

Keyin Georges & Associates
Architecture & Planning DRAINAGE CERTIFICATION I, JEFFREY G. MORTENSEN, NMPE 8547, OF THE FIRM JEFF MORTENSEN & ASSOCIATES, INC., HEREBY CERTIFY THAT THIS PROJECT HAS BEEN GRADED

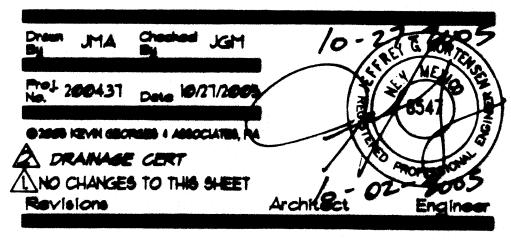
AND DRAINED IN SUBSTANTIAL COMPLIANCE WITH AND IN ACCORDANCE WITH THE DESIGN INTENT OF THE APPROVED PLAN DATED 10-02-2005 AND 10-27-2005. THE RECORD INFORMATION EDITED ONTO THE ORIGINAL DESIGN DOCUMENT HAS BEEN OBTAINED BY ME OR UNDER MY DIRECT SUPERVISION AND IS TRUE AND CORRECT TO THE BEST OF MY KNOWLEDGE AND BELIEF. THIS CERTIFICATION IS SUBMITTED IN RESPONSE TO A CONDITION OF APPROVALE SET FORTH IN THE CITY APPROVAL LETTER (J19/D2D) DATED OCTOBER 31, 20

THE RECORD INFORMATION PRESENTED HEREON IS NOT NECESSARILY COMPLETE AND INTENDED ONLY TO VERIFY SUBSTANTIAL COMPLIANCE OF THE GRADING AND DRAINAGE ASPECTS OF THIS PROJECT. THIS CERTIFICATION DOES NOT EVALUATE ADA COMPLIANCE. THOSE RELYING ON THIS RECORD DOCUMENT ARE ADVISED TO OBTAIN INDEPENDENT VERIFICATION OF ITS ACCURACY BEFORE USING IT FOR

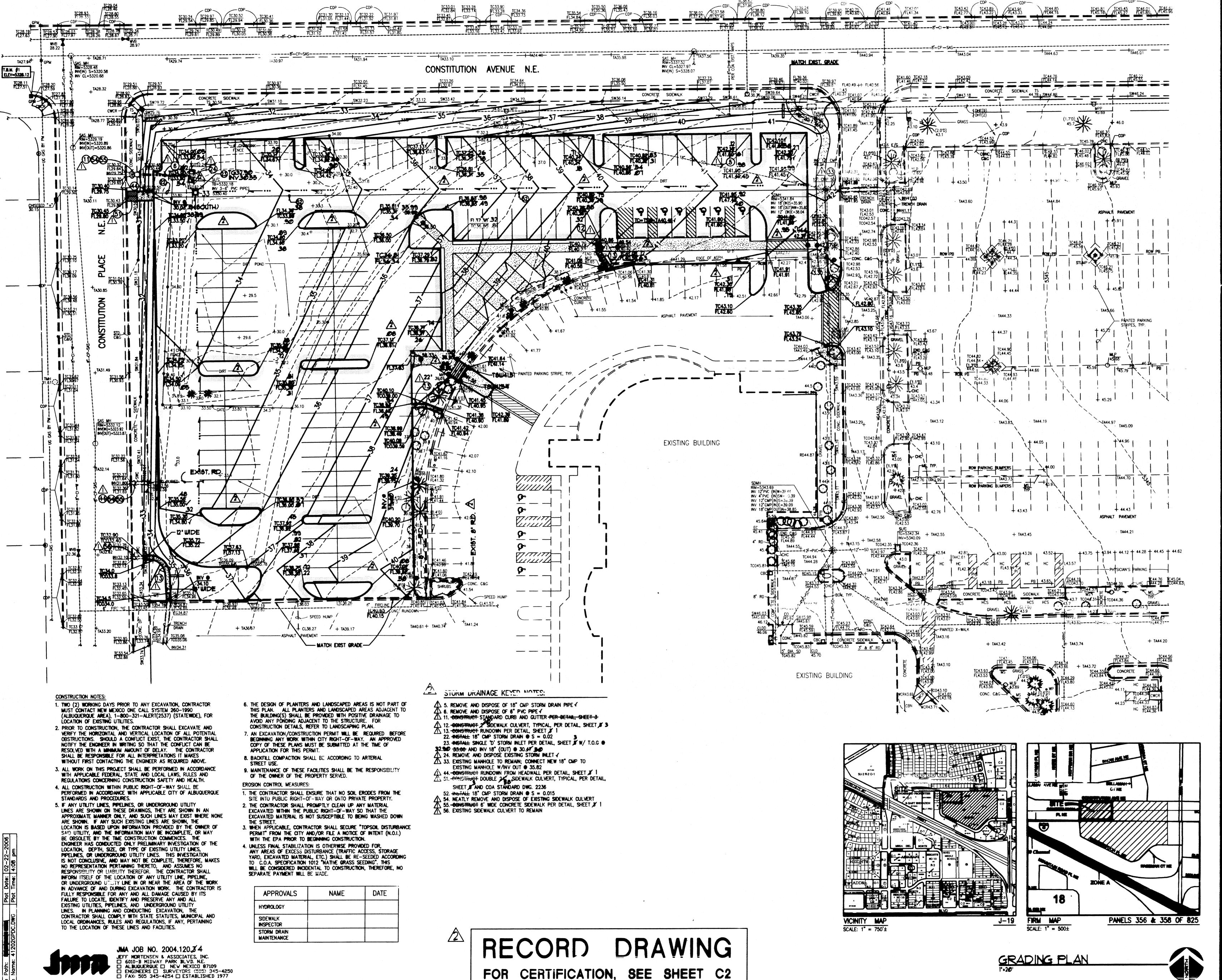
G. MORTENSEN, NMPE 8547

Final Construction Documents

Parking Area A
PMG Constitution
8300 Constitution Avenue NE
Albuquerque, New Mexico



PAVING SECTIONS AND DETAILS



FOR CERTIFICATION, SEE SHEET C2

AREA LIGHT ON CONCRETE ANTI-SIPHON VALVE BUILDING OVERHANG BIKE RACK CURB AND GUTTER CONCRETE BENCH CONCRETE DRIVEPAD CONCRETE HEADER CURB CAST IRON PIPE CENTERLINE OF DOOR CENTERLINL OF DOUBLE DOOR CHAIN LINK FENCE SANITARY SEWER CLEANOUT CONCRETE CURB OPENING CONCRETE PIPE CONCRETE RUNDOWN CONCRETE RETAINING WALL CONCRETE WHEELCHAIR RAMP EDGE OF GRASS

EDGE OF ASPHALT ELECTRIC CONDUIT ELECTRIC OUTLET ELECTRIC OUTLET IN CONCRETE ELECTRIC PULLBOX FLOWLINE FLAG POLE GAS PAINT MARK HANDICAP PARKING SIGN

IRRIGATION METAL BUILDING COLUMN MAILBOX METAL LIGHT POLE METAL POLE METAL POWER POLE METAL SIGN

OVERHEAD ELECTRIC (NO. OF LINES) OVERHEAD TELEPHONE (NO. OF LINES) PARKING BUMPER PAINTED ISLAND PLASTIC BENCH PAINT MARK

SANITARY SEWER MANHOLE SPRINKLER CONTROL TIMER STORM DRAIN STORM DRAIN MANHOLE STORM INLET STORM INLET IN CONCRETE

STANDARD CURB AND GUTTE TOP OF SIDEWALK SIDEWALK CULVERT TOP OF ASPHALT TOP OF CURB TOP OF CONCRETE TOP OF GRATE

TELEPHONE MANHOLE TELEPHONE PULLBOX TELEPHONE RISER/PULLBOX TRAFFIC SIGN TELEPHONE VAULT UNDERGROUND

VALLEY GUTTER WITH METAL HANDRAIL WOOD POLE WATER VALVE BOX CROSSWALK **EXISTING CONTOUR** ____ EXISTING SPOT ELEVATION EXISTING BOULDER

> EXISTING SHRUB **EXISTING DECIDUOUS TREE** (CALIPER SIZE)

(CALIPER SIZE)

PROJECT BENCHMARK

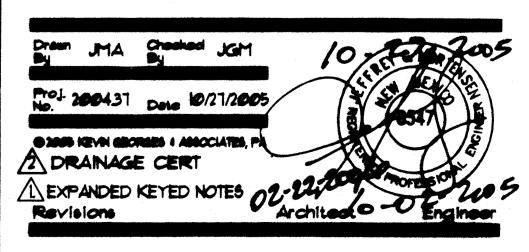
CITY OF ALBUQUERQUE Brees testet, storged "1-J19", SET IN TOP OF A CONCRETE POST WHICH SETS 0.8" NORTH OF THE NORTH EDGE OF A CONCRETE SIDEWALK, AND FLUSH WITH THE TOP OF SIDEWALK PAVEMENT ON THE NORTH SIDE OF CONSTITUTION AVE. N.E. AND BETWEEN PENNSYLVANIA BLVD. N.E. AND CONSTITUTION PL. N.E. ON TH SOUTH SIDE OF SNOW HEIGHTS PARK.

ELEVATION = \$320.05 FEET (M.S.L.D.)

TOP OF CURB ELEVATION LOCATED AT THE S.S.E. RETURN OF THE INTERSECTION OF CONSTITUTION AVE. N.E. AND CONSTITUTION PLACE N.E. AS SHOWN ON THE BRAINING ELEVATION - 5329.86 FEET (M.S.L.D.)

Final Construction Documents

Parking Area A PMG Constitution 8300 Constitution Avenue NE Albuquerque, New Mexico Probyterien Project No. MRSS62



GRADING PLAN

THIS DRAINAGE SUBMITTAL IS MADE IN SUPPORT OF A GRADING AND PAVING PERMIT AND SO #19 APPROVAL.

II. PROJECT DESCRIPTION

THE SITE IS LOCATED ON THE SOUTHEAST CORNER OF CONSTITUTION AVE NE AND CONSTITUTION PLACE NE. THE CURRENT LEGAL DESCRIPTION OF THE SITE IS TRACT 1, EAST END ADDITION. THE SITE IS CURRENTLY LOCATED IN PANEL 356 OF 825 OF THE NATIONAL FLOOD INSURANCE PROGRAM FLOOD INSURANCE RATE MAPS PUBLISHED BY FEMA FOR BERNALILLO COUNTY, NEW MEXICO, NOVEMBER 19, 2003 AND IS ZONED AS ZONE X. AN AREA DETERMINED TO BE OUTSIDE THE 0.2% ANNUAL CHANCE FLOODPLAIN. THE DEVELOPMENT PROPOSED WILL INCREASE DOWNSTREAM FLOW BUT WILL NOT ADVERSELY IMPACT DOWNSTREAM FLOW OR DOWNSTREAM PROPERTIES. RUNOFF FROM THE SITE DRAINS INTO CONSTITUTION PLACE NE., AND THEN FLOWS NORTH TO CONSTITUTION AVENUE NE. AT CONSTITUTION AVENUE NE THE FLOW IS DIRECTED WEST TO THE INTERSECTION WITH PENNSYLVANIA STREET. FLOW PROCEEDS TO ENTER THREE LARGE CURB STORM INLETS ALONG CONSTITUTION AVE JUST BEFORE THE PENNSYLVANIA INTERSECTION. THESE INLETS DRAIN INTO THE 1-40 CHANNEL.

NI. BACKGROUND DOCUMENTS

THE FOLLOWING ITEMS WERE REVIEWED IN THE PREPARATION OF THIS

A. GRADING AND DRAINAGE PLAN FOR PRESBYTERIAN KASEMAN HOSPITAL - RADIATION THERAPY CENTER ADDITION, PREPARED BY JMA AND DATED 05-08-96 FOR BUILDING PERMIT. THE 1996 PLAN RECOGNIZED THAT THE EXISTING POND REQUIRED ENLARGEMENT TO CONTAIN THE DESIGN VOLUME OF RUNOFF. THE PLAN ALSO IDENTIFIED A 100-YEAR PEAK DISCHARGE OF 10.7 CFS FROM THIS PORTION OF TRACT 1. B. DRAINAGE PLAN FOR PRESBYTERIAN KASEMAN HOSPITAL -

RADIATION THERAPY CENTER WEST EXPANSION POND ANALYSIS PREPARED BY JMA AND DATED 01-11-05. THIS PLAN CONCLUDED THAT THE NMDOT 1-40/PENNSYLVANIA OVERPASS PROJECT COMPLETED IN THE SPRING OF 2005 HAS ALLEVIATED PREVIOUS FLOODING CONCERNS ON THE CONSTITUTION AVE/PENNSYLVANIA STREET INTERSECTION. THIS PLAN USED THE CONJUGATE DEPTH EQUATION TO EVALUATE THE EFFECTS OF HYDRAULIC JUMP IN CONSTITUTION AVENUE NE. THE PLAN DETERMINED THAT THE DOWNSTREAM DRAINAGE IMPROVEMENTS THAT WERE MADE AT THE CONCLUSION OF THE 1-40/PENNSYLVANIA PROJECT ALLOWED FOR SUFFICIENT FLOW IN CONSTITUTION AVENUE WITH NO HYDRAULIC JUMP ISSUES. THIS CONCLUSION RESULTED IN THE ALLOWANCE OF THE

WELL AS FREE DISCHARGE INTO THE STREET. C. TOPOGRAPHIC SURVEY OF THE EXISTING SITE PREPARED BY JMA DATED 4/13/2005. THE SUBJECT SURVEY SHOWS THE EXISTING **IMPROVEMENTS**

ELIMINATION OF THE DETENTION POND IN THIS PROJECT AS

IV. EXISTING CONDITIONS

THE SITE IS LOCATED AT THE SOUTHEAST CORNER OF THE INTERSECTION OF CONSTITUTION AVENUE NE AND CONSTITUTION PLACE NE. THE SITE IS PARTIALLY DEVELOPED. THE JMA TOPOGRAPHIC SURVEY DATED 4/13/2005 SHOWS THAT THE SITE CONSISTS OF AN EXISTING HOSPITAL BUILDING AND AN ASPHALT PAVED PARKING LOT, AS WELL AS AN UNDEVELOPED VEGETATED AREA AT THE NORTH AND WEST THAT IS GRADED FOR THE PURPOSE OF CREATING A DETENTION PONDING AREA FOR BOTH ON-SITE RUNOFF AND OFF-SITE FLOWS. THE SITE SLOPES FROM SOUTHEAST TO NORTHWEST AND DRAINS INTO THE POND AT THE NORTHWEST CORNER. THIS POND DRAINS WA STORM DRAIN THROUGH AN EXISTING SIDWALK CULVERT INTO CONSTITUTION PLACE NE. CONSTITUTION AVENUE IS A FULLY IMPROVED ROADWAY, 40 FEET WIDE WITH CURB AND GUTTER ON BOTH SIDES. CONSTITUTION PLACE FLOWS ARE CARRIED BY CURB AND GUTTER NORTH INTO CONSTITUTION AVENUE, WHERE IT FLOWS WEST TO AN INTERSECTION WITH PENNSYLVANIA STREET. JUST BEFORE THIS INTERSECTION THERE ARE THREE LARGE CURB STORM INLETS ALONG CONSTITUTION AVE NE WHERE THE FLOWS WILL DRAIN IN TO, THE INLETS DRAIN SOUTH INTO THE 1-40 CHANNEL.

ONSITE CONTRIBUTING AREA FLOWS FROM A PARKING LOT ALONG THE SOUTH EDGE OF THE SITE ENTER AT THE SOUTHWEST CORNER OF THE SITE THROUGH A TRENCH DRAIN AND CURB CUT; THE FLOWS EMPTY NORTH INTO THE DETENTION POND. ONSITE CONTRIBUTING AREA FLOWS FROM AN ADJACENT PARKING LOT ALSO ENTER THE SITE THROUGH A TRENCH DRAIN INTO A 12" STORM DRAIN THAT RELEASES INTO THE MANHOLE AT THE EAST EDGE OF THE SITE; THESE FLOWS ENTER INTO A 18" CMP STORM DRAIN FROM THE ONSITE MANHOLE AND EMPTIES THESE FLOWS INTO THE SITE AT THE NORTH EDGE OF THE SITE: THESE FLOWS DRAIN WEST INTO THE DETENTION POND AS SHOWN ON THE TCL.

V. DEVELOPED CONDITIONS

THE PROJECT CONSIST OF THE CONSTRUCTION OF A NEW ASPHALT PAVED PARKING LOT THAT WILL SERVE THE EXISITING HOSPITAL BUILDINGS IN THE AREA. THIS NEW LOT WILL REPLACE THE NATURALLY VEGETATED AREA ALONG THE NORTH AND WEST OF THE SITE, AND WILL ELIMINATE THE EXISTING DETENTION POND. AS A RESULT OF THESE IMPROVEMENTS THERE WILL BE AN INCREASE IN THE RUNOFF THE SITE GENERATES AS WELL AS A REDIRECTION OF ONSITE CONTRIBUTING AREA FLOWS AROUND THE SITE THAT CURRENTLY TRAVERSE THE SITE. A SMALL PORTION OF THE ONSITE RUNOFF WILL BE PONDED AT THE NORTHWEST CORNER OF THE DEVELOPED PARKING LOT, WHICH WILL BE DRAINED THROUGH A STORM INLET AND STORM DRAIN INTO A RUNDOWN AND SIDWALK CULVERT INTO CONSTITUTION PLACE NE AS SHOWN ON THE DEVELOPED GRADING PLAN. THIS STORM DRAIN PIPE WILL DRAIN THE CAPTURED STORMWATER RUNOFF IN A PERIOD LESS THAN SIX HOURS. FLOWS INTO CONSTITUTION PLACE NE WILL FOLLOW THE SAME DRAINAGE PATH AS STATED IN THE EXISTING CONDITIONS.

AS THE HYDROGRAPH CALCULATIONS SHOW, THE VOLUME REQUIRING PONDING WILL EQUAL 220 CF. THIS VOLUME WILL REACH A HEIGHT ABOVE THE STORM INLET OF 5333.2 FT. THE LOWEST POINT OF THE TOP OF CURB IN THE PONDING AREA IS AT 5334 FT, THIS IS ALSO THE LOWEST TOP OF CURB IN THE ENTIRE PROJECT SITE. THEREFORE, THERE IS NO PROBLEM OF RUNOFF OVERFLOWING THE CURB.

ONSITE CONTRIBUTING AREA FLOWS THAT ENTER THE MANHOLE AT THE EAST CORNER OF THE SITE WILL BE DIRECTED THROUGH THE SITE VIA A NEW 18" CMP STORM DRAIN THAT WILL TRAVERSE THE SITE FROM EAST TO WEST AND DRAIN INTO CONSTITUTION PLACE NE AT THE SAME LOCATION AS THE ONSITE FLOW STORM DRAIN RELEASES ITS FLOWS AS SHOWN ON THE DEVELOPED GRADING PLAN.

THE OVERALL RESULT OF THE DEVELOPMENT WILL INCREASE THE VOLUME AND PEAK DISCHARGE RATE INTO CONSTITUTION PLACE NE. THIS INCREASE IS ALLOWED DUE TO THE COMPLETION OF DEVELOPMENT AT PENNSYLVANIA STREET AND 1-40 THAT ALLEVIATED PAST DOWNSTREAM FLOW ISSUES: ISSUES THAT HAD PREVIOUSLY RESULTED IN THE CREATION OF THE EXISTING ONSITE DETENTION POND. U

THE GRADING PLAN SHOWS: 1.) EXISTING GRADES INDICATED BY SPOT ELEVATIONS AND CONTOURS AT 1'-0" INTERVALS BASED THE TOPOGRAPHIC SURYEY PREPARED BY JMA, DATED 4/13/2005, 2.) PROPOSED GRADES INDICATED BY CONTOURS AT 1'-0" INTERVALS, 3.) THE LIMIT AND CHARACTER OF THE EXISTING IMPROVEMENTS AS TAKEN FROM THE AFORE MENTIONED JMA SURVEY, 4.) THE LIMIT AND CHARACTER OF THE PROPOSED IMPROVEMENTS, 5.) CONTINUITY BETWEEN EXISTING AND PROPOSED GRADES. AS SHOWN BY THIS PLAN, THE PROPOSED DEVELOPMENT CONSISTS OF PAVED PARKING, ALONG WITH ASSOCIATED LANDSCAPING AND SIDEWALK. THESE MODIFICATIONS TO THE SITE WILL EXPAND AVAILABLE ONSITE PARKING AND VEHICLE ACCESS. AN EROSION CONTROL PLAN WILL BE PREPARED AND ATTACHED TO THE SEPARATE STORM WATER POLLUTION PREVENTION PLAN FOR THIS

JMA JOB NO. 2004.120.3/4 MEFF MORTENSEN & ASSOCIATES, INC. 6010-B MIDWAY PARK BLVD. N.E. ALBUQUERQUE | NEW MEXICO 87109 | ENGINEERS | SURVEYORS (505) 345-4250 | FAX: 505 345-4254 | ESTABLISHED 1977 VI. GRADING PLAN

THE GRADING PLAN SHOWS: 1.) EXISTING GRADES INDICATED BY SPOT ELEVATIONS AND CONTOURS AT 1'-0" INTERVALS BASED THE TOPOGRAPHIC SURVEY PREPARED BY JMA, DATED 4/13/2005, 2.) PROPOSED GRADES INDICATED BY CONTOURS AT 1'-0" INTERVALS, 3.) THE LIMIT AND CHARACTER OF THE EXISTING IMPROVEMENTS AS TAKEN FROM THE AFORE MENTIONED JMA SURVEY, 4.) THE LIMIT AND CHARACTER OF THE PROPOSED IMPROVEMENTS, 5.) CONTINUITY BETWEEN EXISTING AND PROPOSED GRADES. AS SHOWN BY THIS PLAN, THE PROPOSED DEVELOPMENT CONSISTS OF PAVED PARKING, ALONG WITH ASSOCIATED LANDSCAPING AND SIDEWALK, THESE MODIFICATIONS TO THE SITE WILL EXPAND AVAILABLE ONSITE PARKING AND VEHICLE ACCESS. AN EROSION CONTROL PLAN WILL BE PREPARED AND ATTACHED TO THE SEPARATE STORM WATER POLLUTION PREVENTION PLAN FOR THIS PROJECT.

VII. CALCULATIONS

THE CALCULATIONS WHICH APPEAR HEREON ANALYZE BOTH THE EXISTING AND DEVELOPED CONDITIONS FOR THE 100-YEAR. 6-HOUR RAINFALL EVENT, AS WELL AS THE 10-YEAR, 6 HOUR RAINFALL EVENT FOR CULVERT DESIGN PURPOSES. THE PROCEDURE FOR 40-ACRE AND SMALLER BASINS, AS SET FOR THE IN THE REVISION OF SECTION 22.2. HYDROLOGY OF THE DEVELOPMENT PROCESS MANUAL, VOLUME 2. DESIGN CRITERIA, DATED JANUARY, 1993, HAS BEEN USED TO QUANTIFY THE PEAK RATE OF DISCHARGE AND VOLUME OF RUNOFF GENERATED BY THIS DEVELOPMENT. THE CAPACITY OF THE DEVELOPED 18" CMP STORM DRAINS WAS ANALYZED USING THE ORIFICE AND MANNING'S EQUATIONS. THE CAPACITIES OF THE NEW STORM INLETS WERE DETERMINED USING THE ORIFICE EQUATION. THE CAPACITY OF THE NEW 24" CULVERTS ALONG CONSTITUTION PLACE NE WERE DETERMINED USING MANNING'S EQUATION. THE PERFORMANCE OF THE PRIVATE STORMWATER DETENTION SYSTEM IN THE NORTH EAST CORNER OF THE PARKING AREA HAS BEEN EVALUATED FOR OUTFLOW AND STORAGE CAPACITY BASED UP THE HYDROGRAPH ANALYSIS CONTAINED ON THIS SHEET, AND PONDING IS NECESSARY ONLY DUE TO DISCHARGE PIPE SIZING CONSTRAINTS.

THE JANUARY 2005 CALCULATIONS ARE SHOWN HEREON TO SHOW THAT THE DOWNSTREAM STREET (CONSTITUTION AVENUE) IS CAPABLE OF HANDLING FLOWS FROM ONSITE AND OFFSITE, THE CONJUGATE DEPTH EQUATION WAS USED TO EVALUATE THE EFFECTS OF HYDRAULIC JUMP. AS SHOWN BY THESE CALCULATIONS, THE NORMAL DEPTH OF FLOW PLUS THE EFFECTS OF A HYDRAULIC JUMP COMPLY WITH DPM CRITERIA TO FALL BELOW THE TOP-OF-CURB PLUS 0.2 FEET. THIS SUGGESTS THAT THE STREET CAN HANDLE THE FULLY DEVELOPED DESIGN FLOW AS CALCULATED AND THAT PONDING IS NO LONGER NECESSARY.

