

# CITY OF ALBUQUERQUE

Planning Department  
Brennon Williams, Director



Mayor Timothy M. Keller

February 20, 2020

Holden Rennaker  
Short Elliot Hendrickson Inc.  
934 Main Avenue, Unit C  
Durango, CO 81301

RE: **Monterey Motel**  
**2402 Central SW**  
**Grading Plan Stamp Date: 2/4/20**  
**Drainage Report Stamp Date: 2/4/20**  
**Hydrology File: J12D030**

Dear Mr. Rennaker,

PO Box 1293

Based on the submittal received on 2/13/20 the above-referenced Grading Plan and Drainage Report cannot be approved until the following corrections are made:

Prior to Site Plan for Building Permit and Building Permit:

Albuquerque

NM 87103

www.cabq.gov

1. Provide written and signed permission from the adjoining property owner (El Vado Place) for the work on their property.
2. The COA inlets in the valley pan should be called out as Type-D, not C.
3. Payment in Lieu (Amount =  $207\text{CF} \times \$8/\text{CF} = \$1656$ , per sheet C-002) of onsite management of the SWQV must be made. Take three copies of the treasury deposit slip to the Treasury and then include one copy of the paid deposit slip when resubmitting.
4. A waterblock, 0.87' high, per COA Paving Detail No. 2426, is required at the driveway entrances.
5. Include project benchmark and datum; all existing survey, proposed grades, and benchmarks must be provided in NAVD 88.
6. Please provide the FIRM Map and floodplain note with effective date.
7. Please provide the legal description of the property on the Grading Plan.
8. Since the site is extremely flat, please provide spot elevations in enough density to verify the drainage areas and outfalls that you have indicated (top of curb, flow line, top of grate, bottom of wall, etc...).

# CITY OF ALBUQUERQUE

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9. As a reminder, if the project total area of disturbance (including the staging area and any work within the adjacent Right-of-Way) is 1 acre or more, then an Erosion and Sediment Control (ESC) Plan and Owner's certified Notice of Intent (NOI) is required to be submitted to the Stormwater Quality Engineer (Doug Hughes, PE, [jhughes@cabq.gov](mailto:jhughes@cabq.gov), 924-3420) 14 days prior to any earth disturbance.

Prior to Certificate of Occupancy (For Information):

10. Engineer's Certification, per the DPM Chapter 22.7: *Engineer's Certification Checklist For Non-Subdivision* is required.
11. City acceptance and close-out of the public Work Order will be required, unless a financial guarantee has been posted.

If you have any questions, please contact me at 924-3695 or [dpeterson@cabq.gov](mailto:dpeterson@cabq.gov).

PO Box 1293

Sincerely,

Albuquerque



Dana M. Peterson  
Senior Engineer, Planning Dept.  
Development Review Services

NM 87103

[www.cabq.gov](http://www.cabq.gov)



TREASURY DIVISION DAILY DEPOSIT

Transmittals for:  
PROJECTS Only

# Payment In-Lieu for Storm Water Quality Volume Requirement

CASH COUNT	AMOUNT	ACCOUNT NUMBER	FUND NUMBER	BUSINESS UNIT	PROJECT ID	ACTIVITY ID	AMOUNT
TOTAL CHECKS	\$ 4000.00	461615	305	PCDMD	24_MS4	7547210	\$ 4000.00
TOTAL AMOUNT						TOTAL DEPOSIT	<b>\$4000.00</b>

Hydrology#: J12D032 Name: Monterey Motel, 23087 sf imp.  
Payment In-Lieu For Storm Water Quality  
Volume Requirement

Address/Legal Description: 2402 Central Avenue SW  
LOTS 11-A, TRACTION PARK AND CITY ELECTRIC ADDITION

DEPARTMENT NAME: Planning Department/Development Review Services, Hydrology

PREPARED BY Dana Peterson PHONE 924-3695

BUSINESS DATE 2/20/20

DUAL VERIFICATION OF DEPOSIT   
EMPLOYEE SIGNATURE

AND BY \_\_\_\_\_  
EMPLOYEE SIGNATURE

REMITTER: \_\_\_\_\_

AMOUNT: \_\_\_\_\_

BANK: \_\_\_\_\_

CHECK #: \_\_\_\_\_ DATE ON CHECK: \_\_\_\_\_

The Payment-in-Lieu can be paid at the Plaza del Sol Treasury, 600 2<sup>nd</sup> St. NW. **Bring three copies of this invoice to the Treasury** and provide a copy of the receipt to Hydrology, Suite 201, 600 2<sup>nd</sup> St. NW, or e-mail with the Hydrology submittal to [PLNDRS@cabq.gov](mailto:PLNDRS@cabq.gov).



