

CITY OF ALBUQUERQUE

Planning Department
Alan Varela, Interim Director



Mayor Timothy M. Keller

August 16, 2021

David Soule, P.E.
Rio Grande Engineering
P.O. Box 93924
Albuquerque, NM 87199

**RE: Gomez Ave Subdivision
Grading & Drainage Plan and Drainage Report
Engineer's Stamp Date: 06/17/21
Hydrology File: J14D097**

Dear Mr. Soule:

Based upon the information provided in your submittal received 06/18/2021, the Grading & Drainage Plan and Drainage Report are approved for Grading Permit and for action by the DRB on Preliminary Plat.

Prior to Pad Certification/Building Permit

The DRB needs to approve the Preliminary Plat prior to Hydrology's approval of Pad Certification and concurrent Building Permit

Prior to Permanent Release of Occupancy

The Final Plat needs to be recorded prior to Hydrology's approval of Permanent Release of Occupancy.

As a reminder, if the project total area of disturbance (including the staging area and any work within the adjacent Right-of-Way) is 1 acre or more, then an Erosion and Sediment Control (ESC) Plan and Owner's certified Notice of Intent (NOI) is required to be submitted to the Stormwater Quality Engineer (Doug Hughes, PE, jhughes@cabq.gov, 924-3420) 14 days prior to any earth disturbance.

If you have any questions, please contact me at 924-3995 or rbrissette@cabq.gov.

Sincerely,

Renée C. Brissette, P.E. CFM
Senior Engineer, Hydrology
Planning Department



City of Albuquerque

Planning Department
Development & Building Services Division

DRAINAGE AND TRANSPORTATION INFORMATION SHEET (REV 6/2018)

Project Title: GOMEZ SUDVISION **Building Permit #:** _____ **Hydrology File #:** _____
DRB#: _____ **EPC#:** _____ **Work Order#:** _____
Legal Description: LOTS A, B, C, D, E, F LANDS OF GONZALES SUBDIVISION
City Address: 402 GOMEZ

Applicant: _____ **Contact:** _____
Address: _____
Phone#: _____ **Fax#:** _____ **E-mail:** _____

Other Contact: RIO GRANDE ENGINEERING **Contact:** DAVID SOULE
Address: PO BOX 93924 ALB NM 87199
Phone#: 505.321.9099 **Fax#:** 505.872.0999 **E-mail:** david@riograndeengineering.com

TYPE OF DEVELOPMENT: _____ PLAT ☒ RESIDENCE _____ DRB SITE _____ ADMIN SITE
6-LOTS

Check all that Apply:

DEPARTMENT:

☒ HYDROLOGY/ DRAINAGE
_____ TRAFFIC/ TRANSPORTATION

TYPE OF SUBMITTAL:

_____ ENGINEER/ARCHITECT CERTIFICATION
_____ PAD CERTIFICATION
_____ CONCEPTUAL G & D PLAN
☒ GRADING PLAN
☒ DRAINAGE REPORT
_____ DRAINAGE MASTER PLAN
_____ FLOODPLAIN DEVELOPMENT PERMIT APPLIC
_____ ELEVATION CERTIFICATE
_____ CLOMR/LOMR
_____ TRAFFIC CIRCULATION LAYOUT (TCL)
_____ TRAFFIC IMPACT STUDY (TIS)
_____ STREET LIGHT LAYOUT
_____ OTHER (SPECIFY) _____
_____ PRE-DESIGN MEETING?

IS THIS A RESUBMITTAL?: _____ Yes ☒ No

TYPE OF APPROVAL/ACCEPTANCE SOUGHT:

☒ BUILDING PERMIT APPROVAL
_____ CERTIFICATE OF OCCUPANCY
_____ PRELIMINARY PLAT APPROVAL
_____ SITE PLAN FOR SUB'D APPROVAL
_____ SITE PLAN FOR BLDG. PERMIT APPROVAL
_____ FINAL PLAT APPROVAL
_____ SIA/ RELEASE OF FINANCIAL GUARANTEE
_____ FOUNDATION PERMIT APPROVAL
_____ GRADING PERMIT APPROVAL
_____ SO-19 APPROVAL
_____ PAVING PERMIT APPROVAL
_____ GRADING/ PAD CERTIFICATION
_____ WORK ORDER APPROVAL
_____ CLOMR/LOMR
_____ FLOODPLAIN DEVELOPMENT PERMIT
_____ OTHER (SPECIFY) _____

DATE SUBMITTED: _____ **By:** _____

COA STAFF:

ELECTRONIC SUBMITTAL RECEIVED: _____

FEE PAID: _____

DRAINAGE REPORT

For

**LOTS A,B,C,D,E,F
GONZALES SUBDIVISION
Albuquerque, New Mexico**

Prepared by

Rio Grande Engineering
PO Box 93924
Albuquerque, New Mexico 87199



6/18/21

JUNE 2021

David Soule P.E. No. 14522

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Site Hydrology	A
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Map

Site Grading and Drainage Plan

PURPOSE

The purpose of this report is to provide the Drainage Management Plan for the development of a 0.8 acre, 6 lot subdivision located on Gomez avenue between Broadway and Edith NE. This plan was prepared in accordance with the City of Albuquerque design regulations, utilizing the City of Albuquerque's Development Process Manual drainage guidelines. This report will demonstrate that the grading does not adversely affect the surrounding properties, nor the upstream or downstream facilities.

INTRODUCTION

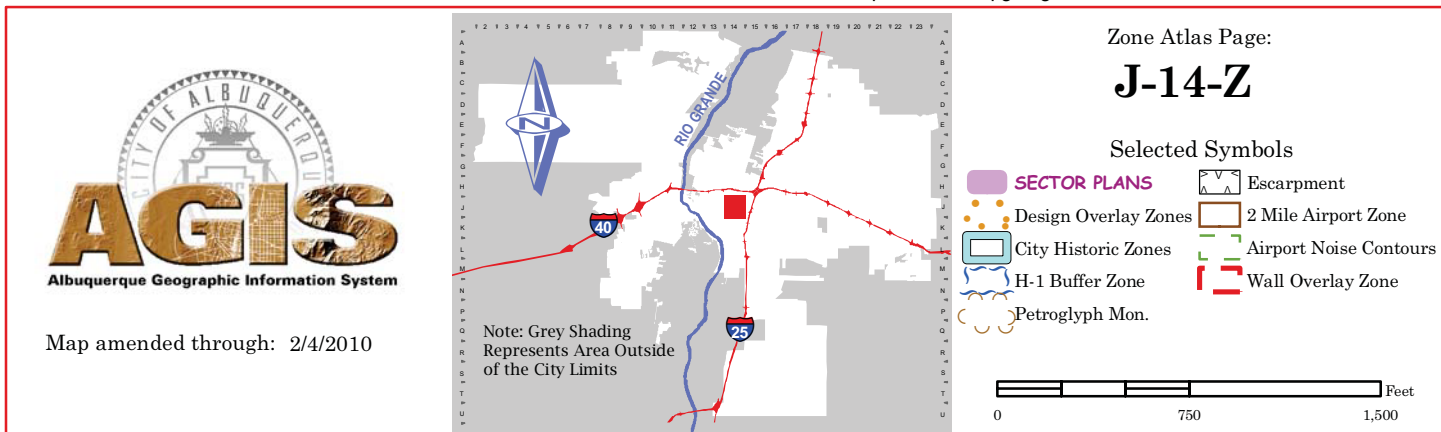
The subject of this report, as shown on the Exhibit A, is a 0.8-acre parcel of land located Gomez avenue between Broadway and Edith NE. The legal description of this site is Lots A,B,C,D,E,F GONZLES SUBDIVISION. As shown on FIRM map35001C0332G, the entire site is located within Flood Zone X. The site is an existing fully developed lot. The site contains structured, landscaping, trees and hard packed paths. The site is surrounded by fully developed sites on all sides with walls on the south side, west and east. The site currently free discharges as sheet flow to the adjacent Gomez Avenue roadway. The development of the site will require the site to discharge at a rate equal to or less than the existing conditions and retain the first flush water quality volume onsite.

EXISTING CONDITIONS

The site is currently developed and not impacted by upland flows. The site is located in flood zone x. The site currently discharges 2.23 cfs as sheet flow to the site to the north. All downstream improvements are in place and maintained by the city of Albuquerque.



For more current information and more details visit: <http://www.cabq.gov/gis>



PROPOSED CONDITIONS

The proposed improvements consist of a new 6 lot subdivision. Each lot will retain in excess of the first flush amount and discharge to Gomez. Each lots discharge is being throttled by introducing a wall and 4" outlet pipe. A summary of the proposed basin data is included in appendix A. The combined discharge from the site will be 2.23 cfs, while retaining 2106 cf. The site drainage and detention pond functions were modeled using ahymo, which is found in appendix B. The stage-storage data utilized in the model is included in appendix b .

SUMMARY AND RECOMMENDATIONS

This project is a development of single family residential infill redevelopment within the fully developed near northeast watershed. The development of this site will retain the first flush volume onsite. The site will discharge 2.23 cfs which is matching existing conditions of 2.23 cfs via 4" pvc drains in a front yard. The site will outfall over the wall and out the driveway to Gomez as an emergency overflow. The drainage structures have been adequately sized. The development of this site will not negatively impact the upstream nor down stream facilities. Since this site does not exceed 1 acre, an erosion and sediment Control Plan may not be required.

APPENDIX A
SITE HYDROLOGY

BASIN DATA

Basin	Area (sf)	Area (acres)	Treatment A		Treatment B		Treatment C		Treatment D		Q100 GENERATED	Q100 DISCHARGED	FIRST FLUSH REQUIRED	FIRST FLUSH PROVIDED	MWESL			
			%	(acres)	%	(acres)	%	(acres)	%	(acres)								
EXISTING LOT	35174.00	0.807	0%	0	36%	0.291	50%	0.4037	14%	0.113	2.23	2.23						
	5672.00	0.130	0%	0	26%	0.034	25%	0.0326	55%	0.072	0.45	0.36	125	350	70.10			
	6120.00	0.140	0%	0	20%	0.028	29%	0.0407	51%	0.072	0.48	0.35	125	403	70.09			
	6111.00	0.140	0%	0	20%	0.028	29%	0.0407	51%	0.072	0.48	0.39	125	342	70.34			
	6103.00	0.140	0%	0	20%	0.028	29%	0.0406	51%	0.071	0.48	0.39	125	370	70.84			
	5763.00	0.132	0%	0	20%	0.026	26%	0.0344	54%	0.071	0.46	0.39	124	327	71.41			
LOT F	5405.00	0.124	0%	0	20%	0.025	22%	0.0273	58%	0.072	0.44	0.35	125	314	72.25			
TOTAL PROPOSED															2.23	749	2106	

Narrative

The subject property is located within a fully developed area of NE Albuquerque. All down stream drainage improvements have been completed. This is a redevelopment of an site that discharges 2.23 cfs to the Gomez roadway. The proposed improvements consist of 6 lots, Each lot retains the first flush then discharges through a 4" pipe to gomez. The detention ponds on each lot reduce the com total discharge to 2.23 cfs, matching existing while retainin in excess the required water quality volume. Each lot will flow out the driveway in the event of clogging or storm greater than the design event

APPENDIX B
HYDRAULIC CALCULATIONS
AND AHYMO MODEL

STAGE/STORAGE VOLUME CALCULATIONS

POND A

OUTLET

ACTUAL ELEV.	DEPTH (FT)	AREA SF	VOLUME PER UNIT	VOLUME CUMULATIVE	VOLUME AC-FT	Q (CFS)
69.00	0.00	135.00	0	169	0.004	0.00
69.25	0.00	228.00	181.50	350.50	0.0080	0.00
70.25	1.00	816.00	182.70	533.20	0.012	0.42

Orifice Equation

$Q = CA \sqrt{2gH}$

C = 0.6

Diameter (in) 4

Area (ft²)= 0.087266463

g = 32.2

H (Ft) = Depth of water above center of orifice

Q (CFS)= Flow

STAGE/STORAGE VOLUME CALCULATIONS

POND B

OUTLET

ACTUAL ELEV.	DEPTH (FT)	AREA SF	VOLUME PER UNIT	VOLUME CUMULATIVE	VOLUME AC-FT	Q (CFS)
69.00	0.00	179.00	0	169	0.004	0.00
69.25	0.00	290.00	234.50	403.50	0.0093	0.00
70.25	1.00	847.00	198.98	602.48	0.014	0.42

Orifice Equation

Q = CA SQRT(2gH)

C = 0.6
Diameter (in) 4
Area (ft^2)= 0.087266463
g = 32.2
H (Ft) = Depth of water above center of orifice
Q (CFS)= Flow

STAGE/STORAGE VOLUME CALCULATIONS

POND C

OUTLET

ACTUAL ELEV.	DEPTH (FT)	AREA SF	VOLUME PER UNIT	VOLUME CUMULATIVE	VOLUME AC-FT	Q (CFS)
69.00	0.00	107.00	0	169	0.004	0.00
69.35	0.00	239.00	173.00	342.00	0.0079	0.00
70.50	1.15	777.00	177.80	519.80	0.012	0.45