VIII. CONCLUSION

THE DRAINAGE CONCEPT SHOWN BY THE PLAN ABOVE WILL ACCOMPLISH

- ITS PURPOSE DUE TO THE FOLLOWING FACTORS: A.) MODIFICATION TO AN EXISTING SITE WITHIN AN INFILL AREA B.) THE PEAK RATE OF DISCHARGE FROM THE SITE WILL BE
 - INCREASED ABOVE THAT OF THE EXISTING CONDITION C.) REMOVAL OF THE EXISTING POND AS ALLOWED BY APPROVED DRAINAGE SUBMITTAL DATED 01-11-05
- D.) ADEQUATE STREET CAPACITY IS AVAILABLE IN CONSTITUTION PLACE NE TO ACCOMMODATE PEAK DISCHARGE UNDER FULLY
- DEVELOPED CONDITIONS E.) DOWNSTREAM CAPACITY OF CONSTITUTION PLACE AND CONSTITUTION AVENUE AND THE STORM DRAIN STRUCTURES LOCATED WITHIN ARE ADEQUATE FOR INCREASED FLOWS DUE
- TO THE NMDOT 1-40/PENINSYLVANIA PROJECT IMPROVEMENTS F.) FREE DISCHARGE INTO CONSTITUTION PLACE AND CONSTITUTION AVENUE PER THE DRAINAGE PLAN DATED 1-11-05
- DOWNSTREAM PROPERTIES
- H.) CULVERTS AT CONSTITUTIONS PLACE SUFFICIENT TO HANDLE PEAK DISCHARGE FROM BOTH STORM DRAINS UNDER FULLY **DEVELOPED CONDITIONS**
- I.) GRADING AND CURB ABOUT STORM INLET AREA SUFFICIENT TO CONTAIN MORE THAN THE 220 CF OF PONDING REQUIRED DURING A 100-YEAR STORM

J.) OFFSITE FLOWS WILL CONTINUE TO BE INTERCEPTED AND CONVEYED THROUGH THE PROJECT TO DOWNSTREAM EXISTING DRAINAGE FACILITIES

CALCULATIONS

SITE CHARACTERISTICS

1. PRECIPITATION ZONE = 3

2. $P_{8,160} = P_{360} = 2.60$ 2. $P_{8,10} = P_{360} = 1.73$

3. TOTAL AREA (At) = 143,065 SF

4. EXISTING LAND TREATMENT

IREALMENT

A. PROJECT SITE AREA (SF/AC) 16,779 / 0.39 12 61,626 / 1.41 43 64,660 / 1.48 45

B. ONSITE CONTRIBUTING AREAS TREATMENT AREA (SF/AC) % 70,000 / 1.61 100

5. DEVELOPED LAND TREATMENT

A. PROJECT SITE TREATMENT AREA (SF/AC) 23,552 / 0.54 16 119,533 / 2.74 84

6. EXISTING CONDITION

A. PROJECT SITE 1. VOLUME

 $Ew = (E_AA_A + E_BA_B + E_CA_C + E_BA_D)/AT$ EW = ((0.92*0.39)+(1.29*1.41)+(2.36*1.48))/3.28 = 1.73 IN $V_{100} = (Ew/12)AT = (1.73/12)3.28 = 0.2317 AC-FT 20.628 CF$

2. PEAK DISCHARGE QP = QPA AA + QPBAB + QPCAC + QPBAD

QP = Q100 = (2.60*0.39)+(3.45*1.41)+(5.02*1.48) = 13.33 CFS

B. ONSITE CONTRIBUTING AREAS (100-YEAR STORM) 1. VOLUME Ew = (EAAA+EBAB+ECAC+EBAD)/AT

Ew = (2.36*1.61)/1.61 = 2.36 IN $V_{100} = (Ew/12)AT = (2.36/12)1.61 = 0.3160 AC-FT = 13,767 CF$

2. PEAK DISCHARGE Qr = Qrx Ax + QreAs + QrcAc + QreAo

QP = Q100 = (5.02 + 1.61) = 8.07 CFS

C. ONSITE CONTRIBUTING AREAS (10-YEAR STORM) 1. VOLUME

Ew = (EAAA+EBAB+EcAc+EBAB)/ATEw = (1.50*1.61)/1.61 = 1.50 IN

 $V_{10} = (Ew/12)AT = (1.50/12)1.61 = 0.2009 AC-FT = 8,750 CF$

2. PEAK DISCHARGE QP = QPA AA + QPBAB + QPCAC + QPBAD QP = Q160 = (3.39*1.61) = 5.45 CFS

7. DEVELOPED CONDITION

A. PROJECT SITE (100-YEAR STORM)

1. VOLUME Ew = (EAAA+EBAB+EcAc+EBAD)/AT

EW = ((0.92*0.54)+(2.36*2.74))/3.28 = 2.12 IN $V_{100} = (Ew/12)AT = (2.12/12)3.28 = 0.5811 AC-FT = 25,314 CF$

2. PEAK DISCHARGE

Qr = Qm An + QraAs + QrcAc + QraAo QP = Q100 = (2.60*0.54)+(5.02*2.74) = 15.18 CFS

B. ONSITE CONTRIBUTING AREAS (10-YEAR STORM)

1. VOLUME EW = (EAA+EBAB+EcAc+EDAD)/AT

Ew = ((0.36*0.54)+(1.50*2.74))/3.28 = 1.31 IN $V_{10} = (Ew/12)AT = (1.31/12)3.28 = 0.3592 AC-FT = 15,648 CF$

2. PEAK DISCHARGE Qr = Qm An + QraAs + QrcAc + QraAo QP = Q10 = (1.19 + 0.54) + (3.39 + 2.74) = 9.95 CFS

RAINSTORM PONDING CAPACITY

AREA (SQ FT) VOLUME (CF) TOTAL VOL. (CF) **5333.**5 2675 [(2675+0)/2] + 0.5 = 6705334 **6**575 [(6575+2675)/2]+0.5 = 9751625

VPROVIDED = 1,625 CF>VNEQ = 240 CF (SEE HYDROGRAPH)

A HYDROGRAPH CALCULATIONS $T_P = 0.7*Tc+(1.6-Ap/At)/12$

TP = 0.7 + 0.2 + (1.6 - 2.74/3.28)/12TP = 0.2037 hr

DP = 0.25 + Ao/ATDP = 0.25 + 2.74 / 3.28 $DP = 0.2088 \ hr$

TB = 2.017 + E + AT/QP - 0.25 + AD/AT $T_8 = 2.017 + 2.12 + 3.28 / 15.18 - 0.25 + 2.74 / 3.28$ Ta = 0.7563 hr

AREA OF HYDROGRAPH = V100 = 26.410 CF

VOLUME REQUIRING PONDING Vector = (15.18-14.90) * ((((25.16-11.98)+(24.75-12.22))/2) * 60)VAREOUD = 215.6 CF

PEAK DISCHARGE CAPACITY OF ON-SITE FLOW DISCHARGE PIPE $Q_{CAP} = 1.49/n AR^{2/3} S^{1/2}$ n = 0.013

 $A = pi + r^2 = pi + (0.75)^2 = 1.767 FT^2$ P = 2 * pi * r = 2 * pi * 0.75 = 4.712 FTR = A/P = .375 FT

S = 0.02 FT/FT $Q_{CAP} = 14.9 CFS$ Q100 = 15.2 CFS>QCP =14.9 CFS, 215 CF OF PONDING REQ'D

CULVERT CALCULATIONS FOR SITE DRAINAGE A. PEAK DISCHARGE CAPACITY OF SINGLE CULVERT

 $Q_{CAP} = 1.49/n AR^{2/3} S^{1/2}$ n = 0.013 $A = 0.667 \text{ FT+2 FT} = 1.33 \text{ FT}^2$ P = 0.667 FT + 0.667 FT + 2 FT = 3.33 FTR = A/P = 0.4 FT

S = 0.02 FT/FTQcr = 11.73 CFSB. CULVERT DESIGN TO HOLD 10-YEAR, 6-HOUR STORM

 $Q_{10,ON-SITE} = 9.95 CFS$ Q10,000000 = 5.45 CFS $Q_{10,REQ'D} = 9.95 + 5.45 = 15.4 CFS$ Q10,REQ10 = 15.4 CFS>QCAP = 11.75 CFS

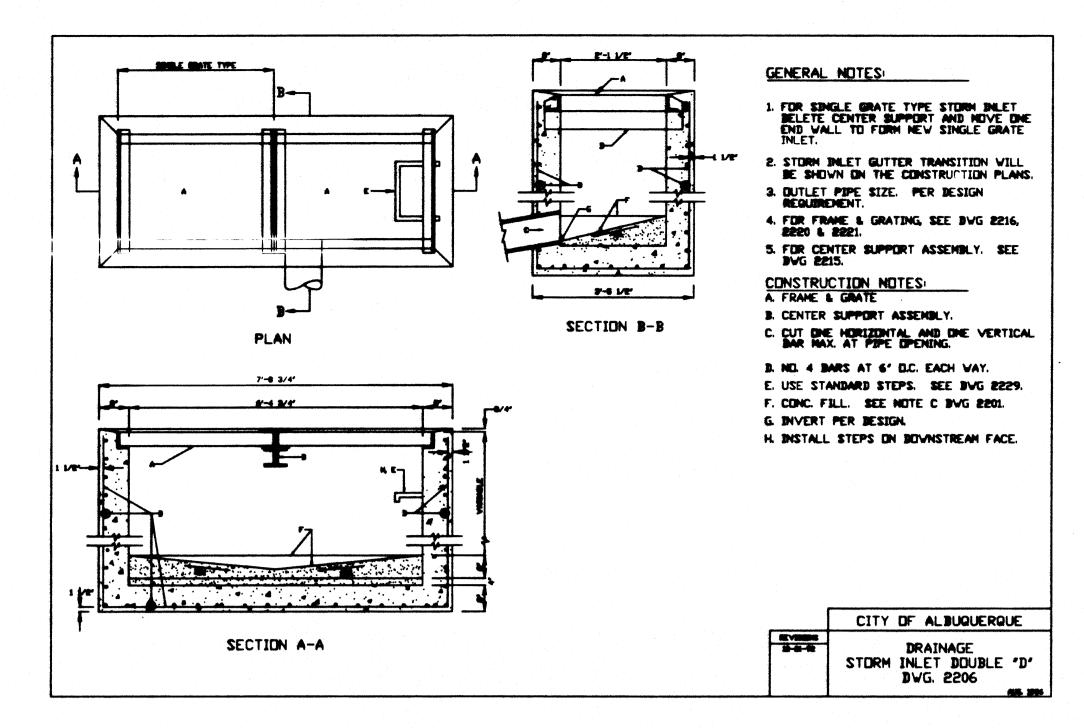
Q16,REQ10 = 15.4 CFS<2*QCAP = 23.46 CFS

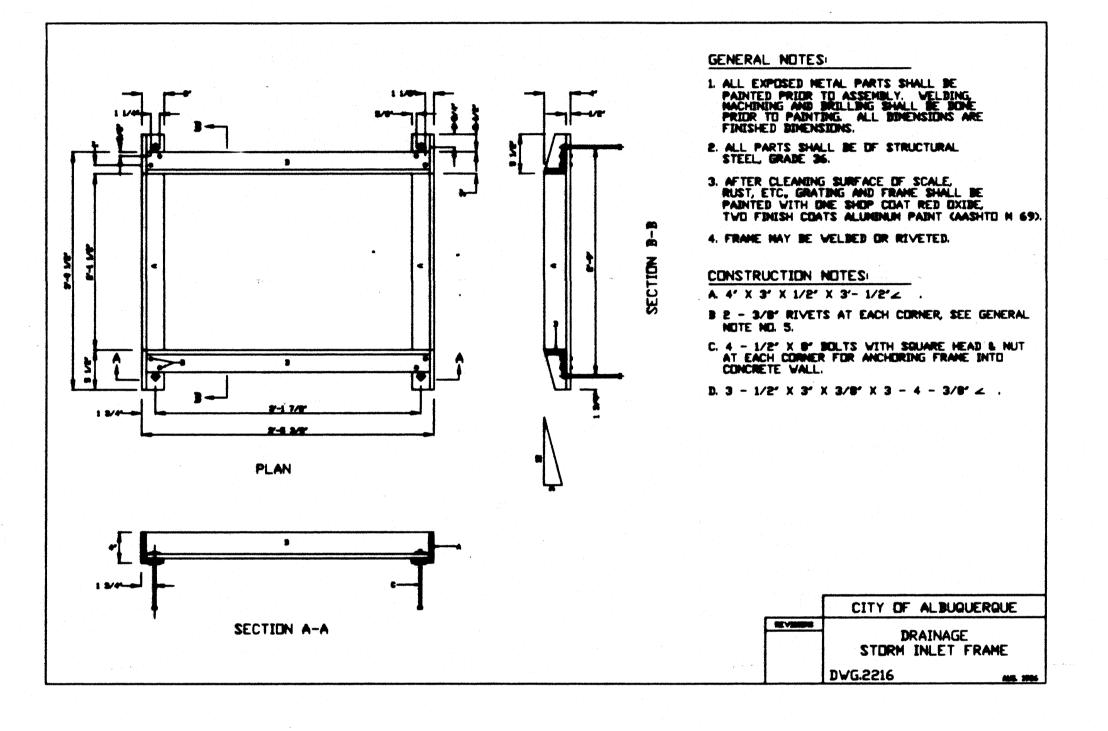
DESIGN REQUIRES TWO CULVERTS 8. COMPARISON

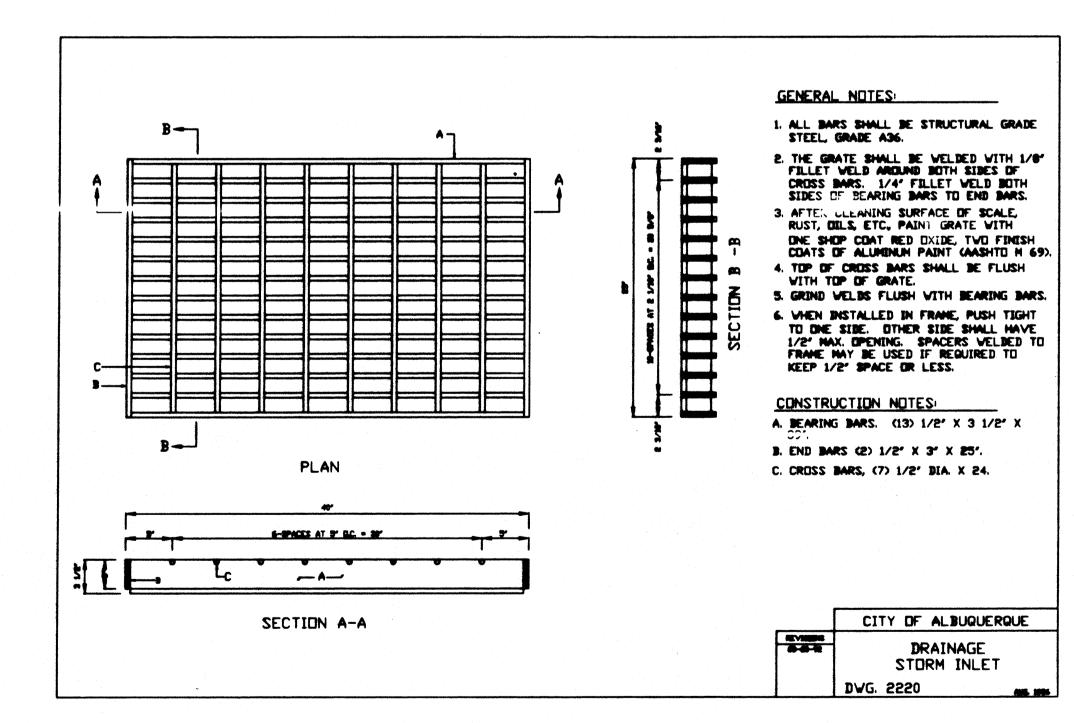
A. VOLUME \triangle V₁₀₀ = 25,315-20,628 = 4,687 CF (INCREASE)

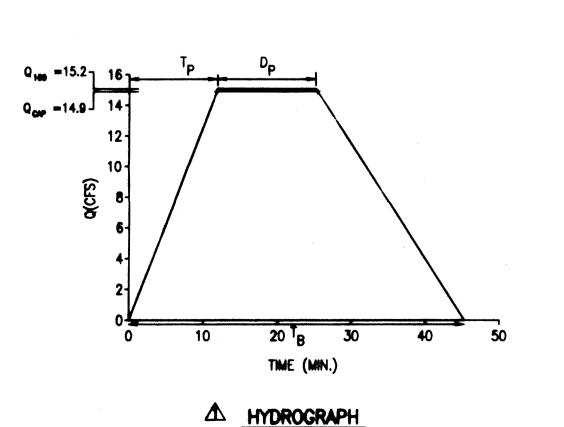
B. PEAK DISCHARGE Δ Q100 = 15.18-13.33 = 1.85 CFS (INCREASE)

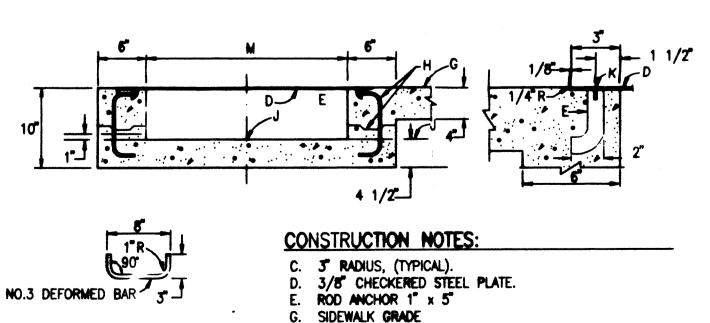
CERTIFICATION, SEE SHEET C2











TYPICAL SIDEWALK CULVERT DETAILS NOT TO SCALE

DOWEL AND JOINT, (OPTIONAL)

M. DRAIN WIDTH, 24" MAX. 12" MIN.

K. 3/8" x 1" F.H. C'SUNK STAINLESS STEEL MACHINE SCREW.

GUTTER FLOWLINE ELEV.

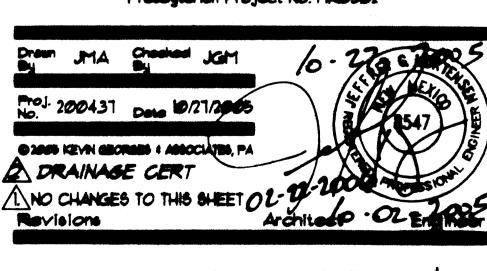
214 Trumen Street NE - Albuquerque, New Mexico STIGS-1933 969/200-4875

Kevin Georges & Associates

Architecture 4 Planning

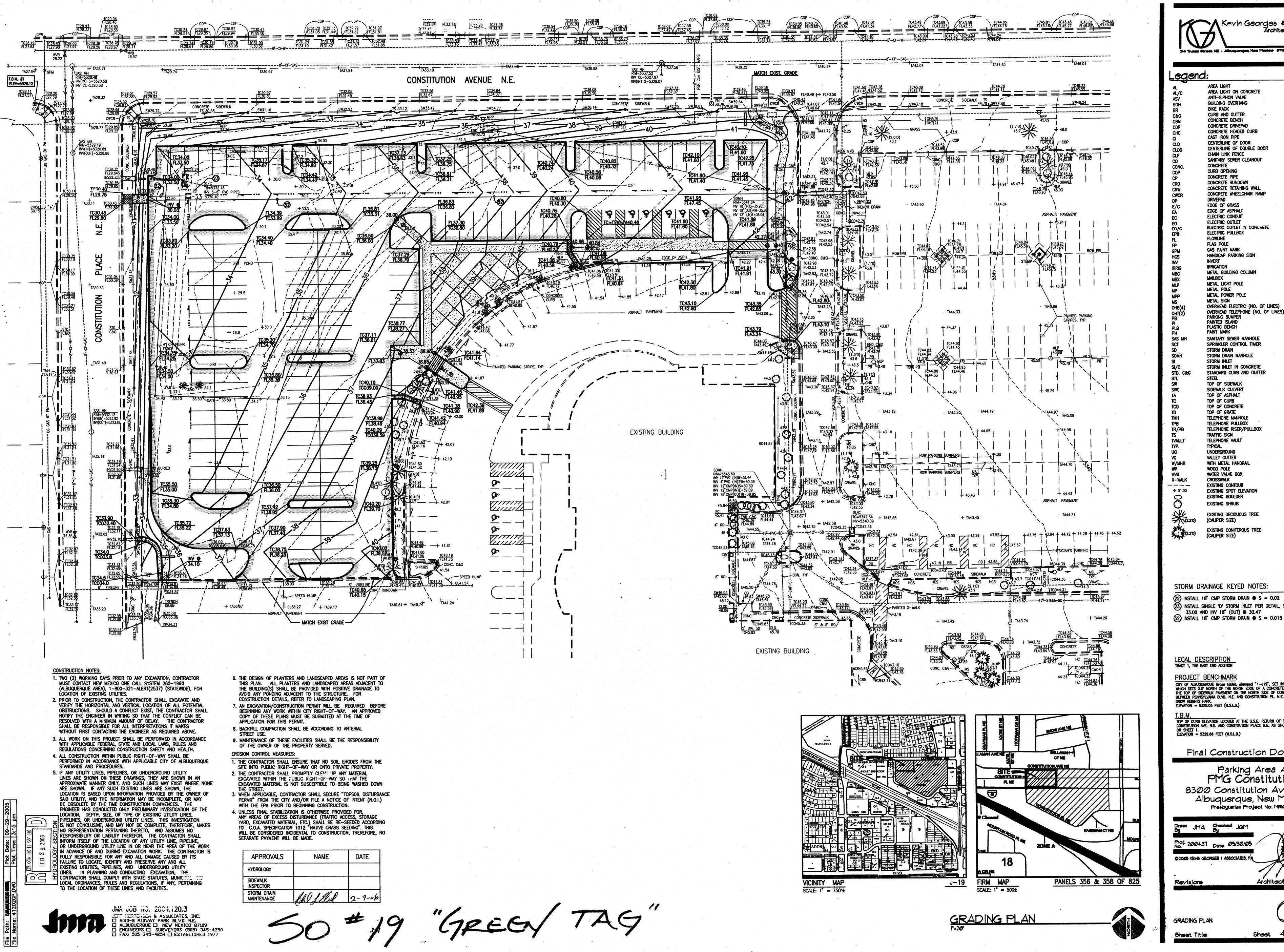
Final Construction Documents

Parking Area A PMG Constitution 8300 Constitution Avenue NE Albuquerque, New Mexico Presbyterian Project No. MR6562



CALCULATIONS, DRAINAGE PLAN AND SECTIONS AND DETAILS Sheet Title





OVERHEAD ELECTRIC (NO. OF LINES)

(22) INSTALL 18" CMP STORM DRAIN • S = 0.02 (23) INSTALL SINGLE 'D' STORM INLET PER DETAIL, SHEET 4 W/ T.O.G

CITY OF ALBUQUERQUE Bross tablet, stamped "1-J19", SET IN TOP OF A CONCRETE POST WHICH SETS 0.6" NORTH OF THE NORTH EDGE OF A CONCRETE SIDEWALK, AND FLUSH WITH THE TOP OF SIDEWALK PAVEMENT ON THE NORTH SIDE OF CONSTITUTION AVE. N.E. AND BETWEEN PENNSYLVANIA BLVD. N.E. AND CONSTITUTION PL. N.E. ON TH SOUTH SIDE OF SNOW HEIGHTS PARK.

ELEVATION = 5320.05 FEET (M.S.L.D.)

TOP OF CURB ELEVATION LOCATED AT THE S.S.E. RETURN OF THE INTERSECTION OF CONSTITUTION AVE. N.E. AND CONSTITUTION PLACE N.E. AS SHOWN ON THE DRAWING ON SHEET 1.

ELEVATION = 5329.86 FEET (M.S.L.D.)

Final Construction Documents

Parking Area A PMG Constitution 8300 Constitution Avenue NE

Albuquerque, New Mexico Presbyterian Project No. MRØ502

GENERAL NOTES

1. OBTAIN ARCHITECT'S APPROVAL OF SIDEWALK LAYOUT AND JOINTS PRIOR TO PLACEMENT OF CONCRETE

SITE PLAN KEYED NOTES:

1. NEATLY SANCUT, REMOVE AND DISPOSE OF EXISTING CURB AND 2. NEATLY SAWCUT, REMOVE AND DISPOSE OF EXISTING SIDEWALK 3. REMOVE AND DISPOSE OF EXISTING TREES AND SHRUBS 4. NEATLY SAWCUT, REMOVE AND DISPOSE OF EXISTING ASPHALT PAVING, 1'-0" MIN. WIDTH

5. REMOVE AND DISPOSE OF 18" CMP STORM DRAIN PIPE

6. REMOVE AND DISPOSE OF 6" PVC PIPE 7. REMOVE AND DISPOSE OF 4' CHAINLINK FENCE 8. EXISTING CONCRETE RUNDOWN TO REMAIN 9. EXISTING CURB AND GUTTER TO REMAIN 10. CONSTRUCT MULTI-DIRECTIONAL WHEELCHAIR RAMP PER DETAIL, SHEET 3 11. CONSTRUCT STANDARD CURB AND GUTTER PER DETAIL, SHEET 3
12. CONSTRUCT 2' SIDEWALK CULVERT, TYPICAL, PER DETAIL, SHEET 5

13. CONSTRUCT RUNDOWN PER DETAIL, SHEET 3 14. CONSTRUCT 5' WIDE CONCRETE SIDEWALK PER DETAIL, SHEET 3 15. CONSTRUCT CONCRETE STAIRWAY PER DETAIL, SHEET 3 16. CONSTRUCT CONCRETE SIDEWALK PER DETAIL, SHEET 3: SEE LANDSCAPING PLAN FOR LAYOUT

17. CONSTRUCT NEW ASPHALT PAVEMENT PER TYPICAL PARKING LOT PAVEMENT SECTION, SHEET 3 18. PAINT WHITE DIRECTIONAL ARROWS WITH WHITE TRAFFIC PAINT, MIN 2 COATS PER MUTCD DEC 2000 19. PAINT 4" PAVEMENT MARKING W/ WHITE TRAFFIC PAINT, MIN. 2

20. PAINT CROSSWALK PAVEMENT MARKING PER DETAIL, SHEET 3 21. NEW LANDSCAPING, SEE LANDSCAPING PLAN 22. INSTALL 18" CMP STORM DRAIN • S = 0.02 23. INSTALL SINGLE "D" STORM INLET PER DETAIL, SHEET 5·W/ T.O.G ● 33.00 AND MV 18" (OUT) @ 30.47

24. REMOVE AND DISPOSE EXISTING STORM INLET 25. STENCIL "COMPACT" ON PARKING LOT PAVEMENT 26. PAINT TOP AND FACE OF CURB W/ YELLOW TRAFFIC PAINT, MIN. 2 27. BEGIN PAINTING TOP AND FACE OF CURB WITH YELLOW TRAFFIC

28. END PAINTING TOP AND FACE OF CURB WITH YELLOW TRAFFIC 29. STENCIL "NO PARKING" ON TOP AND FACE OF CURB AT 50' SPACING 30. BEGIN STENCILING "NO PARKING" ON TOP AND FACE OF CURB

31. END STENCILING "NO PARKING" ON TOP AND FACE OF CURB

32. REMOVE AND DISPOSE 4' DOUBLE SWING GATE

A 33. EXISTING MANHOLE TO REMAIN; CONNECT NEW 18" CMP TO EXISTING MANHOLE W/INV OUT ● 35.82 34. INSTALL SALVAGED CONCRETE PARKING BUMPERS • EDGE OF FLUSH SIDEWALK PER DETAIL, SHEET 3 35. CONSTRUCT RAMPING CONCRETE SIDEWALK PER DETAIL, SHEET 3 (MAX 12:1 SLOPE) 36. NEATLY SAWCUT, REMOVE AND DISPOSE OF EXISTING CONCRETE

37. EXISTING CONCRETE RUNDOWN TO REMAIN (3')
38. NEW LIGHT STANDARD AND BASE, SEE ELECTRICAL
39. CONSTRUCT CONCRETE SIDEWALK FLUSH WITH PAVING PER DETAIL, SHEET 3 40. NEATLY REMOVE AND SALVAGE EXISTING PARKING BUMPERS
41. INSTALL 1 - ADA COMPLIANT HANDICAP PARKING SIGN 42. INSTALL 1 - ADA COMPLIANT HANDICAP PARKING SIGN W/ VAN

ACCESSIBLE PLACARD

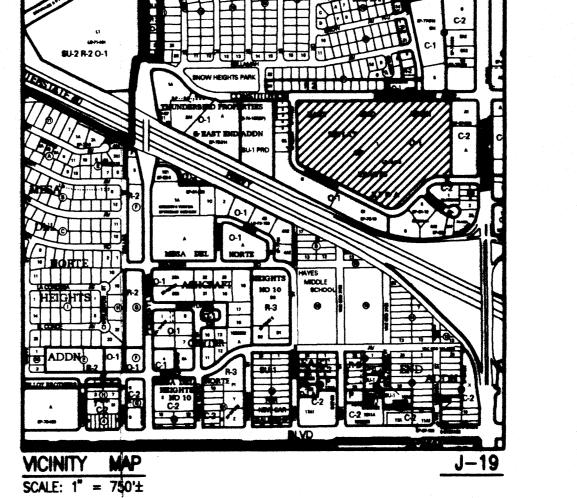
43. NEW GROUND MOUNTED SIGN BY OWNER, PROVIDE POWER, COORDINATE REQUIREMENTS WITH SIGN COMPANY 44. CONSTRUCT RUNDOWN FROM HEADWALL PER DETAIL, SHEET 3 45. REMOVE AND DISPOSE EXISTING CONCRETE RUNDOWN 46. EXISTING TRENCH DRAIN TO REMAIN 47. INSTALL 1 - ADA COMPLIANT HANDICAP PARKING SPACE PAVEMENT MARKING, TYPICAL 48. PAINT 4" WIDE CROSS-HATCH (2' CC) PAVEMENT MARKINGS @ 45

