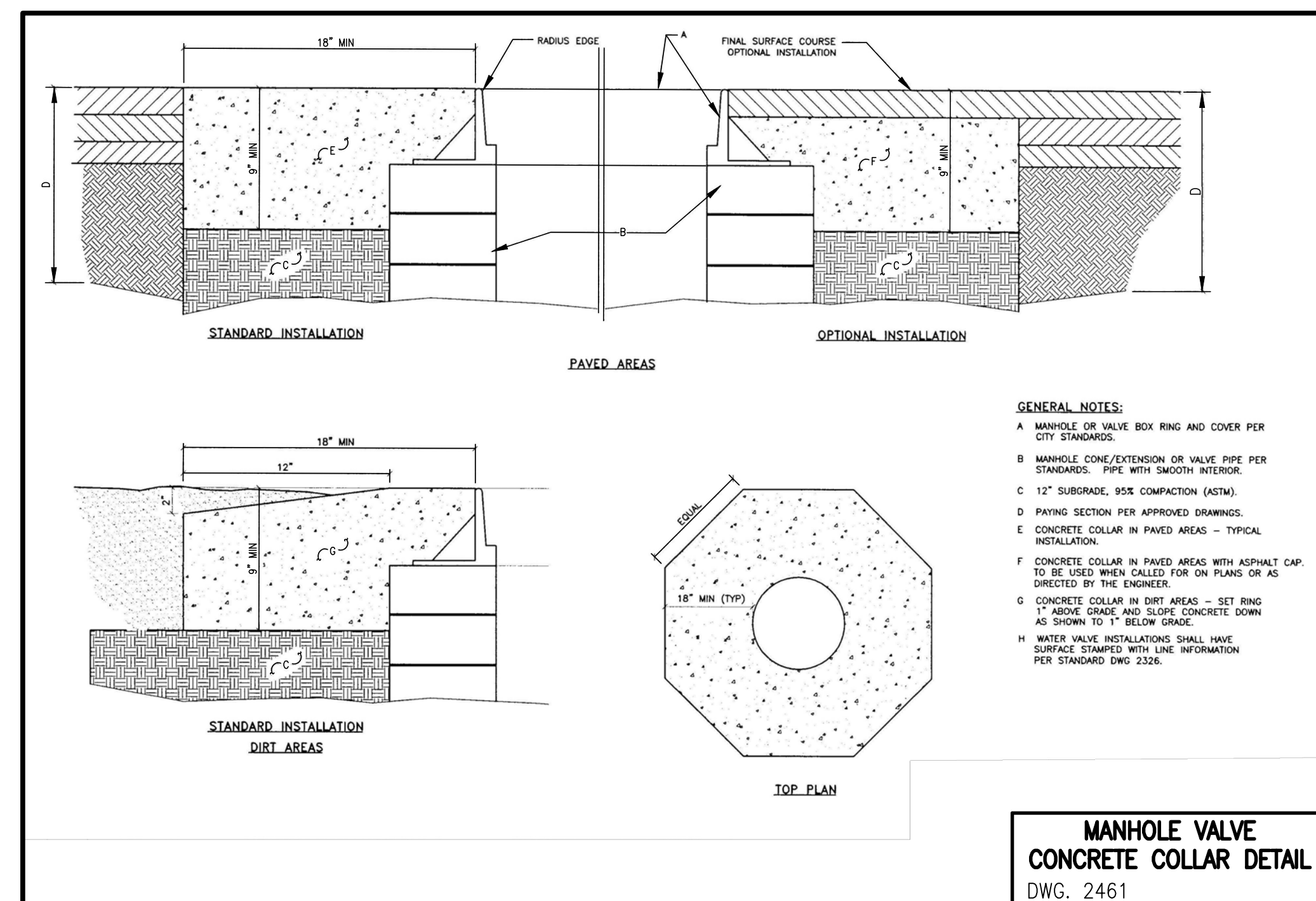
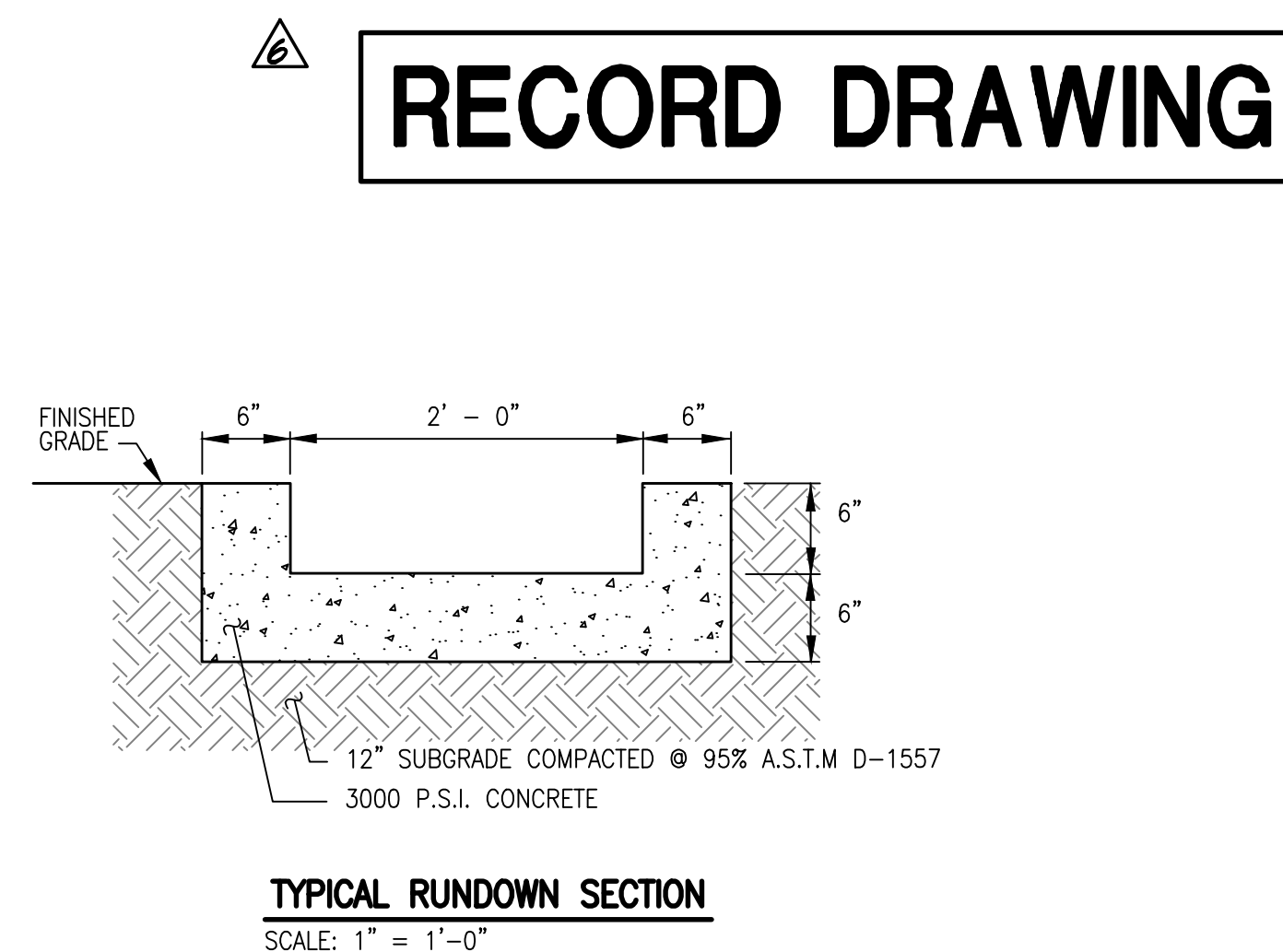
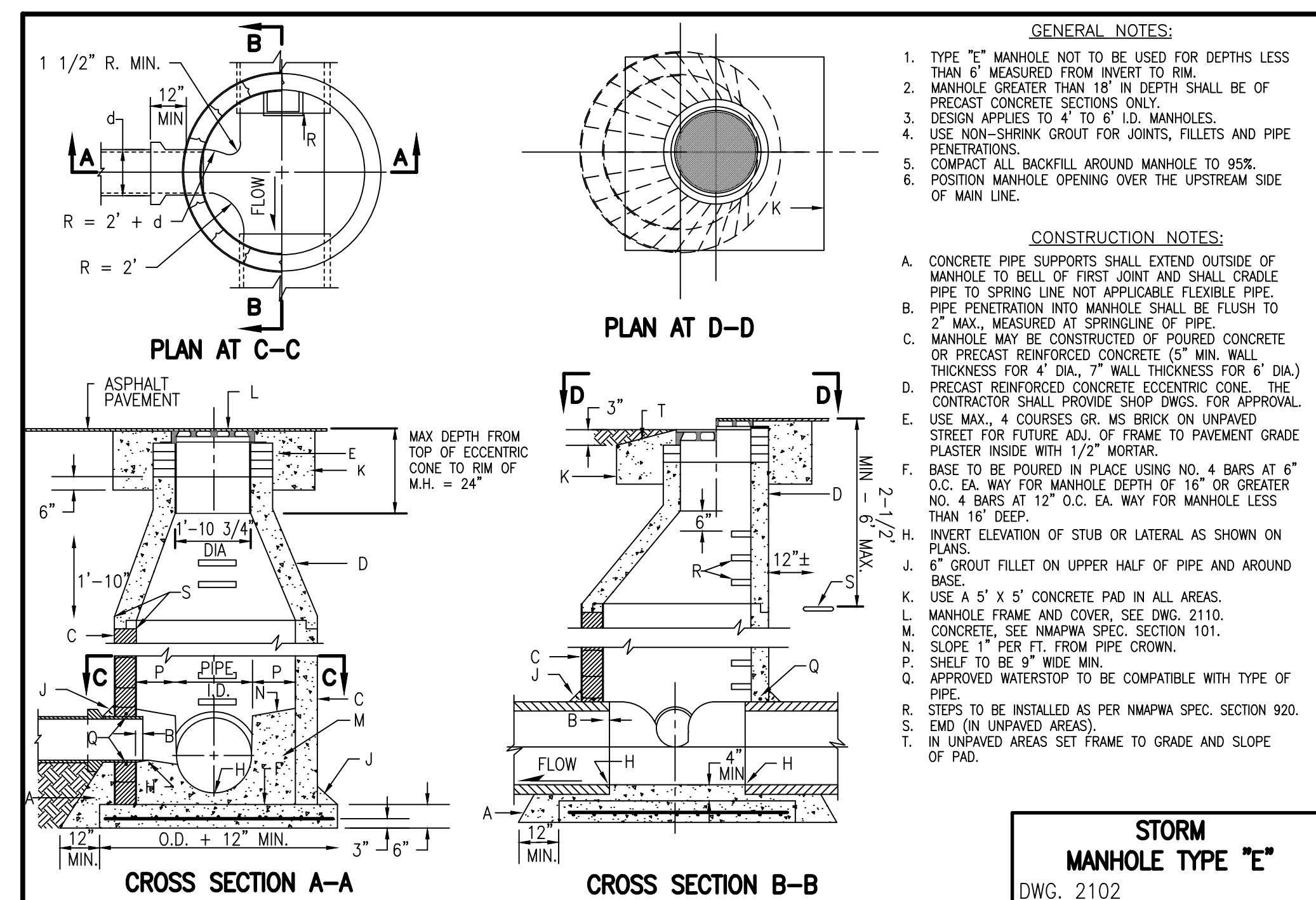
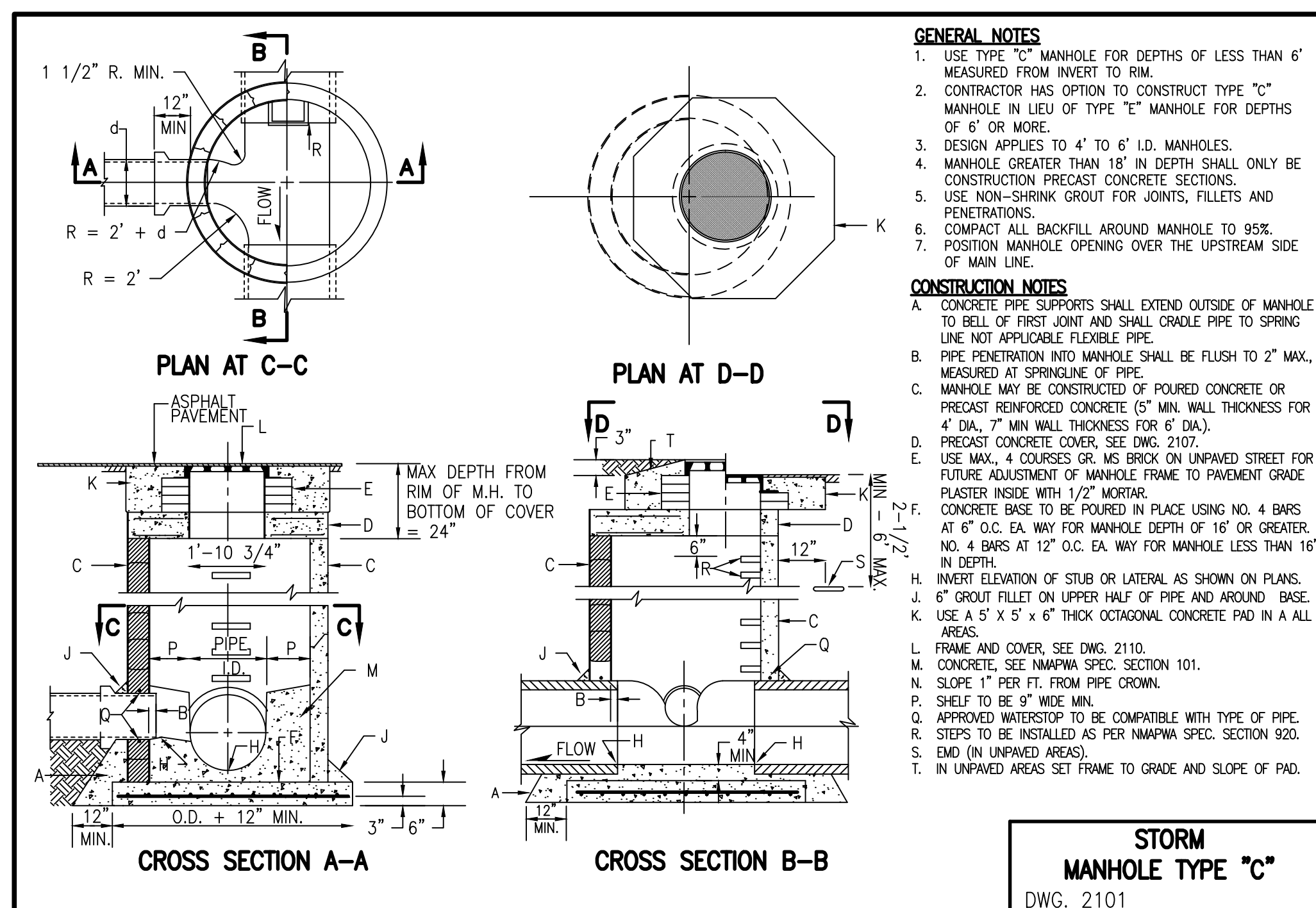
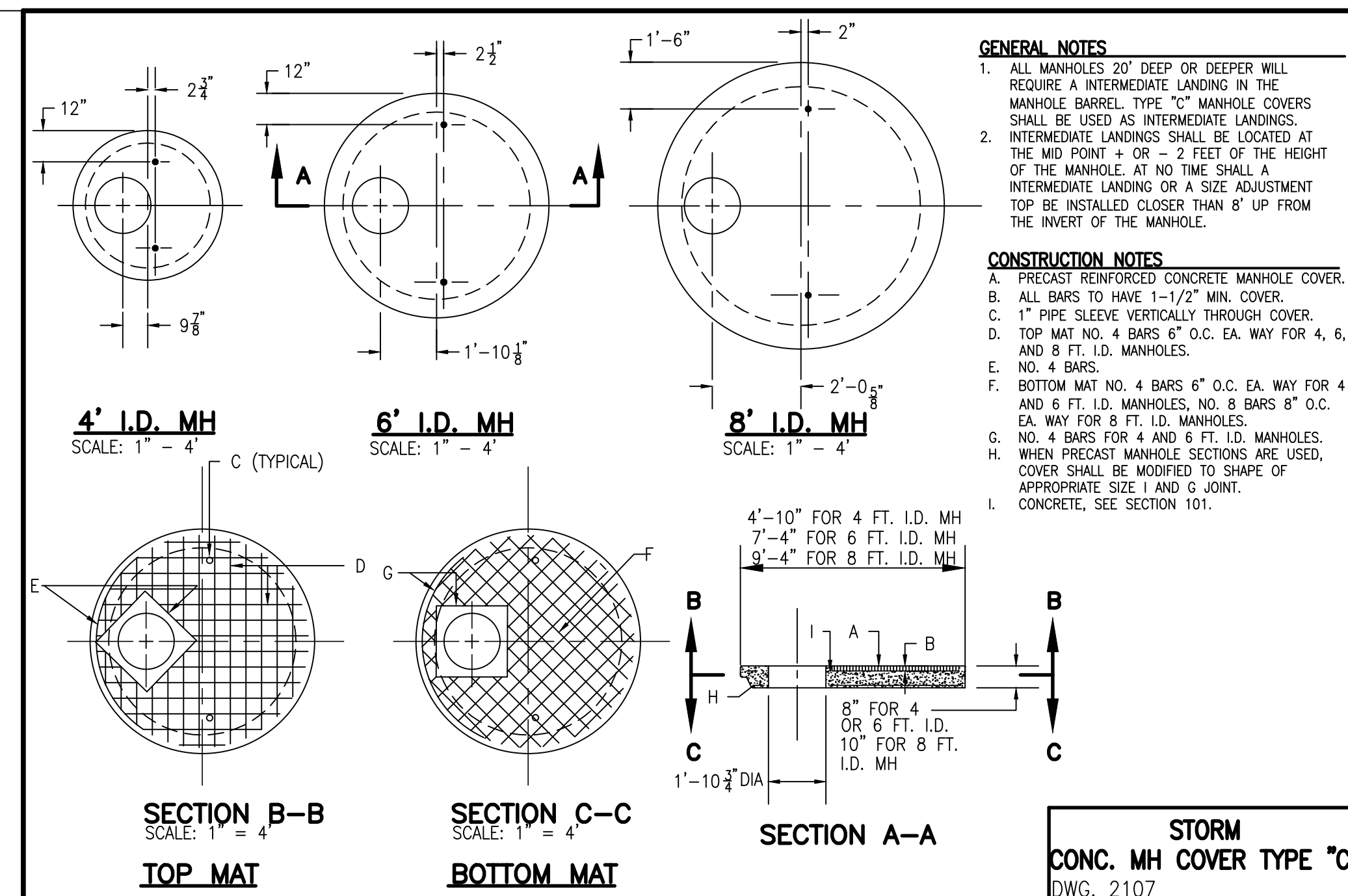
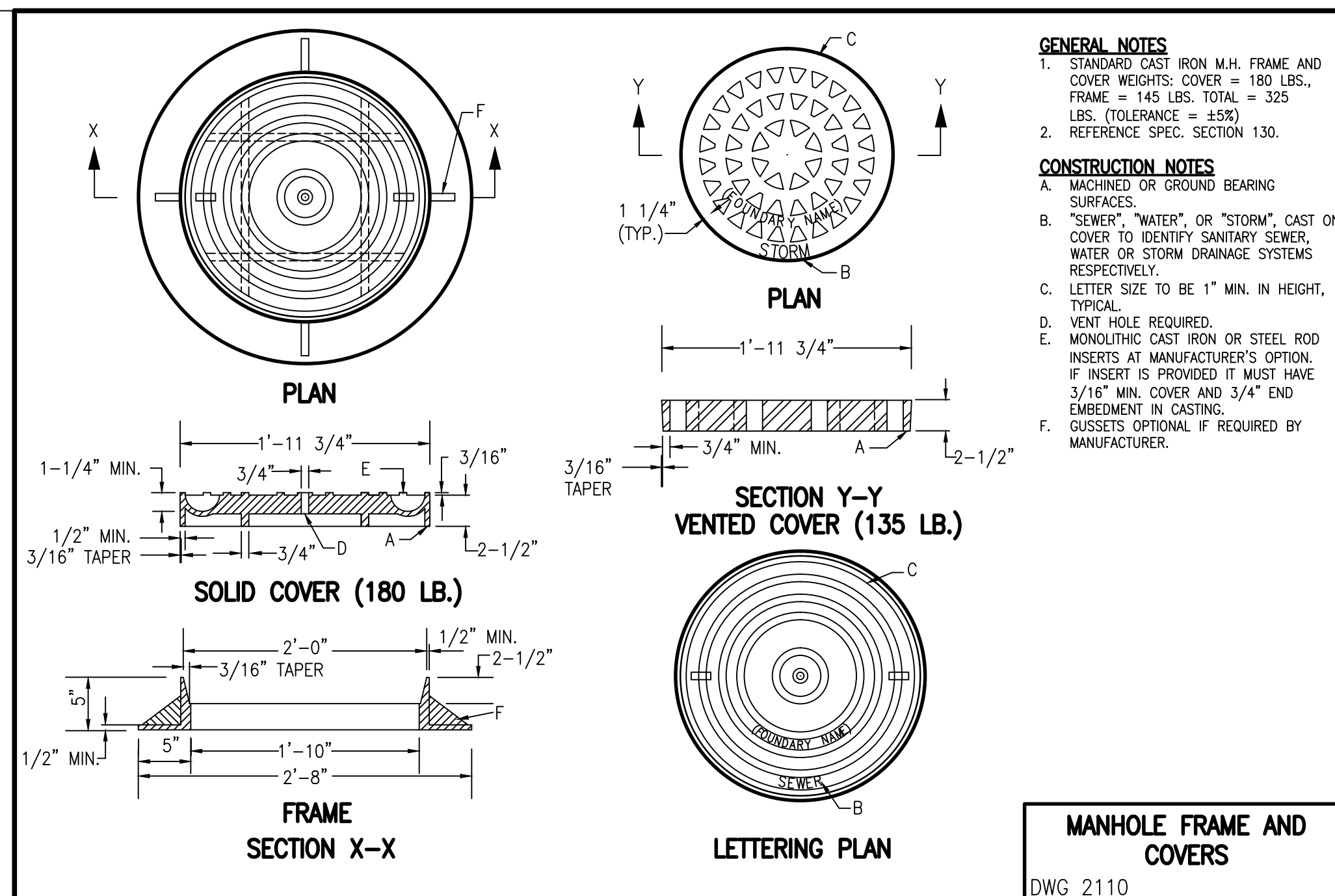
LEGEND

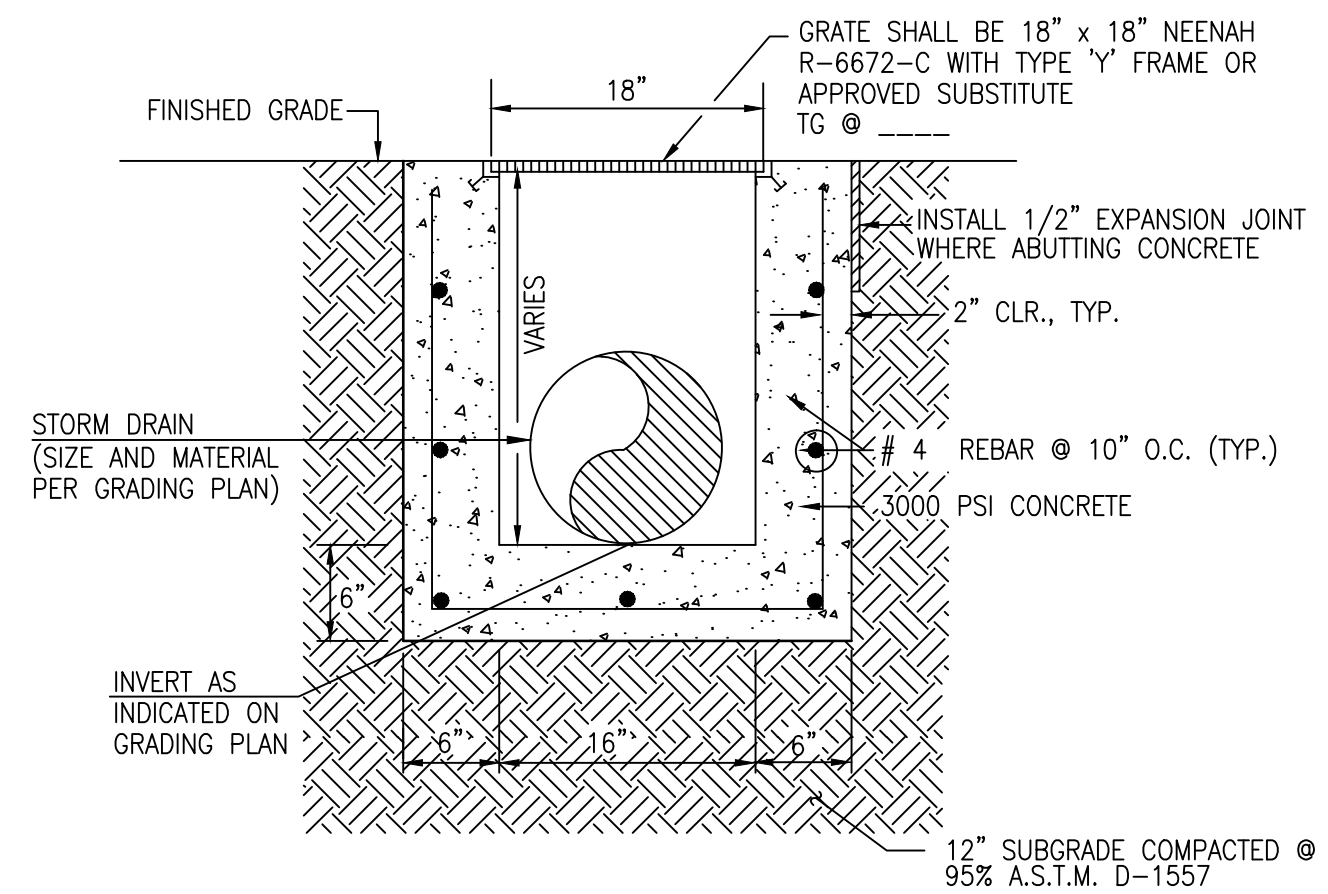
| | | | |
|---------|---|---|----------------------------|
| ADO | AUTOMATIC DOOR OPENER | PS | PAINTED PARKING STRIPE |
| AR | ASPHALT RAMP | RRT | LANDSCAPING RAILROAD TIES |
| BLDG | BUILDING | SAS | SANITARY SEWER |
| BH | BUILDING OVERHANG | SAT | SATELLITE DISH |
| BW | BARBED WIRE | SD | STORM DRAIN |