Orifice Equation

$Q = CA \sqrt{2gH}$

C = 0.6

Diameter (in) 4

Area (ft²)= 0.087266463

g = 32.2

H (Ft) = Depth of water above center of orifice

Q (CFS)= Flow

STAGE/STORAGE VOLUME CALCULATIONS

POND D

OUTLET

ACTUAL ELEV.	DEPTH (FT)	AREA SF	VOLUME PER UNIT	VOLUME CUMULATIVE	VOLUME AC-FT	Q (CFS)
69.50	0.00	144.00	0	169	0.004	0.00
69.85	0.00	258.00	201.00	370.00	0.0085	0.00
71.00	1.15	668.00	162.05	532.05	0.012	0.45

Orifice Equation

$Q = CA \sqrt{2gH}$

C = 0.6

Diameter (in) 4

Area (ft²)= 0.087266463

g = 32.2

H (Ft) = Depth of water above center of orifice

Q (CFS)= Flow

STAGE/STORAGE VOLUME CALCULATIONS

POND E

OUTLET

ACTUAL ELEV.	DEPTH (FT)	AREA SF	VOLUME PER UNIT	VOLUME CUMULATIVE	VOLUME AC-FT	Q (CFS)
70.00	0.00	78.00	0	169	0.004	0.00
70.45	0.00	238.00	158.00	327.00	0.0075	0.00
71.50	1.05	645.00	154.53	481.53	0.011	0.43

Orifice Equation

$Q = CA \sqrt{2gH}$

C = 0.6

Diameter (in) 4

Area (ft²)= 0.087266463

g = 32.2

H (Ft) = Depth of water above center of orifice

Q (CFS)= Flow

STAGE/STORAGE VOLUME CALCULATIONS

POND F

OUTLET

ACTUAL ELEV.	DEPTH (FT)	AREA SF	VOLUME PER UNIT	VOLUME CUMULATIVE	VOLUME AC-FT	Q (CFS)
71.00	0.00	98.00	0	169	0.004	0.00
71.35	0.00	192.00	145.00	314.00	0.0072	0.00
72.50	1.15	761.00	166.78	480.78	0.011	0.45

Orifice Equation

$$Q = CA \sqrt{2gH}$$

$$C = 0.6$$

$$\text{Diameter (in)} = 4$$

$$\text{Area (ft}^2\text{)} = 0.087266463$$

$$g = 32.2$$

$$H \text{ (Ft)} = \text{Depth of water above center of orifice}$$

$$Q \text{ (CFS)} = \text{Flow}$$

AHYMO PROGRAM (AHYMO-S4)

- Version: S4.01a - Rel: 01a

RUN DATE (MON/DAY/YR) = 06/18/2021

START TIME (HR:MIN:SEC) = 06:52:53

USER NO.=

RioGrandeSingleA41963517

INPUT FILE = uments and Settings\Owner\Desktop\2021 JOBS\2021175-402

gomez\pondrout061721.txt

*S AHYMO - DETENTION-GOMEZ APTS
 *S POND ROUTING

START TIME=0.0 PUNCH CODE=0

RAINFALL TYPE=2
 QUARTER=0.0 ONE= 1.74 IN
 SIX=2.21 IN DAY= 3.49 IN DT = 0.05 HR

24-HOUR RAINFALL DIST. - BASED ON NOAA ATLAS 14 FOR CONVECTIVE

AREAS

(NM & AZ) - D1

DT = 0.050000 HOURS			END TIME = 24.000002 HOURS			
0.0000	0.0031	0.0064	0.0098	0.0135	0.0174	0.0218
0.0280	0.0377	0.0481	0.0590	0.0707	0.0827	0.0950
0.1077	0.1206	0.1350	0.1499	0.1661	0.1883	0.2134
0.2471	0.2853	0.3325	0.3955	0.4663	0.5886	0.7787
1.1045	1.3335	1.5140	1.6048	1.6843	1.7415	1.7869
1.8266	1.8556	1.8822	1.9042	1.9216	1.9368	1.9504
1.9632	1.9745	1.9852	1.9957	2.0057	2.0140	2.0187
2.0233	2.0278	2.0321	2.0363	2.0404	2.0444	2.0484
2.0521	2.0558	2.0595	2.0631	2.0666	2.0700	2.0733
2.0766	2.0798	2.0830	2.0861	2.0891	2.0921	2.0951
2.0980	2.1008	2.1037	2.1065	2.1093	2.1120	2.1147
2.1174	2.1200	2.1226	2.1252	2.1277	2.1303	2.1328
2.1352	2.1377	2.1401	2.1425	2.1448	2.1472	2.1495
2.1518	2.1540	2.1563	2.1585	2.1607	2.1629	2.1651
2.1672	2.1693	2.1715	2.1735	2.1756	2.1777	2.1797
2.1817	2.1837	2.1857	2.1876	2.1896	2.1915	2.1934
2.1953	2.1972	2.1991	2.2009	2.2028	2.2046	2.2064
2.2082	2.2100	2.2136	2.2171	2.2207	2.2242	2.2278
2.2313	2.2349	2.2384	2.2420	2.2456	2.2491	2.2527
2.2562	2.2598	2.2633	2.2669	2.2704	2.2740	2.2776
2.2811	2.2847	2.2882	2.2918	2.2953	2.2989	2.3024
2.3060	2.3096	2.3131	2.3167	2.3202	2.3238	2.3273
2.3309	2.3344	2.3380	2.3416	2.3451	2.3487	2.3522
2.3558	2.3593	2.3629	2.3664	2.3700	2.3736	2.3771
2.3807	2.3842	2.3878	2.3913	2.3949	2.3984	2.4020
2.4056	2.4091	2.4127	2.4162	2.4198	2.4233	2.4269
2.4304	2.4340	2.4376	2.4411	2.4447	2.4482	2.4518
2.4553	2.4589	2.4624	2.4660	2.4696	2.4731	2.4767
2.4802	2.4838	2.4873	2.4909	2.4944	2.4980	2.5016
2.5051	2.5087	2.5122	2.5158	2.5193	2.5229	2.5264
2.5300	2.5336	2.5371	2.5407	2.5442	2.5478	2.5513
2.5549	2.5584	2.5620	2.5656	2.5691	2.5727	2.5762
2.5798	2.5833	2.5869	2.5904	2.5940	2.5976	2.6011
2.6047	2.6082	2.6118	2.6153	2.6189	2.6224	2.6260
2.6296	2.6331	2.6367	2.6402	2.6438	2.6473	2.6509
2.6544	2.6580	2.6616	2.6651	2.6687	2.6722	2.6758
2.6793	2.6829	2.6864	2.6900	2.6936	2.6971	2.7007
2.7042	2.7078	2.7113	2.7149	2.7184	2.7220	2.7256
2.7291	2.7327	2.7362	2.7398	2.7433	2.7469	2.7504
2.7540	2.7576	2.7611	2.7647	2.7682	2.7718	2.7753
2.7789	2.7824	2.7860	2.7896	2.7931	2.7967	2.8002
2.8038	2.8073	2.8109	2.8144	2.8180	2.8216	2.8251
2.8287	2.8322	2.8358	2.8393	2.8429	2.8464	2.8500
2.8536	2.8571	2.8607	2.8642	2.8678	2.8713	2.8749
2.8784	2.8820	2.8856	2.8891	2.8927	2.8962	2.8998

AHYMO.OUT						
2.9033	2.9069	2.9104	2.9140	2.9176	2.9211	2.9247
2.9282	2.9318	2.9353	2.9389	2.9424	2.9460	2.9496
2.9531	2.9567	2.9602	2.9638	2.9673	2.9709	2.9744
2.9780	2.9816	2.9851	2.9887	2.9922	2.9958	2.9993
3.0029	3.0064	3.0100	3.0136	3.0171	3.0207	3.0242
3.0278	3.0313	3.0349	3.0384	3.0420	3.0456	3.0491
3.0527	3.0562	3.0598	3.0633	3.0669	3.0704	3.0740
3.0776	3.0811	3.0847	3.0882	3.0918	3.0953	3.0989
3.1024	3.1060	3.1096	3.1131	3.1167	3.1202	3.1238
3.1273	3.1309	3.1344	3.1380	3.1416	3.1451	3.1487
3.1522	3.1558	3.1593	3.1629	3.1664	3.1700	3.1736
3.1771	3.1807	3.1842	3.1878	3.1913	3.1949	3.1984
3.2020	3.2056	3.2091	3.2127	3.2162	3.2198	3.2233
3.2269	3.2304	3.2340	3.2376	3.2411	3.2447	3.2482
3.2518	3.2553	3.2589	3.2624	3.2660	3.2696	3.2731
3.2767	3.2802	3.2838	3.2873	3.2909	3.2944	3.2980
3.3016	3.3051	3.3087	3.3122	3.3158	3.3193	3.3229
3.3264	3.3300	3.3336	3.3371	3.3407	3.3442	3.3478
3.3513	3.3549	3.3584	3.3620	3.3655	3.3691	3.3727
3.3762	3.3798	3.3833	3.3869	3.3904	3.3940	3.3975
3.4011	3.4047	3.4082	3.4118	3.4153	3.4189	3.4224
3.4260	3.4295	3.4331	3.4367	3.4402	3.4438	3.4473
3.4509	3.4544	3.4580	3.4615	3.4651	3.4687	3.4722
3.4758	3.4793	3.4829	3.4864	3.4900		

*Basin HISTORICAL
COMPUTE NM HYD

ID=1 HYD NO=101 DA= .001262 SQ MI
PER A=0 PER B=36 PER C=50 PER D=14
TP=-.138 MASSRAIN=-1

K = 0.075210HR TP = 0.138000HR K/TP RATIO = 0.545000 SHAPE

CONSTANT, N = 7.106428
UNIT PEAK = 0.67379 CFS UNIT VOLUME = 0.9842 B = 526.28

P60 = 1.7400
AREA = 0.000177 SQ MI IA = 0.10000 INCHES INF = 0.04000
INCHES

PER HOUR
RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
0.050000

K = 0.120597HR TP = 0.138000HR K/TP RATIO = 0.873888 SHAPE

CONSTANT, N = 4.063882
UNIT PEAK = 2.8266 CFS UNIT VOLUME = 0.9965 B = 359.41

P60 = 1.7400
AREA = 0.001085 SQ MI IA = 0.41279 INCHES INF = 1.00581
INCHES

PER HOUR
RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
0.050000

PRINT HYD ID=1 CODE=3

PARTIAL HYDROGRAPH 101.00

TIME	TIME	FLOW	TIME	FLOW	TIME	FLOW
	FLOW	TIME	FLOW			
HRS	HRS	CFS	HRS	CFS	HRS	CFS

AHYMO.OUT						
	CFS		HRS	CFS		
	0.000		0.0	4.950	0.0	9.900 0.0
14.850	0.0	19.800	0.0	5.100	0.0	10.050 0.0
	0.150	0.0				
15.000	0.0	19.950	0.0	5.250	0.0	10.200 0.0
	0.300	0.0				
15.150	0.0	20.100	0.0	5.400	0.0	10.350 0.0
	0.450	0.0				
15.300	0.0	20.250	0.0	5.550	0.0	10.500 0.0
	0.600	0.0				
15.450	0.0	20.400	0.0	5.700	0.0	10.650 0.0
	0.750	0.0				
15.600	0.0	20.550	0.0	5.850	0.0	10.800 0.0
	0.900	0.0				
15.750	0.0	20.700	0.0	6.000	0.0	10.950 0.0
	1.050	0.0				
15.900	0.0	20.850	0.0	6.150	0.0	11.100 0.0
	1.200	0.1				
16.050	0.0	21.000	0.0	6.300	0.0	11.250 0.0
	1.350	0.5				
16.200	0.0	21.150	0.0	6.450	0.0	11.400 0.0
	1.500	2.2				
16.350	0.0	21.300	0.0	6.600	0.0	11.550 0.0
	1.650	1.4				
16.500	0.0	21.450	0.0	6.750	0.0	11.700 0.0
	1.800	0.5				
16.650	0.0	21.600	0.0	6.900	0.0	11.850 0.0
	1.950	0.3				
16.800	0.0	21.750	0.0	7.050	0.0	12.000 0.0
	2.100	0.2				
16.950	0.0	21.900	0.0	7.200	0.0	12.150 0.0
	2.250	0.1				
17.100	0.0	22.050	0.0	7.350	0.0	12.300 0.0
	2.400	0.1				
17.250	0.0	22.200	0.0	7.500	0.0	12.450 0.0
	2.550	0.0				
17.400	0.0	22.350	0.0	7.650	0.0	12.600 0.0
	2.700	0.0				
17.550	0.0	22.500	0.0	7.800	0.0	12.750 0.0
	2.850	0.0				
17.700	0.0	22.650	0.0	7.950	0.0	12.900 0.0
	3.000	0.0				
17.850	0.0	22.800	0.0	8.100	0.0	13.050 0.0
	3.150	0.0				
18.000	0.0	22.950	0.0	8.250	0.0	13.200 0.0
	3.300	0.0				
18.150	0.0	23.100	0.0	8.400	0.0	13.350 0.0
	3.450	0.0				

				AHYMO.OUT			
18.300	0.0		23.250	0.0			
	3.600	0.0		8.550	0.0	13.500	0.0
18.450	0.0		23.400	0.0			
	3.750	0.0		8.700	0.0	13.650	0.0
18.600	0.0		23.550	0.0			
	3.900	0.0		8.850	0.0	13.800	0.0
18.750	0.0		23.700	0.0			
	4.050	0.0		9.000	0.0	13.950	0.0
18.900	0.0		23.850	0.0			
	4.200	0.0		9.150	0.0	14.100	0.0
19.050	0.0		24.000	0.0			
	4.350	0.0		9.300	0.0	14.250	0.0
19.200	0.0		24.150	0.0			
	4.500	0.0		9.450	0.0	14.400	0.0
19.350	0.0						
	4.650	0.0		9.600	0.0	14.550	0.0
19.500	0.0						
	4.800	0.0		9.750	0.0	14.700	0.0
19.650	0.0						

RUNOFF VOLUME = 1.21752 INCHES = 0.0819 ACRE-FEET
 PEAK DISCHARGE RATE = 2.23 CFS AT 1.500 HOURS BASIN AREA =
 0.0013
 SQ. MI.