DEG W/ WHITE TRAFFIC PAINT, MIN 2 COATS, TYPICAL 49. EXISTING LIGHT POLE TO REMAIN 50. EXISTING TREES AND SHRUBS TO REMAIN 51. CONSTRUCT DOUBLE 24" SIDEWALK CULVERT, TYPICAL, PER DETAIL, SHEET 5 AND COA STANDARD DWG. 2236 52. INSTALL 18" CMP STORM DRAIN • S = 0.015 53. CLEAR SIGHT TRIANGLE

4\(\text{54. NEATLY REMOVE AND DISPOSE OF EXISTING SIDEWALK CULVERT

55. CONSTRUCT 6' WIDE CONCRETE SIDEWALK PER DETAIL, SHEET 3

56. EXISTING SIDEWALK CULVERT TO REMAIN



PAYING SITE PLAN (TCL





ANTI-SIPHON VALVE CONCRETE BENCH CONCRETE DRIVEPAD CONCRETE HEADER CURE CAST IRON PIPE CENTERLINE OF DOOR CENTERLINE OF DOUBLE DOOR CHAIN LINK FENCE SANITARY SEWER CLEANOUT CONCRETE PIPE CONCRETE RUNDOWN CONCRETE RETAINING WAL CONCRETE WHEELCHAIR RAMP ELECTRIC CONDUIT ELECTRIC OUTLET

ELECTRIC OUTLET IN CONCRETE ELECTRIC PULLBOX METAL LIGHT POLE METAL POWER POLE

OVERHEAD ELECTRIC (NO. OF LINES) OVERHEAD TELEPHONE (NO. OF LINE PAINTED ISLAND PLASTIC BENCH SANITARY SEWER MANHOLE

STORM INLET IN CONCRETE STD. CALG STANDARD CURB AND GUTTER TOP OF SIDEWALK SIDEWALK CULVERT TOP OF ASPHALT TOP OF CURB TOP OF CONCRETE TOP OF GRATE TELEPHONE MANHOLE TELEPHONE PULLBOX TELEPHONE RISER/PULLBOX

TRAFFIC SIGN TELEPHONE VAULT UNDERGROUND WALLEY GUTTER WITH METAL HANDRAIL WOOD POLE WATER VALVE BOX CROSSWALK

EXISTING SPOT ELEVATION EXISTING BOULDER EXISTING SHRUB EXISTING DECIDUOUS TREE

EXISTING CONTOUR

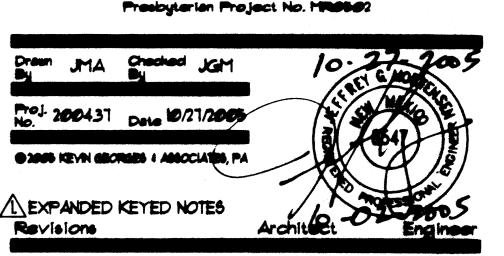
EXISTING DECIDUO (2.20) (CALIPER SIZE) (2.2'd) EXISTING CONIFEROUS TREE (CALIPER SIZE)

PROJECT BENCHMARK CITY OF ALBUQUERQUE Bress tublet, stamped "1-J18", SET IN TOP OF A CONCRETE POST WHICH SETS 0.8' NORTH OF THE NORTH EDGE OF A CONCRETE SIDEWALK, AND FLUSH WITH THE TOP OF SIDEWALK PAWEMENT ON THE NORTH SIDE OF CONSTITUTION AVE. ILE. AND GETWEEN PERMISSIVANIA BLVD. N.E. AND CONSTITUTION PL. N.E. ON TH SOUTH SIDE OF SNOW HEIGHTS PARK. ELEVATION = 5320.05 FEET (M.S.L.D.)

TOP OF CURB ELEVATION LOCATED AT THE S.S.E. RETURN OF THE INTERSECTION OF CONSTITUTION AVE. N.E. AND CONSTITUTION PLACE N.E. AS SHOWN ON THE BRAWING ON SHEET 1.
ELEVATION = 5329.86 FEET (N.S.L.D.)

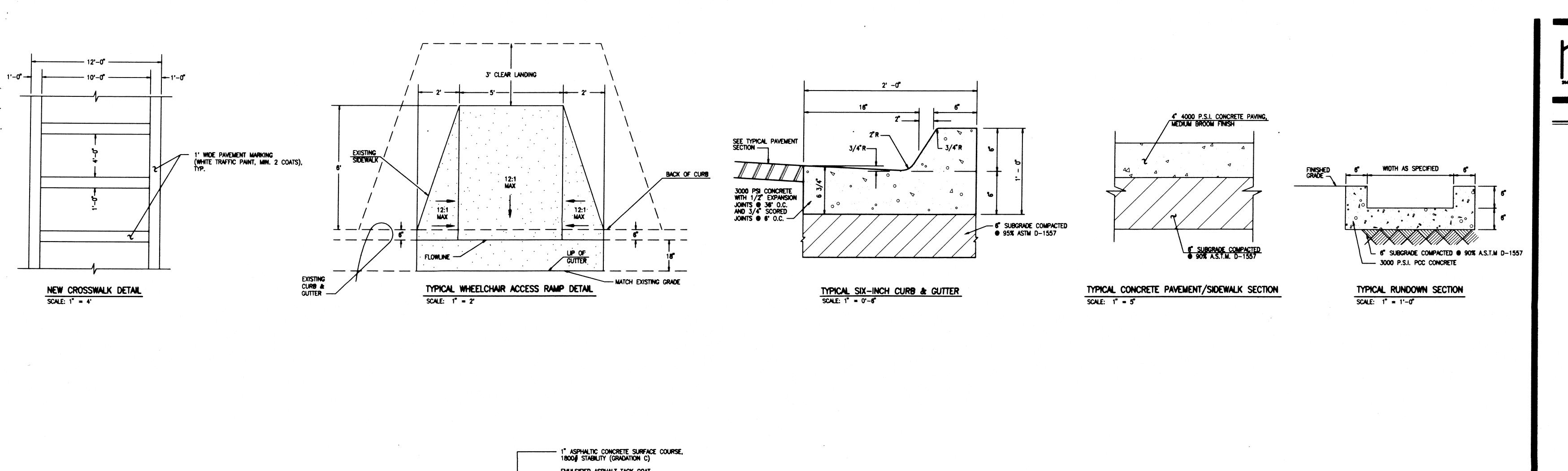
Final Construction Documents

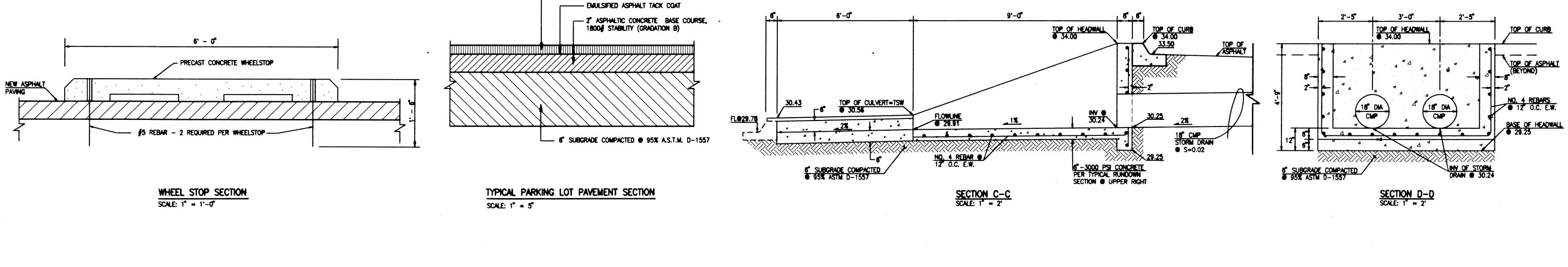
Parking Area A PMG Constitution 8300 Constitution Avenue NE Albuquerque, New Mexico

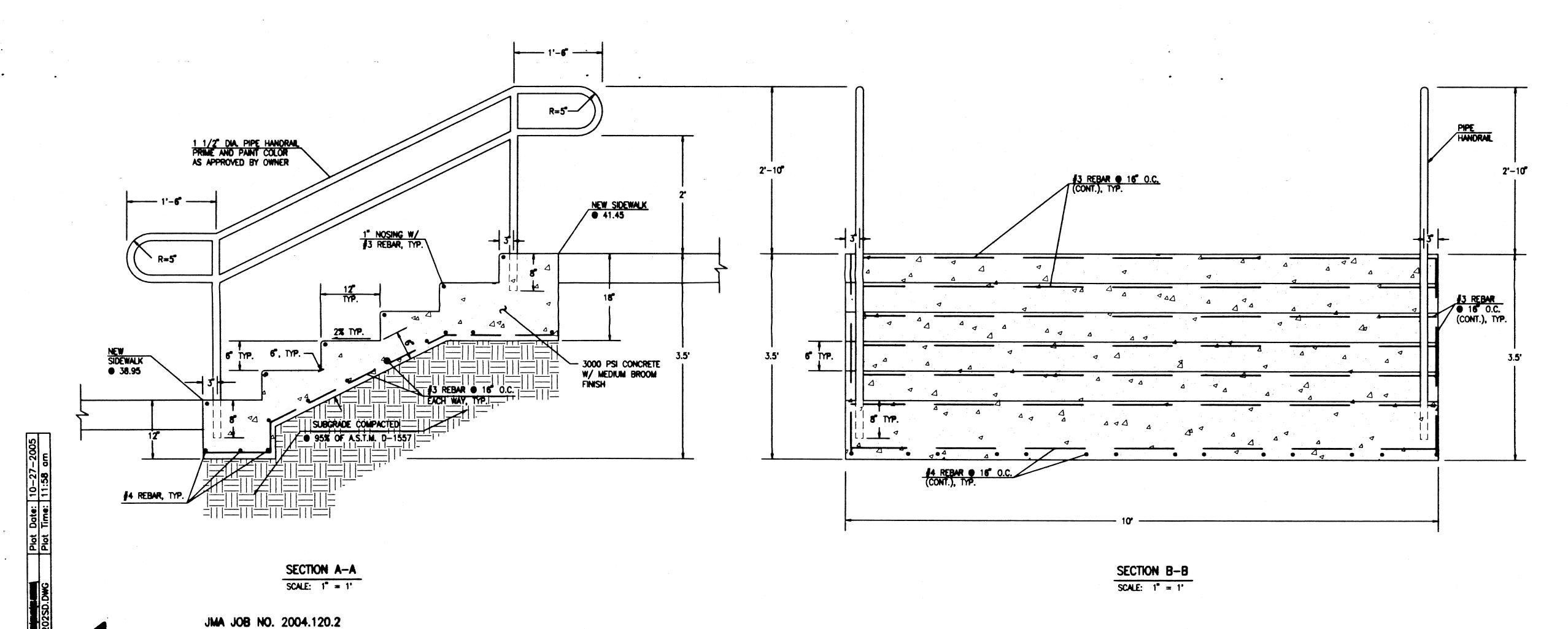


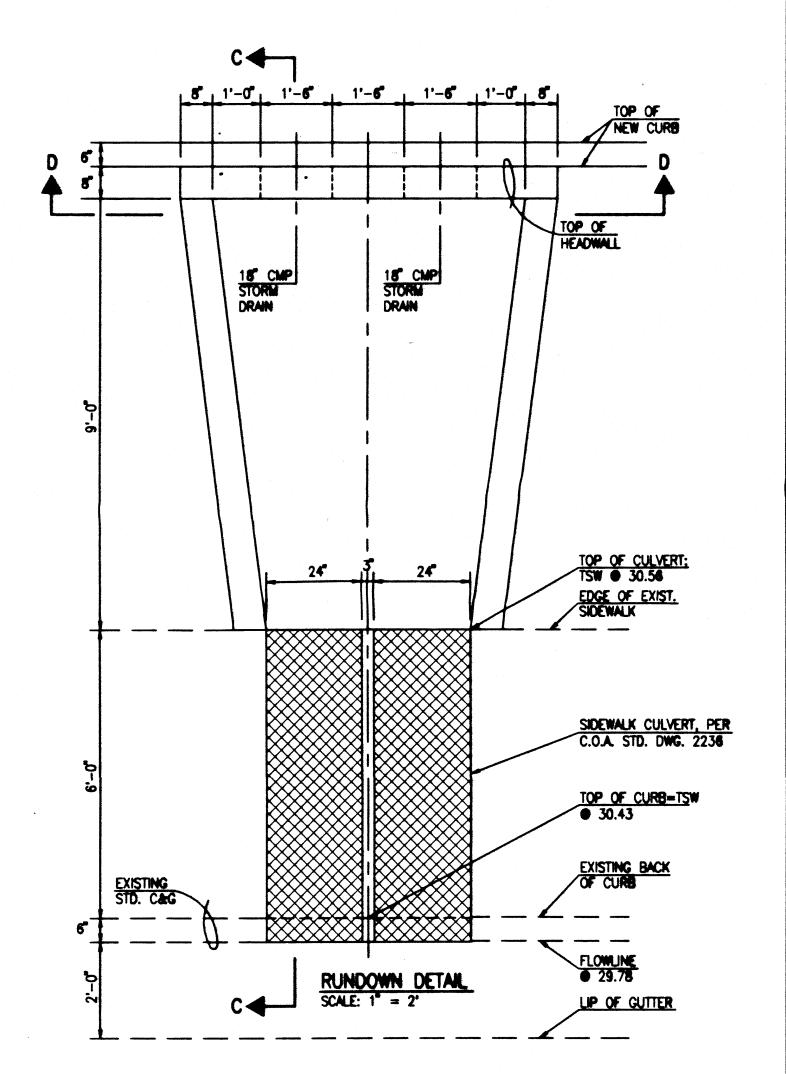
PAYING SITE PLAN (TCL)

JMA JOB NO. 2004.120.2 JEFF MORTENSEN & ASSOCIATES, INC. 6010-B MIDWAY PARK BLVD. N.E. ☐ ALBUQUERQUE ☐ NEW MEXICO 87109
☐ ENGINEERS ☐ SURVEYORS (505) 345-4250
☐ FAX: 505 345-4254 ☐ ESTABLISHED 1977







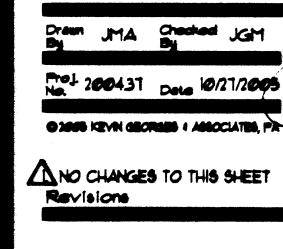


PAYING SECTIONS AND DETAILS

Final Construction Documents

Kevin Georges & Associates
Architecture & Planning

Parking Area A PMG Constitution 8300 Constitution Avenue NE Albuquerque, New Mexico Presbyterian Project No. MR6662



PAVING SECTIONS AND DETAILS Sheet Title



JMA JOB NO. 2004.120.2

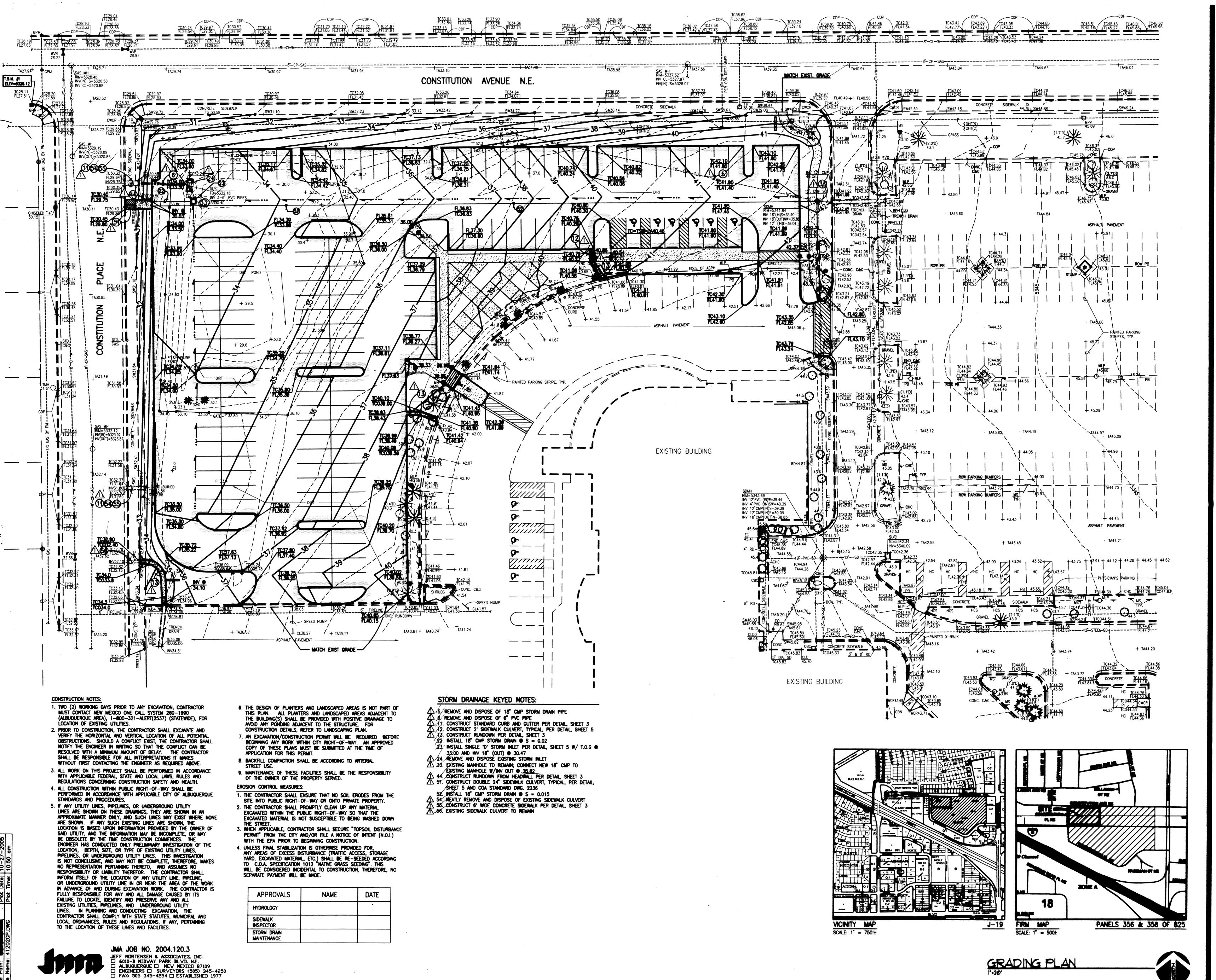
JEFF MORTENSEN & ASSOCIATES, INC.

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ALBUQUERQUE | NEW MEXICO 87109

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FAX: 505 345-4254 | ESTABLISHED 1977



Kevin Georges & Associate Architecture & Plannin

Legen

AREA LIGHT ON CONCRETE
ANTI-SIPHON WALVE
BUILDING OVERHANG
BIKE RACK
CURB AND GUTTER
CONCRETE BENCH
CONCRETE DRIVEPAD
CONCRETE HEADER CURB
CAST IRON PIPE
CENTERLINE OF DOOR
CENTERLINE OF DOUBLE DOOR
CHAIN LINK FENCE
SANITARY SEWER CLEANOUT
CONCRETE

CHAIN LINK FENCE
SANITARY SEWER CLEANOUT
CONCRETE
CURB OPENING
CONCRETE PIPE
CONCRETE RUNDOWN
CONCRETE RETAINING WALL
CONCRETE WHEELCHAIR RAMP
DRIVEPAD
EDGE OF GRASS

EDGE OF ASPHALT
ELECTRIC CONDUIT
ELECTRIC OUTLET
ELECTRIC OUTLET IN CONCRETE
ELECTRIC PULLBOX
FLOWLINE
FLAG POLE
GAS PAINT MARK

GAS PAINT MARK
HANDICAP PARKING SIGN
INVERT
IRRIGATION
METAL BUILDING COLUMN
MAILBOX
METAL LIGHT POLE

METAL POLE
METAL POWER POLE
METAL SIGN
OVERHEAD ELECTRIC (NO. OF LINES)
OVERHEAD TELEPHONE (NO. OF LINES)
PARKING BUMPER

PARKING BUMPER
PAINTED ISLAND
PLB PLASTIC BENCH
PAINT MARK
AS MH SANITARY SEWER MANHOLE
CT SPRINKLER CONTROL TIMER
D STORM DRAIN

STORM DRAIN MANHOLE
STORM INLET
STORM INLET IN CONCRETE
STORM INLET IN CONCRETE
STEEL
TOP OF SIDEWALK
SIDEWALK CULVERT
TOP OF ASPHALT

TOP OF CURB
TOP OF CONCRETE
TOP OF GRATE
TELEPHONE MANHOLE
TELEPHONE PULLBOX
TELEPHONE RISER/PULLBOX
TRAFFIC SIGN

TELEPHONE VAULT
TYPICAL
UNDERGROUND
VALLEY GUTTER
R WITH METAL HANDRAIL
WOOD POLE
WATER VALVE BOX
CROSSWALK
EXISTING CONTOUR

EXISTING SPOT ELEVATION
EXISTING BOULDER
EXISTING SHRUB

EXISTING DECIDUOUS TREE
(CALIPER SIZE)

(CALIPER SIZE)

EXISTING CONIFEROUS TO
(CALIPER SIZE)

LECAL DESCRIPTION

PROJECT BENCHMARK

CITY OF ALBUQUERQUE Breas beliet, storaged "1-J19", SET IN YOP OF A CONCRETE POST WHICH SETS 8.8" MORTH OF THE MORTH EDGE OF A CONCRETE SIDEWALK, AND FLUSH WITH THE TOP OF SIDEWALK PAREMENT ON THE MORTH SIDE OF CONSTITUTION AND ILE, AND SETWEEN PERMISYLVANIA BLVD. N.E. AND CONSTITUTION PL. N.E. ON TH SOUTH SIDE OF SNOW HEIGHTS PARK.

ELEVATION = \$320.05 FEET (M.S.L.D.)

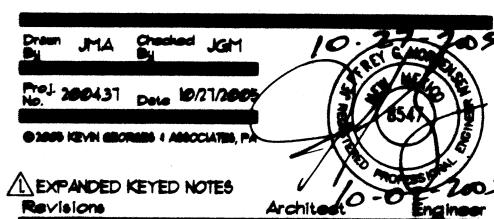
TOP OF CURB ELEVATION LOCATED AT THE S.S.E. RETURN OF THE INTERSECTION OF CONSTITUTION AVE. N.E. AND CONSTITUTION PLACE N.E. AS SHOWN ON THE BRANDING ON SHEET 1.

ELEVATION = \$329.86 FEET (M.S.L.D.)

Final Construction Documents

Parking Area A
PMG Constitution

8300 Constitution Avenue NE Albuquerque, New Mexico Probyterien Project No. MRSS62



GRADING PLAN

W Sheet 4 of 16

VEGETATED AREA GRADED SO AS TO CREATE A DETENTION POND FOR FLOWS FROM BOTH ONSITE AND OFFSITE FLOWS ABOVE THE SITE. THE NEW PARKING LOT WILL REPLACE THE VEGETATED AREA. THE DRAINAGE CONCEPT FOR THIS SITE WILL BE TO ELIMINATE THE EXISTING POND BY PAVING OVER THE ENTIRE VEGETATED LOWER AREA AND DRAIN THE FLOWS INTO THE BORDERING STREET OF CONSTITUTION PLACE NE. THERE WILL BE SOME SLIGHT DETENTION PONDING WITHIN THE PARKING

THIS DRAINAGE SUBMITTAL IS MADE IN SUPPORT OF A GRADING AND PAVING PERMIT AND SO \$19 APPROVAL.

N. PROJECT DESCRIPTION

THE SITE IS LOCATED ON THE SOUTHEAST CORNER OF CONSTITUTION AVE NE AND CONSTITUTION PLACE NE. THE CURRENT LEGAL DESCRIPTION OF THE SITE IS TRACT 1, EAST END ADDITION. THE SITE IS CURRENTLY LOCATED IN PANEL 356 OF 825 OF THE NATIONAL FLOOD INSURANCE PROGRAM FLOOD INSURANCE RATE MAPS PUBLISHED BY FEMA FOR BERNALILLO COUNTY, NEW MEXICO, NOVEMBER 19, 2003 AND IS ZONED AS ZONE X, AN AREA DETERMINED TO BE OUTSIDE THE 0.2% ANNUAL CHANCE FLOODPLAIN. THE DEVELOPMENT PROPOSED WILL INCREASE DOWNSTREAM FLOW BUT WILL NOT ADVERSELY IMPACT DOWNSTREAM FLOW OR DOWNSTREAM PROPERTIES, RUNOFF FROM THE SITE DRAINS INTO CONSTITUTION PLACE NE., AND THEN FLOWS NORTH TO CONSTITUTION AVENUE NE. AT CONSTITUTION AVENUE NE THE FLOW IS DIRECTED WEST TO THE INTERSECTION WITH PENNSYLVANIA STREET. FLOW PROCEEDS TO ENTER THREE LARGE CURB STORM INLETS ALONG CONSTITUTION AVE JUST BEFORE THE PENNSYLVANIA INTERSECTION. THESE INLETS DRAIN INTO THE 1-40 CHANNEL.

III. BACKGROUND DOCUMENTS

THE FOLLOWING ITEMS WERE REVIEWED IN THE PREPARATION OF THIS

A GRADING AND DRAINAGE PLAN FOR PRESBYTERIAN KASEMAN HOSPITAL - RADIATION THERAPY CENTER ADDITION, PREPARED BY JMA AND DATED 05-08-96 FOR BUILDING PERMIT. THE 1996 PLAN RECOGNIZED THAT THE EXISTING POND REQUIRED ENLARGEMENT TO CONTAIN THE DESIGN VOLUME OF RUNOFF. THE PLAN ALSO IDENTIFIED A 100-YEAR PEAK DISCHARGE OF 10.7 CFS FROM THIS PORTION OF TRACT 1.