# City of Albuquerque

Planning Department

Development & Building Services Division

## DRAINAGE AND TRANSPORTATION INFORMATION SHEET (REV 11/2018)

**Project Title:** \_\_\_\_\_ **Building Permit #:** \_\_\_\_\_ **Hydrology File #:** \_\_\_\_\_

**DRB#:** \_\_\_\_\_ **EPC#:** \_\_\_\_\_ **Work Order#:** \_\_\_\_\_

**Legal Description:** \_\_\_\_\_

**City Address:** \_\_\_\_\_

**Applicant:** \_\_\_\_\_ **Contact:** \_\_\_\_\_

**Address:** \_\_\_\_\_

**Phone#:** \_\_\_\_\_ **Fax#:** \_\_\_\_\_ **E-mail:** \_\_\_\_\_

**Owner:** \_\_\_\_\_ **Contact:** \_\_\_\_\_

**Address:** \_\_\_\_\_

**Phone#:** \_\_\_\_\_ **Fax#:** \_\_\_\_\_ **E-mail:** \_\_\_\_\_

**TYPE OF SUBMITTAL:** \_\_\_\_\_ PLAT (\_\_\_# OF LOTS) \_\_\_\_\_ RESIDENCE \_\_\_\_\_ DRB SITE \_\_\_\_\_ ADMIN SITE

**IS THIS A RESUBMITTAL?:** \_\_\_\_\_ Yes \_\_\_\_\_ No

**DEPARTMENT:** \_\_\_\_\_ TRAFFIC/ TRANSPORTATION \_\_\_\_\_ HYDROLOGY/ DRAINAGE

Check all that Apply:

**TYPE OF SUBMITTAL:**

- \_\_\_\_\_ ENGINEER/ARCHITECT CERTIFICATION
- \_\_\_\_\_ PAD CERTIFICATION
- \_\_\_\_\_ CONCEPTUAL G & D PLAN
- \_\_\_\_\_ GRADING PLAN
- \_\_\_\_\_ DRAINAGE MASTER PLAN
- \_\_\_\_\_ DRAINAGE REPORT
- \_\_\_\_\_ FLOODPLAIN DEVELOPMENT PERMIT APPLIC
- \_\_\_\_\_ ELEVATION CERTIFICATE
- \_\_\_\_\_ CLOMR/LOMR
- \_\_\_\_\_ TRAFFIC CIRCULATION LAYOUT (TCL)
- \_\_\_\_\_ TRAFFIC IMPACT STUDY (TIS)
- \_\_\_\_\_ OTHER (SPECIFY) \_\_\_\_\_
- \_\_\_\_\_ PRE-DESIGN MEETING?

**TYPE OF APPROVAL/ACCEPTANCE SOUGHT:**

- \_\_\_\_\_ BUILDING PERMIT APPROVAL
- \_\_\_\_\_ CERTIFICATE OF OCCUPANCY
- \_\_\_\_\_ PRELIMINARY PLAT APPROVAL
- \_\_\_\_\_ SITE PLAN FOR SUB'D APPROVAL
- \_\_\_\_\_ SITE PLAN FOR BLDG. PERMIT APPROVAL
- \_\_\_\_\_ FINAL PLAT APPROVAL
- \_\_\_\_\_ SIA/ RELEASE OF FINANCIAL GUARANTEE
- \_\_\_\_\_ FOUNDATION PERMIT APPROVAL
- \_\_\_\_\_ GRADING PERMIT APPROVAL
- \_\_\_\_\_ SO-19 APPROVAL
- \_\_\_\_\_ PAVING PERMIT APPROVAL
- \_\_\_\_\_ GRADING/ PAD CERTIFICATION
- \_\_\_\_\_ WORK ORDER APPROVAL
- \_\_\_\_\_ CLOMR/LOMR
- \_\_\_\_\_ FLOODPLAIN DEVELOPMENT PERMIT
- \_\_\_\_\_ OTHER (SPECIFY) \_\_\_\_\_

**DATE SUBMITTED:** \_\_\_\_\_ **By:** \_\_\_\_\_

COA STAFF:

ELECTRONIC SUBMITTAL RECEIVED: \_\_\_\_\_

FEE PAID: \_\_\_\_\_

**NOTES:**  
 1. NO WORK SHALL BE PERFORMED IN PUBLIC ROW WITHOUT AN APPROVED WORK ORDER OR EXCAVATION PERMIT.

**EXISTING CONDITIONS:**

THE PROPOSED DEVELOPMENT IS PRESENTLY AN EXISTING MOTEL THAT IS PREDOMINANTLY ASPHALT AND CONCRETE COVERED, WITH AREAS OF COMPACTED GRAVEL AND MINIMAL LANDSCAPING. THE EXISTING SITE DRAINS TO A LOW POINT IN THE SOUTH DRIVE AISLE WHERE IT IS CAPTURED BY A PRIVATE STORM DRAIN INLET. FLOWS FROM THIS INLET ARE THEN CONVEYED VIA A 6-IN STORM DRAIN LINE TO AN EXISTING CITY 18-IN STORM DRAIN TO THE EAST OF THE PROJECT SITE. MINOR NUISANCE FLOWS SURFACE FLOW TO BOTH NEW YORK AVENUE AND CENTRAL AVENUE.