*basin A
 COMPUTE NM HYD ID=2 HYD NO=102 DA= .000203 SQ MI
 PER A=0 PER B=20 PER C=25 PER D=55
 TP=-.138 MASSRAIN=-1

K = 0.075210HR TP = 0.138000HR K/TP RATIO = 0.545000 SHAPE
 CONSTANT, N = 7.106428
 UNIT PEAK = 0.42579 CFS UNIT VOLUME = 0.9762 B = 526.28

P60 = 1.7400
 AREA = 0.000112 SQ MI IA = 0.10000 INCHES INF = 0.04000
 INCHES

PER HOUR
 RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
 0.050000

K = 0.121369HR TP = 0.138000HR K/TP RATIO = 0.879486 SHAPE
 CONSTANT, N = 4.036064
 UNIT PEAK = 0.23669 CFS UNIT VOLUME = 0.9509 B = 357.57

P60 = 1.7400
 AREA = 0.000091 SQ MI IA = 0.41667 INCHES INF = 1.01667
 INCHES

PER HOUR
 RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =

0.050000

PRINT HYD

ID=2 CODE=3

PARTIAL HYDROGRAPH 102.00

TIME	TIME	FLOW	TIME	FLOW	TIME	FLOW
	FLOW	TIME	FLOW			
HRS	HRS	CFS	HRS	CFS	HRS	CFS
	CFS	HRS	CFS			
	0.000	0.0	4.950	0.0	9.900	0.0
14.850	0.0	19.800	0.0			
	0.150	0.0	5.100	0.0	10.050	0.0
15.000	0.0	19.950	0.0			
	0.300	0.0	5.250	0.0	10.200	0.0
15.150	0.0	20.100	0.0			
	0.450	0.0	5.400	0.0	10.350	0.0
15.300	0.0	20.250	0.0			
	0.600	0.0	5.550	0.0	10.500	0.0
15.450	0.0	20.400	0.0			
	0.750	0.0	5.700	0.0	10.650	0.0
15.600	0.0	20.550	0.0			
	0.900	0.0	5.850	0.0	10.800	0.0
15.750	0.0	20.700	0.0			
	1.050	0.0	6.000	0.0	10.950	0.0
15.900	0.0	20.850	0.0			
	1.200	0.0	6.150	0.0	11.100	0.0
16.050	0.0	21.000	0.0			
	1.350	0.1	6.300	0.0	11.250	0.0
16.200	0.0	21.150	0.0			
	1.500	0.5	6.450	0.0	11.400	0.0
16.350	0.0	21.300	0.0			
	1.650	0.3	6.600	0.0	11.550	0.0
16.500	0.0	21.450	0.0			
	1.800	0.1	6.750	0.0	11.700	0.0
16.650	0.0	21.600	0.0			
	1.950	0.1	6.900	0.0	11.850	0.0
16.800	0.0	21.750	0.0			
	2.100	0.0	7.050	0.0	12.000	0.0
16.950	0.0	21.900	0.0			
	2.250	0.0	7.200	0.0	12.150	0.0
17.100	0.0	22.050	0.0			
	2.400	0.0	7.350	0.0	12.300	0.0
17.250	0.0	22.200	0.0			
	2.550	0.0	7.500	0.0	12.450	0.0
17.400	0.0	22.350	0.0			
	2.700	0.0	7.650	0.0	12.600	0.0
17.550	0.0	22.500	0.0			
	2.850	0.0	7.800	0.0	12.750	0.0

AHYMO.OUT						
17.700	0.0	22.650	0.0	0.0	12.900	0.0
17.850	0.0	22.800	0.0	0.0	13.050	0.0
18.000	0.0	22.950	0.0	0.0	13.200	0.0
18.150	0.0	23.100	0.0	0.0	13.350	0.0
18.300	0.0	23.250	0.0	0.0	13.500	0.0
18.450	0.0	23.400	0.0	0.0	13.650	0.0
18.600	0.0	23.550	0.0	0.0	13.800	0.0
18.750	0.0	23.700	0.0	0.0	13.950	0.0
18.900	0.0	23.850	0.0	0.0	14.100	0.0
19.050	0.0	24.000	0.0	0.0	14.250	0.0
19.200	0.0	24.150	0.0	0.0	14.400	0.0
19.350	0.0			0.0	14.550	0.0
19.500	0.0			0.0	14.700	0.0
19.650	0.0					

RUNOFF VOLUME = 2.17882 INCHES = 0.0236 ACRE-Feet
 PEAK DISCHARGE RATE = 0.45 CFS AT 1.500 HOURS BASIN AREA =
 0.0002
 SQ. MI.

* ROUTE BASIN A THRU DETENTION

* ROUTE THE TOTAL FLOW THROUGH THE PROPOSED RESERVOIR
 ROUTE RESERVOIR ID=3 HYD NO=103 INFLOW=2 CODE=3
 OUTFLOW(CFS) STORAGE(AC-FT) ELEV(FT)
 0.00 0.008 69.25
 0.42 0.012 70.25

* * * * *

TIME (HRS)	INFLOW (CFS)	ELEV (FEET)	VOLUME (AC-FT)	OUTFLOW (CFS)
0.00	0.00	69.25	0.008	0.00
0.15	0.00	69.25	0.008	0.00
0.30	0.00	69.25	0.008	0.00
0.45	0.00	69.25	0.008	0.00
0.60	0.00	69.25	0.008	0.00
0.75	0.00	69.25	0.008	0.00

AHYMO.OUT

0.90	0.01	69.27	0.008	0.01
1.05	0.02	69.29	0.008	0.02
1.20	0.05	69.33	0.008	0.03
1.35	0.14	69.42	0.009	0.07
1.50	0.45	69.88	0.011	0.26
1.65	0.29	70.07	0.011	0.35
1.80	0.13	69.79	0.010	0.22
1.95	0.07	69.55	0.009	0.13
2.10	0.04	69.42	0.009	0.07
2.25	0.02	69.34	0.008	0.04
2.40	0.01	69.30	0.008	0.02
2.55	0.01	69.28	0.008	0.01
2.70	0.00	69.27	0.008	0.01
2.85	0.00	69.26	0.008	0.00

PEAK DISCHARGE = 0.357 CFS - PEAK OCCURS AT HOUR 1.60
 MAXIMUM WATER SURFACE ELEVATION = 70.100
 MAXIMUM STORAGE = 0.0114 AC-FT INCREMENTAL TIME= 0.050000HRS

*basin B
 COMPUTE NM HYD ID=4 HYD NO=104 DA= .000220 SQ MI
 PER A=0 PER B=20 PER C=29 PER D=51
 TP=-.138 MASSRAIN=-1

K = 0.075210HR TP = 0.138000HR K/TP RATIO = 0.545000 SHAPE
 CONSTANT, N = 7.106428
 UNIT PEAK = 0.42788 CFS UNIT VOLUME = 0.9762 B = 526.28
 P60 = 1.7400
 AREA = 0.000112 SQ MI IA = 0.10000 INCHES INF = 0.04000
 INCHES
 PER HOUR
 RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
 0.050000

K = 0.120284HR TP = 0.138000HR K/TP RATIO = 0.871626 SHAPE
 CONSTANT, N = 4.075250
 UNIT PEAK = 0.28134 CFS UNIT VOLUME = 0.9582 B = 360.16
 P60 = 1.7400
 AREA = 0.000108 SQ MI IA = 0.41122 INCHES INF = 1.00143
 INCHES
 PER HOUR
 RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
 0.050000

PRINT HYD ID=4 CODE=3

PARTIAL HYDROGRAPH 104.00

TIME	TIME	FLOW	TIME	FLOW	TIME	FLOW
	FLOW	TIME	FLOW			
HRS	HRS	CFS	HRS	CFS	HRS	CFS
	CFS	HRS	CFS			
	0.000	0.0	4.950	0.0	9.900	0.0
14.850	0.0		19.800	0.0		
	0.150	0.0	5.100	0.0	10.050	0.0
15.000	0.0		19.950	0.0		

			AHYMO.OUT			
	0.300	0.0	5.250	0.0	10.200	0.0
15.150	0.0	20.100	0.0			
	0.450	0.0	5.400	0.0	10.350	0.0
15.300	0.0	20.250	0.0			
	0.600	0.0	5.550	0.0	10.500	0.0
15.450	0.0	20.400	0.0			
	0.750	0.0	5.700	0.0	10.650	0.0
15.600	0.0	20.550	0.0			
	0.900	0.0	5.850	0.0	10.800	0.0
15.750	0.0	20.700	0.0			
	1.050	0.0	6.000	0.0	10.950	0.0
15.900	0.0	20.850	0.0			
	1.200	0.0	6.150	0.0	11.100	0.0
16.050	0.0	21.000	0.0			
	1.350	0.1	6.300	0.0	11.250	0.0
16.200	0.0	21.150	0.0			
	1.500	0.5	6.450	0.0	11.400	0.0
16.350	0.0	21.300	0.0			
	1.650	0.3	6.600	0.0	11.550	0.0
16.500	0.0	21.450	0.0			
	1.800	0.1	6.750	0.0	11.700	0.0
16.650	0.0	21.600	0.0			
	1.950	0.1	6.900	0.0	11.850	0.0
16.800	0.0	21.750	0.0			
	2.100	0.0	7.050	0.0	12.000	0.0
16.950	0.0	21.900	0.0			
	2.250	0.0	7.200	0.0	12.150	0.0
17.100	0.0	22.050	0.0			
	2.400	0.0	7.350	0.0	12.300	0.0
17.250	0.0	22.200	0.0			
	2.550	0.0	7.500	0.0	12.450	0.0
17.400	0.0	22.350	0.0			
	2.700	0.0	7.650	0.0	12.600	0.0
17.550	0.0	22.500	0.0			
	2.850	0.0	7.800	0.0	12.750	0.0
17.700	0.0	22.650	0.0			
	3.000	0.0	7.950	0.0	12.900	0.0
17.850	0.0	22.800	0.0			
	3.150	0.0	8.100	0.0	13.050	0.0
18.000	0.0	22.950	0.0			
	3.300	0.0	8.250	0.0	13.200	0.0
18.150	0.0	23.100	0.0			
	3.450	0.0	8.400	0.0	13.350	0.0
18.300	0.0	23.250	0.0			
	3.600	0.0	8.550	0.0	13.500	0.0
18.450	0.0	23.400	0.0			
	3.750	0.0	8.700	0.0	13.650	0.0
18.600	0.0	23.550	0.0			
	3.900	0.0	8.850	0.0	13.800	0.0

AHYMO.OUT

18.750	0.0	23.700	0.0			
	4.050	0.0	9.000	0.0	13.950	0.0
18.900	0.0	23.850	0.0			
	4.200	0.0	9.150	0.0	14.100	0.0
19.050	0.0	24.000	0.0			
	4.350	0.0	9.300	0.0	14.250	0.0
19.200	0.0	24.150	0.0			
	4.500	0.0	9.450	0.0	14.400	0.0
19.350	0.0					
	4.650	0.0	9.600	0.0	14.550	0.0
19.500	0.0					
	4.800	0.0	9.750	0.0	14.700	0.0
19.650	0.0					

RUNOFF VOLUME = 2.08858 INCHES = 0.0245 ACRE-FEET
 PEAK DISCHARGE RATE = 0.48 CFS AT 1.500 HOURS BASIN AREA =
 0.0002
 SQ. MI.