B. DRAINAGE PLAN FOR PRESBYTERIAN KASEMAN HOSPITAL RADIATION THERAPY CENTER WEST EXPANSION POND ANALYSIS PREPARED BY JMA AND DATED 01-11-05. THIS PLAN CONCLUDED THAT THE NMOOT 1-40/PENNSYLVANIA OVERPASS PROJECT COMPLETED IN THE SPRING OF 2005 HAS ALLEVIATED PREVIOUS FLOODING CONCERNS ON THE CONSTITUTION AVE/PENNSYLVANIA STREET INTERSECTION. THIS PLAN USED THE CONJUGATE DEPTH EQUATION TO EVALUATE THE EFFECTS OF HYDRAULIC JUMP IN CONSTITUTION AVENUE NE. THE PLAN DETERMINED THAT THE DOWNSTREAM DRAINAGE IMPROVEMENTS THAT WERE MADE AT THE CONCLUSION OF THE 1-40/PENINSYLVANIA PROJECT ALLOWED FOR SUFFICIENT FLOW IN CONSTITUTION AVENUE WITH NO HYDRAULIC JUMP ISSUES. THIS CONCLUSION RESULTED IN THE ALLOWANCE OF THE ELIMINATION OF THE DETENTION POND IN THIS PROJECT AS

WELL AS FREE DISCHARGE INTO THE STREET. C. TOPOGRAPHIC SURVEY OF THE EXISTING SITE PREPARED BY JMA

IV. EXISTING CONDITIONS

THE SITE IS LOCATED AT THE SOUTHEAST CORNER OF THE INTERSECTION OF CONSTITUTION AVENUE NE AND CONSTITUTION PLACE NE. THE SITE IS PARTIALLY DEVELOPED. THE JMA TOPOGRAPHIC SURVEY DATED 4/13/2005 SHOWS THAT THE SITE CONSISTS OF AN EXISTING HOSPITAL BUILDING AND AN ASPHALT PAVED PARKING LOT, AS WELL AS AN UNDEVELOPED VEGETATED AREA AT THE NORTH AND WEST THAT IS GRADED FOR THE PURPOSE OF CREATING A DETENTION PONDING AREA FOR BOTH ON-SITE RUNOFF AND OFF-SITE FLOWS. THE SITE SLOPES FROM SOUTHEAST TO NORTHWEST AND DRAINS INTO THE POND AT THE NORTHWEST CORNER. THIS POND DRAINS WA STORM DRAIN THROUGH AN EXISTING SIDWALK CULVERT INTO CONSTITUTION PLACE NE. CONSTITUTION AVENUE IS A FULLY IMPROVED ROADWAY, 40 FEET WIDE WITH CURB AND GUTTER ON BOTH SIDES. CONSTITUTION PLACE FLOWS ARE CARRIED BY CURB AND GUTTER NORTH INTO CONSTITUTION AVENUE, WHERE IT FLOWS WEST TO AN INTERSECTION WITH PENNSYLVANIA STREET, JUST BEFORE THIS INTERSECTION THERE ARE THREE LARGE CURB STORM INLETS ALONG CONSTITUTION AVE NE WHERE THE FLOWS WILL DRAIN IN TO, THE INLETS DRAIN SOUTH INTO THE 1-40 CHANNEL.

ONSITE CONTRIBUTING AREA FLOWS FROM A PARKING LOT ALONG THE SOUTH EDGE OF THE SITE ENTER AT THE SOUTHWEST CORNER OF THE SITE THROUGH A TRENCH DRAIN AND CURB CUT; THE FLOWS EMPTY NORTH INTO THE DETENTION POND. ONSITE CONTRIBUTING AREA FLOWS FROM AN ADJACENT PARKING LOT ALSO ENTER THE SITE THROUGH A TRENCH DRAIN INTO A 12" STORM DRAIN THAT RELEASES INTO THE MANHOLE AT THE EAST EDGE OF THE SITE; THESE FLOWS ENTER INTO A 18° CMP STORM DRAIN FROM THE ONSITE MANHOLE AND EMPTIES THESE FLOWS INTO THE SITE AT THE NORTH EDGE OF THE SITE: THESE FLOWS DRAIN WEST INTO THE DETENTION POND AS SHOWN ON THE TCL.

V. DEVELOPED CONDITIONS

THE PROJECT CONSIST OF THE CONSTRUCTION OF A NEW ASPHALT PAVED PARKING LOT THAT WILL SERVE THE EXISITING HOSPITAL BUILDINGS IN THE AREA THIS NEW LOT WILL REPLACE THE NATURALLY VEGETATED AREA ALONG THE NORTH AND WEST OF THE SITE, AND WELL ELIMINATI THE EXISTING DETENTION POND. AS A RESULT OF THESE IMPROVEMENTS. THERE WILL BE AN INCREASE IN THE RUNOFF THE SITE GENERATES AS WELL AS A REDIRECTION OF ONSITE CONTRIBUTING AREA FLOWS AROUND THE SITE THAT CURRENTLY TRAVERSE THE SITE, A SMALL PORTION OF THE ONSITE RUNOFF WILL BE PONDED AT THE NORTHWEST CORNER OF THE DEVELOPED PARKING LOT, WHICH WILL BE DRAINED THROUGH A STORM INLET AND STORM DRAIN INTO A RUNDOWN AND SIDWALK CULVERT INTO CONSTITUTION PLACE NE AS SHOWN ON THE DEVELOPED GRADING PLAN, THIS STORM DRAIN PIPE WILL DRAIN THI CAPTURED STORMWATER RUNOFF IN A PERIOD LESS THAN SIX HOURS FLOWS INTO CONSTITUTION PLACE NE WILL FOLLOW THE SAME DRAINAGE PATH AS STATED IN THE EXISTING CONDITIONS.

AS THE HYDROGRAPH CALCULATIONS SHOW, THE VOLUME REQUIRING PONDING WILL EQUAL 220 CF. THIS VOLUME WILL REACH A HEIGHT ABOVE THE STORM INLET OF 5333.2 FT. THE LOWEST POINT OF THE TOP OF CURB IN THE PONDING AREA IS AT 5334 FT. THIS IS ALSO THE LOWEST TOP OF CURB IN THE ENTIRE PROJECT SITE. THEREFORE, THERE IS NO PROBLEM OF RUNOFF OVERFLOWING THE CURB.

ONSITE CONTRIBUTING AREA FLOWS THAT ENTER THE MANHOLE AT THE EAST CORNER OF THE SITE WILL BE DIRECTED THROUGH THE SITE VIA A NEW 18" CMP STORM DRAIN THAT WILL TRAVERSE THE SITE FROM EAST TO WEST AND DRAIN INTO CONSTITUTION PLACE NE AT THE SAME LOCATION AS THE ONSITE FLOW STORM DRAIN RELEASES ITS FLOWS AS SHOWN ON THE DEVELOPED GRADING PLAN.

THE OVERALL RESULT OF THE DEVELOPMENT WILL INCREASE THE VOLUME AND PEAK DISCHARGE RATE INTO CONSTITUTION PLACE NE. THIS INCREASE IS ALLOWED DUE TO THE COMPLETION OF DEVELOPMENT AT PENNSYLVANIA STREET AND 1-40 THAT ALLEVIATED PAST DOWNSTREAM FLOW ISSUES; ISSUES THAT HAD PREVIOUSLY RESULTED IN THE CREATION OF THE EXISTING ONSITE DETENTION POND. U

VI. GRADING PLAN

PROJECT.

THE GRADING PLAN SHOWS: 1.) EXISTING GRADES INDICATED BY SPOT ELEVATIONS AND CONTOURS AT 1'-OF INTERVALS BASED THE TOPOGRAPHIC SURVEY PREPARED BY JMA, DATED 4/13/2005, 2.) PROPOSED GRADES INDICATED BY CONTOURS AT 1'-O" INTERVALS, 3.) THE LIMIT AND CHARACTER OF THE EXISTING IMPROVEMENTS AS TAKEN FROM THE AFORE MENTIONED JMA SURVEY, 4.) THE LIMIT AND CHARACTER OF THE PROPOSED IMPROVEMENTS, 5.) CONTINUITY BETWEEN EXISTING AND PROPOSED GRADES, AS SHOWN BY THIS PLAN, THE PROPOSED DEVELOPMENT CONSISTS OF PAVED PARKING, ALONG WITH ASSOCIATED LANDSCAPING AND SIDEWALK, THESE MODIFICATIONS TO THE SITE WILL EXPAND AVAILABLE ONSITE PARKING AND VEHICLE ACCESS. AN EROSION CONTROL PLAN WILL BE PREPARED AND ATTACHED TO THE SEPARATE STORM WATER POLLUTION PREVENTION PLAN FOR THIS

VI. GRADING PLAN

THE GRADING PLAN SHOWS: 11) EXISTING GRADES INDICATED BY SPOT ELEVATIONS AND CONTOURS AT 1'-0" INTERVALS BASED THE TOPOGRAPHIC SURVEY PREPARED BY JMA, DATED 4/13/2005, 2.) PROPOSED GRADES INDICATED BY CONTOURS AT 1'-0" INTERVALS, 3.) THE LIMIT AND CHARACTER OF THE EXISTING IMPROVEMENTS AS TAKEN FROM THE AFORE MENTIONED JMA SURVEY, 4) THE LIMIT AND CHARACTER OF THE PROPOSED IMPROVEMENTS, 5,) CONTINUITY BETWEEN EXISTING AND PROPOSED GRADES. AS SHOWN BY THIS PLAN, THE PROPOSED DEVELOPMENT CONSISTS OF PAVED PARKING, ALONG WITH ASSOCIATED -LANDSCAPING AND SIDEWALK. THESE MODIFICATIONS TO THE SITE WILL EXPAND AVAILABLE ONSITE PARKING AND VEHICLE ACCESS. AN EROSION CONTROL PLAN WILL BE PREPARED AND ATTACHED TO THE SEPARATE STORM WATER POLLUTION PREVENTION PLAN FOR THIS PROJECT.

VII. CALCULATIONS

THE CALCULATIONS WHICH APPEAR HEREON ANALYZE BOTH THE EXISTING AND DEVELOPED CONDITIONS FOR THE 100-YEAR. 6-HOUR rainfall event, as well as the 10–year, 6 hour rainfall event FOR CULVERT DESIGN PURPOSES. THE PROCEDURE FOR 40-ACRE AND SMALLER BASINS. AS SET FOR THE IN THE REVISION OF SECTION 22.2 HYDROLOGY OF THE DEVELOPMENT PROCESS MANUAL VOLUME 2. DESIGN CRITERIA, DATED JANUARY, 1993, HAS BEEN USED TO QUANTIFY THE PEAK RATE OF DISCHARGE AND VOLUME OF RUNOFF GENERATED BY THIS DEVELOPMENT. THE CAPACITY OF THE DEVELOPED 18 CMP STORM EQUATIONS. THE CAPACITIES OF THE NEW STORM INLETS WERE DETERMINED USING THE ORIFICE EQUATION. THE CAPACITY OF THE NEW 24" CULVERTS ALONG CONSTITUTION PLACE NE WERE DETERMINED USING MANNING'S EQUATION. THE PERFORMANCE OF THE PRIVATE STORMWATER DETENTION SYSTEM IN THE NORTH EAST CORNER OF THE PARKING AREA HAS BEEN EVALUATED FOR OUTFLOW AND STORAGE CAPACITY BASED UP THE HYDROGRAPH ANALYSIS CONTAINED ON THIS sheet, and ponding is necessary only due to discharge PIPE SIZING CONSTRAINTS.

THE JANUARY 2005 CALCULATIONS ARE SHOWN HEREON TO SHOW THAT THE DOWNSTREAM STREET (CONSTITUTION AVENUE) IS CAPABLE OF HANDLING FLOWS FROM ONSITE AND OFFSITE. THE CONJUGATE DEPTH EQUATION WAS USED TO EVALUATE THE EFFECTS OF HYDRAULIC JUMP. AS SHOWN BY THESE CALCULATIONS, THE NORMAL DEPTH OF FLOW PLUS THE EFFECTS OF A HYDRAULIC JUMP COMPLY WITH DPM CRITERIA TO FALL BELOW THE TOP-OF-CURB PLUS 0.2 FEET. THIS SUGGESTS THAT THE STREET CAN HANDLE THE FULLY DEVELOPED DESIGN FLOW AS CALCULATED AND THAT PONDING IS NO LONGER NECESSARY.

VIII. CONCLUSION

THE DRAINAGE CONCEPT SHOWN BY THE PLAN ABOVE WILL ACCOMPLISH

ITS PURPOSE DUE TO THE FOLLOWING FACTORS: A.) MODIFICATION TO AN EXISTING SITE WITHIN AN INFILL AREA B.) THE PEAK RATE OF DISCHARGE FROM THE SITE WILL BE

INCREASED ABOVE THAT OF THE EXISTING CONDITION C.) REMOVAL OF THE EXISTING POND AS ALLOWED BY APPROVED DRAMAGE SUBMITTAL DATED 01-11-05 D.) ADEQUATE STREET CAPACITY IS AVAILABLE IN CONSTITUTION PLACE NE. TO ACCOMMODATE. PEAK DISCHARGE UNDER FULLY

DEVELOPED CONDITIONS E.) DOWNSTREAM CAPACITY OF CONSTITUTION PLACE AND CONSTITUTION AVENUE AND THE STORM DRAIN STRUCTURES LOCATED WITHIN ARE ADEQUATE FOR INCREASED FLOWS DUE TO THE NMOOT 1-40/PENNSYLVANIA PROJECT IMPROVEMENTS

F.) FREE DISCHARGE INTO CONSTITUTION PLACE AND CONSTITUTION AVENUE PER THE DRAINAGE PLAN DATED 1-11-05 G.) NO ADVERSE IMPACT ON DOWNSTREAM CAPACITY OR DOWNSTREAM PROPERTIES

H.) CULVERTS AT CONSTITUTIONS PLACE SUFFICIENT TO HANDLE PEAK DISCHARGE FROM BOTH STORM DRAINS UNDER FULLY DEVELOPED CONDITIONS

I.) Grading and curb about storm inlet area sufficient to CONTAIN MORE THAN THE 220 CF OF PONDING REQUIRED DURING A 100-YEAR STORM

J.) OFFSITE FLOWS WILL CONTINUE TO BE INTERCEPTED AND CONVEYED THROUGH THE PROJECT TO DOWNSTREAM EXISTING DRAINAGE FACILITIES

SITE CHARACTERISTICS

1. PRECIPITATION ZONE = 3

CALCULATIONS

2. Pa.100 = P360 = 2.60 2. Pa.10 = P360 = 1.73

3. TOTAL AREA (At) = 143,065 SF

4. EXISTING LAND TREATMENT

B. ONSITE CONTRIBUTING AREAS

A. PROJECT SITE TREATMENT AREA (SF/AC) 16,779 / 0.39 12 / 61,626 / 1.41 43/ 64,660 / 1.48

5. DEVELOPED LAND TREATMENT

TREATMENT

TREATMENT

A. PROJECT SITE AREA (SF/AC) % 23,552 / 0.54 119,533 / 2.74 84

AREA (SF/AC) %

70,000 / 1.61

6. EXISTING CONDITION

A. PROJECT SITE 1. VOLUME

Ew = (EAAA+EBAB+EcAc+EbAb)/ArEw = ((0.92*0.39)+(1.29*1.41)+(2.36*1.48))/3.28 = 1.73 IN $V_{100} = (Ew/12)Ar = (1.73/12)3.28 = 0.2317 AC-FT 20,628 CF$

2. PEAK DISCHARGE QP = QPA AA + QPBAB + QPCAC + QPBAB

QP = Q100 = (2.60 + 0.39) + (3.45 + 1.41) + (5.02 + 1.48) = 13.33 CFS

B. ONSITE CONTRIBUTING AREAS (100-YEAR STORM) 1. VOLUME

 $Ew = (E_AA_A + E_BA_B + E_CA_C + E_DA_B)/Ar$ Ew = (2.36*1.61)/1.61 = 2.36 IN

 $V_{160} = (Ew/12)Ar = (2.36/12)1.61 = 0.3160 AC-FT = 13,767 CF$

2. PEAK DISCHARGE QP = QPA AA + QPBAB + QPCAC + QPBAB QP = Qtee = (5.02+1.61) = 8.07 CFS

C. ONSITE CONTRIBUTING AREAS (10-YEAR STORM)

1. VOLUME EW = (EAA+EBAB+EcAc+EDAB)/Ar

 $EW = (1.50 \cdot 1.61)/1.61 = 1.50 \text{ IN}$ $V_{10} = (E_W/12)A_T = (1.50/12)1.61 = 0.2009 AC-FT = 8,750 CF$

2. PEAK DISCHARGE

QP = QPA AA + QPBAB + QPCAC + QPBAB QP = Q166 = (3.39 + 1.61) = 5.45 CFS

7. DEVELOPED CONDITION

A. PROJECT SITE (100-YEAR STORM) 1. VOLUME

EW = (EAA+EBAG+EcAc+EBAG)/A EW = ((0.92*0.54)+(2.36*2.74))/3.28 = 2.12 IN

 $V_{100} = (Ew/12)Ar = (2.12/12)3.28 = 0.5811 AC-FT = 25,314 CF$

2. PEAK DISCHARGE

QP = QPA AA + QPBAB + QPCAC + QPBABQP = Q100 = (2.60 + 0.54) + (5.02 + 2.74) = 15.18 CFS

B. ONSITE CONTRIBUTING AREAS (10-YEAR STORM) 1. VOLUME

 $Ew = (E_AA_A + E_BA_B + E_CA_C + E_DA_B)/Ar$

Ew = ((0.36*0.54)+(1.50*2.74))/3.28 = 1.31 IN $V_{10} = (Ew/12)Ar = (1.31/12)3.28 = 0.3592 AC-FT = 15.648 CF$

2. PEAK DISCHARGE QP = QPA AA + QPBAB + QPCAC + QPBAB

QP = Q10 = (1.19 + 0.54) + (3.39 + 2.74) = 9.95 CFS

RAINSTORM PONDING CAPACITY TOTAL VOL. (CF) 5333.5 [(2675+0)/2] *0.5 = 670670 5334 [(6575+2675)/2] + 0.5 = 9751625

VINOVOER = 1,625 CF>VINER = 240 CF (SEE HYDROGRAPH)

1 HYDROGRAPH CALCULATIONS

TP = 0.7*Tc+(1.6-Ap/Ar)/12TP = 0.7*0.2+(1.6-2.74/3.28)/12TP = 0.2037 hr

DP = 0.25 + Ac/Ac

DP = 0.25 + 2.74 / 3.28DP = 0.2088 hrTe = 2.017 + E + AT/QP - 0.25 + Ae/Ar

 $T_8 = 2.017 \cdot 2.12 \cdot 3.28 / 15.18 - 0.25 \cdot 2.74 / 3.28$ $T_B = 0.7563 \text{ hr}$

AREA OF HYDROGRAPH = Vigs = 26,410 CE **VOLUME REQUIRING PONDING**

 $\forall \textbf{rese} = (15.18-14.90) \cdot ((((25.16-11.96)+(24.75-12.22))/2) \cdot 60)$

VNEOTS = 215.6 CE/ PEAK DISCHARGE CAPACITY OF ON-SITE FLOW DISCHARGE PIPE

 $Q_{CP} = 1.49/n AR^{2/3} S^{1/2}$

 $A = pier^2 = pie(0.75)^2 = 1.767 FT^2$ P = 2*pi*r = 2*pi*0.75 = 4.712 FT

R = A/P = .375 FTS = 0.02 FT/FT

Qce = 14.9 CFS Q100 = 15.2 CFS>QC# =14.9 CFS, 215 CF OF PONDING REQ'D

CULVERT CALCULATIONS FOR SITE DRAINAGE

A. PEAK DISCHARGE CAPACITY OF SINGLE CULVERT $Q_{CP} = 1.49/n AR^{2/3} S^{1/2}$ n = 0.013 $A = 0.667 \text{ FT+2 FT} = 1.33 \text{ FT}^2$

P = 0.667 FT + 0.667 FT + 2 FT = 3.33 FT

S = 0.02 FT/FTQce = 11.73 CFS

R = A/P = 0.4 FT

B. CULVERT DESIGN TO HOLD 10-YEAR, 6-HOUR STORM Q10,08-SRE = 9.95 CFS Q10,comme = 5.45 CFS Q10,EQ20 = 9.95 + 5.45 = 15.4 CFS

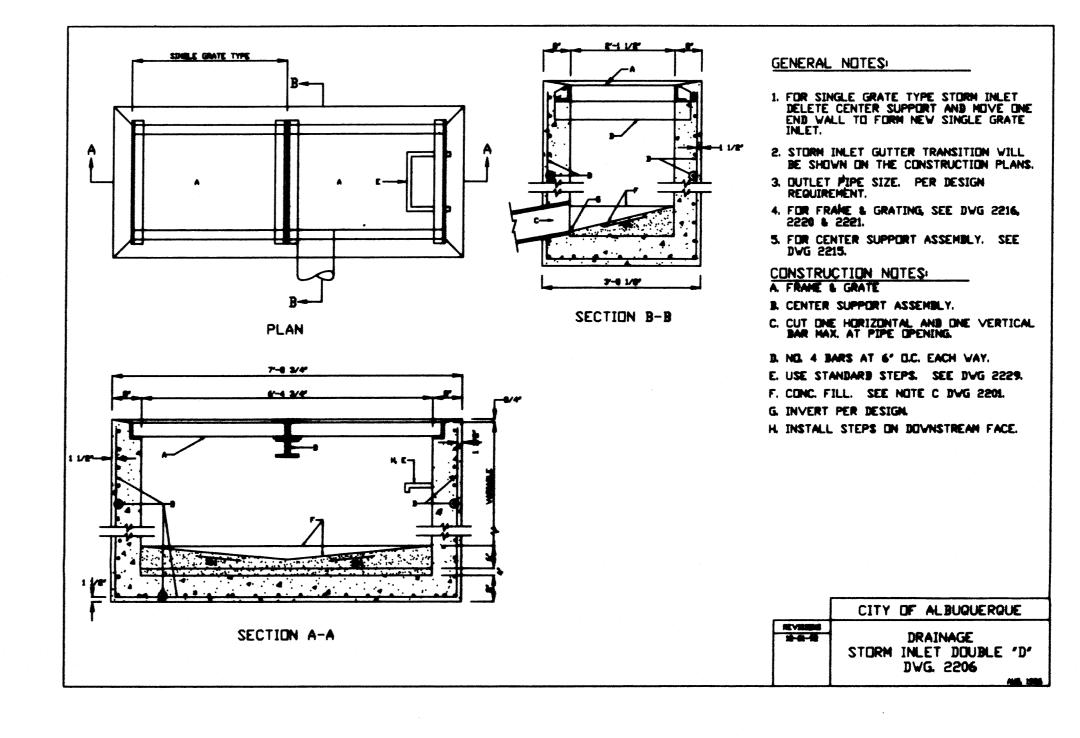
Q10,0000 = 15.4 CFS>QCO = 11.73 CFS

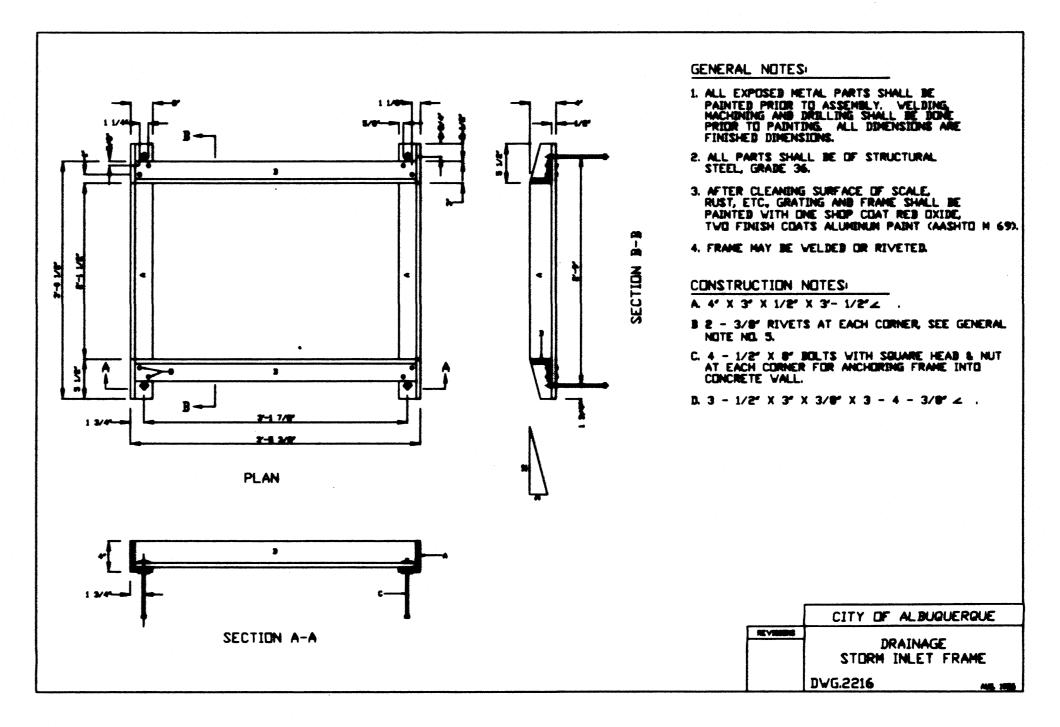
Q10,0000 = 15.4 CFS<2+Qcar = 23.46 CFS

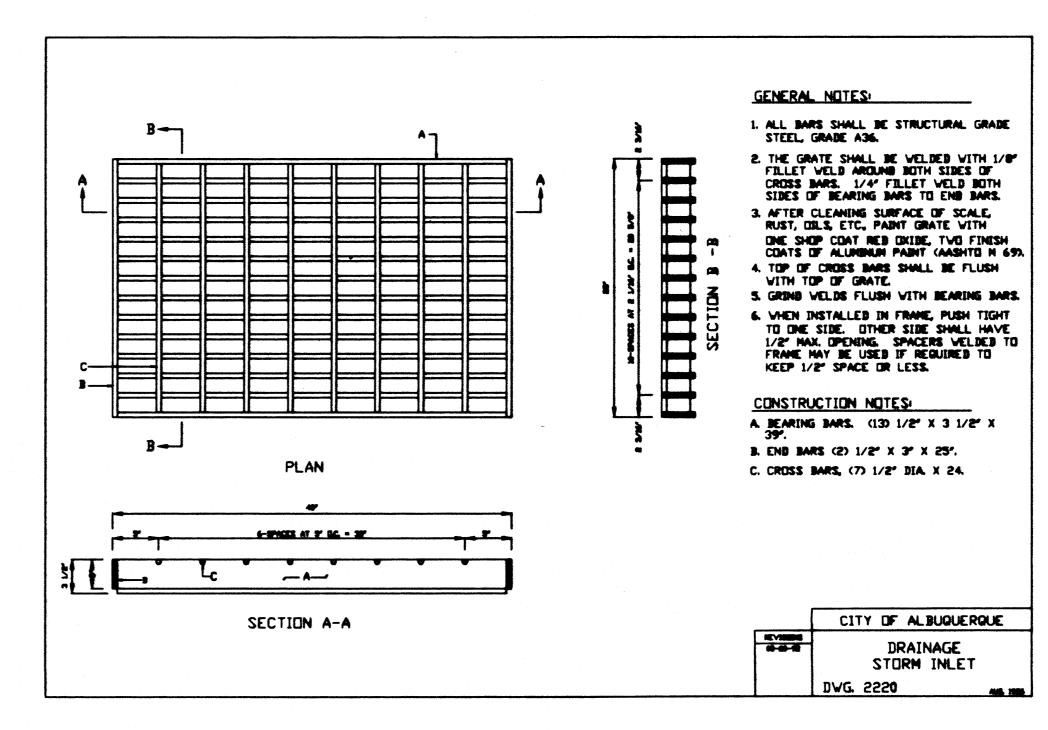
DESIGN REQUIRES TWO CULVERTS 8. COMPARISON