TOTAL RUNOFF FROM THE EXISTING SITE WAS CALCULATED IN TABLE 1 OF THIS PLAN.

ADJACENT PROPERTIES WERE NOT FOUND TO DISCHARGE ONTO THE PROJECT SITE. THE EAST AND WEST LOT LINES HAVE AN EXISTING CMU WALL THAT PREVENT FLOWS FROM ENTERING OR LEAVING THE SITE.

**PROPOSED CONDITIONS:**

THE PROJECT WILL CONSIST OF TWO PHASES. PHASE 1 IS THE RENOVATION OF THE EXISTING MOTEL BUILDING WHICH RESULTS IN A SMALLER BUILDING AREA BASED ON THE DEMOLITION OF AN ADDITION ON THE WEST MOTEL BUILDING. PHASE 2 IS FOR A NEW BUILDING AROUND THE EXISTING POOL. THE PRELIMINARY FOOTPRINT OF THE BUILDING FOR PHASE 2 WAS INCLUDED IN ALL STORM DRAIN CALCULATIONS TO ENSURE ADEQUATE FEATURE SIZING AND FOR DEVELOPED RUNOFF CALCULATIONS.

THE PROPOSED SITE WAS DESIGNED TO HAVE SLIGHTLY REDUCED IMPERVIOUS AREA FROM EXISTING CONDITIONS.

THE MIDDLE PORTION OF THE SITE WAS DIVIDED INTO BASINS A1, A2, A4, AND A5 WHICH WERE DESIGNED TO DRAIN TO THEIR RESPECTIVELY NAMED INLETS. INLET A3 IS NEAR A HIGH POINT IN THE GUTTER PAN WHICH RESULTS IN NEGLIGIBLE FLOW. THIS INLET WAS DESIGNED PREMARILY TO PROVIDE A CHANGE OF DIRECTION OF THE STORM DRAIN PIPE.

FLOWS FROM THE ROOF AND FROM BEHIND EACH BUILDING WILL BE CONVEYED VIA A STORM DRAIN COLLECTION SYSTEM THAT RUNS BEHIND EACH BUILDING. ROOF DOWNSPOUTS WERE DESIGNED TO DISCHARGE INTO A SMALL AREA DRAIN WHICH WILL TIE INTO THESE COLLECTION LINES.

PONDING FEATURES ON-SITE WERE NOT REASONABLY ABLE TO BE DESIGNED BASED ON THE CONSTRAINTS OF THE SITE. 23,087-SF OF IMPERVIOUS AREA WILL NOT BE TREATED ON-SITE AND THE DEVELOPER IS REQUESTING TO PAY A FEE IN-LIEU OF MANAGING ON SITE PER TABLE 6.17 OF THE DRAFT DPM. THIS AREA INCLUDES THE IMPERVIOUS AREA OF THE FUTURE BUILDING AND POOL DECK.

STORM VOLUMES AND PEAK FLOW CALCULATIONS ARE SHOWN IN TABLES 3 AND 4. WATER QUALITY VOLUME CALCULATIONS ARE SHOWN IN TABLE 5.

THE DEVELOPED OVERALL FLOWS WILL BE SLIGHTLY REDUCED FROM THE EXISTING CONDITIONS, AS SHOWN IN TABLE 2. THE RUNOFF PATTERN WAS ALTERED SLIGHTLY TO CONVEY FLOWS TO AN EXISTING STORM DRAIN MANHOLE AS OPPOSED TO THE EXISTING 6-IN STORM DRAIN CONNECTION.

**PROJECT BACKGROUND INFORMATION**

PROJECT AREA: 0.66 ACRES  
 ZONE: 2

TREATMENT	EXCESS PRECIPITATION (INCHES)		
	2-YR	10-YR	100-YR
A	0	0.15	0.62
B	0.02	0.3	0.8
C	0.16	0.48	1.03
D	0.98	1.51	2.33

TREATMENT	PEAK DISCHARGE (CFS/ACRE)		
	2-YR	10-YR	100-YR
A	0	0.41	1.71
B	0.08	0.95	2.36
C	0.61	1.59	3.05
D	1.66	2.71	4.34

TABLE 1 - EXISTING SITE OVERALL RUNOFF

LAND TREATMENT	AREA (ACRES)	PEAK (CFS)		
		2-YR	10-YR	100-YR
B	0.01	0.00	0.01	0.02
C	0.11	0.07	0.17	0.34
D	0.55	0.91	1.49	2.39
<b>TOTAL</b>	<b>0.67</b>	<b>0.98</b>	<b>1.67</b>	<b>2.75</b>