* ROUTE BASIN B THRU DETENTION

* ROUTE THE TOTAL FLOW THROUGH THE PROPOSED RESERVOIR

ROUTE RESERVOIR	ID=5	HYD NO=105	INFLOW=4	CODE=3
	OUTFLOW(CFS)	STORAGE(AC-FT)	ELEV(FT)	
	0.00	0.009	69.25	
	0.42	0.014	70.25	

* * * * *

TIME (HRS)	INFLOW (CFS)	ELEV (FEET)	VOLUME (AC-FT)	OUTFLOW (CFS)
0.00	0.00	69.25	0.009	0.00
0.15	0.00	69.25	0.009	0.00
0.30	0.00	69.25	0.009	0.00
0.45	0.00	69.25	0.009	0.00
0.60	0.00	69.25	0.009	0.00
0.75	0.00	69.25	0.009	0.00
0.90	0.01	69.27	0.009	0.01
1.05	0.02	69.29	0.009	0.01
1.20	0.05	69.32	0.009	0.03
1.35	0.14	69.41	0.010	0.07
1.50	0.48	69.84	0.012	0.25
1.65	0.30	70.09	0.013	0.35
1.80	0.13	69.84	0.012	0.25
1.95	0.07	69.60	0.011	0.15
2.10	0.04	69.46	0.010	0.09
2.25	0.02	69.37	0.010	0.05
2.40	0.01	69.32	0.009	0.03
2.55	0.01	69.29	0.009	0.02
2.70	0.00	69.27	0.009	0.01
2.85	0.00	69.26	0.009	0.01
3.00	0.00	69.26	0.009	0.00

PEAK DISCHARGE = 0.352 CFS - PEAK OCCURS AT HOUR 1.60
 MAXIMUM WATER SURFACE ELEVATION = 70.087
 MAXIMUM STORAGE = 0.0132 AC-FT INCREMENTAL TIME= 0.050000HRS

*basin C

COMPUTE NM HYD ID=6 HYD NO=106 DA= .000219 SQ MI

AHYMO.OUT
 PER A=0 PER B=20 PER C=29 PER D=51
 TP=-.138 MASSRAIN=-1

K = 0.075210HR TP = 0.138000HR K/TP RATIO = 0.545000 SHAPE
 CONSTANT, N = 7.106428
 UNIT PEAK = 0.42594 CFS UNIT VOLUME = 0.9762 B = 526.28

P60 = 1.7400
 AREA = 0.000112 SQ MI IA = 0.10000 INCHES INF = 0.04000
 INCHES

PER HOUR
 RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
 0.050000

K = 0.120284HR TP = 0.138000HR K/TP RATIO = 0.871626 SHAPE
 CONSTANT, N = 4.075250
 UNIT PEAK = 0.28006 CFS UNIT VOLUME = 0.9582 B = 360.16

P60 = 1.7400
 AREA = 0.000107 SQ MI IA = 0.41122 INCHES INF = 1.00143
 INCHES

PER HOUR
 RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
 0.050000

PRINT HYD ID=6 CODE=3

PARTIAL HYDROGRAPH 106.00

TIME	TIME	FLOW	TIME	FLOW	TIME	FLOW
	FLOW	TIME	FLOW			
HRS	HRS	CFS	HRS	CFS	HRS	CFS
	CFS	HRS	CFS			
	0.000	0.0	4.950	0.0	9.900	0.0
14.850	0.0	0.0	19.800	0.0		
	0.150	0.0	5.100	0.0	10.050	0.0
15.000	0.0	0.0	19.950	0.0		
	0.300	0.0	5.250	0.0	10.200	0.0
15.150	0.0	0.0	20.100	0.0		
	0.450	0.0	5.400	0.0	10.350	0.0
15.300	0.0	0.0	20.250	0.0		
	0.600	0.0	5.550	0.0	10.500	0.0
15.450	0.0	0.0	20.400	0.0		
	0.750	0.0	5.700	0.0	10.650	0.0
15.600	0.0	0.0	20.550	0.0		
	0.900	0.0	5.850	0.0	10.800	0.0
15.750	0.0	0.0	20.700	0.0		
	1.050	0.0	6.000	0.0	10.950	0.0
15.900	0.0	0.0	20.850	0.0		
	1.200	0.0	6.150	0.0	11.100	0.0

				AHYMO.OUT			
16.050	0.0		21.000	0.0			
	1.350	0.1		6.300	0.0	11.250	0.0
16.200	0.0		21.150	0.0			
	1.500	0.5		6.450	0.0	11.400	0.0
16.350	0.0		21.300	0.0			
	1.650	0.3		6.600	0.0	11.550	0.0
16.500	0.0		21.450	0.0			
	1.800	0.1		6.750	0.0	11.700	0.0
16.650	0.0		21.600	0.0			
	1.950	0.1		6.900	0.0	11.850	0.0
16.800	0.0		21.750	0.0			
	2.100	0.0		7.050	0.0	12.000	0.0
16.950	0.0		21.900	0.0			
	2.250	0.0		7.200	0.0	12.150	0.0
17.100	0.0		22.050	0.0			
	2.400	0.0		7.350	0.0	12.300	0.0
17.250	0.0		22.200	0.0			
	2.550	0.0		7.500	0.0	12.450	0.0
17.400	0.0		22.350	0.0			
	2.700	0.0		7.650	0.0	12.600	0.0
17.550	0.0		22.500	0.0			
	2.850	0.0		7.800	0.0	12.750	0.0
17.700	0.0		22.650	0.0			
	3.000	0.0		7.950	0.0	12.900	0.0
17.850	0.0		22.800	0.0			
	3.150	0.0		8.100	0.0	13.050	0.0
18.000	0.0		22.950	0.0			
	3.300	0.0		8.250	0.0	13.200	0.0
18.150	0.0		23.100	0.0			
	3.450	0.0		8.400	0.0	13.350	0.0
18.300	0.0		23.250	0.0			
	3.600	0.0		8.550	0.0	13.500	0.0
18.450	0.0		23.400	0.0			
	3.750	0.0		8.700	0.0	13.650	0.0
18.600	0.0		23.550	0.0			
	3.900	0.0		8.850	0.0	13.800	0.0
18.750	0.0		23.700	0.0			
	4.050	0.0		9.000	0.0	13.950	0.0
18.900	0.0		23.850	0.0			
	4.200	0.0		9.150	0.0	14.100	0.0
19.050	0.0		24.000	0.0			
	4.350	0.0		9.300	0.0	14.250	0.0
19.200	0.0		24.150	0.0			
	4.500	0.0		9.450	0.0	14.400	0.0
19.350	0.0						
	4.650	0.0		9.600	0.0	14.550	0.0
19.500	0.0						
	4.800	0.0		9.750	0.0	14.700	0.0
19.650	0.0						

AHYMO.OUT

RUNOFF VOLUME = 2.08858 INCHES = 0.0244 ACRE-Feet
 PEAK DISCHARGE RATE = 0.48 CFS AT 1.500 HOURS BASIN AREA =
 0.0002
 SQ. MI.

* ROUTE BASIN C THRU DETENTION

* ROUTE THE TOTAL FLOW THROUGH THE PROPOSED RESERVOIR
 ROUTE RESERVOIR ID=7 HYD NO=107 INFLOW=6 CODE=3
 OUTFLOW(CFS) STORAGE(AC-FT) ELEV(FT)
 0.00 0.008 69.35
 0.45 0.012 70.50

* * * * *

TIME (HRS)	INFLOW (CFS)	ELEV (FEET)	VOLUME (AC-FT)	OUTFLOW (CFS)
0.00	0.00	69.35	0.008	0.00
0.15	0.00	69.35	0.008	0.00
0.30	0.00	69.35	0.008	0.00
0.45	0.00	69.35	0.008	0.00
0.60	0.00	69.35	0.008	0.00
0.75	0.00	69.35	0.008	0.00
0.90	0.01	69.37	0.008	0.01
1.05	0.02	69.39	0.008	0.02
1.20	0.05	69.43	0.008	0.03
1.35	0.14	69.55	0.009	0.08
1.50	0.48	70.08	0.011	0.29
1.65	0.30	70.30	0.011	0.37
1.80	0.13	69.95	0.010	0.23
1.95	0.07	69.67	0.009	0.13
2.10	0.04	69.53	0.009	0.07
2.25	0.02	69.45	0.008	0.04
2.40	0.01	69.40	0.008	0.02
2.55	0.01	69.38	0.008	0.01
2.70	0.00	69.37	0.008	0.01
2.85	0.00	69.36	0.008	0.00

PEAK DISCHARGE = 0.386 CFS - PEAK OCCURS AT HOUR 1.60
 MAXIMUM WATER SURFACE ELEVATION = 70.337
 MAXIMUM STORAGE = 0.0114 AC-FT INCREMENTAL TIME= 0.050000HRS

*basin D

COMPUTE NM HYD ID=8 HYD NO=108 DA= .000219 SQ MI
 PER A=0 PER B=20 PER C=29 PER D=51
 TP=-.138 MASSRAIN=-1

K = 0.075210HR TP = 0.138000HR K/TP RATIO = 0.545000 SHAPE
 CONSTANT, N = 7.106428
 UNIT PEAK = 0.42594 CFS UNIT VOLUME = 0.9762 B = 526.28

P60 = 1.7400
 AREA = 0.000112 SQ MI IA = 0.10000 INCHES INF = 0.04000
 INCHES

PER HOUR
 RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
 0.050000

K = 0.120284HR TP = 0.138000HR K/TP RATIO = 0.871626 SHAPE

AHYMO.OUT

CONSTANT, N = 4.075250

UNIT PEAK = 0.28006 CFS UNIT VOLUME = 0.9582 B = 360.16

P60 = 1.7400

AREA = 0.000107 SQ MI IA = 0.41122 INCHES INF = 1.00143 INCHES

PER HOUR

RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = 0.050000

PRINT HYD

ID=8 CODE=3

PARTIAL HYDROGRAPH 108.00

TIME	TIME	FLOW	TIME	FLOW	TIME	FLOW
	FLOW	TIME	FLOW			
HRS	HRS	CFS	HRS	CFS	HRS	CFS
	CFS	HRS	CFS			
	0.000	0.0	4.950	0.0	9.900	0.0
14.850	0.0	19.800	0.0			
	0.150	0.0	5.100	0.0	10.050	0.0
15.000	0.0	19.950	0.0			
	0.300	0.0	5.250	0.0	10.200	0.0
15.150	0.0	20.100	0.0			
	0.450	0.0	5.400	0.0	10.350	0.0
15.300	0.0	20.250	0.0			
	0.600	0.0	5.550	0.0	10.500	0.0
15.450	0.0	20.400	0.0			
	0.750	0.0	5.700	0.0	10.650	0.0
15.600	0.0	20.550	0.0			
	0.900	0.0	5.850	0.0	10.800	0.0
15.750	0.0	20.700	0.0			
	1.050	0.0	6.000	0.0	10.950	0.0
15.900	0.0	20.850	0.0			
	1.200	0.0	6.150	0.0	11.100	0.0
16.050	0.0	21.000	0.0			
	1.350	0.1	6.300	0.0	11.250	0.0
16.200	0.0	21.150	0.0			
	1.500	0.5	6.450	0.0	11.400	0.0
16.350	0.0	21.300	0.0			
	1.650	0.3	6.600	0.0	11.550	0.0
16.500	0.0	21.450	0.0			
	1.800	0.1	6.750	0.0	11.700	0.0
16.650	0.0	21.600	0.0			
	1.950	0.1	6.900	0.0	11.850	0.0
16.800	0.0	21.750	0.0			
	2.100	0.0	7.050	0.0	12.000	0.0
16.950	0.0	21.900	0.0			
	2.250	0.0	7.200	0.0	12.150	0.0

				AHYMO.OUT			
17.100	0.0	22.050	0.0	0.0	12.300	0.0	
	2.400		0.0	7.350			
17.250	0.0	22.200	0.0	0.0	12.450	0.0	
	2.550		0.0	7.500			
17.400	0.0	22.350	0.0	0.0	12.600	0.0	
	2.700		0.0	7.650			
17.550	0.0	22.500	0.0	0.0	12.750	0.0	
	2.850		0.0	7.800			
17.700	0.0	22.650	0.0	0.0	12.900	0.0	
	3.000		0.0	7.950			
17.850	0.0	22.800	0.0	0.0	13.050	0.0	
	3.150		0.0	8.100			
18.000	0.0	22.950	0.0	0.0	13.200	0.0	
	3.300		0.0	8.250			
18.150	0.0	23.100	0.0	0.0	13.350	0.0	
	3.450		0.0	8.400			
18.300	0.0	23.250	0.0	0.0	13.500	0.0	
	3.600		0.0	8.550			
18.450	0.0	23.400	0.0	0.0	13.650	0.0	
	3.750		0.0	8.700			
18.600	0.0	23.550	0.0	0.0	13.800	0.0	
	3.900		0.0	8.850			
18.750	0.0	23.700	0.0	0.0	13.950	0.0	
	4.050		0.0	9.000			
18.900	0.0	23.850	0.0	0.0	14.100	0.0	
	4.200		0.0	9.150			
19.050	0.0	24.000	0.0	0.0	14.250	0.0	
	4.350		0.0	9.300			
19.200	0.0	24.150	0.0	0.0	14.400	0.0	
	4.500		0.0	9.450			
19.350	0.0			0.0	14.550	0.0	
	4.650		0.0	9.600			
19.500	0.0			0.0	14.700	0.0	
	4.800		0.0	9.750			
19.650	0.0						

RUNOFF VOLUME = 2.08858 INCHES = 0.0244 ACRE-FEET
 PEAK DISCHARGE RATE = 0.48 CFS AT 1.500 HOURS BASIN AREA =
 0.0002
 SQ. MI.