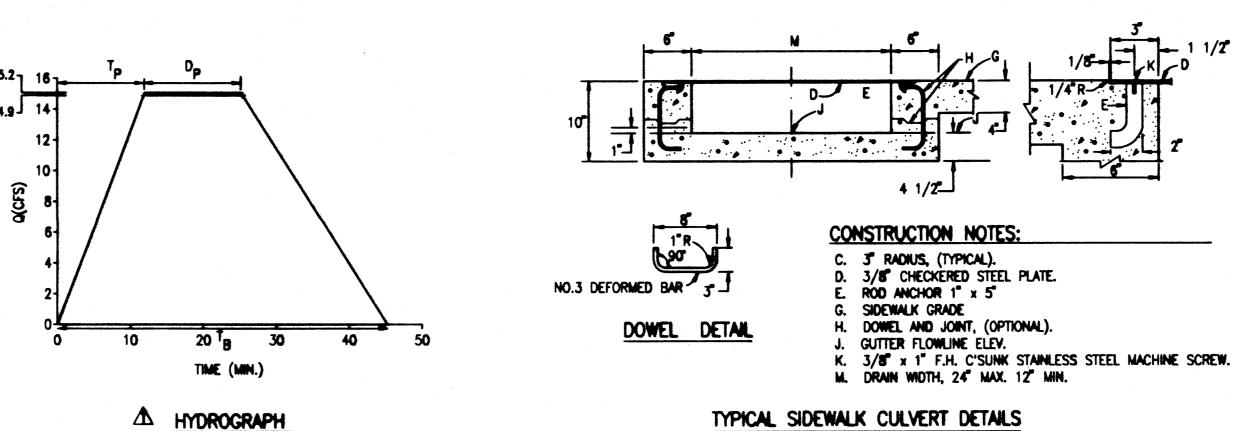
A. VOLUME Δ Vies = 25.315-20,628 = 4,687 CF (INCREASE)

B. PEAK DISCHARGE \triangle Q100 = 15.18-13.33 = 1.85 CFS (INCREASE)

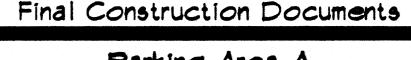










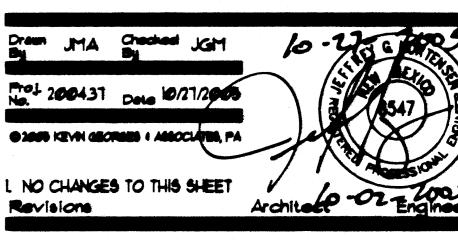


Kevin Georges & Associates

14 Trumen Street NE - Albuquerque, New Mexico 81100-1333 500/250-4875

Architecture 4 Planning

Parking Area A PMG Constitution 8300 Constitution Avenue NE Albuquerque, New Mexico Presbyterian Project No. MR6862



CALCULATIONS, DRAINAGE PLAN AND SECTIONS AND DETAILS Sheet 5 of 0 Sheet Title

NOT TO SCALE

SCALE: 1" = 750'±

PAVING SITE PLAN (TCL

No. 200437 Date 10/27/2005 @ 2005 KEYIN GEORGES & ASSOCIATES, PA EXPANDED KEYED NOTES

Parking Area A PMG Constitution

Albuquerque, New Mexico

PAYING SITE PLAN (TCL)

JMA JOB NO. 2004.120.2 JEFF MORTENSEN & ASSOCIATES, INC. 6010-B MIDWAY PARK BLVD, N.E. ALBUQUERQUE | NEW MEXICO 87109 ☐ ENGINEERS ☐ SURVEYORS (505) 345-4250 ☐ FAX: 505 345-4254 ☐ ESTABLISHED 1977 19. PAINT 4" PAVEMENT MARKING W/ WHITE TRAFFIC PAINT, MIN. 2 20. PAINT CROSSWALK PAVEMENT MARKING PER DETAIL, SHEET 3

21. NEW LANDSCAPING, SEE LANDSCAPING PLAN 22. INSTALL 18" CMP STORM DRAIN @ S = 0.02 23. INSTALL SINGLE "D' STORM INLET PER DETAIL, SHEET 5 W/ T.O.G • 33.00 AND INV 18" (OUT) @ 30.47 24. REMOVE AND DISPOSÉ EXISTING STORM INLET 25. STENCIL "COMPACT" ON PARKING LOT PAVEMENT

26. PAINT TOP AND FACE OF CURB W/ YELLOW TRAFFIC PAINT, MIN. 2 27. BEGIN PAINTING TOP AND FACE OF CURB WITH YELLOW TRAFFIC

28. END PAINTING TOP AND FACE OF CURB WITH YELLOW TRAFFIC 29. STENCIL "NO PARKING" ON TOP AND FACE OF CURB AT 50' SPACING 30. BEGIN STENCILING "NO PARKING" ON TOP AND FACE OF CURB 31. END STENCILING "NO PARKING" ON TOP AND FACE OF CURB

32. REMOVE AND DISPOSE 4' DOUBLE SWING GATE

48. PAINT 4" WIDE CROSS-HATCH (2' CC) PAVEMENT MARKINGS @ 45 DEG W/ WHITE TRAFFIC PAINT, MIN 2 COATS, TYPICAL 49. EXISTING LIGHT POLE TO REMAIN

50. EXISTING TREES AND SHRUBS TO REMAIN 51. CONSTRUCT DOUBLE 24" SIDEWALK CULVERT, TYPICAL, PER DETAIL, SHEET 5 AND COA STANDARD DWG. 2236 52. INSTALL 18" CMP STORM DRAIN @ S = 0.015 53. CLEAR SIGHT TRIANGLE

54. NEATLY REMOVE AND DISPOSE OF EXISTING SIDEWALK CULVERT

55. CONSTRUCT 6' WIDE CONCRETE SIDEWALK PER DETAIL, SHEET 3

56. EXISTING SIDEWALK CULVERT TO REMAIN



Architecture 4 Planning

AREA LIGHT ON CONCRETE ANTI-SIPHON VALVE BUILDING OVERHANG

BIKE RACK

CONCRETE BENCH CONCRETE DRIVEPAD CONCRETE HEADER CURB

CAST IRON PIPE

CENTERLINE OF DOOR

CHAIN LINK FENCE

CONCRETE RUNDOWN CONCRETE RETAINING WALL CONCRETE WHEELCHAIR RAMP

CURB OPENING

EDGE OF GRASS

ELECTRIC OUTLET

ELECTRIC PULLBOX

METAL POWER POLE

STORM DRAIN MANHOLE

TOP OF SIDEWALK

TRAFFIC SIGN TELEPHONE VAULT

UNDERGROUND WALLEY GUTTER

WOOD POLE WATER VALVE BOX

CROSSWALK

WITH METAL HANDRAIL

EXISTING CONTOUR EXISTING SPOT ELEVATION

EXISTING BOULDER

EXISTING DECIDUOUS TREE

EXISTING SHRUB

SIDEWALK CULVERT TOP OF ASPHALT TOP OF CURB TOP OF CONCRETE TOP OF GRATE TELEPHONE MANHOLE TELEPHONE PULLBOX TELEPHONE RISER/PULLBOX

STORM INLET IN CONCRETE

OVERHEAD ELECTRIC (NO. OF LINES) OVERHEAD TELEPHONE (NO. OF LINES)

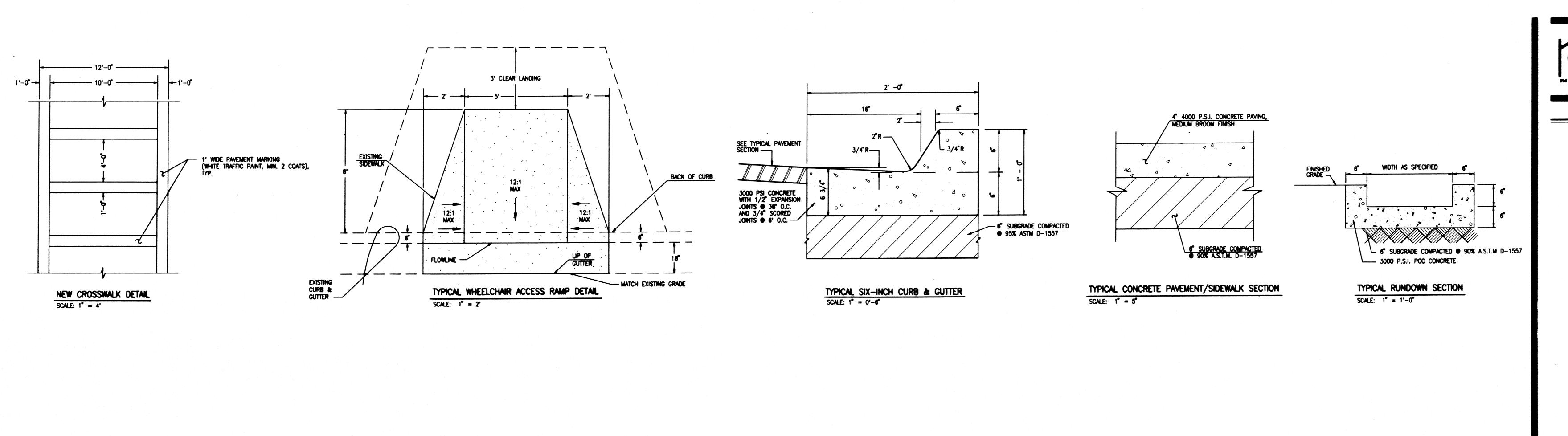
FLOWLINE FLAG POLE GAS PAINT MARK

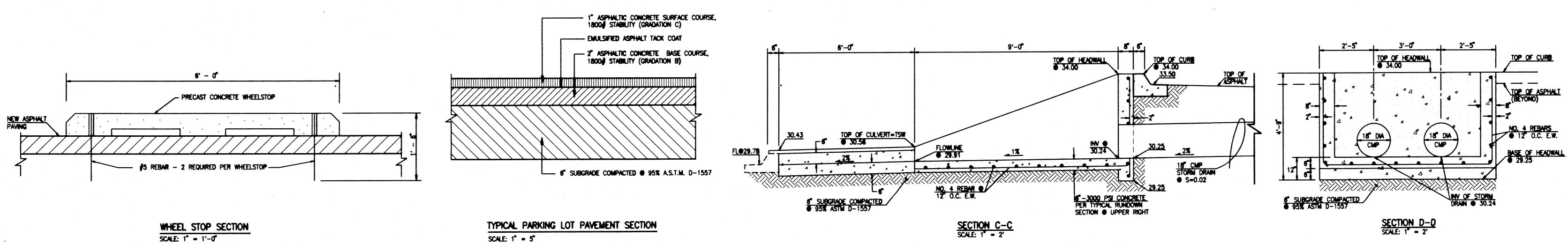
ELECTRIC OUTLET IN CONCRETE

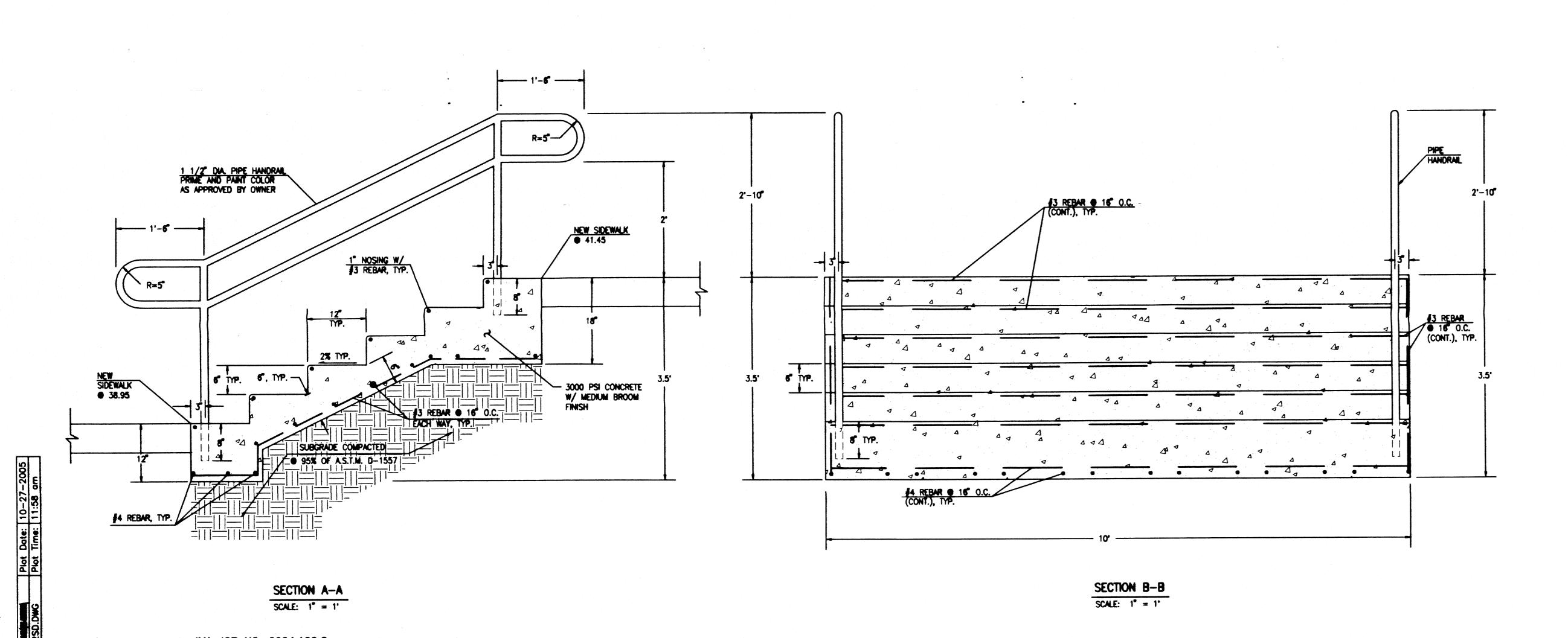
HANDICAP PARKING SIGN

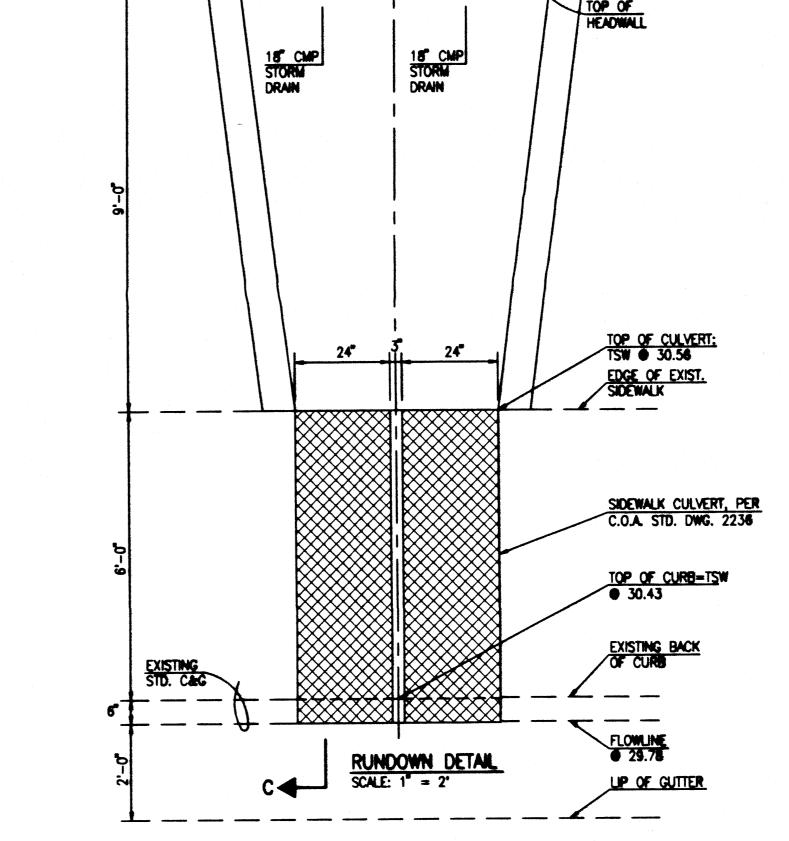
CENTERLINE OF DOUBLE DOOR

SANITARY SEWER CLEANOUT







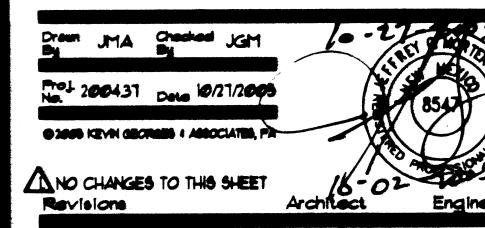


8 1'-0 1'-6 1'-6 1'-6 1'-6 5

Final Construction Documents

Kevin Georges & Associates Architecture & Planning

Parking Area A PMG Constitution 8300 Constitution Avenue NE Albuquerque, New Mexico Problemen Project No. MRSS62

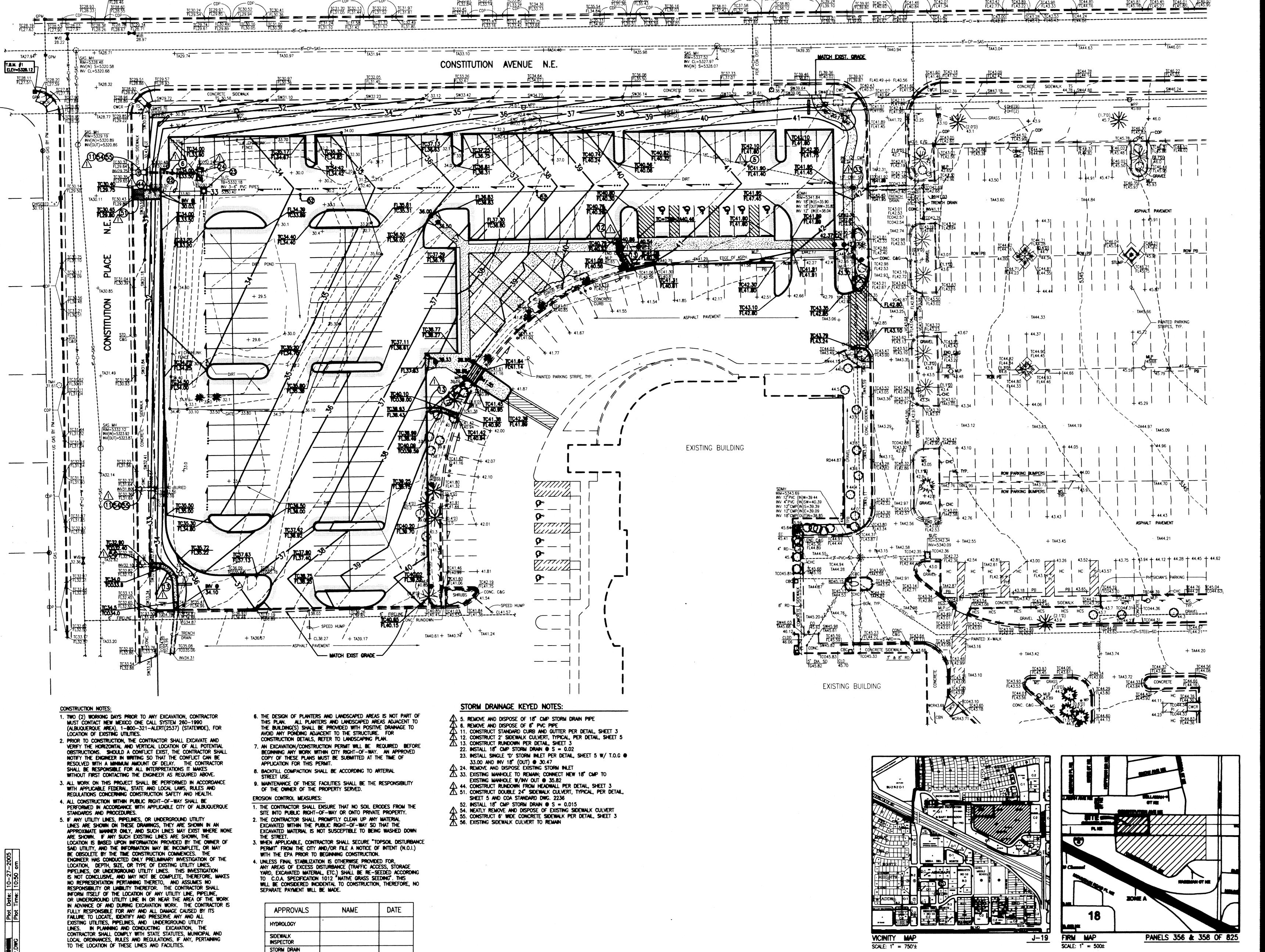


PAVING SECTIONS AND DETAILS

Sheet 3 of 10



PAVING SECTIONS AND DETAILS



MAINTENANCE

JMA JOB NO. 2004.120.3

JEFF MORTENSEN & ASSICIATES, INC.

6010-B MIDWAY PARK BLVD. N.E.

ALBUQUERQUE | NEW MEXICO 87109

ENGINEERS | SURVEYORS (505) 345-4250

FAX: 505 345-4254 | ESTABLISHED 1977

Kevin Georges & Associate Architecture & Plannin

Legeno

AREA LIGHT ON CONCRETE ANTI-SIPHON VALVE BUILDING OVERHANG BIKE RACK CURB AND GUTTER CONCRETE BENCH CONCRETE DRIVEPAD CONCRETE HEADER CURB CAST IRON PIPE CENTERLINE OF DOOR CENTERLINE OF DOUBLE DOOR CHAIN LINK FENCE SANITARY SEWER CLEANOUT CONCRETE CURB OPENING CONCRETE PIPE CONCRETE RUNDOWN

CONCRETE RETAINING WALL
CONCRETE WHEELCHAIR RAMP
DRIVEPAD
EDGE OF GRASS
EDGE OF ASPHALT
ELECTRIC CONDUIT
ELECTRIC OUTLET
ELECTRIC OUTLET IN CONCRETE
ELECTRIC PULLBOX
FLOWLINE

FLAG POLE
GAS PAINT MARK
HANDICAP PARKING SIGN
INVERT
IRRIGATION
METAL BUILDING COLUMN
MAILBOX
METAL LIGHT POLE
METAL POLE

MPP METAL POWER POLE
MS METAL SIGN
OHE(4) OVERHEAD ELECTRIC (NO. OF LINES)
OHT(2) PARKING BUMPER
PI PAINTED ISLAND
PLB PLASTIC BENCH

PLB PLASTIC BENCH
PM PAINT MARK

SAS MH SANITARY SEWER MANHOLE
SCT SPRINKLER CONTROL TIMER
SD STORM DRAIN
SDMH STORM DRAIN MANHOLE '
SI STORM INLET
SI/C STORM INLET IN CONCRETE
STD. C&G STANDARD CURB AND GUTTER

TOP OF SIDEWALK
SIDEWALK CULVERT
TOP OF ASPHALT
TOP OF CURB
TOP OF CONCRETE
TOP OF GRATE
TELEPHONE MANHOLE
TELEPHONE BULLBOY

TELEPHONE PULLBOX
TELEPHONE RISER/PULLBOX
TRAFFIC SIGN
TELEPHONE VAULT
TYPICAL
UNDERGROUND
VALLEY GUTTER

WITH METAL HANDRAIL

WOOD POLE
WATER VALVE BOX
CROSSWALK
EXISTING CONTOUR
EXISTING SPOT ELEVATION
EXISTING BOULDER
EXISTING SHRUB

EXISTING DECIDUOUS TREE (CALIPER SIZE)

EXISTING CONIFEROUS TREE

(CALIPER SIZE)

LECAL DECODIDATION

PROJECT BENCHMARK

CITY OF ALBUQUERQUE Brees teblet, stemped "1-J19", SET IN TOP OF A CONCRETE POST WHICH SETS G.G' NORTH OF THE NORTH EDGE OF A CONCRETE SIDEWALK, AND FLUSH WITH THE TOP OF SIDEWALK PAMEMENT ON THE NORTH SIDE OF CONSTITUTION AME. N.E. AND SETWEEN PENNSYLVANIA BLVD. N.E. AND CONSTITUTION PL. N.E. ON TH SOUTH SIDE OF SNOW HEIGHTS PANK.

ELEVATION = \$320.05 FEET (M.S.L.D.)

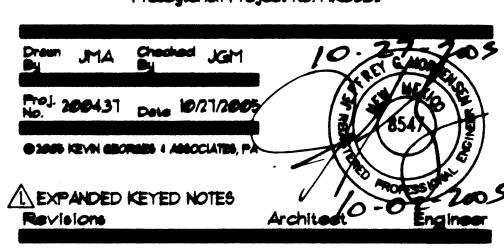
TOP OF CURB ELEVATION LOCATED AT THE S.S.E. RETURN OF THE INTERSECTION OF CONSTITUTION AVE. N.E. AND CONSTITUTION PLACE N.E. AS SHOWN ON THE BRAWING ON SHEET 1.

ELEVATION = 5329.86 FEET (M.S.L.D.)

Final Construction Documents

Parking Area A PMG Constitution

8300 Constitution Avenue NE Albuquerque, New Mexico



GRADING IPLAN

6heet 4 of 10

GRADING PLAN

- E

THIS DRAINAGE SUBMITTAL IS MADE IN SUPPORT OF A GRADING AND PAVING PERMIT AND SO #19 APPROVAL

N. PROJECT DESCRIPTION

THE SITE IS LOCATED ON THE SOUTHEAST CORNER OF CONSTITUTION AVE NE AND CONSTITUTION PLACE NE. THE CURRENT LEGAL DESCRIPTION OF THE SITE IS TRACT 1, EAST END ADDITION. THE SITE IS CURRENTLY LOCATED IN PANEL 356 OF 825 OF THE NATIONAL FLOOD INSURANCE PROGRAM FLOOD INSURANCE RATE MAPS PUBLISHED BY FEMA FOR BERNALILLO COUNTY, NEW MEXICO, NOVEMBER 19, 2003 AND IS ZONED AS ZONE X, AN AREA DETERMINED TO BE OUTSIDE THE 0.2% ANNUAL CHANCE FLOODPLAIN. THE DEVELOPMENT PROPOSED WILL INCREASE DOWNSTREAM FLOW BUT WILL NOT ADVERSELY IMPACT DOWNSTREAM FLOW OR DOWNSTREAM PROPERTIES. RUNOFF FROM THE SITE DRAINS INTO CONSTITUTION PLACE NE., AND THEN FLOWS NORTH TO CONSTITUTION AVENUE NE. AT CONSTITUTION AVENUE NE THE FLOW IS DIRECTED WEST TO THE INTERSECTION WITH PENNSYLVANIA STREET. FLOW PROCEEDS TO ENTER THREE LARGE CURB STORM INLETS ALONG CONSTITUTION AVE JUST BEFORE THE PENNSYLVANIA INTERSECTION. THESE INLETS DRAIN INTO THE 1-40 CHANNEL.