TABLE 2 - DEVELOPED SITE OVERALL RUNOFF

LAND TREATMENT	AREA (ACRES)	PEAK (CFS)		
		2-YR	10-YR	100-YR
B	0.14	0.01	0.13	0.33
C	0	0.00	0.00	0.00
D	0.53	0.88	1.44	2.30
<b>TOTAL</b>	<b>0.67</b>	<b>0.89</b>	<b>1.57</b>	<b>2.63</b>

TABLE 3 - DEVELOPED BASIN VOLUME TABLE

BASIN	TREATMENT B AREA (ACRES)	TREATMENT D AREA (ACRES)	TOTAL AREA (ACRES)	EXCESS PRECIPITATION (IN)	V100 - 360 (CF)	V100 - 1440 (CF)	V100 - 4 DAY (CF)	V100 - 10 DAY (CF)
A1	0.01	0.11	0.12	2.203	959	1134	1373	1656
A2	0.02	0.08	0.1	2.024	735	880	1080	1316
A4	0	0.1	0.1	2.33	846	991	1191	1427
A5	0	0.08	0.08	2.33	677	793	953	1141
WEST BUILDING	0.02	0.06	0.08	1.948	566	682	841	1030
EAST BUILDING	0.02	0.08	0.1	2.024	735	880	1080	1316
FUTURE BUILDING	0.02	0.06	0.08	1.948	566	682	841	1030

TABLE 4 - DEVELOPED BASIN RUNOFF TABLE

BASIN	Q2 (CFS)	Q10 (CFS)	Q100 (CFS)
A1	0.18	0.31	0.50
A2	0.13	0.24	0.39
A4	0.17	0.27	0.43
A5	0.13	0.22	0.35
WEST BUILDING	0.10	0.18	0.31
EAST BUILDING	0.13	0.24	0.39
FUTURE BUILDING	0.10	0.18	0.31

TABLE 5 - DEVELOPED WATER QUALITY TABLE

BASIN SITE	PRIVATE IMPERVIOUS AREA (SF)	80TH PERCENTILE EVENT (IN)	VOLUME REQUIRED (CF)	VOLUME PROVIDED (CF)	FEE-IN LIEU OF VOLUME (CF)	FEE-IN LIEU OF AREA (SF)
23087	23087	0.26	500	0	500	23087

SEE SHEET C-201 FOR NORTH GRADING DETAIL

PROPOSED TRENCH DRAIN TO CAPTURE MONTEREY MOTEL FLOWS FROM ENTERING EL VADO PLACE PROPERTY

PROPOSED SPILL CURB & GUTTER

MATCH GRADE AT EXISTING SIDEWALK (TYP.)

PROPOSED 8-IN PVC ROOF DRAIN COLLECTOR (TYP.)

PROPOSED INLET A3 COA SINGLE C TYPE INLET WITH VALLEY PAN GRATE

PROPOSED 8-IN PVC STUB FOR FUTURE BUILDING ROOF DRAIN CONNECTIONS

PROPOSED 12-IN PVC ROOF DRAIN COLLECTOR LINE

PROPERTY LINE (TYP.)

BASIN A5

BASIN A4

BASIN WEST BUILDING

BASIN EAST BUILDING

BASIN A1

BASIN A2

BASIN A5

WEST BUILDING

EAST BUILDING

FUTURE BUILDING

ROOF DOWNSPOUT (TYP.) 8-IN NYOPLAST DROP GRATE INLET TO TIE INTO ROOF DRAIN TRUNK LINE WITH 8X8X8 PVC TEE

PROPOSED INLET A5 COA SINGLE C TYPE INLET WITH VALLEY PAN GRATE

PROPOSED 12-IN PVC STORM DRAIN LINE A SEE SHEET C-301

PROPOSED 8-IN PVC ROOF DRAIN COLLECTOR (TYP.)

PROPOSED INLET A4 COA SINGLE C TYPE INLET WITH VALLEY PAN GRATE

PAVE UP TO PROPOSED SIDEWALK

PROPOSED INLET A2 COA SINGLE C TYPE INLET WITH VALLEY PAN GRATE

EXISTING STORM INLET TO BE REMOVED EXISTING 6-IN PIPE TO BE CAPPED AND ABANDONED

SEE SHEET C-202 FOR SOUTH GRADING DETAIL

PROPOSED INLET A1 COA SINGLE C TYPE INLET WITH VALLEY PAN GRATE

PROPOSED 12-IN PVC STORM DRAIN (TYP.)