| C&G | CURB & GUTTER | SDI | STORM DRAIN INLET |
| C | COMMUNICATIONS LINE | SGP | STEEL GUARD POST |
| C / C/M | COMMUNICATIONS LINE BY PAINT MARK | SP | STEEL POLE |
| CM | CAMERA | SPB | SPEED BUMP |
| CBC | CONCRETE BUILDING COLUMN | SW | SIDEWALK |
| CC | COMMUNICATIONS CONDUIT | TA | TOP OF ASPHALT |
| CCP | CONCRETE DRIVE PAD | TC | TOP OF CURB |
| CGP | CONCRETE GUARD POST | TG | TOP OF CONCRETE |
| CMH | COMMUNICATIONS MANHOLE | TO | TOP OF GRATE |
| CHC | CONCRETE HEADER CURB | TRN | ELECTRIC TRANSFORMER |
| CLD | CENTERLINE DOOR | TS | TRAFFIC SIGN |
| CLDL | CENTERLINE DOUBLE DOOR | TV | TELEVISION |
| CLM | COMMUNICATIONS LINE MARKER | TV/M | BY PAINT MARK |
| CL | CONCRETE LANDING | TVR | CABLE TELEVISION RISER |
| CMP | COMMUNICATIONS PANEL | UNK | UNKNOWN |
| CMU | CONCRETE MASONRY UNIT WALL | W | WITH |
| CO | SANITARY SEWER CLEANOUT | W/M | WATER LINE BY PAINT MARK |
| C/CNC | COMMUNICATIONS PULLBOX | WCR | CONCRETE WHEELCHAIR RAMP |
| CR | CONCRETE RAMP | WHB | WATER HOT BOX |
| CRD | CONCRETE ROUNDOWN | WMB | WATER METER BOX |
| CS | CONCRETE STEPS | WPP | WOOD POWER POLE |
| CSP | CONCRETE SPLASH PAD | WV | WATER VALVE |
| CSW | CONCRETE SIDEWALK | WVB | WATER VALVE BOX |