III. BACKGROUND DOCUMENTS

THE FOLLOWING ITEMS WERE REVIEWED IN THE PREPARATION OF THIS

A. GRADING AND DRAINAGE PLAN FOR PRESBYTERIAN KASEMAN HOSPITAL - RADIATION THERAPY CENTER ADDITION, PREPARED BY JMA AND DATED 05-08-96 FOR BUILDING PERMIT. THE 1996 PLAN RECOGNIZED THAT THE EXISTING POND REQUIRED ENLARGEMENT TO CONTAIN THE DESIGN VOLUME OF RUNOFF. THE PLAN ALSO IDENTIFIED A 100-YEAR PEAK DISCHARGE OF

10.7 CFS FROM THIS PORTION OF TRACT 1. B. DRAINAGE PLAN FOR PRESBYTERIAN KASEMAN HOSPITAL RADIATION THERAPY CENTER WEST EXPANSION POND ANALYSIS PREPARED BY JMA AND DATED 01-11-05. THIS PLAN CONCLUDED THAT THE NMOOT 1-40/PENNSYLVANIA OVERPASS PROJECT COMPLETED IN THE SPRING OF 2005 HAS ALLEVIATED PREVIOUS FLOODING CONCERNS ON THE CONSTITUTION AVE/PENINSYLVANIA STREET INTERSECTION. THIS PLAN USED THE CONJUGATE DEPTH EQUATION TO EVALUATE THE EFFECTS OF HYDRAULIC JUMP IN CONSTITUTION AVENUE NE. THE PLAN DETERMINED THAT THE DOWNSTREAM DRAINAGE IMPROVEMENTS THAT WERE MADE AT THE CONCLUSION OF THE 1-40/PENNSYLVANIA PROJECT ALLOWED FOR SUFFICIENT FLOW IN CONSTITUTION AVENUE WITH NO HYDRAULIC JUMP ISSUES. THIS CONCLUSION RESULTED IN THE ALLOWANCE OF THE ELIMINATION OF THE DETENTION POND IN THIS PROJECT AS WELL AS FREE DISCHARGE INTO THE STREET.

C. TOPOGRAPHIC SURVEY OF THE EXISTING SITE PREPARED BY JMA

DATED 4/13/2005. THE SUBJECT SURVEY SHOWS THE EXISTIN

IV. EXISTING CONDITIONS

THE SITE IS LOCATED AT THE SOUTHEAST CORNER OF THE INTERSECTION OF CONSTITUTION AVENUE NE AND CONSTITUTION PLACE NE. THE SITE IS PARTIALLY DEVELOPED. THE JMA TOPOGRAPHIC SURVEY DATED 4/13/2005 SHOWS THAT THE SITE CONSISTS OF AN EXISTING HOSPITAL BUILDING AND AN ASPHALT PAVED PARKING LOT, AS WELL AS AN UNDEVELOPED VEGETATED AREA AT THE NORTH AND WEST THAT IS GRADED FOR THE PURPOSE OF CREATING A DETENTION PONDING AREA FOR BOTH ON-SITE RUNOFF AND OFF-SITE FLOWS. THE SITE SLOPES FROM SOUTHEAST TO NORTHWEST AND DRAINS INTO THE POND AT THE NORTHWEST CORNER. THIS POND DRAINS VIA STORM DRAIN THROUGH AN EXISTING SIDWALK CULVERT INTO CONSTITUTION PLACE NE. CONSTITUTION AVENUE IS A FULLY IMPROVED ROADWAY, 40 FEET WIDE WITH CURB AND GUTTER ON BOTH SIDES. CONSTITUTION PLACE FLOWS ARE CARRIED BY CURB AND GUTTER NORTH INTO CONSTITUTION AVENUE, WHERE IT FLOWS WEST TO AN INTERSECTION WITH PENNSYLVANIA STREET, JUST BEFORE THIS INTERSECTION THERE ARE THREE LARGE CURB STORM INLETS ALONG CONSTITUTION AVE NE WHERE THE FLOWS WILL DRAIN IN TO. THE INLETS DRAIN SOUTH INTO THE 1-40 CHANNEL.

ONSITE CONTRIBUTING AREA FLOWS FROM A PARKING LOT ALONG THE SOUTH EDGE OF THE SITE ENTER AT THE SOUTHWEST CORNER OF THE SITE THROUGH A TRENCH DRAIN AND CURB CUT; THE FLOWS EMPTY NORTH INTO THE DETENTION POND, ONSITE CONTRIBUTING AREA FLOWS FROM AN ADJACENT PARKING LOT ALSO ENTER THE SITE THROUGH A TRENCH DRAIN INTO A 12" STORM DRAIN THAT RELEASES INTO THE MANHOLE AT THE EAST EDGE OF THE SITE; THESE FLOWS ENTER INTO A 18" CMP STORM DRAIN FROM THE ONSITE MANHOLE AND EMPTIES THESE FLOWS INTO THE SITE AT THE NORTH EDGE OF THE SITE; THESE FLOWS DRAIN WEST INTO THE DETENTION POND AS SHOWN ON THE TCL.

V. DEVELOPED CONDITIONS

THE PROJECT CONSIST OF THE CONSTRUCTION OF A NEW ASPHALT PAVED PARKING LOT THAT WILL SERVE THE EXISITING HOSPITAL BUILDINGS IN THE AREA THIS NEW LOT WILL REPLACE THE NATURALLY VEGETATED AREA ALONG THE NORTH AND WEST OF THE SITE, AND WILL ELIMINATE THE EXISTING DETENTION POND. AS A RESULT OF THESE IMPROVEMENTS, THERE WILL BE AN INCREASE IN THE RUNOFF THE SITE GENERATES AS WELL AS A REDIRECTION OF ONSITE CONTRIBUTING AREA FLOWS AROUND THE SITE THAT CURRENTLY TRAVERSE THE SITE. A SMALL PORTION OF THE ONSITE RUNOFF WILL BE PONDED AT THE NORTHWEST. CORNER OF THE DEVELOPED PARKING LOT, WHICH WILL BE DRAINED THROUGH A STORM INLET AND STORM DRAIN INTO A RUNDOWN AND SIDWALK CULVERT INTO CONSTITUTION PLACE NE AS SHOWN ON THE DEVELOPED GRADING PLAN. THIS STORM DRAIN PIPE WILL DRAIN THE CAPTURED STORMWATER RUNOFF IN A PERIOD LESS THAN SIX HOURS. FLOWS INTO CONSTITUTION PLACE NE WILL FOLLOW THE SAME DRAINAGE PATH AS STATED IN THE EXISTING CONDITIONS.

AS THE HYDROGRAPH CALCULATIONS SHOW, THE VOLUME REQUIRING PONDING WILL EQUAL 220 CF. THIS VOLUME WILL REACH A HEIGHT ABOVE THE STORM INLET OF 5333.2 FT. THE LOWEST POINT OF THE TOP OF CURB IN THE PONDING AREA IS AT 5334 FT, THIS IS ALSO THE LOWEST TOP OF CURB IN THE ENTIRE PROJECT SITE. THEREFORE, THERE IS NO PROBLEM OF RUNOFF OVERFLOWING THE CURB.

ONSITE CONTRIBUTING AREA FLOWS THAT ENTER THE MANHOLE AT THE EAST CORNER OF THE SITE WILL BE DIRECTED THROUGH THE SITE VIA A NEW 18" CMP STORM DRAIN THAT WILL TRAVERSE THE SITE FROM EAST TO WEST AND DRAIN INTO CONSTITUTION PLACE NE AT THE SAME LOCATION AS THE ONSITE FLOW STORM DRAIN RELEASES ITS FLOWS AS SHOWN ON THE DEVELOPED GRADING PLAN.

THE OVERALL RESULT OF THE DEVELOPMENT WILL INCREASE THE VOLUME AND PEAK DISCHARGE RATE INTO CONSTITUTION PLACE NE. THIS INCREASE IS ALLOWED DUE TO THE COMPLETION OF DEVELOPMENT AT PENNSYLVANIA STREET AND 1-40 THAT ALLEVIATED PAST DOWNSTREAM FLOW ISSUES; ISSUES THAT HAD PREVIOUSLY RESULTED IN THE CREATION OF THE EXISTING ONSITE DETENTION POND. U

VI. GRADING PLAN

THE GRADING PLAN SHOWS: 1.) EXISTING GRADES INDICATED BY SPOT ELEVATIONS AND CONTOURS AT 1'-0" INTERVALS BASED THE TOPOGRAPHIC SURVEY PREPARED BY JMA. DATED 4/13/2005, 2.) PROPOSED GRADES INDICATED BY CONTOURS AT 1'-O" INTERVALS, 3.) THE LIMIT AND CHARACTER OF THE EXISTING IMPROVEMENTS AS TAKEN FROM THE AFORE MENTIONED JMA SURVEY, 4.) THE LIMIT AND CHARACTER OF THE PROPOSED IMPROVEMENTS, 5.) CONTINUITY BETWEEN EXISTING AND PROPOSED GRADES. AS SHOWN BY THIS PLAN, THE PROPOSED DEVELOPMENT CONSISTS OF PAVED PARKING, ALONG WITH ASSOCIATED LANDSCAPING AND SIDEWALK. THESE MODIFICATIONS TO THE SITE WILL EXPAND AVAILABLE ONSITE PARKING AND VEHICLE ACCESS. AN EROSION CONTROL PLAN WILL BE PREPARED AND ATTACHED TO THE SEPARATE STORM WATER POLLUTION PREVENTION PLAN FOR THIS

VI. GRADING PLAN

THE GRADING PLAN SHOWS: 1.) EXISTING GRADES INDICATED BY SPOT ELEVATIONS AND CONTOURS AT 1'-0" INTERVALS BASED THE TOPOGRAPHIC SURVEY PREPARED BY JMA, DATED 4/13/2005, 2.) PROPOSED GRADES INDICATED BY CONTOURS AT 1'-0" INTERVALS, 3.) THE LIMIT AND CHARACTER OF THE EXISTING IMPROVEMENTS AS TAKEN FROM THE AFORE MENTIONED JMA SURVEY, 4.) THE LIMIT AND CHARACTER OF THE PROPOSED IMPROVEMENTS, 5.) CONTINUITY BETWEEN EXISTING AND PROPOSED GRADES. AS SHOWN BY THIS PLAN, THE PROPOSED DEVELOPMENT CONSISTS OF PAVED PARKING, ALONG WITH ASSOCIATED LANDSCAPING AND SIDEWALK. THESE MODIFICATIONS TO THE SITE WILL EXPAND AVAILABLE ONSITE PARKING AND VEHICLE ACCESS. AN EROSION CONTROL PLAN WILL BE PREPARED AND ATTACHED TO THE SEPARATE STORM WATER POLLUTION PREVENTION PLAN FOR THIS

VII. CALCULATIONS

THE CALCULATIONS WHICH APPEAR HEREON ANALYZE BOTH THE EXISTING AND DEVELOPED CONDITIONS FOR THE 100-YEAR, 6-HOUR rainfall event, as well as the 10-year, 6 hour rainfall event FOR CULVERT DESIGN PURPOSES. THE PROCEDURE FOR 40-ACRE AND SMALLER BASINS, AS SET FOR THE IN THE REVISION OF SECTION 22.2. HYDROLOGY OF THE DEVELOPMENT PROCESS MANUAL, VOLUME 2, DESIGN CRITERIA, DATED JANUARY, 1993, HAS BEEN USED TO QUANTIFY the peak rate of discharge and volume of runoff generated by THIS DEVELOPMENT. THE CAPACITY OF THE DEVELOPED 18° CMP STORM drains was analyzed using the orifice and manning's EQUATIONS. THE CAPACITIES OF THE NEW STORM INLETS WERE determined using the orifice equation, the capacity of the New 24° CULVERTS ALONG CONSTITUTION PLACE NE WERE DETERMINED USING MANNING'S EQUATION. THE PERFORMANCE OF THE PRIVATE STORMMATER DETENTION SYSTEM IN THE NORTH EAST CORNER OF THE parking area has been evaluated for outflow and storage CAPACITY BASED UP THE HYDROGRAPH ANALYSIS CONTAINED ON THIS sheet, and ponding is necessary only due to discharge PIPE SIZING CONSTRAINTS.

THE JANUARY 2005 CALCULATIONS ARE SHOWN HEREON TO SHOW THAT THE DOWNSTREAM STREET (CONSTITUTION AVENUE) IS CAPABLE OF handling flows from onsite and offsite, the conjugate depth-EQUATION WAS USED TO EVALUATE THE EFFECTS OF HYDRAULIC JUMP. AS SHOWN BY THESE CALCULATIONS, THE NORMAL DEPTH OF FLOW PLUS THE EFFECTS OF A HYDRAULIC JUMP COMPLY WITH DPM CRITERIA TO FALL BELOW THE TOP-OF-CURB PLUS 0.2 FEET. THIS SUGGESTS THAT THE STREET CAN HANDLE THE FULLY DEVELOPED DESIGN FLOW AS: CALCULATED AND THAT PONDING IS NO LONGER NECESSARY.

VIII. CONCLUSION

THE DRAINAGE CONCEPT SHOWN BY THE PLAN ABOVE WILL ACCOMPLISH ITS PURPOSE DUE TO THE FOLLOWING FACTORS:

- A.) MODIFICATION TO AN EXISTING SITE WITHIN AN INFILL AREA B.) THE PEAK RATE OF DISCHARGE FROM THE SITE WILL BE INCREASED ABOVE THAT OF THE EXISTING CONDITION
- C.) REMOVAL OF THE EXISTING POND AS ALLOWED BY APPROVED DRAINAGE SUBMITTAL DATED 01-11-05 D.) ADEQUATE STREET CAPACITY IS AVAILABLE IN CONSTITUTION PLACE NE TO ACCOMMODATE PEAK DISCHARGE UNDER FULLY DEVELOPED CONDITIONS
- E.) DOWNSTREAM CAPACITY OF CONSTITUTION PLACE AND CONSTITUTION AVENUE AND THE STORM DRAIN STRUCTURES LOCATED WITHIN ARE ADEQUATE FOR INCREASED FLOWS DUE TO THE NMOOT 1-40/PENNSYLVANIA PROJECT IMPROVEMENTS
- F.) FREE DISCHARGE INTO CONSTITUTION PLACE AND CONSTITUTION AVENUE PER THE DRAINAGE PLAN DATED 1-11-05 G.) NO ADVERSE IMPACT ON DOWNSTREAM CAPACITY OR
- DOWNSTREAM PROPERTIES H.) CULVERTS AT CONSTITUTIONS PLACE SUFFICIENT TO HANDLE PEAK DISCHARGE FROM BOTH STORM DRAINS UNDER FULLY DEVELOPED CONDITIONS
- I.) GRADING AND CURB ABOUT STORM INLET AREA SUFFICIENT TO CONTAIN MORE THAN THE 220 CF OF PONDING REQUIRED DURING A 100-YEAR STORM
- J.) OFFSITE FLOWS WILL CONTINUE TO BE INTERCEPTED AND CONVEYED THROUGH THE PROJECT TO DOWNSTREAM EXISTING DRAINAGE FACILITIES

CALCULATIONS

SITE CHARACTERISTICS

1. PRECIPITATION ZONE = 3

2. $P_{6,100} = P_{300} = 2.60$ 2. $P_{6,10} = P_{300} = 1.73$

3. TOTAL AREA (At) = 143,065 SI

4. EXISTING LAND TREATMENT

A. PROJECT SITE TREATMENT AREA (SF/AC) 16,779 / 0.39 61,626 / 1.41 43 64,660 / 1.48 45

B. ONSITE CONTRIBUTING AREAS TREATMENT AREA (SF/AC) 70,000 / 1.61

5. DEVELOPED LAND TREATMENT

A. PROJECT SITE TREATMENT AREA (SF/AC) 23,552 / 0.54 119,533 / 2.74 84

6. EXISTING CONDITION

A PROJECT SITE 1. VOLUME

Ew = (EAA+EBAe+EcAc+EbAe)/AtEW = ((0.92*0.39)+(1.29*1.41)+(2.36*1.48))/3.26 = 1.73 IN

 $V_{100} = (Ew/12)Ar = (1.73/12)3.28 = 0.2317 AC-FT 20,628 CF$

2. PEAK DISCHARGE OP = QPA AA + QPBAB + QPCAC + QPBAB

 $QP = Q100 = (2.60 \cdot 0.39) + (3.45 \cdot 1.41) + (5.02 \cdot 1.48) = 13.33 CFS$

B. ONSITE CONTRIBUTING AREAS (100-YEAR STORM)

1. VOLUME EW = (EAA+EBA+EcAc+EBA)/Ar

 $Ew = (2.36 \cdot 1.61)/1.61 = 2.36 \text{ IN}$ $V_{100} = (Ew/12)Ar = (2.36/12)1.61 = 0.3160 AC-FT = 13,767 CF$

2. PEAK DISCHARGE QP = QPA AA + QPaAs + QPcAc + QPaAs

QP = Q100 = (5.02*1.61) = 8.07 CFSC. ONSITE CONTRIBUTING AREAS (10-YEAR STORM)

1. VOLUME EW = (EAA+EBA+EcAc+EBA)/AT

 $Ew = (1.50 \cdot 1.61)/1.61 = 1.50 \text{ IN}$

 $V_{10} = (E_W/12)Ar = (1.50/12)1.61 = 0.2009 AC-FT = 8,750 CF$ 2. PEAK DISCHARGE

QP = Qm An + Qmale + QmcAc + Qmale $QP = Q160 = (3.39 \cdot 1.61) = 5.45 CFS$

7. DEVELOPED CONDITION

1. VOLUME EW = (EAA+EBA+EcAc+EBA)/Ar

Ew = ((0.92*0.54)+(2.36*2.74))/3.28 = 2.12 IN $V_{100} = (Ew/12)Ar = (2.12/12)3.28 = 0.5811 AC-FT = 25,314 CF$

2. PEAK DISCHARGE

QP = QPA AA + QPBAB + QPCAC + QPBAB QP = Q100 = (2.60 + 0.54) + (5.02 + 2.74) = 15.18 CFS

B. ONSITE CONTRIBUTING AREAS (10-YEAR STORM)

1. VOLUME $Ew = (E_AA_A + E_BA_B + E_CA_C + E_BA_B)/Ar$

 $Ew = ((0.36 \cdot 0.54) + (1.50 \cdot 2.74))/3.28 = 1.31 \text{ IN}$ $V_{10} = (Ew/12)Ar = (1.31/12)3.28 = 0.3592 AC-FT = 15,648 CF$

2. PEAK DISCHARGE

Qr = Qru Au + QreAs + QrcAc + QreAs $QP = Q10 = (1.19 \cdot 0.54) + (3.39 \cdot 2.74) = 9.95 CFS$

RAINSTORM PONDING CAPACITY

TOTAL VOL. (CF) 5333.5 [(2675+0)/2] *0.5 = 6701625 [(6575+2675)/2] + 0.5 = 975

VINOVEED = 1,625 CF>VINES = 240 CF (SEE HYDROGRAPH)

A HYDROGRAPH CALCULATIONS

TP = 0.7*Tc+(1.6-Ap/At)/12 $T_P = 0.7*0.2+(1.6-2.74/3.28)/12$ TP = 0.2037 hr

DP = 0.2546/R

 $DP = 0.25 \cdot 2.74 / 3.28$ DP = 0.2086 hr

 $T_B = 2.017 + E + AT/QP - 0.25 + AD/AT$

 $T_8 = 2.017 + 2.12 + 3.28 / 15.18 - 0.25 + 2.74 / 3.28$ $T_8 = 0.7563 \text{ hr}$

AREA OF HYDROGRAPH = Vice = 26,410 CF

VOLUME REQUIRING PONDING $\forall \textbf{wears} = (15.18-14.90) \circ ((((25.16-11.98)+(24.75-12.22))/2) \circ 60)$ VNEETS = 215.6 CF

PEAK DISCHARGE CAPACITY OF ON-SITE FLOW DISCHARGE PIPE

 $Qop = 1.49/n AR^{2/3} S^{1/2}$ $A = pier^2 = pie(0.75)^2 = 1.767 FT^2$

 $P = 2 \cdot pier = 2 \cdot pie 0.75 = 4.712 FT$ R = A/P = .375 FTS = 0.02 FT/FTQce = 14.9 CFS

Q100 = 15.2 CFS>QCM =14.9 CFS, 215 CF OF PONDING REQ'D

CULVERT CALCULATIONS FOR SITE DRAINAGE

A. PEAK DISCHARGE CAPACITY OF SINGLE CULVERT $Q_{CP} = 1.49/n AR^{2/3} S^{1/2}$ n = 0.013 $A = 0.667 \text{ FT+2 FT} = 1.33 \text{ FT}^2$

> P = 0.667 FT + 0.667 FT + 2 FT = 3.33 FTR = AP = 0.4 FT S = 0.02 FT/FT $Q_{CP} = 11.73 \text{ CFS}$

B. CULVERT DESIGN TO HOLD 10-YEAR, 6-HOUR STORM Q10,00-500 = 9.95 CFS

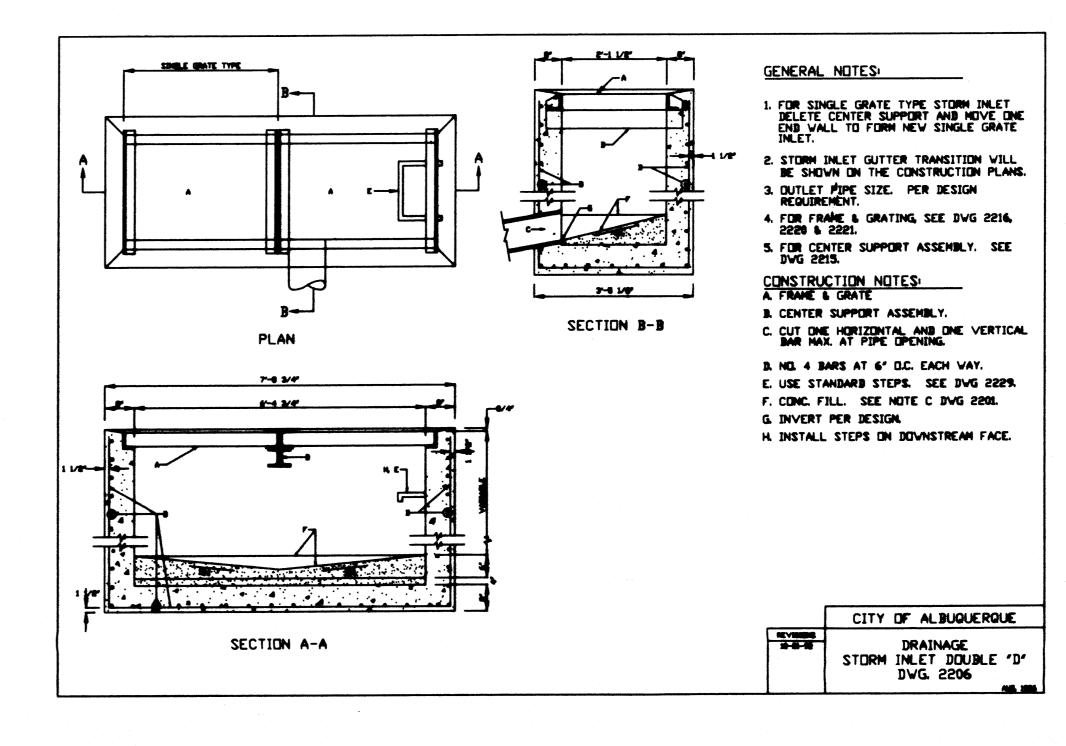
Q1e,comms = 5.45 CFS $Q_{10,REP9} = 9.95 + 5.45 = 15.4 CFS$ Q16,0000 = 15.4 CFS>Qce = 11.73 CFS Q10,0000 = 15.4 CFS<2+Qcut = 23.46 CFS DESIGN REQUIRES TWO CULVERTS

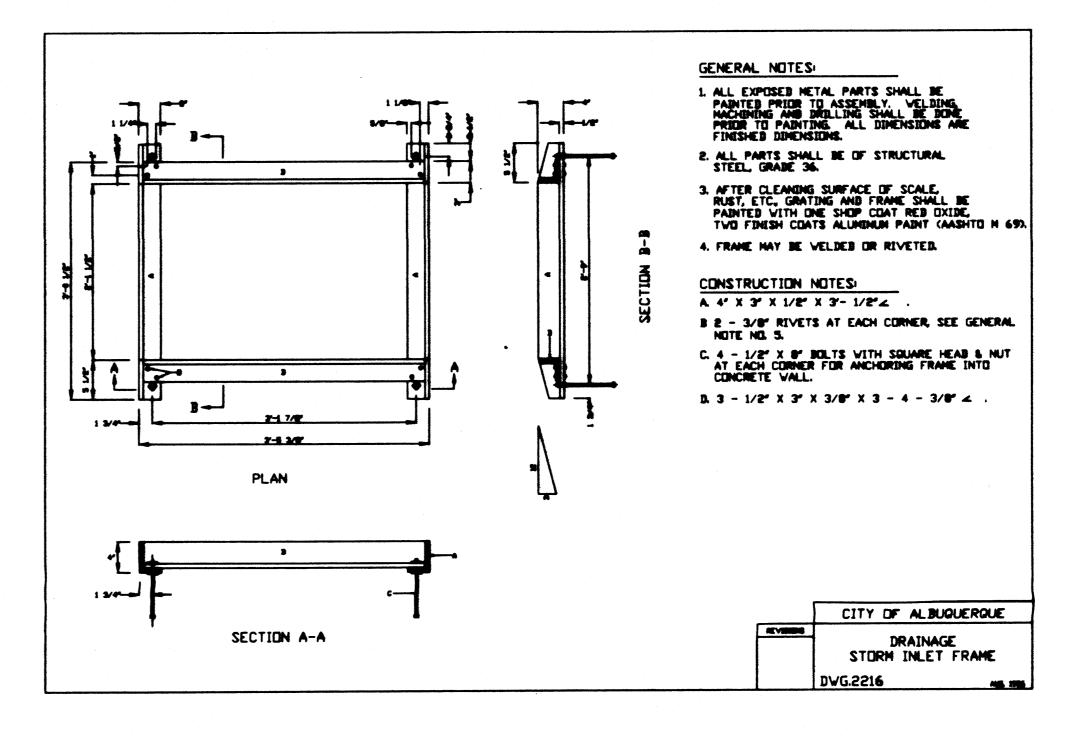
8. COMPARISON

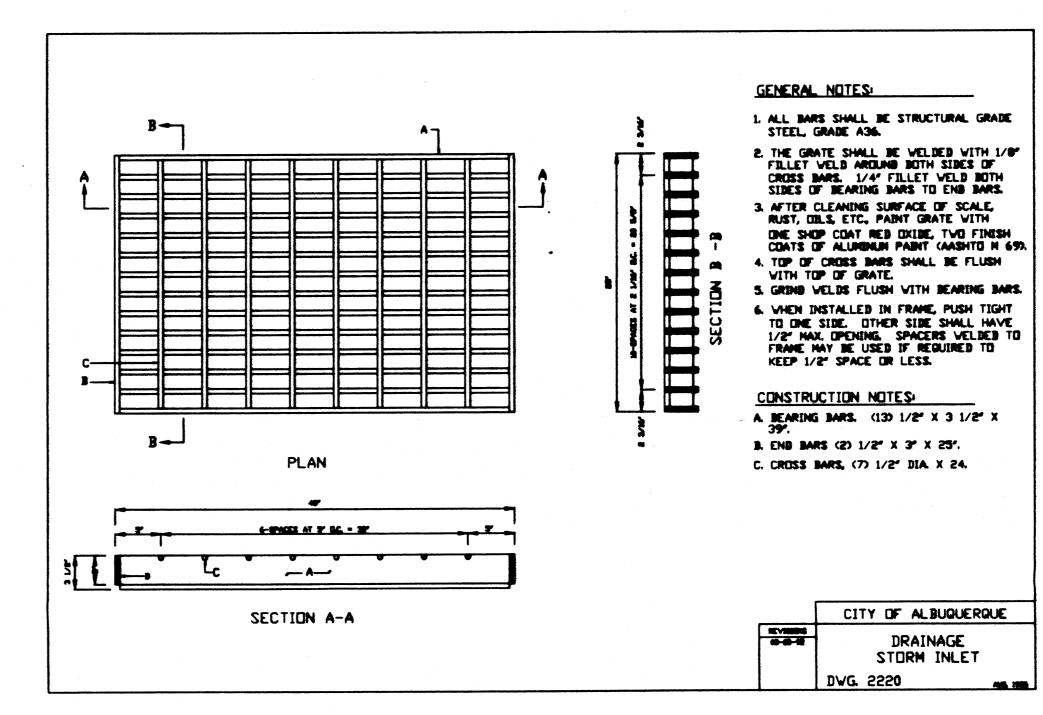
A. VOLUME $\Delta V_{100} = 25,315-20,628 = 4,687 \text{ CF (INCREASE)}$