* ROUTE BASIN D THRU DETENTION

* ROUTE THE TOTAL FLOW THROUGH THE PROPOSED RESERVOIR
 ROUTE RESERVOIR ID=9 HYD NO=109 INFLOW=8 CODE=3
 OUTFLOW(CFS) STORAGE(AC-FT) ELEV(FT)
 0.00 0.008 69.85
 0.45 0.012 71.00

* * * * *

AHYMO.OUT

TIME (HRS)	INFLOW (CFS)	ELEV (FEET)	VOLUME (AC-FT)	OUTFLOW (CFS)
0.00	0.00	69.85	0.008	0.00
0.15	0.00	69.85	0.008	0.00
0.30	0.00	69.85	0.008	0.00
0.45	0.00	69.85	0.008	0.00
0.60	0.00	69.85	0.008	0.00
0.75	0.00	69.85	0.008	0.00
0.90	0.01	69.87	0.008	0.01
1.05	0.02	69.89	0.008	0.02
1.20	0.05	69.93	0.008	0.03
1.35	0.14	70.05	0.009	0.08
1.50	0.48	70.58	0.011	0.29
1.65	0.30	70.80	0.011	0.37
1.80	0.13	70.45	0.010	0.23
1.95	0.07	70.17	0.009	0.13
2.10	0.04	70.03	0.009	0.07
2.25	0.02	69.95	0.008	0.04
2.40	0.01	69.90	0.008	0.02
2.55	0.01	69.88	0.008	0.01
2.70	0.00	69.87	0.008	0.01
2.85	0.00	69.86	0.008	0.00

PEAK DISCHARGE = 0.386 CFS - PEAK OCCURS AT HOUR 1.60
 MAXIMUM WATER SURFACE ELEVATION = 70.837
 MAXIMUM STORAGE = 0.0114 AC-FT INCREMENTAL TIME= 0.050000HRS

*basin E
 COMPUTE NM HYD ID=10 HYD NO=110 DA= .000207 SQ MI
 PER A=0 PER B=20 PER C=26 PER D=54
 TP=-.138 MASSRAIN=-1

K = 0.075210HR TP = 0.138000HR K/TP RATIO = 0.545000 SHAPE
 CONSTANT, N = 7.106428
 UNIT PEAK = 0.42628 CFS UNIT VOLUME = 0.9762 B = 526.28

P60 = 1.7400
 AREA = 0.000112 SQ MI IA = 0.10000 INCHES INF = 0.04000
 INCHES

PER HOUR
 RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
 0.050000

K = 0.121080HR TP = 0.138000HR K/TP RATIO = 0.877393 SHAPE
 CONSTANT, N = 4.046413
 UNIT PEAK = 0.24719 CFS UNIT VOLUME = 0.9512 B = 358.25

P60 = 1.7400
 AREA = 0.000095 SQ MI IA = 0.41522 INCHES INF = 1.01261
 INCHES

PER HOUR
 RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
 0.050000

PRINT HYD ID=10 CODE=3

PARTIAL HYDROGRAPH 110.00

TIME	TIME	FLOW	TIME	FLOW	TIME	FLOW
TIME	TIME	FLOW	TIME	FLOW	TIME	FLOW

HRS	AHYMO.OUT		HRS	CFS		HRS	CFS
	HRS	CFS		HRS	CFS		
	CFS	HRS	CFS				
	0.000	0.0	4.950	0.0	9.900	0.0	
14.850	0.0	19.800	0.0	5.100	0.0	10.050	0.0
	0.150	0.0					
15.000	0.0	19.950	0.0	5.250	0.0	10.200	0.0
	0.300	0.0					
15.150	0.0	20.100	0.0	5.400	0.0	10.350	0.0
	0.450	0.0					
15.300	0.0	20.250	0.0	5.550	0.0	10.500	0.0
	0.600	0.0					
15.450	0.0	20.400	0.0	5.700	0.0	10.650	0.0
	0.750	0.0					
15.600	0.0	20.550	0.0	5.850	0.0	10.800	0.0
	0.900	0.0					
15.750	0.0	20.700	0.0	6.000	0.0	10.950	0.0
	1.050	0.0					
15.900	0.0	20.850	0.0	6.150	0.0	11.100	0.0
	1.200	0.0					
16.050	0.0	21.000	0.0	6.300	0.0	11.250	0.0
	1.350	0.1					
16.200	0.0	21.150	0.0	6.450	0.0	11.400	0.0
	1.500	0.5					
16.350	0.0	21.300	0.0	6.600	0.0	11.550	0.0
	1.650	0.3					
16.500	0.0	21.450	0.0	6.750	0.0	11.700	0.0
	1.800	0.1					
16.650	0.0	21.600	0.0	6.900	0.0	11.850	0.0
	1.950	0.1					
16.800	0.0	21.750	0.0	7.050	0.0	12.000	0.0
	2.100	0.0					
16.950	0.0	21.900	0.0	7.200	0.0	12.150	0.0
	2.250	0.0					
17.100	0.0	22.050	0.0	7.350	0.0	12.300	0.0
	2.400	0.0					
17.250	0.0	22.200	0.0	7.500	0.0	12.450	0.0
	2.550	0.0					
17.400	0.0	22.350	0.0	7.650	0.0	12.600	0.0
	2.700	0.0					
17.550	0.0	22.500	0.0	7.800	0.0	12.750	0.0
	2.850	0.0					
17.700	0.0	22.650	0.0	7.950	0.0	12.900	0.0
	3.000	0.0					
17.850	0.0	22.800	0.0	8.100	0.0	13.050	0.0
	3.150	0.0					
18.000	0.0	22.950	0.0	8.250	0.0	13.200	0.0
	3.300	0.0					
18.150	0.0	23.100	0.0				

				AHYMO.OUT		
	3.450	0.0	8.400	0.0	13.350	0.0
18.300	0.0	23.250	0.0			
	3.600	0.0	8.550	0.0	13.500	0.0
18.450	0.0	23.400	0.0			
	3.750	0.0	8.700	0.0	13.650	0.0
18.600	0.0	23.550	0.0			
	3.900	0.0	8.850	0.0	13.800	0.0
18.750	0.0	23.700	0.0			
	4.050	0.0	9.000	0.0	13.950	0.0
18.900	0.0	23.850	0.0			
	4.200	0.0	9.150	0.0	14.100	0.0
19.050	0.0	24.000	0.0			
	4.350	0.0	9.300	0.0	14.250	0.0
19.200	0.0	24.150	0.0			
	4.500	0.0	9.450	0.0	14.400	0.0
19.350	0.0					
	4.650	0.0	9.600	0.0	14.550	0.0
19.500	0.0					
	4.800	0.0	9.750	0.0	14.700	0.0
19.650	0.0					

RUNOFF VOLUME = 2.15626 INCHES = 0.0238 ACRE-FEET
 PEAK DISCHARGE RATE = 0.46 CFS AT 1.500 HOURS BASIN AREA =
 0.0002
 SQ. MI.

* ROUTE BASIN E THRU DETENTION

* ROUTE THE TOTAL FLOW THROUGH THE PROPOSED RESERVOIR
 ROUTE RESERVOIR ID=11 HYD NO=110 INFLOW=10 CODE=3
 OUTFLOW(CFS) STORAGE(AC-FT) ELEV(FT)
 0.00 0.008 70.45
 0.43 0.011 71.50

* * * * *

TIME (HRS)	INFLOW (CFS)	ELEV (FEET)	VOLUME (AC-FT)	OUTFLOW (CFS)
0.00	0.00	70.45	0.008	0.00
0.15	0.00	70.45	0.008	0.00
0.30	0.00	70.45	0.008	0.00
0.45	0.00	70.45	0.008	0.00
0.60	0.00	70.45	0.008	0.00
0.75	0.00	70.45	0.008	0.00
0.90	0.01	70.47	0.008	0.01
1.05	0.02	70.49	0.008	0.02
1.20	0.05	70.54	0.008	0.04
1.35	0.14	70.65	0.009	0.08
1.50	0.46	71.20	0.010	0.31
1.65	0.29	71.34	0.011	0.37
1.80	0.13	70.96	0.009	0.21
1.95	0.07	70.71	0.009	0.11
2.10	0.04	70.60	0.008	0.06
2.25	0.02	70.53	0.008	0.03
2.40	0.01	70.49	0.008	0.02

AHYMO.OUT

2.55	0.01	70.47	0.008	0.01
2.70	0.00	70.46	0.008	0.01
2.85	0.00	70.46	0.008	0.00

PEAK DISCHARGE = 0.392 CFS - PEAK OCCURS AT HOUR 1.60
 MAXIMUM WATER SURFACE ELEVATION = 71.408
 MAXIMUM STORAGE = 0.0107 AC-FT INCREMENTAL TIME= 0.050000HRS

*basin F
 COMPUTE NM HYD ID=12 HYD NO=112 DA= .000194 SQ MI
 PER A=0 PER B=20 PER C=22 PER D=58
 TP=-.138 MASSRAIN=-1

K = 0.075210HR TP = 0.138000HR K/TP RATIO = 0.545000 SHAPE
 CONSTANT, N = 7.106428
 UNIT PEAK = 0.42911 CFS UNIT VOLUME = 0.9762 B = 526.28

P60 = 1.7400
 AREA = 0.000113 SQ MI IA = 0.10000 INCHES INF = 0.04000
 INCHES

PER HOUR
 RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
 0.050000

K = 0.122318HR TP = 0.138000HR K/TP RATIO = 0.886364 SHAPE
 CONSTANT, N = 4.002361
 UNIT PEAK = 0.20980 CFS UNIT VOLUME = 0.9427 B = 355.33

P60 = 1.7400
 AREA = 0.000081 SQ MI IA = 0.42143 INCHES INF = 1.03000
 INCHES

PER HOUR
 RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
 0.050000

PRINT HYD ID=12 CODE=3

PARTIAL HYDROGRAPH 112.00

TIME	TIME	FLOW	TIME	FLOW	TIME	FLOW
	FLOW	TIME	FLOW			
HRS	HRS	CFS	HRS	CFS	HRS	CFS
	CFS	HRS	CFS			
	0.000	0.0	4.950	0.0	9.900	0.0
14.850	0.0		19.800	0.0		
	0.150	0.0	5.100	0.0	10.050	0.0
15.000	0.0		19.950	0.0		
	0.300	0.0	5.250	0.0	10.200	0.0
15.150	0.0		20.100	0.0		
	0.450	0.0	5.400	0.0	10.350	0.0
15.300	0.0		20.250	0.0		
	0.600	0.0	5.550	0.0	10.500	0.0
15.450	0.0		20.400	0.0		
	0.750	0.0	5.700	0.0	10.650	0.0

				AHYMO.OUT			
15.600	0.0 0.900	0.0	20.550	0.0 5.850	0.0	10.800	0.0
15.750	0.0 1.050	0.0	20.700	0.0 6.000	0.0	10.950	0.0
15.900	0.0 1.200	0.0	20.850	0.0 6.150	0.0	11.100	0.0
16.050	0.0 1.350	0.1	21.000	0.0 6.300	0.0	11.250	0.0
16.200	0.0 1.500	0.4	21.150	0.0 6.450	0.0	11.400	0.0
16.350	0.0 1.650	0.3	21.300	0.0 6.600	0.0	11.550	0.0
16.500	0.0 1.800	0.1	21.450	0.0 6.750	0.0	11.700	0.0
16.650	0.0 1.950	0.1	21.600	0.0 6.900	0.0	11.850	0.0
16.800	0.0 2.100	0.0	21.750	0.0 7.050	0.0	12.000	0.0
16.950	0.0 2.250	0.0	21.900	0.0 7.200	0.0	12.150	0.0
17.100	0.0 2.400	0.0	22.050	0.0 7.350	0.0	12.300	0.0
17.250	0.0 2.550	0.0	22.200	0.0 7.500	0.0	12.450	0.0
17.400	0.0 2.700	0.0	22.350	0.0 7.650	0.0	12.600	0.0
17.550	0.0 2.850	0.0	22.500	0.0 7.800	0.0	12.750	0.0
17.700	0.0 3.000	0.0	22.650	0.0 7.950	0.0	12.900	0.0
17.850	0.0 3.150	0.0	22.800	0.0 8.100	0.0	13.050	0.0
18.000	0.0 3.300	0.0	22.950	0.0 8.250	0.0	13.200	0.0
18.150	0.0 3.450	0.0	23.100	0.0 8.400	0.0	13.350	0.0
18.300	0.0 3.600	0.0	23.250	0.0 8.550	0.0	13.500	0.0
18.450	0.0 3.750	0.0	23.400	0.0 8.700	0.0	13.650	0.0
18.600	0.0 3.900	0.0	23.550	0.0 8.850	0.0	13.800	0.0
18.750	0.0 4.050	0.0	23.700	0.0 9.000	0.0	13.950	0.0
18.900	0.0 4.200	0.0	23.850	0.0 9.150	0.0	14.100	0.0
19.050	0.0 4.350	0.0	24.000	0.0 9.300	0.0	14.250	0.0
19.200	0.0		24.150	0.0			

	4.500	0.0	9.450	0.0	14.400	0.0
19.350	0.0					
	4.650	0.0	9.600	0.0	14.550	0.0
19.500	0.0					
	4.800	0.0	9.750	0.0	14.700	0.0
19.650	0.0					

RUNOFF VOLUME = 2.24655 INCHES = 0.0232 ACRE-FEET
 PEAK DISCHARGE RATE = 0.44 CFS AT 1.500 HOURS BASIN AREA =
 0.0002
 SQ. MI.