| CTC | CONCRETE TRASH CAN | XW | PAINTED CROSS WALK |
| DBL | DOUBLE | 0.5'φ | TREE TRUNK DIAMETER |
| DCO | DOUBLE SANITARY SEWER CLEANOUT | | |
| E/P/M | ELECTRIC LINE BY PAINT MARK |  | DECIDUOUS TREE |
| EA | EDGE OF ASPHALT |  | SMALL DECIDUOUS TREE |
| EC | ELECTRIC CONDUIT |  | CONIFEROUS TREE |
| EO | ELECTRIC OUTLET |  | SMALL CONIFEROUS TREE |
| EP | ELECTRIC PANEL |  | SMALL SHRUB |
| EPB | ELECTRIC PULLBOX |  | BOULDER |
| FH | FIRE HYDRANT |  | PAINTED UTILITY MARKER |
| FL | FLOWLINE |  | STUMP |
| F/LC | FIRE LINE CONNECTION | INVT | INVERT |
| FNT | FOUNTAIN | TA | TOP OF ASPHALT PAVEMENT |
| FP | FLAG POLE | TC | TOP OF CURB |
| FRD | FROM RECORD DRAWING | TG | TOP OF GRATE |
| G/M | GAS LINE BY PAINT MARK | + 20.05 | EXISTING SPT ELEVATION |
| GL | GAS LINE | 5105- | EXISTING CONTOUR |
| GRV | GRAVEL |  | EXISTING DIRECTION OF FLOW |
| GS | GAS SERVICE |  | EXISTING STORM DRAIN MH |
| HCS | HANDICAPPED PARKING SIGN |  | EXISTING FIRE HYDRANT |
| ICB | IRRIGATION CONTROL BOX |  | EXISTING GAS MANHOLE |
| INVT | INVERT |  | EXISTING VALVE BOX |
| INVT | INVERT |  | EXISTING DOUBLE CLEANOUT |
| IVB | IRRIGATION VALVE BOX |  | EXISTING SINGLE CLEANOUT |