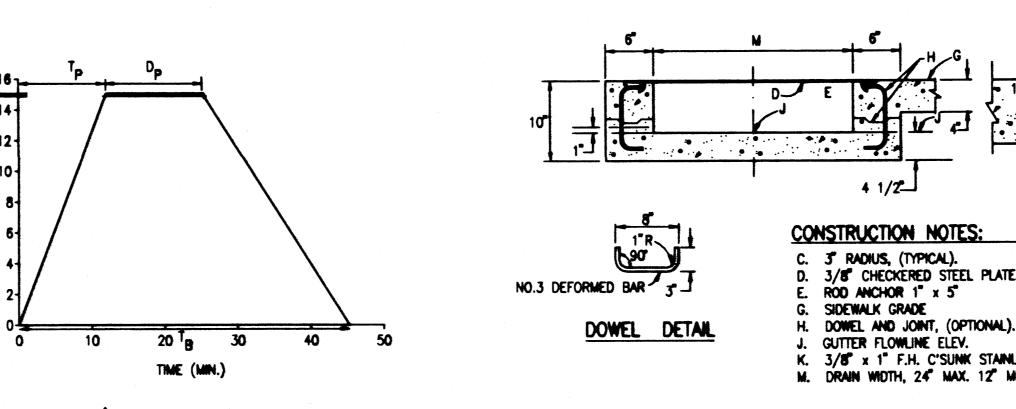
B. PEAK DISCHARGE

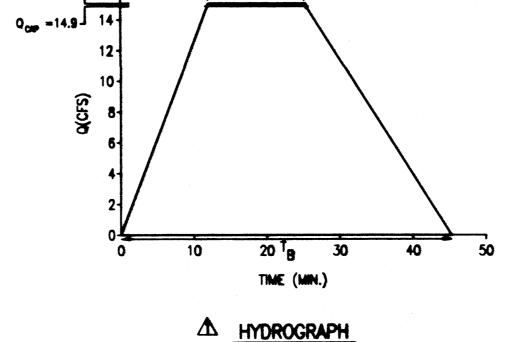
 $\Delta_{Q100} = 15.18-13.33 = 1.85 \text{ CFS (INCREASE)}$

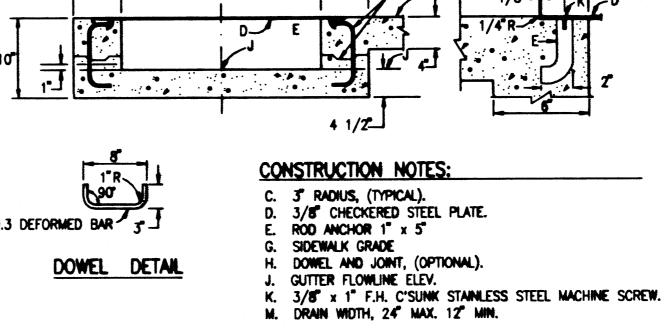












TYPICAL SIDEWALK CULVERT DETAILS NOT TO SCALE

CALCULATIONS, DRAINAGE PLAN AND SECTIONS AND DETAILS Sheet Title

Revisions

92005 KEVN GEORGES 4 ASSOCIATES. PA

L NO CHANGES TO THIS SHEET

Sheet 5 of 12

Final Construction Documents

Parking Area A PMG Constitution

8300 Constitution Avenue NE

Albuqueraus, New Mexico

Presbyterian Project No. MR6867

Kevin Georges 4 Associates

214 Trumen Street NE - Albuquerque, New Mexico 81106-1333 506/256-4978

Architecture 4 Planning



☐ FAX: 505 345-4254 ☐ ESTABLISHED 1977

5. REMOVE AND DISPOSE OF 18" CMP STORM DRAIN PIPE 6. REMOVE AND DISPOSE OF 6" PVC PIPE 7. REMOVE AND DISPOSE OF 4" CHAINLINK FENCE 8. EXISTING CONCRETE RUNDOWN TO REMAIN 9. EXISTING CURB AND GUTTER TO REMAIN

10. CONSTRUCT MULTI-DIRECTIONAL WHEELCHAIR RAMP PER DETAIL, SHEET 11. CONSTRUCT STANDARD CURB AND GUTTER PER DETAIL, SHEET 2 12. CONSTRUCT 2' SIDEWALK CULVERT, TYPICAL, PER DETAIL, SHEET 2 13. CONSTRUCT RUNDOWN PER DETAIL, SHEET 2 14. CONSTRUCT 5' WIDE CONCRETE SIDEWALK 15. CONSTRUCT CONCRETE STAIRWAY PER DETAIL, SHEET 2 16. CONSTRUCT CONCRETE SIDEWALK PER DETAIL, SHEET 2:

SEE LANDSCAPING PLAN FOR LAYOUT 17. CONSTRUCT NEW ASPHALT PAVEMENT PER TYPICAL PARKING LOT PAVEMENT SECTION, SHEET 2

18. PAINT WHITE DIRECTIONAL ARROWS WITH WHITE TRAFFIC PAINT, MIN 2 COATS PER MUTCD DEC 2000

19. PAINT 4" PAVEMENT MARKING W/ WHITE TRAFFIC PAINT, MIN. 2 20. PAINT CROSSWALK PAVEMENT MARKING PER DETAIL, SHEET 2 21. NEW LANDSCAPING, SEE LANDSCAPING PLAN 22. INSTALL 18" CMP STORM DRAIN • S = 0.02 23. INSTALL SINGLE 'D' STORM INLET PER DETAIL, SHEET 4 W/ T.O.G .

33.00 AND INV 18" (OUT) @ 30.47 24. REMOVE AND DISPOSÈ EXISTING STORM INLET 25. STENCIL "COMPACT" ON PARKING LOT PAVEMENT 26. PAINT TOP AND FACE OF CURB W/ YELLOW TRAFFIC PAINT, MIN. 2

27. BEGIN PAINTING TOP AND FACE OF CURB WITH YELLOW TRAFFIC 28. END PAINTING TOP AND FACE OF CURB WITH YELLOW TRAFFIC 29. STENCIL "NO PARKING" ON TOP AND FACE OF CURB AT 50' SPACING 30. BEGIN STENCILING "NO PARKING" ON TOP AND FACE OF CURB

31. END STENCILING "NO PARKING" ON TOP AND FACE OF CURB

32. REMOVE AND DISPOSE 4' DOUBLE SWING GATE

37. EXISTING CONCRETE RUNDOWN TO REMAIN (3')
38. NEW LIGHT STANDARD AND BASE, SEE ELECTRICAL 39. CONSTRUCT CONCRETE SIDEWALK FLUSH WITH PAVING PER

DETAIL, SHEET 2

53. CLEAR SIGHT TRIANGLE

40. NEATLY REMOVE AND SALVAGE EXISTING PARKING BUMPERS 41. INSTALL 1 - ADA COMPLIANT HANDICAP PARKING SIGN 42. INSTALL 1 - ADA COMPLIANT HANDICAP PARKING SIGN W/ VAN ACCESSIBLE PLACARD 43. NEW GROUND MOUNTED SIGN BY OWNER, PROVIDE POWER, COORDINATE REQUIREMENTS WITH SIGN COMPANY 44. CONSTRUCT RUNDOWN FROM HEADWALL PER DETAIL, SHEET 2

45. REMOVE AND DISPOSE EXISTING CONCRETE RUNDOWN 46. EXISTING TRENCH DRAIN TO REMAIN 47. INSTALL 1 - ADA COMPLIANT HANDICAP PARKING SPACE PAVEMENT MARKING, TYPICAL 48. PAINT 4" WIDE CROSS-HATCH (2' CC) PAVEMENT MARKINGS • 45

DEG W/ WHITE TRAFFIC PAINT, MIN 2 COATS, TYPICAL 49. EXISTING LIGHT POLE TO REMAIN 50. EXISTING TREES AND SHRUBS TO REMAIN 51. CONSTRUCT SIDEWALK CULVERT, TYPICAL, PER DETAIL, SHEET 2 AND COA STANDARD DWG. 2236

52. INSTALL 18" CMP STORM DRAIN • S = 0.015

VICINITY MAP SCALE: 1" = 750'±

PAVING SITE PLAN (TCL)





AREA LIGHT AREA LIGHT ON CONCRETE ANTI-SIPHON VALVE BUILDING OVERHANG BIKE RACK CURB AND CUTTER CONCRETE DRIVEPAD CONCRETE HEADER CURB CAST IRON PIPE CENTERLINE OF DOOR CENTERLINE OF DOUBLE DOOR CHAIN LINK FENCE SANITARY SEWER CLEANOUT CONCRETE CURB OPENING CONCRETE PIPE

CONCRETE RUNDOWN CONCRETE RETAINING WALL CONCRETE WHEELCHAIR RAME EDGE OF ASPHALT ELECTRIC CONDUIT ELECTRIC OUTLET ELECTRIC OUTLET IN CONCRETE ELECTRIC PULLBOX

FLAG POLE GAS PAINT MARK HANDICAP PARKING SIGN PRRIGATION METAL BUILDING COLUMN METAL LIGHT POLE METAL POLE

METAL POWER POLE METAL SIGN OVERHEAD ELECTRIC (NO. OF LINES) OVERHEAD TELEPHONE (NO. OF LINES) PAINTED ISLAND PLASTIC BENCH SANITARY SEWER MANHOLE

SPRINKLER CONTROL TIMER STD. CAIG STANDARD CURB AND GUTTER TOP OF SIDEWALK

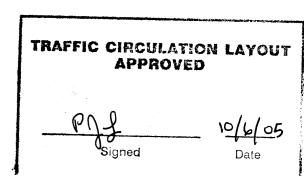
SIDEWALK CULVERT TOP OF ASPHALT TOP OF CURB TOP OF CONCRETE TOP OF GRATE TELEPHONE MANHOLE TELEPHONE PULLBOX TELEPHONE RISER/PULLBOX TRAFFIC SIGN TELEPHONE VAULT

UNDERGROUND WALLEY GUTTER WITH METAL HANDRAIL WOOD POLE WATER WALVE BOX CROSSWALK EXISTING CONTOUR EXISTING SPOT ELEVATION

EXISTING SHRUB EXISTING DECIDUOI (2.20) (CALIPER SIZE) EXISTING DECIDUOUS TREE

EXISTING BOULDER

(2.20) EXISTING CONIFEROUS TREE (CALIPER SIZE)



CITY OF ALBUQUERQUE Breas tablet, stamped "1-J19", SET IN TOP OF A CONCRETE POST SHICH SETS 0.0" NORTH OF THE NORTH EDGE OF A CONCRETE SIDEMALK, AND FLUSH WITH THE TOP OF SIDEMALK PANEMENT ON THE NORTH SIDE OF CONSTITUTION AVE. ILE. AND SETWEEN PEDBISYLVANIA BLVD. ILE. AND CONSTITUTION PL. ILE. ON TH SOUTH SIDE OF SHOW HEIGHTS PANK.

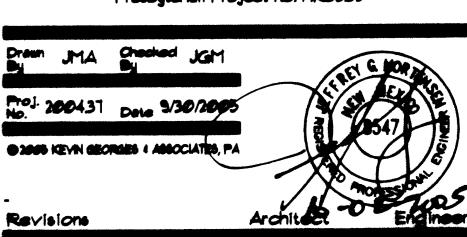
ELEVATION = \$320.05 PEET (M.S.L.D.)

TOP OF CURB ELEVATION LOCATED AT THE S.S.E. RETURN OF THE INTERSECTION OF CONSTITUTION AVE. N.E. AND CONSTITUTION PLACE N.E. AS SHOWN ON THE BRANKING ON SHEET 1.

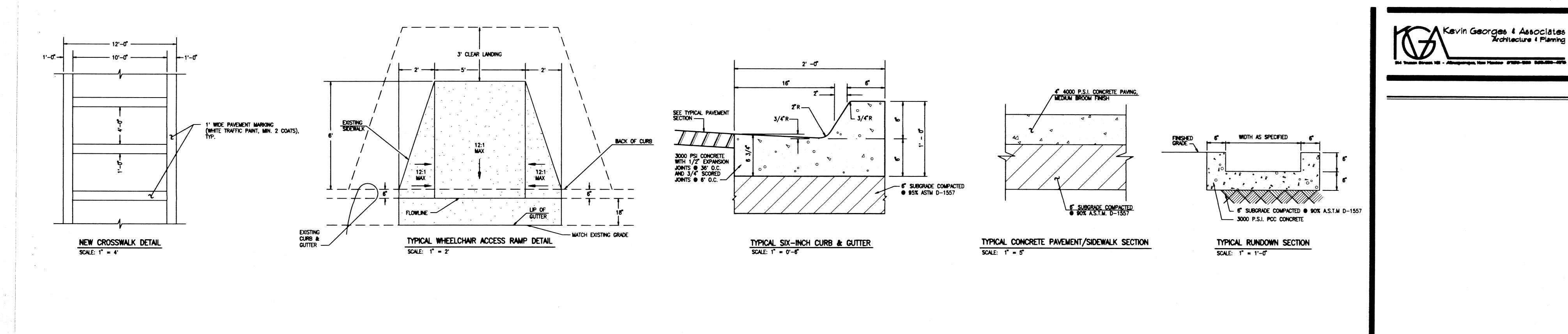
ELEVATION = 5328.86 FEET (M.S.L.D.)

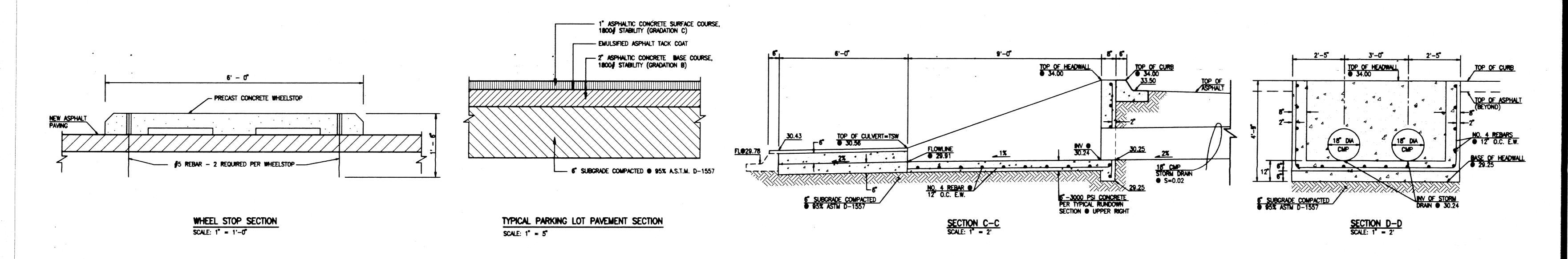
Final Construction Documents

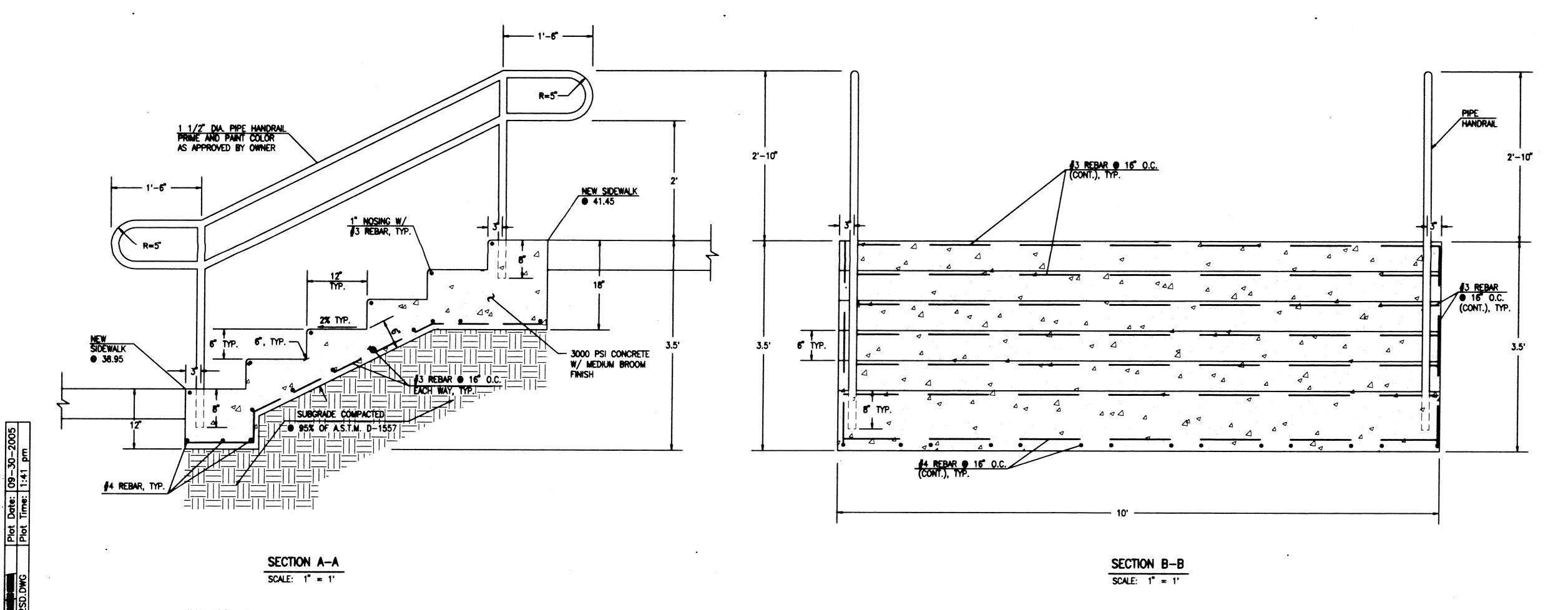
Parking Area A PMG Constitution 8300 Constitution Avenue NE Albuquerque, New Mexico

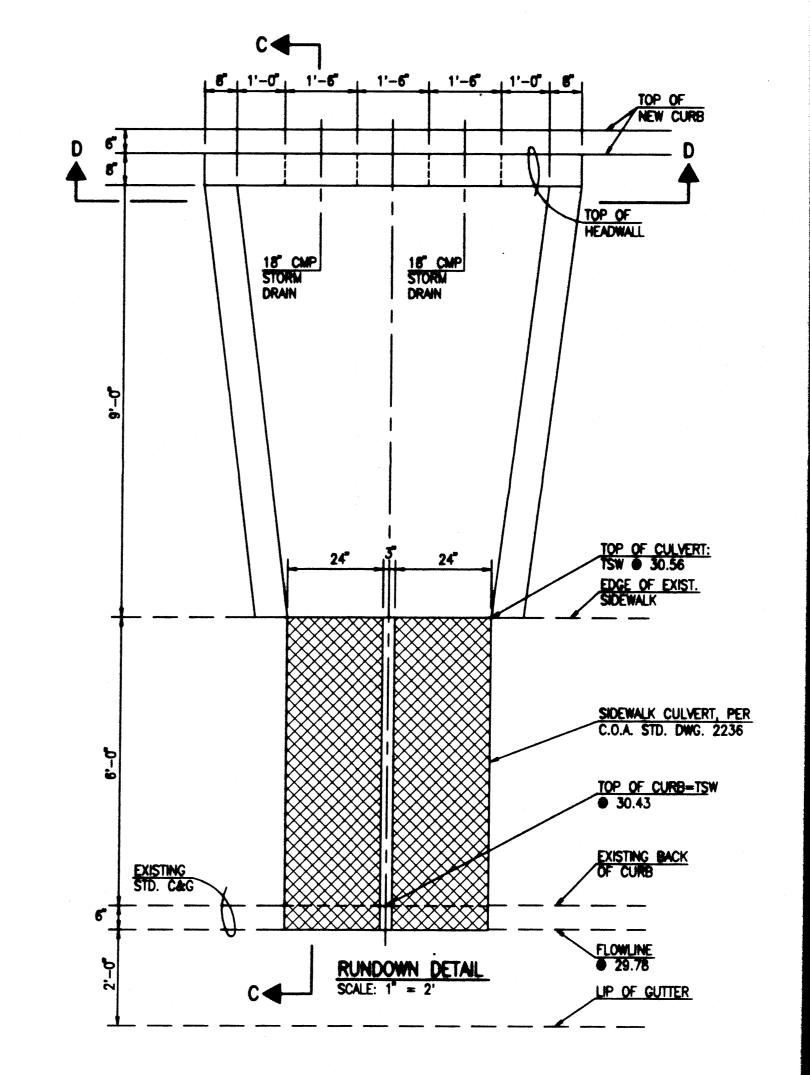


PAYING SITE PLAN (TCL)





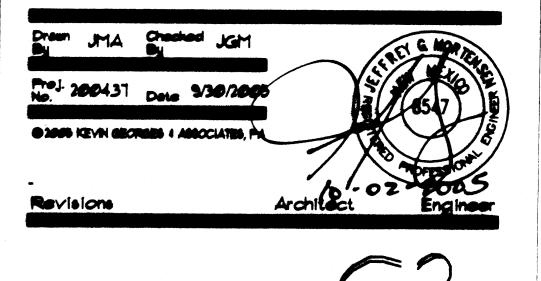




PAYING SECTIONS AND DETAILS

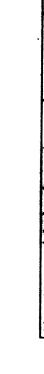


Parking Area A
PMG Constitution
8300 Constitution Avenue NE
Albuquerque, New Mexico



PAVING SECTIONS AND DETAILS

est Title Sheet 3 of 1



JMA JOB NO. 2004.120.2

JEFF MORTENSEN & ASSOCIATES, INC.

6010-B MIDWAY PARK BLVD. N.E.

ALBUQUERQUE | NEW MEXICO 87109

ENGINEERS | SURVEYORS (505) 345-4250

FAX: 505 345-4254 | ESTABLISHED 1977

- 3. ALL WORK ON THIS PROJECT SHALL BE PERFORMED IN ACCORDANCE WITH APPLICABLE FEDERAL, STATE AND LOCAL LAWS, RULES AND REGULATIONS CONCERNING CONSTRUCTION SAFETY AND HEALTH. 4. ALL CONSTRUCTION WITHIN PUBLIC RIGHT-OF-WAY SHALL BE PERFORMED IN ACCORDANCE WITH APPLICABLE CITY OF ALBUQUERQUE
- STANDARDS AND PROCEDURES. 5. IF ANY UTILITY LINES, PIPELINES, OR UNDERGROUND UTILITY LINES ARE SHOWN ON THESE DRAWINGS, THEY ARE SHOWN IN AN APPROXIMATE MANNER ONLY, AND SUCH LINES MAY EXIST WHERE NONE ARE SHOWN. IF ANY SUCH EXISTING LINES ARE SHOWN, THE LOCATION IS BASED UPON INFORMATION PROVIDED BY THE OWNER OF SAID UTILITY, AND THE INFORMATION MAY BE INCOMPLETE, OR MAY BE OBSOLETE BY THE TIME CONSTRUCTION COMMENCES. THE ENGINEER HAS CONDUCTED ONLY PRELIMINARY INVESTIGATION OF TH LOCATION, DEPTH, SIZE, OR TYPE OF EXISTING UTILITY LINES, PIPELINES, OR UNDERGROUND UTILITY LINES. THIS INVESTIGATION IS NOT CONCLUSIVE, AND MAY NOT BE COMPLETE, THEREFORE, MAKES NO REPRESENTATION PERTAINING THERETO, AND ASSUMES NO RESPONSIBILITY OR LIABILITY THEREFOR. THE CONTRACTOR SHALL INFORM ITSELF OF THE LOCATION OF ANY UTILITY LINE, PIPELINE, OR UNDERGROUND UTILITY LINE IN OR NEAR THE AREA OF THE WORK IN ADVANCE OF AND DURING EXCAVATION WORK. THE CONTRACTOR IS FULLY RESPONSIBLE FOR ANY AND ALL DAMAGE CAUSED BY ITS FAILURE TO LOCATE, IDENTIFY AND PRESERVE ANY AND ALL EXISTING UTILITIES, PIPELINES, AND UNDERGROUND UTILITY LINES. IN PLANNING AND CONDUCTING EXCAVATION, THE

LOCAL ORDINANCES, RULES AND REGULATIONS, IF ANY, PERTAINING TO THE LOCATION OF THESE LINES AND FACILITIES. JMA JOB NO. 2004.120.3 JEFF MORTENSEN & ASSOCIATES, INC. 1 6010-B MIDWAY PARK BLVD. N.E. ☐ ALBUQUERQUE ☐ NEW MEXICO 87109
☐ ENGINEERS ☐ SURVEYORS (505) 345-4250
☐ FAX: 505 345-4254 ☐ ESTABLISHED 1977

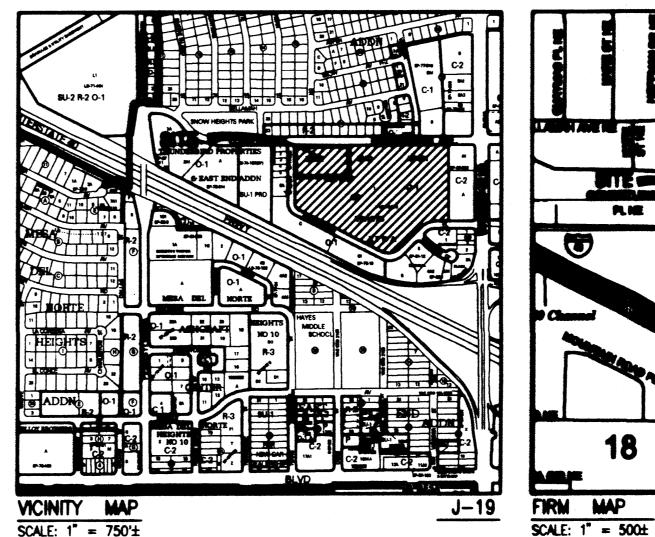
CONTRACTOR SHALL COMPLY WITH STATE STATUTES, MUNICIPAL AND

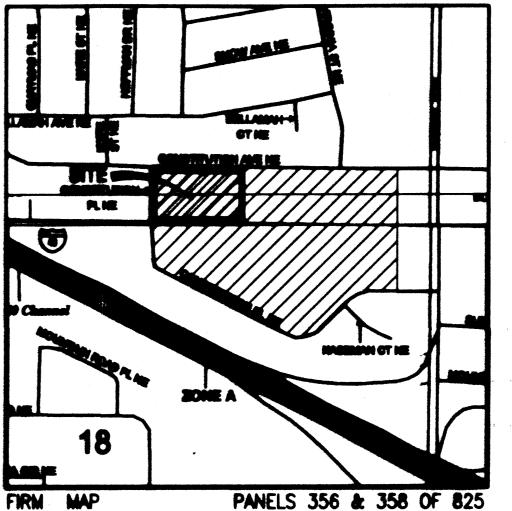
EROSION CONTROL MEASURES: 1. THE CONTRACTOR SHALL ENSURE THAT NO SOIL ERODES FROM THE SITE INTO PUBLIC RIGHT-OF-WAY OR ONTO PRIVATE PROPERTY. 2. THE CONTRACTOR SHALL PROMPTLY CLEAN UP ANY MATERIAL

EXCAVATED WITHIN THE PUBLIC RIGHT-OF-WAY SO THAT THE

- EXCAVATED MATERIAL IS NOT SUSCEPTIBLE TO BEING WASHED DOWN 3. WHEN APPLICABLE, CONTRACTOR SHALL SECURE "TOPSOIL DISTURBANCE PERMIT" FROM THE CITY AND/OR FILE A NOTICE OF INTENT (N.O.I.) WITH THE EPA PRIOR TO BEGINNING CONSTRUCTION.
- 4. UNLESS FINAL STABILIZATION IS OTHERWISE PROVIDED FOR, ANY AREAS OF EXCESS DISTURBANCE (TRAFFIC ACCESS, STORAGE YARD, EXCAVATED MATERIAL, ETC.) SHALL BE RE-SEEDED ACCORDING TO C.O.A. SPECIFICATION 1012 "NATIVE GRASS SEEDING". THIS WILL BE CONSIDERED INCIDENTAL TO CONSTRUCTION, THEREFORE, NO SEPARATE PAYMENT WILL BE MADE.