* ROUTE BASIN F THRU DETENTION

* ROUTE THE TOTAL FLOW THROUGH THE PROPOSED RESERVOIR
 ROUTE RESERVOIR ID=13 HYD NO=113 INFLOW=12 CODE=3
 OUTFLOW(CFS) STORAGE(AC-FT) ELEV(FT)
 0.00 0.007 71.35
 0.45 0.011 72.50

* * * * *

TIME (HRS)	INFLOW (CFS)	ELEV (FEET)	VOLUME (AC-FT)	OUTFLOW (CFS)
0.00	0.00	71.35	0.007	0.00
0.15	0.00	71.35	0.007	0.00
0.30	0.00	71.35	0.007	0.00
0.45	0.00	71.35	0.007	0.00
0.60	0.00	71.35	0.007	0.00
0.75	0.00	71.35	0.007	0.00
0.90	0.01	71.37	0.007	0.01
1.05	0.02	71.39	0.007	0.02
1.20	0.05	71.43	0.007	0.03
1.35	0.13	71.54	0.008	0.07
1.50	0.44	72.02	0.009	0.26
1.65	0.28	72.22	0.010	0.34
1.80	0.13	71.90	0.009	0.21
1.95	0.07	71.65	0.008	0.12
2.10	0.04	71.52	0.008	0.07
2.25	0.02	71.44	0.007	0.03
2.40	0.01	71.40	0.007	0.02
2.55	0.01	71.38	0.007	0.01
2.70	0.00	71.37	0.007	0.01
2.85	0.00	71.36	0.007	0.00

PEAK DISCHARGE = 0.353 CFS - PEAK OCCURS AT HOUR 1.60
 MAXIMUM WATER SURFACE ELEVATION = 72.251
 MAXIMUM STORAGE = 0.0101 AC-FT INCREMENTAL TIME= 0.050000HRS

ADD HYD ID=14 HYD NO=114 ID I=3 ID II=5
 ADD HYD ID=15 HYD NO=115 ID I=7 ID II=9
 ADD HYD ID=16 HYD NO=116 ID I=11 ID II=13
 ADD HYD ID=17 HYD NO=117 ID I=14 ID II=15
 ADD HYD ID=18 HYD NO=118 ID I=16 ID II=17

PRINT HYD ID=18 CODE=3

PARTIAL HYDROGRAPH 118.00

TIME	TIME	FLOW	TIME	FLOW	TIME	FLOW
------	------	------	------	------	------	------

FLOW HRS	HRS	TIME CFS	AHYMO.OUT		HRS	CFS
			FLOW HRS	CFS		
	CFS	HRS	CFS			
	0.000	0.0	4.950	0.0	9.900	0.0
14.850	0.0 0.150	0.0	19.800 5.100	0.0 0.0	10.050	0.0
15.000	0.0 0.300	0.0	19.950 5.250	0.0 0.0	10.200	0.0
15.150	0.0 0.450	0.0	20.100 5.400	0.0 0.0	10.350	0.0
15.300	0.0 0.600	0.0	20.250 5.550	0.0 0.0	10.500	0.0
15.450	0.0 0.750	0.0	20.400 5.700	0.0 0.0	10.650	0.0
15.600	0.0 0.900	0.0	20.550 5.850	0.0 0.0	10.800	0.0
15.750	0.0 1.050	0.1	20.700 6.000	0.0 0.0	10.950	0.0
15.900	0.0 1.200	0.2	20.850 6.150	0.0 0.0	11.100	0.0
16.050	0.0 1.350	0.4	21.000 6.300	0.0 0.0	11.250	0.0
16.200	0.0 1.500	1.7	21.150 6.450	0.0 0.0	11.400	0.0
16.350	0.0 1.650	2.1	21.300 6.600	0.0 0.0	11.550	0.0
16.500	0.0 1.800	1.4	21.450 6.750	0.0 0.0	11.700	0.0
16.650	0.0 1.950	0.8	21.600 6.900	0.0 0.0	11.850	0.0
16.800	0.0 2.100	0.4	21.750 7.050	0.0 0.0	12.000	0.0
16.950	0.0 2.250	0.2	21.900 7.200	0.0 0.0	12.150	0.0
17.100	0.0 2.400	0.1	22.050 7.350	0.0 0.0	12.300	0.0
17.250	0.0 2.550	0.1	22.200 7.500	0.0 0.0	12.450	0.0
17.400	0.0 2.700	0.0	22.350 7.650	0.0 0.0	12.600	0.0
17.550	0.0 2.850	0.0	22.500 7.800	0.0 0.0	12.750	0.0
17.700	0.0 3.000	0.0	22.650 7.950	0.0 0.0	12.900	0.0
17.850	0.0 3.150	0.0	22.800 8.100	0.0 0.0	13.050	0.0
18.000	0.0 3.300	0.0	22.950 8.250	0.0 0.0	13.200	0.0

				AHYMO.OUT			
18.150	0.0	23.100	0.0	0.0	0.0	13.350	0.0
	3.450		0.0	8.400			
18.300	0.0	23.250	0.0	0.0	0.0	13.500	0.0
	3.600		0.0	8.550			
18.450	0.0	23.400	0.0	0.0	0.0	13.650	0.0
	3.750		0.0	8.700			
18.600	0.0	23.550	0.0	0.0	0.0	13.800	0.0
	3.900		0.0	8.850			
18.750	0.0	23.700	0.0	0.0	0.0	13.950	0.0
	4.050		0.0	9.000			
18.900	0.0	23.850	0.0	0.0	0.0	14.100	0.0
	4.200		0.0	9.150			
19.050	0.0	24.000	0.0	0.0	0.0	14.250	0.0
	4.350		0.0	9.300			
19.200	0.0	24.150	0.0	0.0	0.0	14.400	0.0
	4.500		0.0	9.450			
19.350	0.0	24.300	0.0	0.0	0.0	14.550	0.0
	4.650		0.0	9.600			
19.500	0.0						
	4.800		0.0	9.750	0.0	14.700	0.0
19.650	0.0						

RUNOFF VOLUME = 2.13657 INCHES = 0.1438 ACRE-FEET
 PEAK DISCHARGE RATE = 2.23 CFS AT 1.600 HOURS BASIN AREA =
 0.0013
 SQ. MI.

FINISH

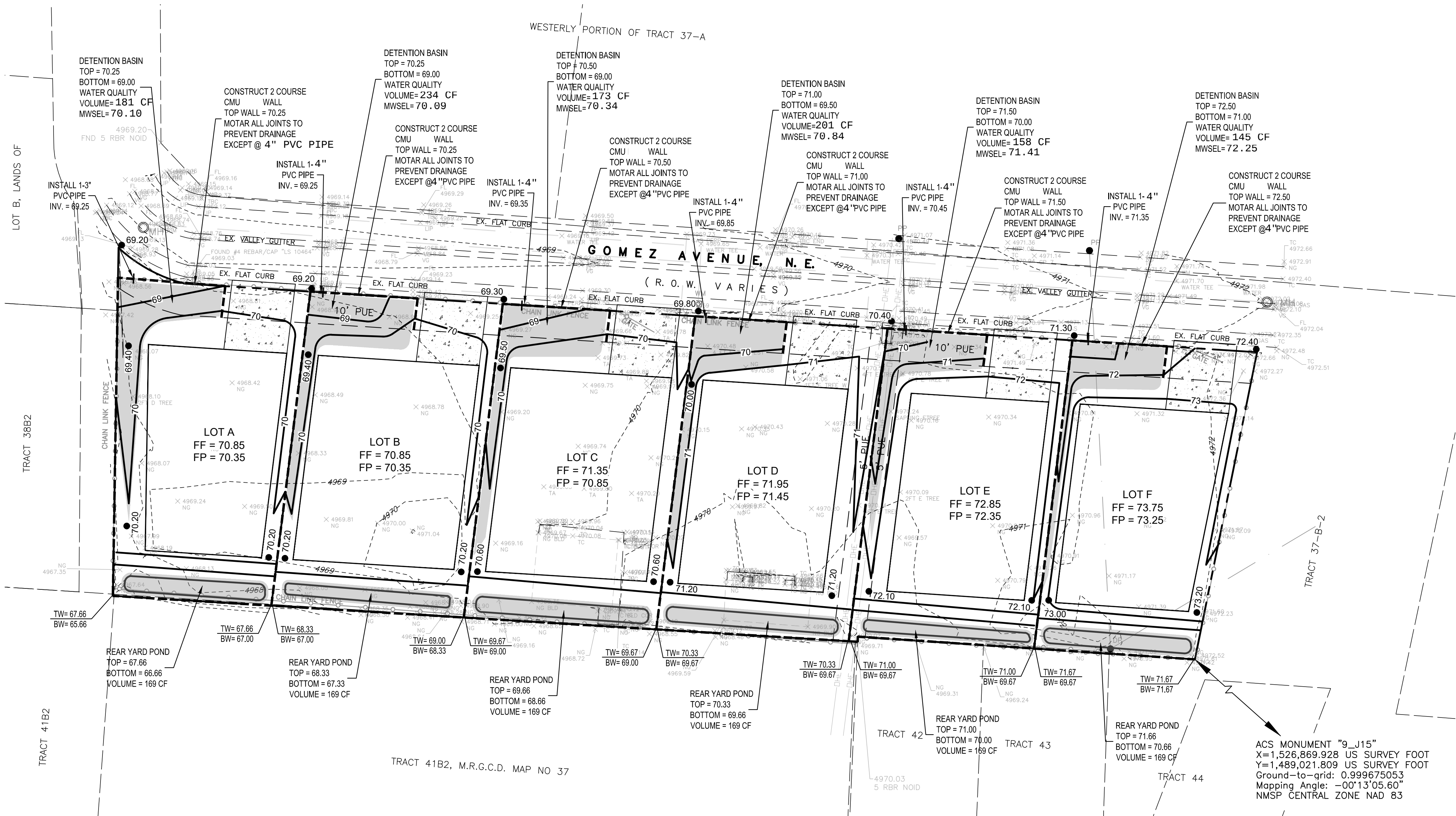
NORMAL PROGRAM FINISH

END TIME (HR:MIN:SEC) = 06:52:53

BASIN DATA

Basin	Area (sf)	Area (acres)	Treatment A % (acres)	Treatment B % (acres)	Treatment C % (acres)	Treatment D % (acres)	Q100 GENERATED	Q100 DISCHARGED	FIRST FLUSH REQUIRED	FIRST FLUSH PROVIDED	MWSEL		
EXISTING LOT	35174.00	0.807	0%	0	36%	0.291	50%	0.4037	14%	0.113	2.23	2.23	
LOT A	5672.00	0.130	0%	0	26%	0.034	25%	0.0326	55%	0.072	0.45	0.36	70.10
LOT B	6120.00	0.140	0%	0	20%	0.028	29%	0.0407	51%	0.072	0.48	0.35	70.09
LOT C	6111.00	0.140	0%	0	20%	0.028	29%	0.0407	51%	0.072	0.48	0.39	70.34
LOT D	6103.00	0.140	0%	0	20%	0.028	29%	0.0406	51%	0.071	0.48	0.39	70.84
LOT E	5763.00	0.132	0%	0	20%	0.026	26%	0.0344	54%	0.071	0.46	0.39	71.41
LOT F	5405.00	0.124	0%	0	20%	0.025	22%	0.0273	58%	0.072	0.44	0.35	72.25
TOTAL PROPOSED								2.23		749		2106	