EXISTING 18-IN STORM DRAIN

PROPOSED TIE INTO EXISTING STORM MANHOLE TO BE CONSTRUCTED UNDER CITY WORK ORDER



2415 PRINCETON DR. NE, SUITE E  
 ALBUQUERQUE, NM 87107  
 505.843.7587  
 www.designplusllc.com

DESIGN PLUS LLC



MONTEREY MOTEL  
 2402 CENTRAL AVE SW  
 ALBUQUERQUE, NM 87104

PERMIT SUBMITTAL

DATE: 02/04/2020

REVISIONS

DRAWN BY: HR

CHECKED BY: PR

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SHEET TITLE

GRADING AND DRAINAGE PLAN

C-102

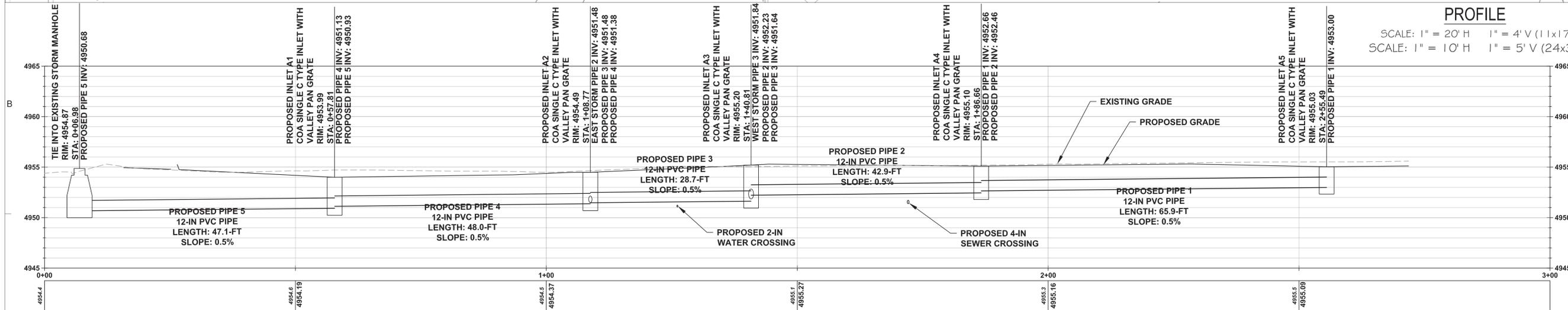
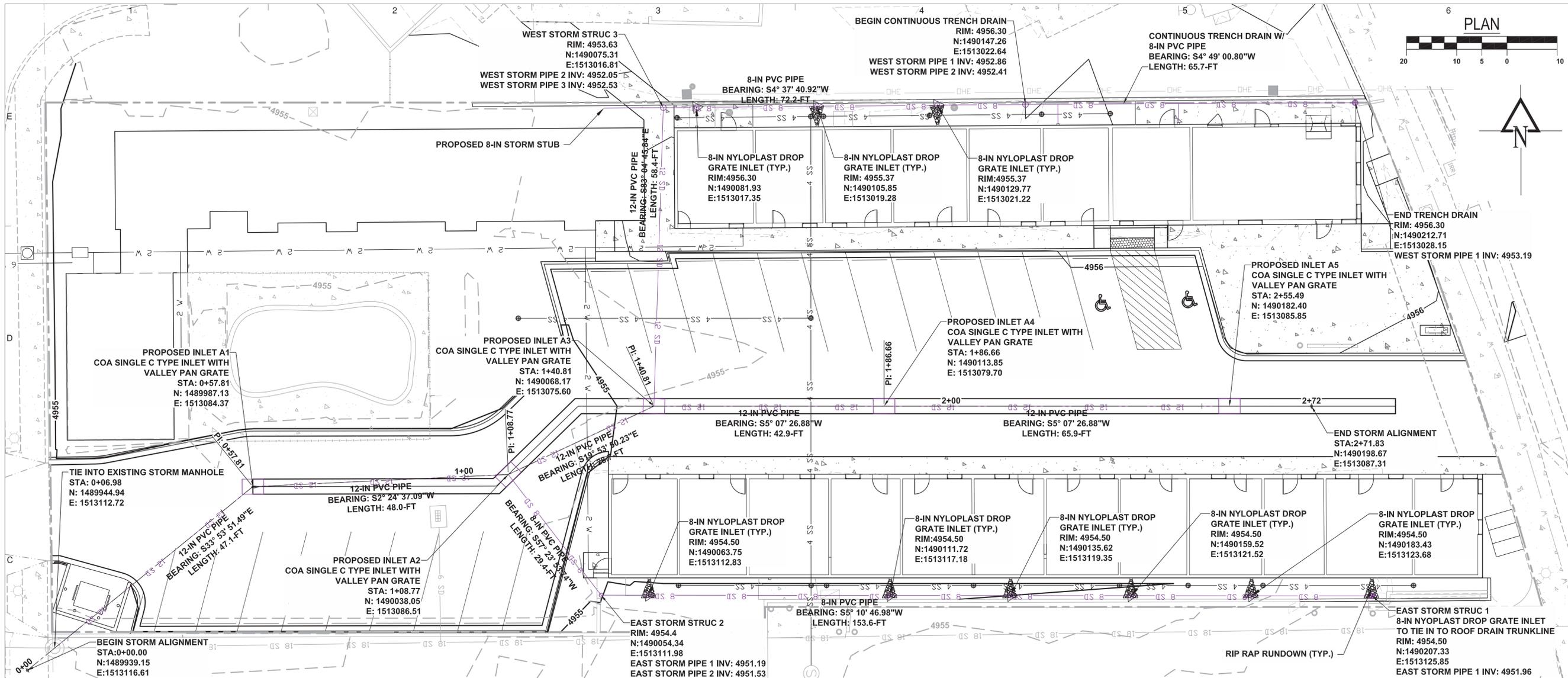
PREPARED BY:



**Short Elliott Hendrickson, Inc.**  
 934 Main Avenue, Unit C  
 Durango, Colorado 81301  
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 Fax: (970) 385-4502

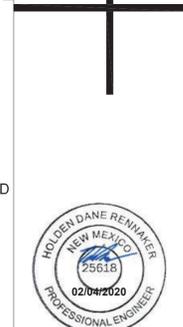






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MONTEREY MOTEL  
2402 CENTRAL AVE SW  
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PERMIT SUBMITTAL

DATE: 02/04/2020

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STORM P-PRO

**C-301**

**MONTEREY MOTEL**

**GRADING AND DRAINAGE PLAN  
SUPPLEMENTAL CALCULATIONS  
AND FINDINGS**

**ALBUQUERQUE, NM**



February 04, 2020

**Prepared by:**  
Short, Elliott, Hendrickson, Inc.  
934 Main Ave., Unit C  
Durango, CO 81301



Building a Better World  
for All of Us®

## **INTRODUCTION**

The Monterey Motel project is a proposed motel renovation and addition located at 2402 Central Avenue SW, Albuquerque, NM 87104. The project includes two existing single story buildings which will be renovated during Phase 1 to have a combined footprint of approximately 5,900-sf. Phase 2 of the project will have an additional building around the existing pool deck that will have a footprint of 2,805-sf.

The following report summarizes calculations and findings supplemental to the submitted Grading and Drainage Plan. The Grading and Drainage Plan highlights both the existing and proposed conditions and flow calculations for each basin. The following sections provide more detail into certain design elements of the drainage scheme.

## **STORMWATER CONVEYANCE**

Basins A1-A5 were all designed to surface flow to a concrete valley pan in the center of the drive aisle. The valley pan was designed to have a series of high and low points with a proposed inlet at each low point. These inlets were designed to be drained by a proposed 12-in storm drain trunk line. Proposed roof drain lines were also designed to tie into this trunk line system.

### Valley Pan Inlets

The *Hydraflow Express Extension for AutoCad Civil3D 2018* was used to model the proposed inlets to determine the water spread during the 100-year storm. The inlets were modelled per COA Type C Single Inlets in a sag condition. *Express* output is attached and shows that the maximum spread of these inlets during the highest 100-year flow for Basins A1-A5 (0.50-cfs) results in a spread of 6-ft – within the drive aisle.

### Storm Drain Trunk Line

The *Hydraflow Express Extension for AutoCad Civil3D 2018* was used to model the highest flow in the trunk line during the 100-year storm. The storm drain pipe was modelled as being 12-in PVC with a slope of 0.5%. *Express* output is attached and shows that the total developed flow of the site (2.63-cfs) flows through the pipe with a flow depth of 0.68-ft.

### Roof Drain Inlets

Each proposed roof drain was designed to flow into an 8-in Nyoplast Drop-In Grate Inlet. The maximum 100-year storm flow of a building basin (0.39 cfs for the existing East Building) was found to pool up just over 3-in from the grate rim – 0.75-ft below the finish floor elevation. This flow assumption was found to be conservative as this basin flow was designed to be split over several drop inlets.

### Roof Drain Collector

The *Hydraflow Express Extension for AutoCad Civil3D 2018* was used to model the proposed roof drain trunk line to verify the lines can adequately convey the 100-year storm. The pipe was modelled as an 8-in PVC line with 0.5% slope with a flow of 0.39-cfs (the largest building basin flow). The *Express* results attached show that the proposed lines can adequately convey the flow.