| MBC | METAL BUILDING COLUMN |  | EXISTING WATER SERVICE |
| MC/B | METER CAN WITH BIB VALVE |  | EXISTING WATER LINE |
| MC/V | METER CAN WITH VALVE |  | EXISTING SANITARY SEWER |
| MH | MANHOLE |  | EXISTING FIRE LINE |
| MHR | METAL HAND RAIL |  | EXISTING P.I.V. |
| MR | METAL RAMP |  | SANITARY SEWER MANHOLE |
| MTS | METAL SIGN |  | PROPOSED VALVE BOX |
| OH(C1) | OVERHEAD COMMUNICATIONS (# OF LINES) |  | PROPOSED DOUBLE CLEANOUT |
| OH(E3) | OVERHEAD ELECTRIC (# OF LINES) |  | PROPOSED SINGLE CLEANOUT |
| PI | PAINTED PARKING LOT ISLAND POST INDICATOR VALVE |  | PROPOSED WATER SERVICE |
| P/V | POST INDICATOR VALVE |  | PROPOSED WATER LINE |
| 14.00 | PROPOSED SPOT ELEVATION |  | PROPOSED SANITARY SEWER |
| --- | EXISTING FLOWLINE | | PROPOSED FIRE LINE |
| --- | PROPOSED FLOWLINE | | PROPOSED P.I.V. |
| --- | PROPOSED CONTOUR | | PROPOSED STORM DRAIN |
| --- | PROPOSED DIRECTION OF FLOW | | PROPOSED CONCRETE |
| --- | RIGHT OF WAY LINE | | PROPOSED CRUSHER FINES |
| --- | PUBLIC EASEMENT LINE | | LANDSCAPED AREA |
| --- | HIGH POINT / DIVIDE | | DRAINAGE BASIN |
| --- | PROPOSED STORM INLET | | |
| --- | PROPOSED STORM DRAIN MANHOLE | | |
| --- | PROPOSED FIRE HYDRANT | | |