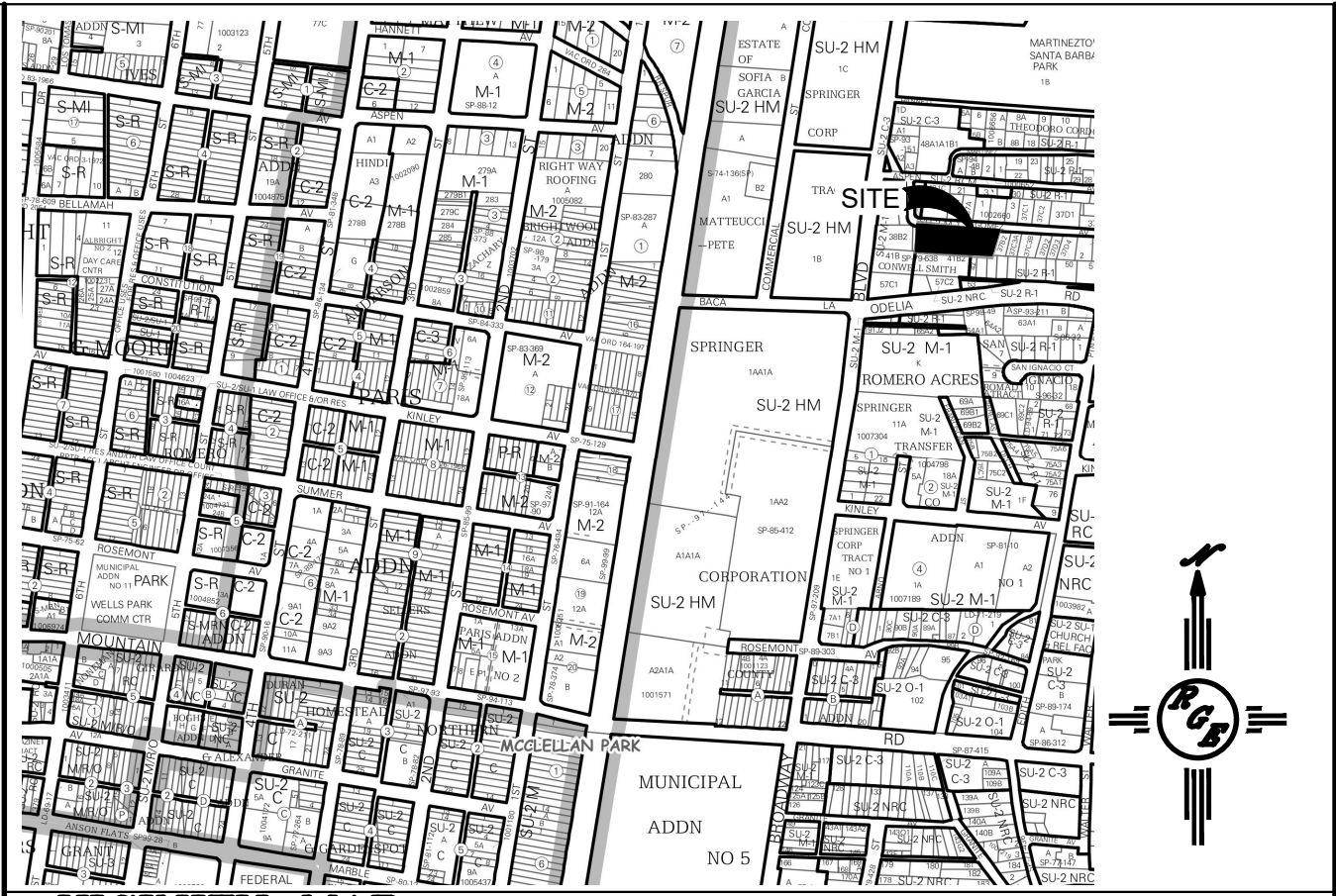
Narrative
The subject property is located within a fully developed area of NE Albuquerque. All down stream drainage improvements have been completed. This is a redevelopment of an site that discharges 2.23 cfs to the Gomez roadway. The proposed improvements consist of 6 lots, Each lot retains the first flush then discharges through a 4" pipe to gomez. The detention ponds on each lot reduce the com total discharge to 2.23 cfs, matching existing while retainin in excess the required water quality volume. Each lot will flow out the driveway in the event of clogging or storm greater than the design event



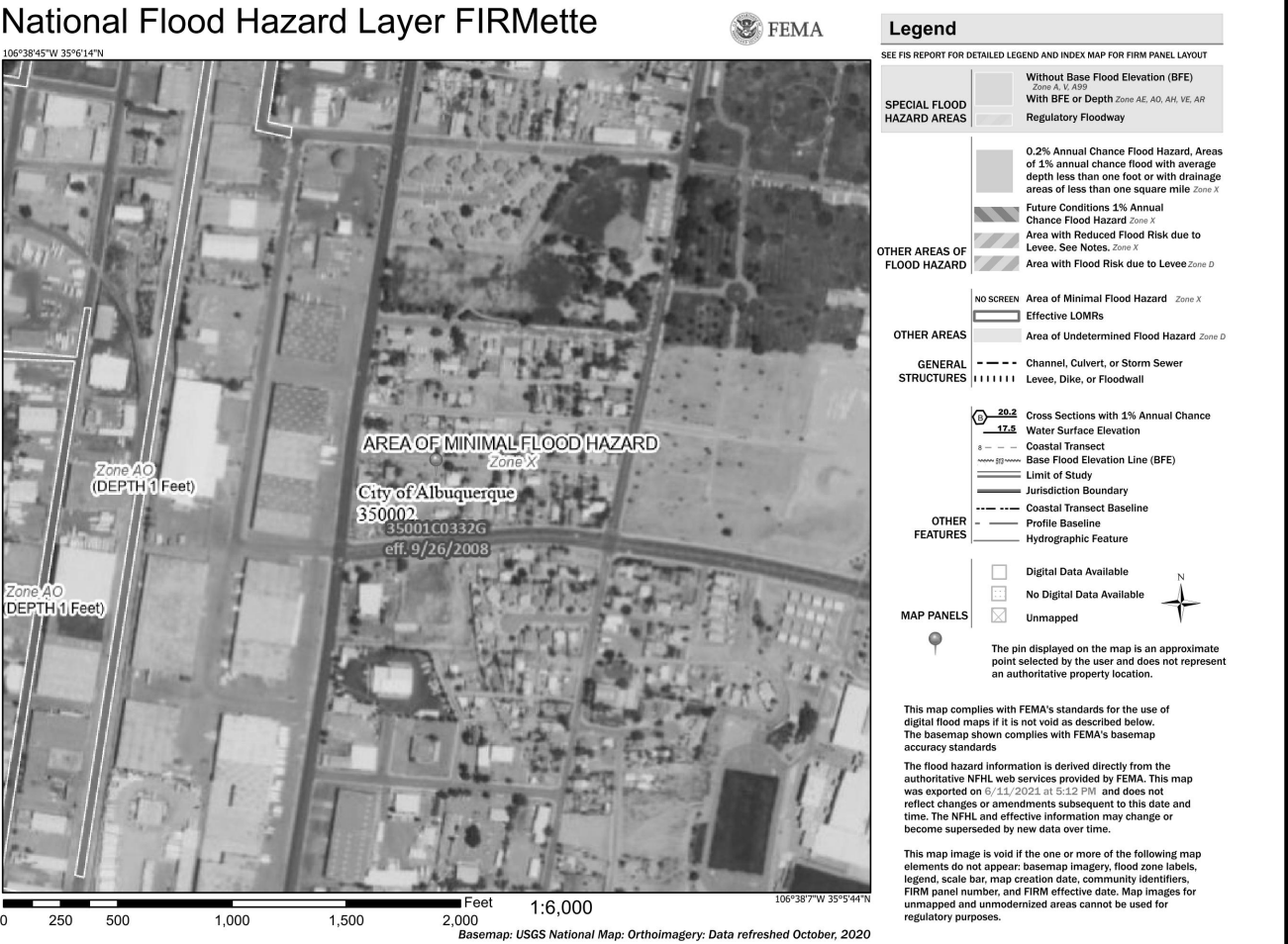
CAUTION:
EXISTING UTILITIES ARE NOT SHOWN. IT SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR TO CONDUCT ALL NECESSARY FIELD INVESTIGATIONS PRIOR TO ANY EXCAVATION TO DETERMINE THE ACTUAL LOCATION OF UTILITIES & OTHER IMPROVEMENTS.

EROSION CONTROL NOTES:

- CONTRACTOR IS RESPONSIBLE FOR OBTAINING A TOPSOIL DISTURBANCE PERMIT PRIOR TO BEGINNING WORK.
- CONTRACTOR IS RESPONSIBLE FOR MAINTAINING RUN-OFF ON SITE DURING CONSTRUCTION.
- CONTRACTOR IS RESPONSIBLE FOR CLEANING ALL SEDIMENT THAT GETS INTO EXISTING RIGHT-OF-WAY.
- REPAIR OF DAMAGED FACILITIES AND CLEANUP OF SEDIMENT ACCUMULATIONS ON ADJACENT PROPERTIES AND IN PUBLIC FACILITIES IS THE RESPONSIBILITY OF THE CONTRACTOR.
- ALL EXPOSED EARTH SURFACES MUST BE PROTECTED FROM WIND AND WATER EROSION PRIOR TO FINAL ACCEPTANCE OF ANY PROJECT.



VICINITY MAP:



FIRM MAP:

LEGAL DESCRIPTION:

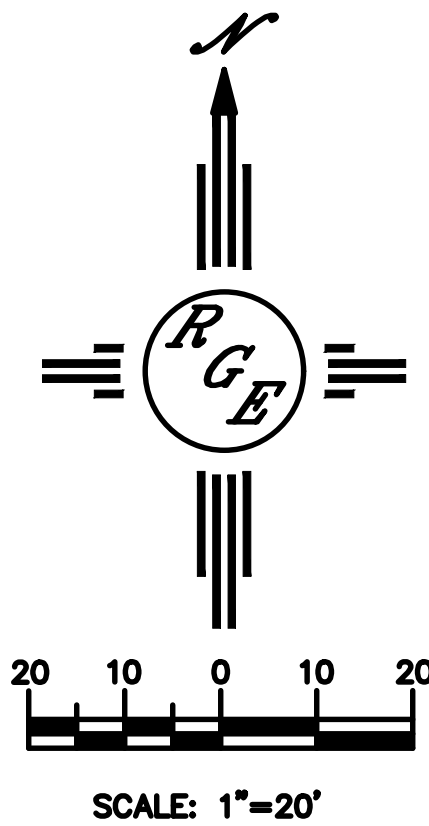
LOTS A, B, C, D, E, F GOMEZ AVENUE SUBDIVISION
CITY OF ALBUQUERQUE, BERNALILLO COUNTY, NEW MEXICO

NOTES:

- ALL SPOT ELEVATIONS REPRESENT FLOWLINE ELEVATION UNLESS OTHERWISE NOTED.
- ALL SLOPES SHALL BE 3:1 MAX. AND GRAVEL OR NATIVE SEEDING PRIOR TO CO.
- ANY PERIMETER WALLS MUST BE PERMITTED SEPARATELY ALL RETAINING WALL DESIGN SHALL BE BY OTHERS.
- SURVEY INFORMATION PROVIDED BY COMMUNITY SCIENCES CORPORATION USING NAVD DATUM 1988.
- A PAD ELEVATION CERTIFICATION SHALL BE REQUIRED PRIOR TO RELEASE OF BUILDING PERMIT.

LEGEND

-----XXXX-----	EXISTING CONTOUR
-----XXXX-----	EXISTING INDEX CONTOUR
-----XXXX-----	PROPOSED CONTOUR
-----XXXX-----	PROPOSED INDEX CONTOUR
+ XXXXX	EXISTING SPOT ELEVATION
● XXXXX	PROPOSED SPOT ELEVATION
-----	BOUNDARY
-----	ADJACENT BOUNDARY
=====	EXISTING EDGE OF CONCRETE
=====	PROPOSED RETAINING WALL
=====	PROPOSED PONDING
=====	PROPOSED CONCRETE