## **ATTACHMENTS**

- Output from *Hydraflow Express Extension for Civil 3D* (Valley Pan Inlet)
- Output from *Hydraflow Express Extension for Civil 3D* (Trunk Line Pipe)
- Design Sheet for Drop Grate Inlet for Nyoplast Drop Inlets
- Output from *Hydraflow Express Extension for Civil 3D* (Roof Drain Collector)

# Inlet Report

## MOMO Valley Pan Inlet

### Drop Grate Inlet

Location	= Sag
Curb Length (ft)	= -0-
Throat Height (in)	= -0-
Grate Area (sqft)	= 4.00
Grate Width (ft)	= 2.08
Grate Length (ft)	= 3.33

### Gutter

Slope, Sw (ft/ft)	= 0.040
Slope, Sx (ft/ft)	= 0.040
Local Depr (in)	= -0-
Gutter Width (ft)	= 3.00
Gutter Slope (%)	= -0-
Gutter n-value	= -0-

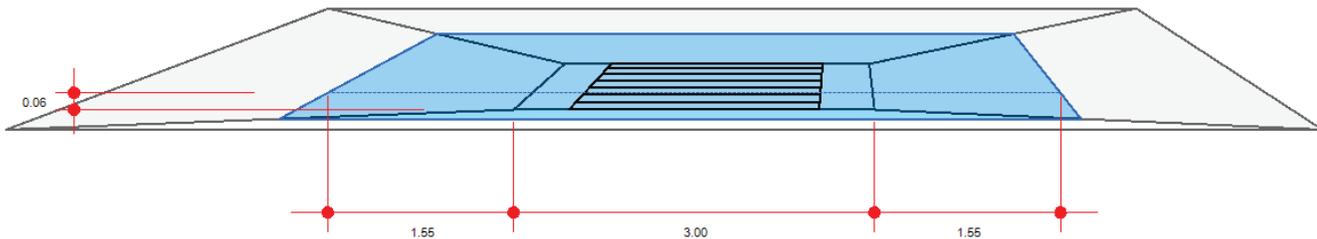
### Calculations

Compute by:	Known Q
Q (cfs)	= 0.50

### Highlighted

Q Total (cfs)	= 0.50
Q Capt (cfs)	= 0.50
Q Bypass (cfs)	= -0-
Depth at Inlet (in)	= 0.74
Efficiency (%)	= 100
Gutter Spread (ft)	= 6.09
Gutter Vel (ft/s)	= -0-
Bypass Spread (ft)	= -0-
Bypass Depth (in)	= -0-