| --- | FIRE DEPARTMENT CONNECTION | | |
| --- | HIGH POINT/WATER BLOCK | | |



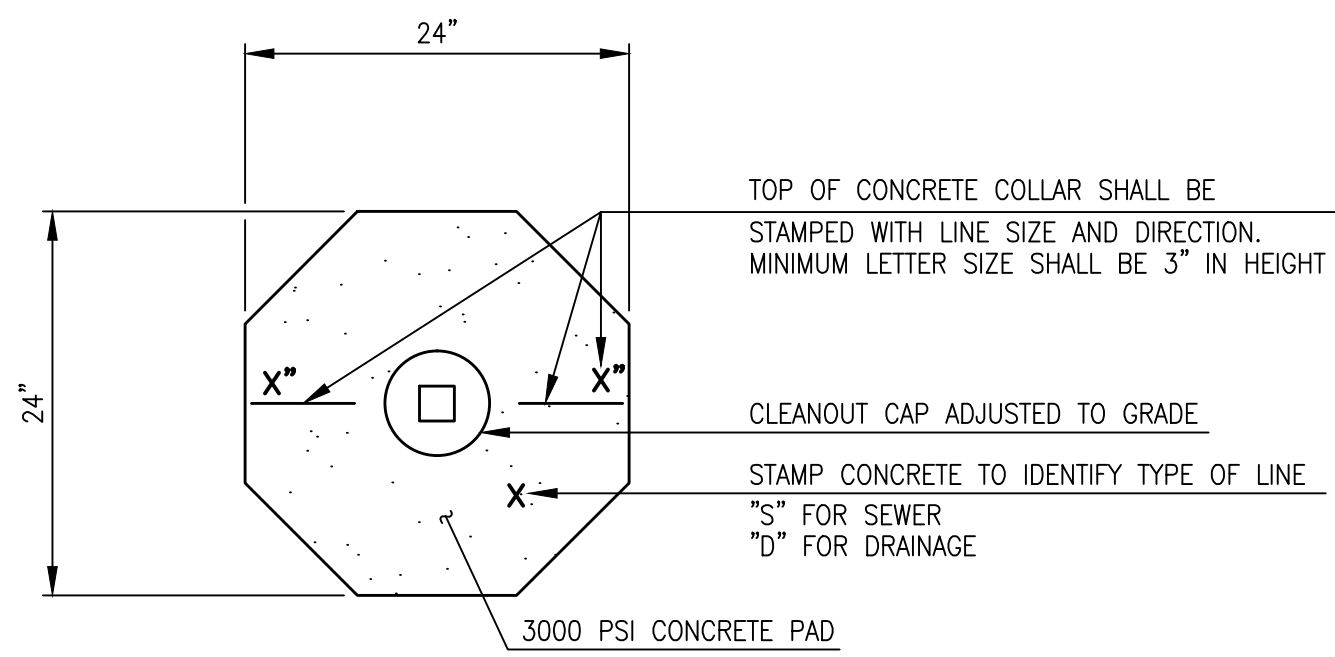
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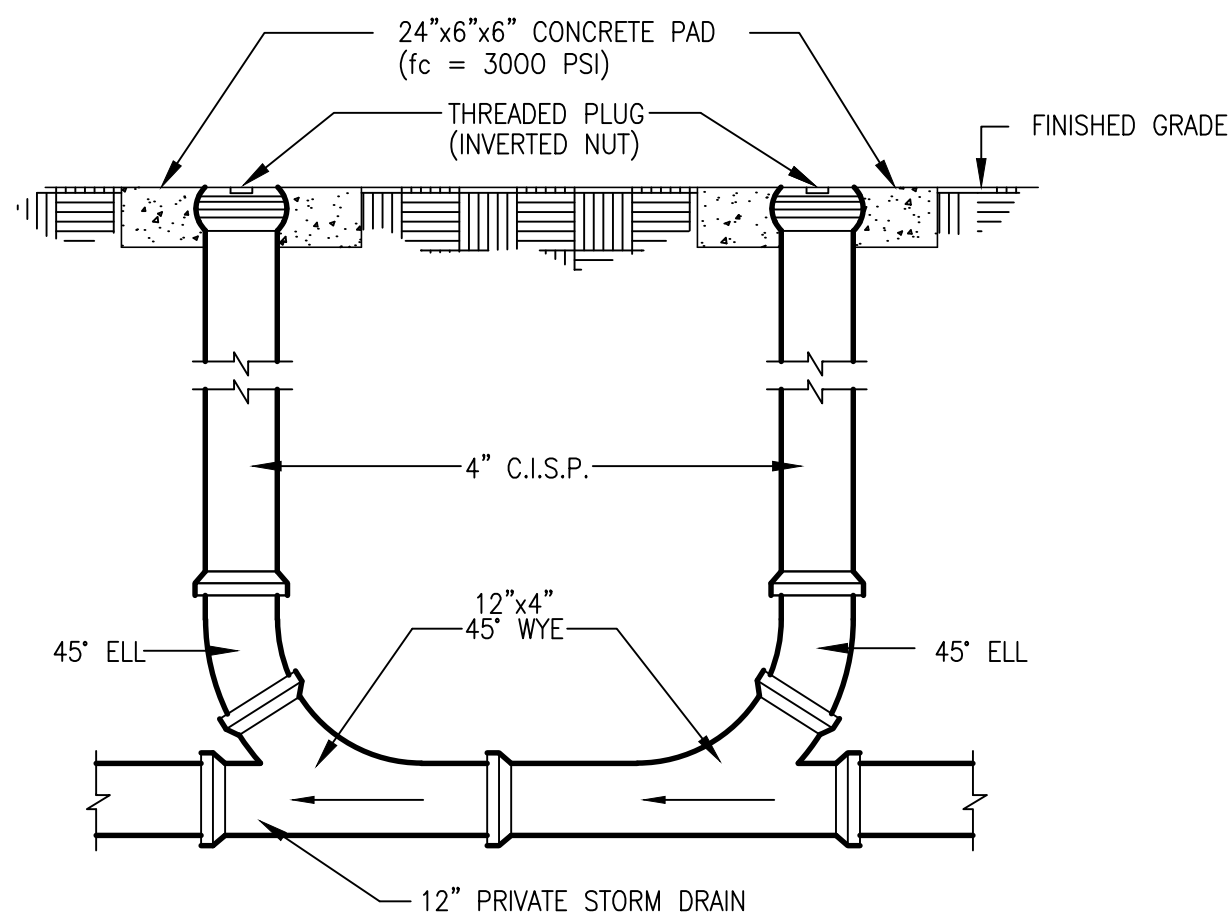




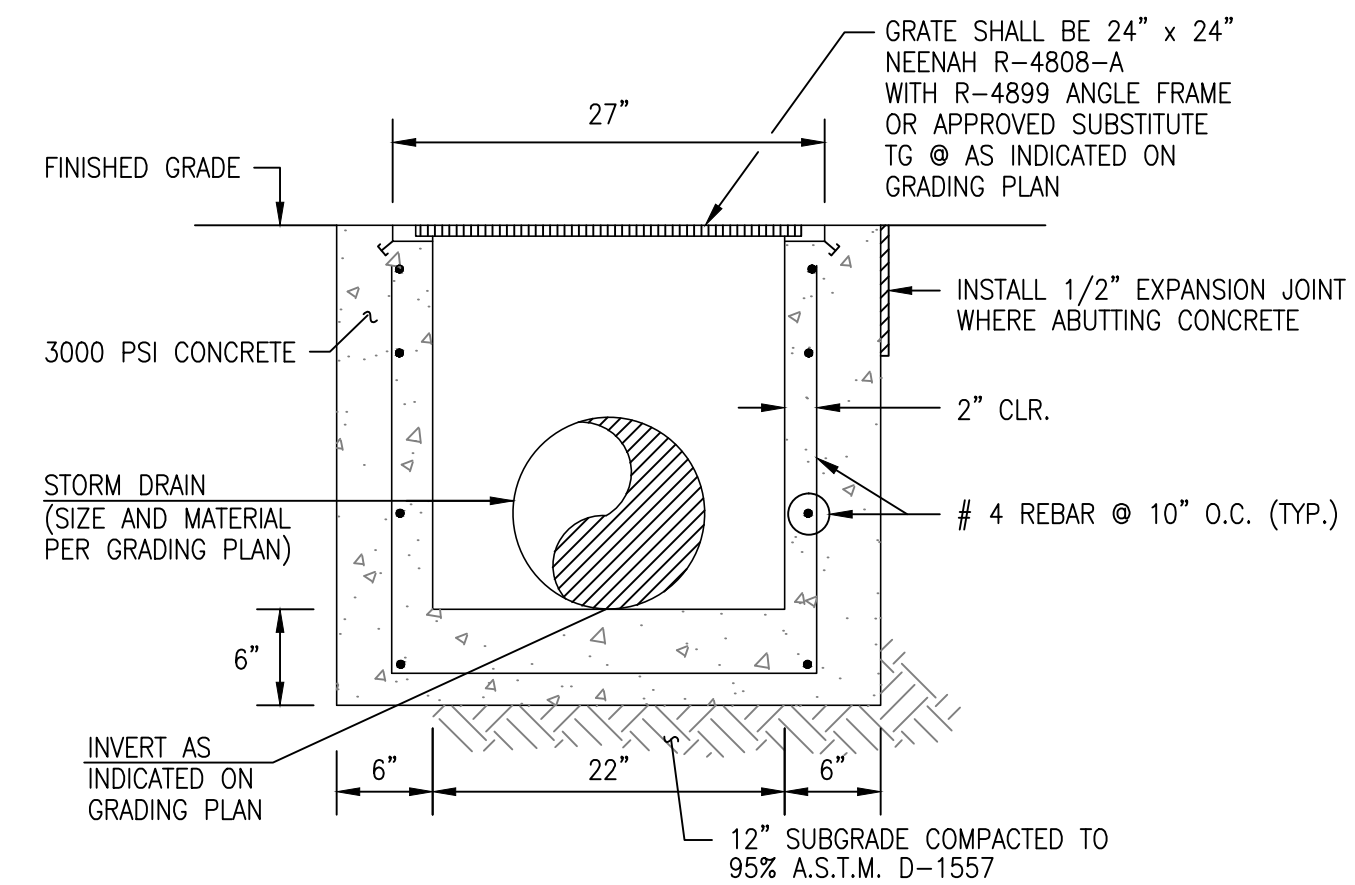
(A1) TYPICAL 18"X18" STORM INLET SECTION
SCALE: 1" = 1' - 0"



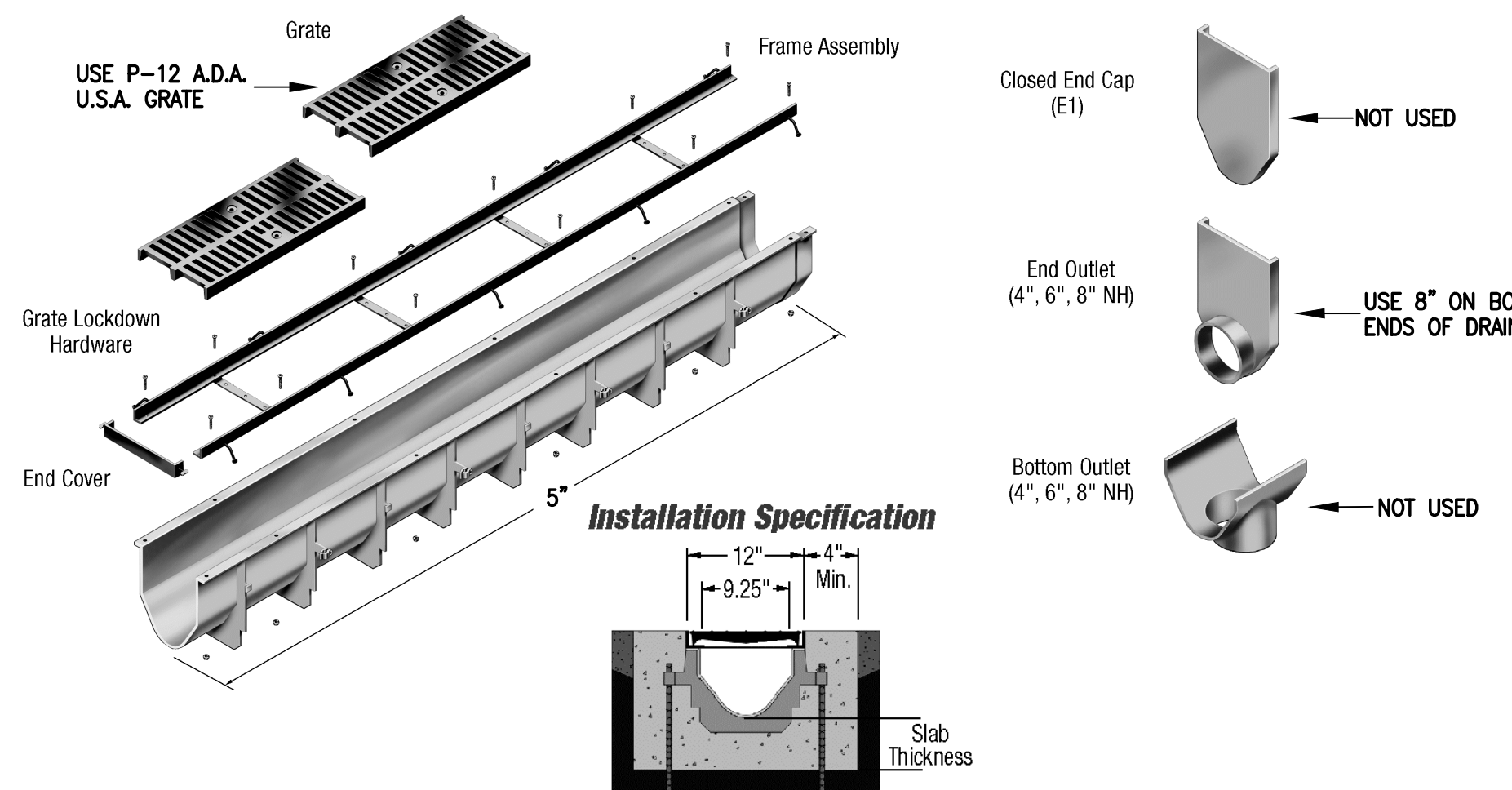
(C3) TYPICAL CLEANOUT COLLAR DETAIL
SCALE: 1" = 1'-0"



A3 TYPICAL DOUBLE CLEANOUT SECTION
NOT TO SCALE



(D5) TYPICAL 24"X24" STORM INLET SECTION
SCALE: 1" = 1' - 0"



(A5) TRENCH DRAIN SECTIONS AND DETAILS (ZURN Z-882)
SCALE: 1" = 2'-0"