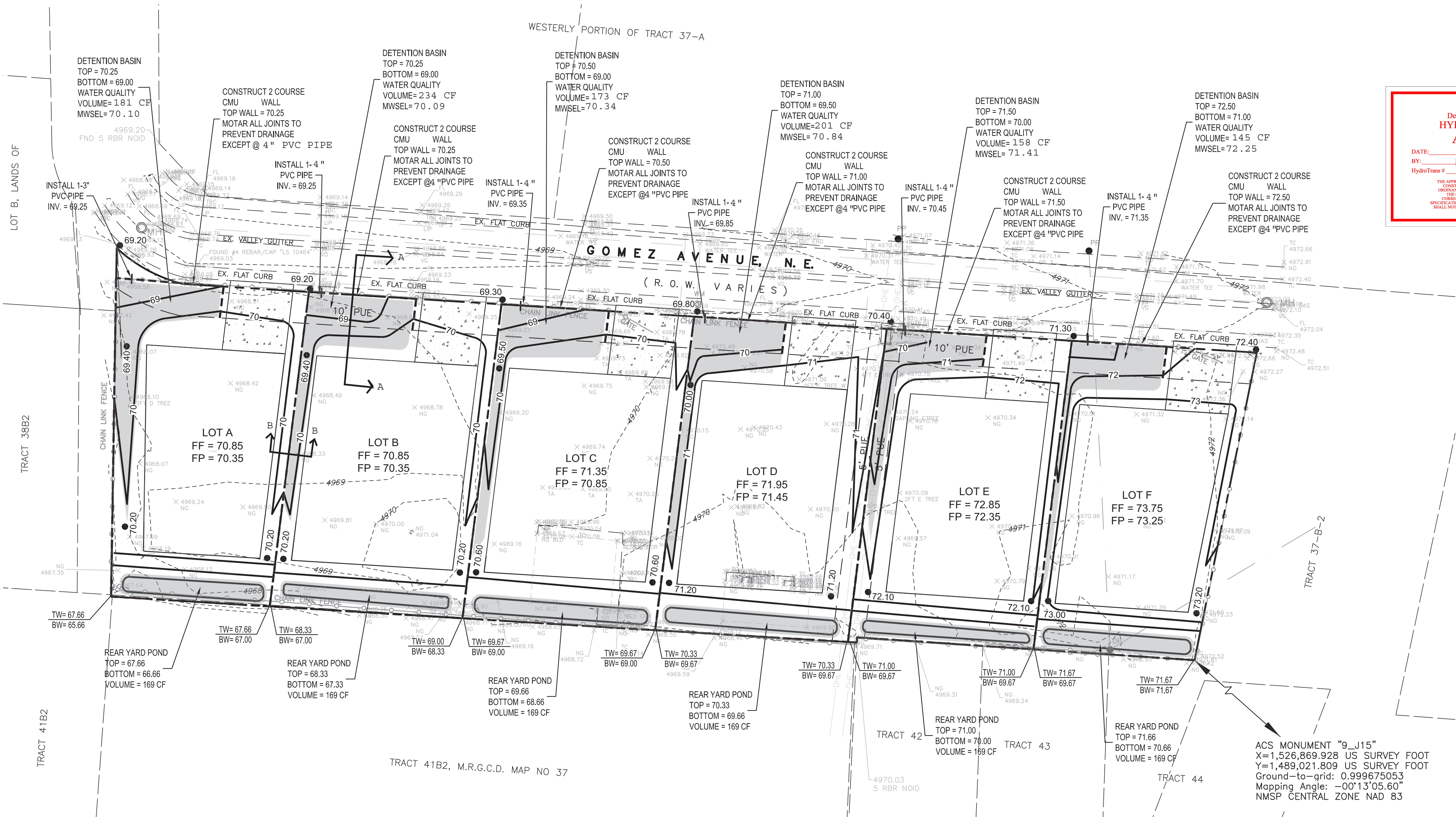


ENGINEER'S SEAL	LOTS A, B, C, D, E, F GOMEZ AVE. SUBDIVSION	DRAWN BY DEM
DAVID SOULE NEW MEXICO 14522 REGISTERED PROFESSIONAL ENGINEER	GRADING AND DRAINAGE PLAN	DATE 6-16-21
6/17/21	Rio Grande Engineering P.O. BOX 53924 ALBUQUERQUE, NM 87199 (505) 321-8099	Gomez Ave.Subdivision.DWG
DAVID SOULE P.E. #14522		SHEET # C1
		JOB #

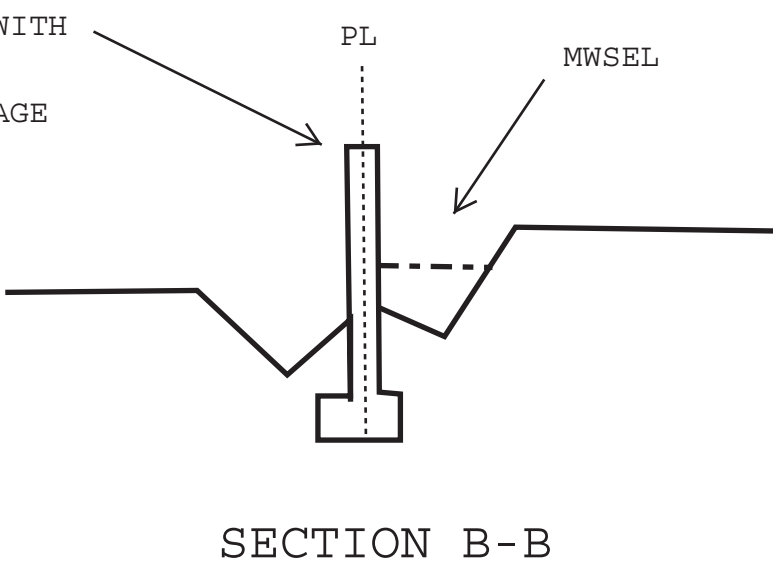
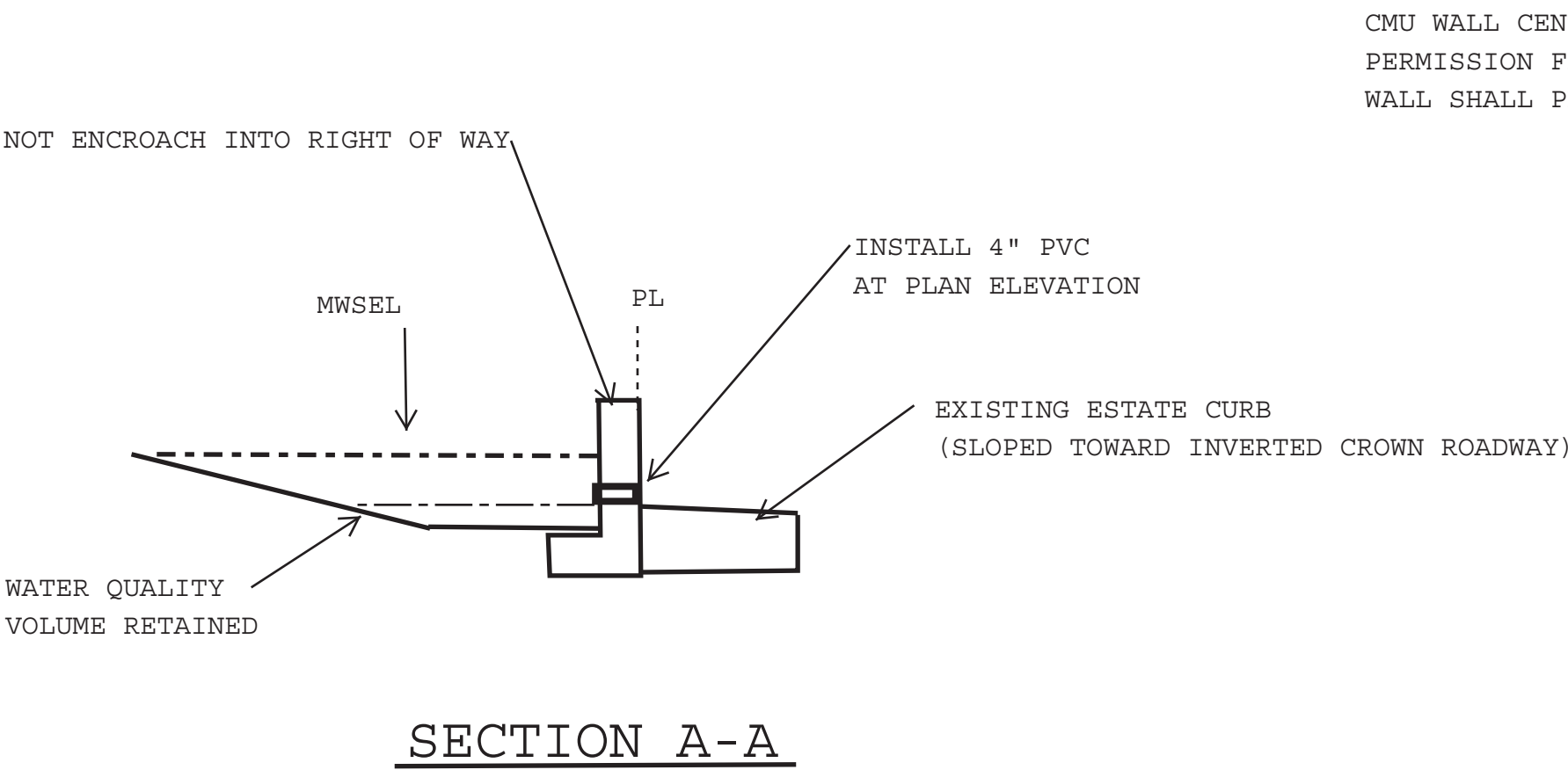
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ACS MONUMENT "9_J15"
X=1,526,869.928 US SURVEY FOOT
Y=1,489,021.809 US SURVEY FOOT
Ground-to-grid: 0.999675053
Mapping Angle: -00°13'05.60"
NMSP CENTRAL ZONE NAD 83

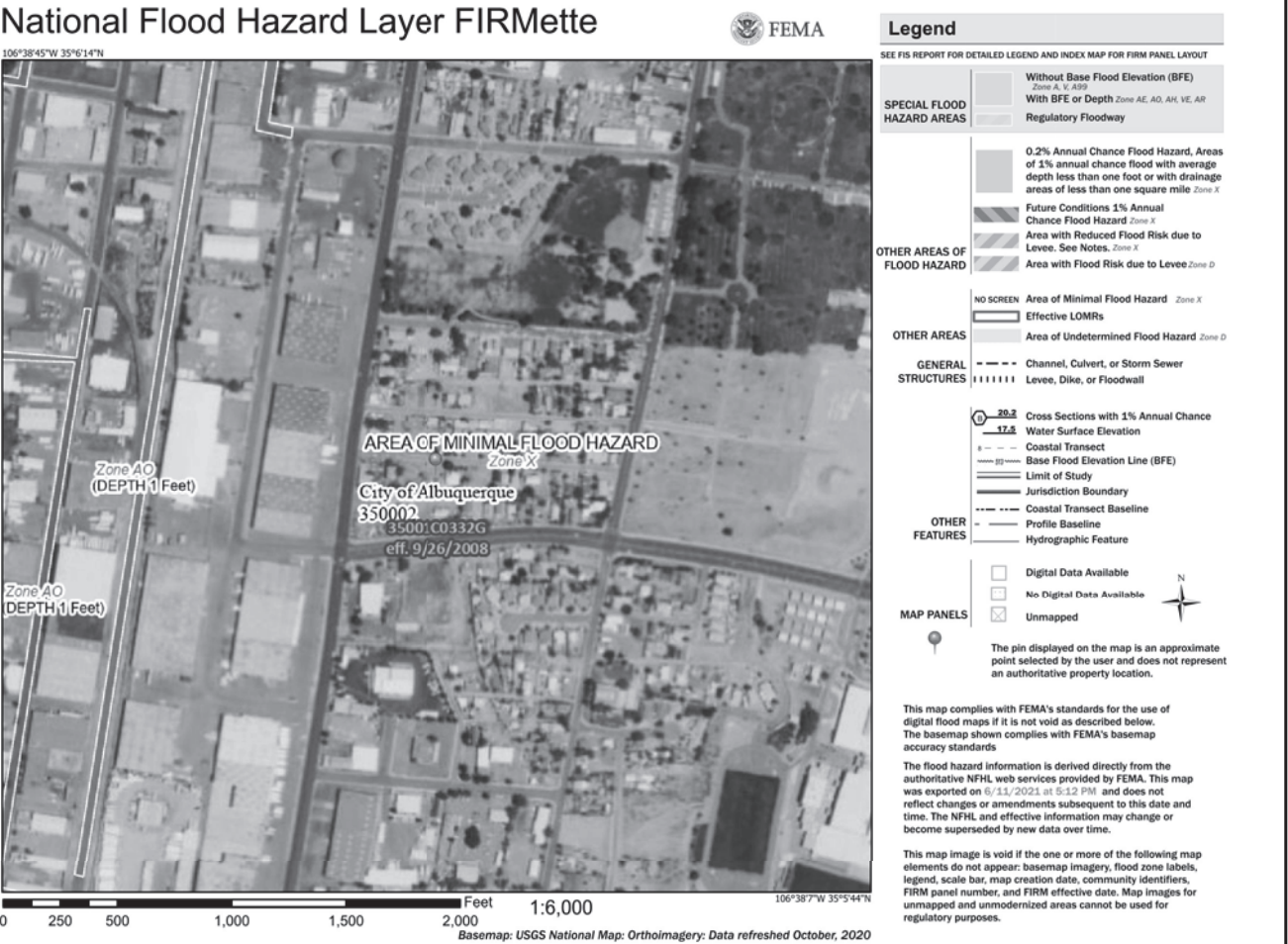


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LEGAL DESCRIPTION:

LOTS A, B, C, D, E, F GOMEZ AVENUE SUBDIVISION
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LEGEND

- XXXX--- EXISTING CONTOUR
- - - - -XXXX- - - EXISTING INDEX CONTOUR
- XXXX----- PROPOSED CONTOUR
- XXXX----- PROPOSED INDEX CONTOUR
- + XXXXX EXISTING SPOT ELEVATION
- XXXX PROPOSED SPOT ELEVATION
- BOUNDARY
- ADJACENT BOUNDARY
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ENGINEER'S SEAL 6/17/21 DAVID SOULE P.E. #14522	LOTS A, B, C, D, E, F GOMEZ AVE. SUBDIVSION GRADING AND DRAINAGE PLAN PO BOX 53924 ALBUQUERQUE, NM 87199 (505) 321-8099	DRAWN BY DEM
		DATE 6-16-21
		Gomez Ave.Subdivision.DWG
		SHEET # C1
		JOB #