All dimensions in feet



# Channel Report

## TRUNK LINE

### Circular

Diameter (ft) = 1.00

Invert Elev (ft) = 1.00

Slope (%) = 0.50

N-Value = 0.010

### Calculations

Compute by: Known Q

Known Q (cfs) = 2.63

### Highlighted

Depth (ft) = 0.68

Q (cfs) = 2.630

Area (sqft) = 0.57

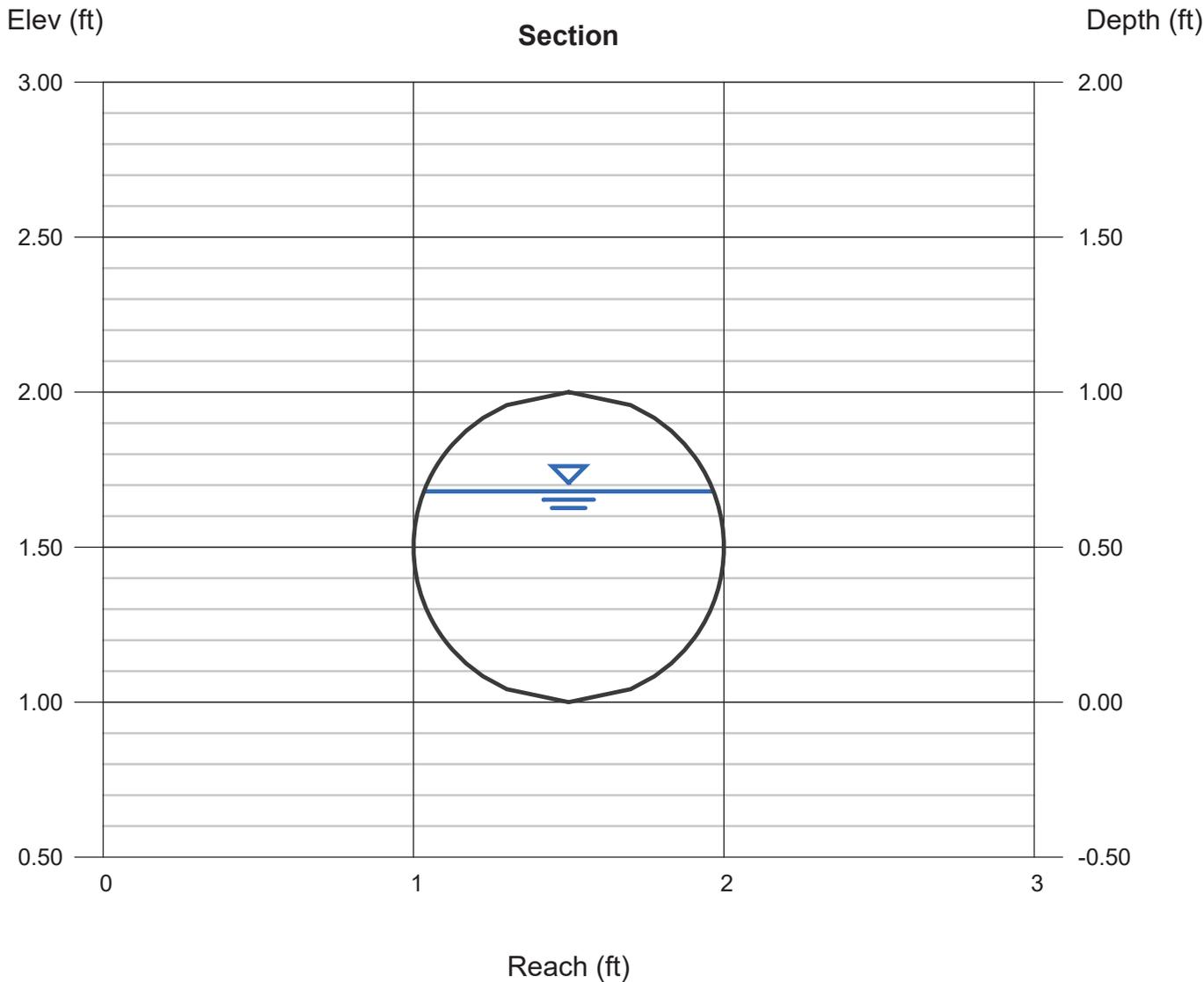
Velocity (ft/s) = 4.62

Wetted Perim (ft) = 1.94

Crit Depth, Yc (ft) = 0.70

Top Width (ft) = 0.93

EGL (ft) = 1.01



## Nyloplast Drop In Grates

### Applications

Nyloplast Drop In Grates are commonly used in non-traffic applications such as green spaces as well as atrium walkway areas. The Drop In grates are designed to fit most pipe types such as ADS N-12, PVC Sewer SDR35, PVC Schedule 40 and so on. They are light weight and easy to install while maintaining the durability of a ductile iron casting making them extremely versatile for an assortment of different applications.

### Specifications

Nyloplast Drop In Grates conform to ASTM A536 grade 70-50-05 for ductile iron castings. These grate designs are not load rated like some of our other casting designs and therefore should not be used in vehicular traffic applications.

### Inlet Capacity Information



6" Drop In Grate



8" Drop In Grate



10" Drop In Grate



12" Drop In Grate



15" Drop In Grate



18" Drop In Grate



24" Drop In Grate

NYLOPLAST INLET CAPACITY CHART DATA									
NYLOPLAST CASTINGS	GRATE SIZES	GRATE OPEN AREA (sq.in.)	PERIMETER OF GRATE OPENINGS (in.)	FLOW RATE AT DIFFERENT HEAD PRESSURES (cfs)				CHANGE OVER FROM WEIR FLOW TO ORIFICE FLOW	
				0.25'	0.50'	0.75'	1.00'	FLOW (cfs)	HEAD (ft)
				(3")	(6")	(9")	(12")		
DROP IN GRATES	6 IN	9.98	16.47	0.165	0.230	0.285	0.330	0.085	0.065
	8 IN	19.30	22.29	0.320	0.450	0.555	0.640	0.180	0.090
	10 IN	32.80	28.58	0.550	0.770	0.950	1.080	0.410	0.130
	12 IN	39.75	33.70	0.680	0.950	1.150	1.290	0.490	0.140
	15 IN	62.03	41.78	1.049	1.450	1.790	2.060	0.855	0.175
	18 IN	84.61	48.69	1.400	1.950	2.430	2.800	1.280	0.210
	24 IN	164.94	66.76	2.300	3.800	4.750	5.450	3.000	0.300

The Most **Advanced** Name in Drainage Systems®

# Channel Report

## ROOF DRAIN COLLECTOR

### Circular

Diameter (ft) = 0.67

Invert Elev (ft) = 1.00

Slope (%) = 0.50

N-Value = 0.010

### Calculations

Compute by: Known Q

Known Q (cfs) = 0.39

### Highlighted

Depth (ft) = 0.28

Q (cfs) = 0.390

Area (sqft) = 0.14

Velocity (ft/s) = 2.78

Wetted Perim (ft) = 0.94

Crit Depth,  $Y_c$  (ft) = 0.29

Top Width (ft) = 0.66

EGL (ft) = 0.40