APPROVALS	NAME	DATE
HYDROLOGY		
SIDEWALK INSPECTOR		
STORM DRAIN MAINTENANCE		





GRADING PLAN





AREA LIGHT ON CONCRETE ANTI-SIPHON VALVE BUILDING OVERHANG CURB AND GUTTER CONCRETE BENCH CONCRETE DRIVEPAD CONCRETE HEADER CURB CAST IRON PIPE CENTERLINE OF DOOR CENTERLINE OF DOUBLE DOOR CHAIN LINK FENCE SANITARY SEWER CLEANOUT

CURB OPENING CONCRETE PIPE CONCRETE RUNDOWN CONCRETE RETAINING WALL CONCRETE WHEELCHAIR RAMP

EDGE OF ASPHALT ELECTRIC CONDUIT ELECTRIC OUTLET ELECTRIC OUTLET IN CONCRETE ELECTRIC PULLBOX FLOWLINE FLAG POLE

GAS PAINT MARK IRRIGATION METAL BUILDING COLUMN METAL LIGHT POLE METAL POLE

METAL POWER POLE METAL SIGN OVERHEAD ELECTRIC (NO. OF LINES) OVERHEAD TELEPHONE (NO. OF LINES)
PARKING BUMPER PAINTED ISLAND

PLASTIC BENCH PAINT MARK SANITARY SEWER MANHOLE SPRINKLER CONTROL TIMER STORM DRAIN STORM DRAIN MANHOLE STORM INLET

STORM INLET IN CONCRETE STANDARD CURB AND GUTTER SIDEWALK CULVERT TOP OF ASPHALT TOP OF CURB TOP OF GRATE

TELEPHONE MANHOLE TELEPHONE PULLBOX TELEPHONE RISER/PULLBOX TRAFFIC SIGN TELEPHONE VAULT UNDERGROUND WALLEY GUTTER

WITH METAL HANDRAIL WOOD POLE WATER VALVE BOX CROSSWALK EXISTING CONTOUR EXISTING SPOT ELEVATION EXISTING BOULDER

EXISTING SHRUB

EXISTING DECIDUOUS TREE (CALIPER SIZE) EXISTING CONIFEROUS TREE

STORM DRAINAGE KEYED NOTES:

(22) Install 18" CMP Storm Drain (0 S = 0.02) 23) Install single 'd' storm inlet per detail, sheet 4 w/ t.o.g • 33.00 AND INV 18" (OUT) ● 30.47 (52) INSTALL 18" CMP STORM DRAIN • S = 0.015

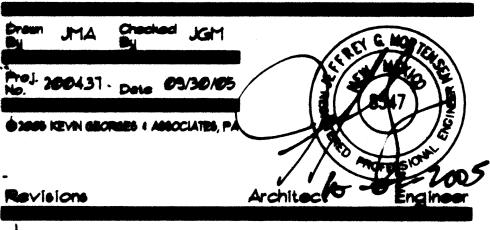
PROJECT BENCHMARK CITY OF ALBUQUERQUE Bross toblet, stomped "1-J19", SET IN TOP OF A CONCRETE POST WHICH SETS Q.B" NORTH OF THE NORTH EDGE OF A CONCRETE SIDEWALK, AND FLUSH WITH THE TOP OF SIDEWALK PAVEMENT ON THE NORTH SIDE OF CONSTITUTION AVE. N.E. AND BETWEEN PENNISYLVANIA BLVD. N.E. AND CONSTITUTION PL. N.E. ON TH SOUTH SIDE OF SHOW HEIGHTS PARK.
ELEWATION = \$320.05 FEET (M.S.L.D.)

TOP OF CURB ELEVATION LOCATED AT THE S.S.E. RETURN OF THE INTERSECTION OF CONSTITUTION AVE. N.E. AND CONSTITUTION PLACE N.E. AS SHOWN ON THE BRAWING ON SMEET 1. ELEWATION = 5329.86 FEET (M.S.L.D.)

Final Construction Documents

Parking Area A PMG Constitution

8300 Constitution Avenue NE Albuquerque, New Mexico Presbyterien Project No. MR6562



GRADING PLAN

THIS DRAINAGE SUBMITTAL IS MADE IN SUPPORT OF A GRADING AND PAVING PERMIT AND SO #19 APPROVAL.

II. PROJECT DESCRIPTION

THE SITE IS LOCATED ON THE SOUTHEAST CORNER OF CONSTITUTION AVE NE AND CONSTITUTION PLACE NE. THE CURRENT LEGAL DESCRIPTION OF THE SITE IS TRACT 1, EAST END ADDITION. THE SITE IS CURRENTLY LOCATED IN PANEL 356 OF 825 OF THE NATIONAL FLOOD INSURANCE PROGRAM FLOOD INSURANCE RATE MAPS PUBLISHED BY FEMA FOR BERNALILLO COUNTY, NEW MEXICO, NOVEMBER 19, 2003 AND is zoned as zone X. An area determined to be outside the 0.2% ANNUAL CHANCE FLOODPLAIN. THE DEVELOPMENT PROPOSED WILL INCREASE DOWNSTREAM FLOW BUT WILL NOT ADVERSELY IMPACT DOWNSTREAM FLOW OR DOWNSTREAM PROPERTIES. RUNOFF FROM THE SITE DRAINS INTO CONSTITUTION PLACE NE., AND THEN FLOWS NORTH TO CONSTITUTION AVENUE NE. AT CONSTITUTION AVENUE NE. THE FLOW IS DIRECTED WEST TO THE INTERSECTION WITH PENNSYLVANIA STREET. FLOW PROCEEDS TO ENTER THREE LARGE CURB STORM INLETS ALONG CONSTITUTION AVE JUST BEFORE THE PENNSYLVANIA INTERSECTION. THESE INLETS DRAIN INTO THE 1-40 CHANNEL.

III. BACKGROUND DOCUMENTS

THE FOLLOWING ITEMS WERE REVIEWED IN THE PREPARATION OF THIS SUBMITTAL:

A. GRADING AND DRAINAGE PLAN FOR PRESBYTERIAN KASEMAN HOSPITAL - RADIATION THERAPY CENTER ADDITION, PREPARED BY JMA AND DATED 05-08-96 FOR BUILDING PERMIT. THE 1996 PLAN RECOGNIZED THAT THE EXISTING POND REQUIRED ENLARGEMENT TO CONTAIN THE DESIGN VOLUME OF RUNOFF THE PLAN ALSO IDENTIFIED A 100-YEAR PEAK DISCHARGE OF 10.7 CFS FROM THIS PORTION OF TRACT 1.

B. DRAINAGE PLAN FOR PRESBYTERIAN KASEMAN HOSPITAL -RADIATION THERAPY CENTER WEST EXPANSION POND ANALYSIS PREPARED BY JMA AND DATED 01-11-05. THIS PLAN CONCLUDED THAT THE NMIDOT 1-40/PENNSYLVANIA OVERPASS PROJECT COMPLETED IN THE SPRING OF 2005 HAS ALLEVIATED PREVIOUS FLOODING CONCERNS ON THE CONSTITUTION AVE/PENNSYLVANIA STREET INTERSECTION. THIS PLAN USED THE CONJUGATE DEPTH EQUATION TO EVALUATE THE EFFECTS OF HYDRAULIC JUMP IN CONSTITUTION AVENUE NE. THE PLAN DETERMINED THAT THE DOWNSTREAM DRAINAGE IMPROVEMENTS THAT WERE MADE AT THE CONCLUSION OF THE 1-40/PENINSYLVANIA PROJECT ALLOWED FOR SUFFICIENT FLOW IN CONSTITUTION AVENUE WITH NO HYDRAULIC JUMP ISSUES. THIS CONCLUSION RESULTED IN THE ALLOWANCE OF THE ELIMINATION OF THE DETENTION POND IN THIS PROJECT AS WELL AS FREE DISCHARGE INTO THE STREET.

C. TOPOGRAPHIC SURVEY OF THE EXISTING SITE PREPARED BY JMA DATED 4/13/2005. THE SUBJECT SURVEY SHOWS THE EXISTING

IV. EXISTING CONDITIONS

THE SITE IS LOCATED AT THE SOUTHEAST CORNER OF THE INTERSECTION OF CONSTITUTION AVENUE NE AND CONSTITUTION PLACE NE. THE SITE IS PARTIALLY DEVELOPED. THE JMA TOPOGRAPHIC SURVEY DATED 4/13/2005 SHOWS THAT THE SITE CONSISTS OF AN EXISTING HOSPITAL BUILDING AND AN ASPHALT PAVED PARKING LOT, AS WELL AS AN UNDEVELOPED VEGETATED AREA AT THE NORTH AND WEST THAT IS GRADED FOR THE PURPOSE OF CREATING A DETENTION PONDING AREA FOR BOTH ON-SITE RUNOFF AND OFF-SITE FLOWS. THE SITE SLOPES FROM SOUTHEAST TO NORTHWEST AND DRAINS INTO THE POND AT THE NORTHWEST CORNER. THIS POND DRAINS VIA STORM DRAIN THROUGH AN EXISTING SIDWALK CULVERT INTO CONSTITUTION PLACE NE. CONSTITUTION AVENUE IS A FULLY IMPROVED ROADWAY, 40 FEET WIDE WITH CURB AND GUTTER ON BOTH SIDES, CONSTITUTION PLACE FLOWS ARE CARRIED BY CURB AND GUTTER NORTH INTO CONSTITUTION AVENUE, WHERE IT FLOWS WEST TO AN INTERSECTION WITH PENNSYLVANIA STREET. JUST BEFORE THIS INTERSECTION THERE ARE THREE LARGE CURB STORM INLETS ALONG CONSTITUTION AVE NE WHERE THE FLOWS WILL DRAIN IN TO. THE INLETS DRAIN SOUTH INTO THE 1-40 CHANNEL.

ONSITE CONTRIBUTING AREA FLOWS FROM A PARKING LOT ALONG THE SOUTH EDGE OF THE SITE ENTER AT THE SOUTHWEST CORNER OF THE SITE THROUGH A TRENCH DRAIN AND CURB CUT: THE FLOWS EMPTY NORTH INTO THE DETENTION POND. ONSITE CONTRIBUTING AREA FLOWS FROM AN ADJACENT PARKING LOT ALSO ENTER THE SITE THROUGH A TRENCH DRAIN INTO A 12" STORM DRAIN THAT RELEASES INTO THE MANHOLE AT THE EAST EDGE OF THE SITE; THESE FLOWS ENTER INTO A 18" CMP STORM DRAIN FROM THE ONSITE MANHOLE AND EMPTIES THESE FLOWS INTO THE SITE AT THE NORTH EDGE OF THE SITE; THESE FLOWS DRAIN WEST INTO THE DETENTION POND AS SHOWN ON THE TCL.

V. DEVELOPED CONDITIONS

THE PROJECT CONSIST OF THE CONSTRUCTION OF A NEW ASPHALT PAVED PARKING LOT THAT WILL SERVE THE EXISITING HOSPITAL BUILDINGS IN THE AREA. THIS NEW LOT WILL REPLACE THE NATURALLY VEGETATED AREA ALONG THE NORTH AND WEST OF THE SITE, AND WILL ELIMINATE THE EXISTING DETENTION POND. AS A RESULT OF THESE IMPROVEMENTS THERE WILL BE AN INCREASE IN THE RUNOFF THE SITE GENERATES AS WELL AS A REDIRECTION OF ONSITE CONTRIBUTING AREA FLOWS AROUND THE SITE THAT CURRENTLY TRAVERSE THE SITE. A SMALL PORTION OF THE ONSITE RUNOFF WILL BE PONDED AT THE NORTHWEST CORNER OF THE DEVELOPED PARKING LOT, WHICH WILL BE DRAINED THROUGH A STORM INLET AND STORM DRAIN INTO A RUNDOWN AND SIDWALK CULVERT INTO CONSTITUTION PLACE NE AS SHOWN ON THE DEVELOPED GRADING PLAN. THIS STORM DRAIN PIPE WILL DRAIN THE CAPTURED STORMWATER RUNOFF IN A PERIOD LESS THAN SIX HOURS. FLOWS INTO CONSTITUTION PLACE NE WILL FOLLOW THE SAME DRAINAGE PATH AS STATED IN THE EXISTING CONDITIONS.

AS THE HYDROGRAPH CALCULATIONS SHOW, THE VOLUME REQUIRING PONDING WILL EQUAL 220 CF. THIS VOLUME WILL REACH A HEIGHT AROVE THE STORM INLET OF 5333.2 FT. THE LOWEST POINT OF THE TOP OF CURB IN THE PONDING AREA IS AT 5334 FT, THIS IS ALSO THE LOWEST TOP OF CURB IN THE ENTIRE PROJECT SITE. THEREFORE, THERE IS NO PROBLEM OF RUNOFF OVERFLOWING THE CURB.

ONSITE CONTRIBUTING AREA FLOWS THAT ENTER THE MANHOLE AT THE EAST CORNER OF THE SITE WILL BE DIRECTED THROUGH THE SITE VIA A NEW 18" CMP STORM DRAIN THAT WILL TRAVERSE THE SITE FROM EAST TO WEST AND DRAIN INTO CONSTITUTION PLACE NE AT THE SAME LOCATION AS THE ONSITE FLOW STORM DRAIN RELEASES ITS FLOWS AS SHOWN ON THE DEVELOPED GRADING PLAN.

THE OVERALL RESULT OF THE DEVELOPMENT WILL INCREASE THE VOLUME AND PEAK DISCHARGE RATE INTO CONSTITUTION PLACE NE. THIS INCREASE IS ALLOWED DUE TO THE COMPLETION OF DEVELOPMENT AT PENNSYLVANIA STREET AND 1-40 THAT ALLEVIATED PAST DOWNSTREAM FLOW ISSUES: ISSUES THAT HAD PREVIOUSLY RESULTED IN THE CREATION OF THE EXISTING ONSITE DETENTION POND. U

VI. GRADING PLAN

THE GRADING PLAN SHOWS: 1.) EXISTING GRADES INDICATED BY SPOT ELEVATIONS AND CONTOURS AT 1'-0" INTERVALS BASED THE TOPOGRAPHIC SURVEY PREPARED BY JMA, DATED 4/13/2005, 2.) PROPOSED GRADES INDICATED BY CONTOURS AT 1'-0" INTERVALS. 3.) THE LIMIT AND CHARACTER OF THE EXISTING IMPROVEMENTS AS TAKEN FROM THE AFORE MENTIONED JMA SURVEY, 4.) THE LIMIT AND CHARACTER OF THE PROPOSED IMPROVEMENTS, 5.) CONTINUITY BETWEEN EXISTING AND PROPOSED GRADES. AS SHOWN BY THIS PLAN, THE PROPOSED DEVELOPMENT CONSISTS OF PAVED PARKING, ALONG WITH ASSOCIATED LANDSCAPING AND SIDEWALK. THESE MODIFICATIONS TO THE SITE WILL EXPAND AVAILABLE ONSITE PARKING AND VEHICLE ACCESS. AN EROSION CONTROL PLAN WILL BE PREPARED AND ATTACHED TO THE SEPARATE STORM WATER POLLUTION PREVENTION PLAN FOR THIS

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VII. CALCULATIONS

THE CALCULATIONS WHICH APPEAR HEREON ANALYZE BOTH THE EXISTING AND DEVELOPED CONDITIONS FOR THE 100-YEAR, 6-HOUR RAINFALL EVENT, AS WELL AS THE 10-YEAR, 6 HOUR RAINFALL EVENT FOR CULVERT DESIGN PURPOSES. THE PROCEDURE FOR 40-ACRE AND SMALLER BASINS. AS SET FOR THE IN THE REVISION OF SECTION 22.2. HYDROLOGY OF THE DEVELOPMENT PROCESS MANUAL, VOLUME 2, DESIGN CRITERIA, DATED JANUARY, 1993, HAS BEEN USED TO QUANTIFY THE PEAK RATE OF DISCHARGE AND VOLUME OF RUNOFF GENERATED BY THIS DEVELOPMENT. THE CAPACITY OF THE DEVELOPED 18" CMP STORM DRAINS WAS ANALYZED USING THE ORIFICE AND MANNING'S EQUATIONS. THE CAPACITIES OF THE NEW STORM INLETS WERE DETERMINED USING THE ORIFICE EQUATION. THE CAPACITY OF THE NEW 24" CULVERTS ALONG CONSTITUTION PLACE NE WERE DETERMINED USING MANNING'S EQUATION, THE PERFORMANCE OF THE PRIVATE STORMWATER DETENTION SYSTEM IN THE NORTH EAST CORNER OF THE PARKING AREA HAS BEEN EVALUATED FOR OUTFLOW AND STORAGE CAPACITY BASED UP THE HYDROGRAPH ANALYSIS CONTAINED ON THIS SHEET, AND PONDING IS NECESSARY ONLY DUE TO DISCHARGE PIPE SIZING CONSTRAINTS.

THE JANUARY 2005 CALCULATIONS ARE SHOWN HEREON TO SHOW THAT THE DOWNSTREAM STREET (CONSTITUTION AVENUE) IS CAPABLE OF HANDLING FLOWS FROM ONSITE AND OFFSITE. THE CONJUGATE DEPTH EQUATION WAS USED TO EVALUATE THE EFFECTS OF HYDRAULIC JUMP. AS SHOWN BY THESE CALCULATIONS, THE NORMAL DEPTH OF FLOW PLUS THE EFFECTS OF A HYDRAULIC JUMP COMPLY WITH DPM CRITERIA TO FALL BELOW THE TOP-OF-CURB PLUS 0.2 FEET. THIS SUGGESTS THAT THE STREET CAN HANDLE THE FULLY DEVELOPED DESIGN FLOW AS CALCULATED AND THAT PONDING IS NO LONGER NECESSARY.

VIII. CONCLUSION

THE DRAINAGE CONCEPT SHOWN BY THE PLAN ABOVE WILL ACCOMPLISH ITS PURPOSE DUE TO THE FOLLOWING FACTORS:

- A.) MODIFICATION TO AN EXISTING SITE WITHIN AN INFILL AREA B.) THE PEAK RATE OF DISCHARGE FROM THE SITE WILL BE INCREASED ABOVE THAT OF THE EXISTING CONDITION C.) REMOVAL OF THE EXISTING POND AS ALLOWED BY APPROVED
- DRAINAGE SUBMITTAL DATED 01-11-05 D.) ADEQUATE STREET CAPACITY IS AVAILABLE IN CONSTITUTION PLACE NE TO ACCOMMODATE PEAK DISCHARGE UNDER FULLY DEVELOPED CONDITIONS E.) DOWNSTREAM CAPACITY OF CONSTITUTION PLACE AND
- CONSTITUTION AVENUE AND THE STORM DRAIN STRUCTURES LOCATED WITHIN ARE ADEQUATE FOR INCREASED FLOWS DUE TO THE NMDOT 1-40/PENNSYLVANIA PROJECT IMPROVEMENTS F.) FREE DISCHARGE INTO CONSTITUTION PLACE AND
- CONSTITUTION AVENUE PER THE DRAINAGE PLAN DATED 1-11-05 G.) NO ADVERSE IMPACT ON DOWNSTREAM CAPACITY OR DOWNSTREAM PROPERTIES
- H.) CULVERTS AT CONSTITUTIONS PLACE SUFFICIENT TO HANDLE PEAK DISCHARGE FROM BOTH STORM DRAINS UNDER FULLY DEVELOPED CONDITIONS
- I.) GRADING AND CURB ABOUT STORM INLET AREA SUFFICIENT TO CONTAIN MORE THAN THE 220 CF OF PONDING REQUIRED DURING A 100-YEAR STORM
- J.) OFFSITE FLOWS WILL CONTINUE TO BE INTERCEPTED AND CONVEYED THROUGH THE PROJECT TO DOWNSTREAM EXISTING DRAINAGE FACILITIES

SITE CHARACTERISTICS

CALCULATIONS

1. PRECIPITATION ZONE = 3

2. $P_{6,100} = P_{300} = 2.60$ 2. $P_{6,10} = P_{300} = 1.73$

3. TOTAL AREA (AT) = 143,065 S

4. EXISTING LAND TREATMENT

A. PROJECT SITE AREA (SF/AC) TREATMENT 16,779 / 0.39 12 61,626 / 1.41 43 64,660 / 1.48 45

B. ONSITE CONTRIBUTING AREAS AREA (SF/AC) TREATMENT 70,000 / 1.61 100

5. DEVELOPED LAND TREATMENT

A. PROJECT SITE AREA (SF/AC) TREATMENT 23,552 / 0.54 16 119,533 / 2.74 84

6. EXISTING CONDITION

1. VOLUME

A. PROJECT SITE 1. VOLUME Ew = (EAAA+EBAB+EcAc+EbAb)/ATEw = ((0.92*0.39)+(1.29*1.41)+(2.36*1.48))/3.28 = 1.73 IN $V_{100} = (E_W/12)A_T = (1.73/12)3.28 = 0.2317 AC-FT 20,628 CF$

2. PEAK DISCHARGE QP = QPA AA + QPBAB + QPCAC + QPDAD $Q_P = Q_{100} = (2.60 \cdot 0.39) + (3.45 \cdot 1.41) + (5.02 \cdot 1.48) = 13.33 CFS$

B. ONSITE CONTRIBUTING AREAS (100-YEAR STORM)

Ew = (EAAA+EBAB+EcAc+EDAD)/ATEw = (2.36*1.61)/1.61 = 2.36 IN $V_{100} = (Ew/12)AT = (2.36/12)1.61 = 0.3160 AC-FT = 13,767 CF$

2. PEAK DISCHARGE QP = QPA AA + QPBAB + QPCAC + QPBAD

QP = Q100 = (5.02 + 1.61) = 8.07 CFS

C. ONSITE CONTRIBUTING AREAS (10-YEAR STORM) 1. VOLUME

EW = (EAAA + EBAB + ECAC + EBAB)/ATEw = (1.50*1.61)/1.61 = 1.50 in $V_{10} = (E_W/12)A_T = (1.50/12)1.61 = 0.2009 AC-FT = 8.750 CF$

2. PEAK DISCHARGE

QP = QPA AA + QPBAB + QPCAC + QPBAD $Q_P = Q_{100} = (3.39 * 1.61) = 5.45 CFS$

7. DEVELOPED CONDITION

A. PROJECT SITE (100-YEAR STORM)

 $Ew = (E_AA_A + E_BA_B + E_CA_C + E_BA_D)/AT$ Ew = ((0.92*0.54)+(2.36*2.74))/3.28 = 2.12 IN $V_{100} = (Ew/12)Ar = (2.12/12)3.28 = 0.5811 AC-FT = 25,314 CF$

2. PEAK DISCHARGE

QP = QPA AA + QPBAB + QPCAC + QPBAD $\Omega_P = \Omega_{100} = (2.60 \cdot 0.54) + (5.02 \cdot 2.74) = 15.18 CFS$

B. ONSITE CONTRIBUTING AREAS (10-YEAR STORM)

EW = (EAAA+EBAB+ECAC+EDAD)/ATEw = ((0.36*0.54)+(1.50*2.74))/3.28 = 1.31 H

 $V_{10} = (E_W/12)A_T = (1.31/12)3.28 = 0.3592 AC-FT = 15,648 CF$

2. PEAK DISCHARGE

QP = QPA AA + QPBAB + QPCAC + QPBAD

 $Q_P = Q_{10} = (1.19*0.54)+(3.39*2.74) = 9.95 CFS$

RAINSTORM PONDING CAPACITY

AREA (SQ FT) VOLUME (CF) TOTAL VOL. (CF) ELEV (FT) 5333 **5333.**5 [(2675+0)/2] *0.5 = 670[(6575+2675)/2] + 0.5 = 9751625 5334

VPROVIDED = 1.625 CF>VREQ = 240 CF (SEE HYDROGRAPH)

AL HYDROGRAPH CALCULATIONS

 $T_P = 0.7*Tc+(1.6-Ao/At)/12$ $T_P = 0.7*0.2+(1.6-2.74/3.28)/12$ TP = 0.2037 hr

DP = 0.25 + Ao/ATDP = 0.25 + 2.74 / 3.28

 $DP = 0.2088 \ hr$

 $T_B = 2.017 * E * AT/QP - 0.25 * AD/AT$ $T_B = 2.017 + 2.12 + 3.28 / 15.18 - 0.25 + 2.74 / 3.28$

Ta = 0.7563 hr

AREA OF HYDROGRAPH = Vieo = 26,410 CF

VOLUME REQUIRING PONDING $\forall \mathbf{megrb} = (15.18-14.90) + ((((25.16-11.98)+(24.75-12.22))/2) + 60)$ $V_{REDYD} = 215.6 CF$

PEAK DISCHARGE CAPACITY OF ON-SITE FLOW DISCHARGE PIPE $Q_{CAP} = 1.49/n AR^{2/3} S^{1/2}$

n = 0.013

 $A = pi + r^2 = pi + (0.75)^2 = 1.767 FT^2$ $P = 2 \cdot pi \cdot r = 2 \cdot pi \cdot 0.75 = 4.712 FT$

R = A/P = .375 FTS = 0.02 FT/FT

 $Q_{CAP} = 14.9 \text{ CFS}$ Q100 = 15.2 CFS>QCM =14.9 CFS, 215 CF OF PONDING REQ'D

CULVERT CALCULATIONS FOR SITE DRAINAGE

A. PEAK DISCHARGE CAPACITY OF SINGLE CULVERT

 $Q_{CAP} = 1.49/n AR^{2/3} S^{1/2}$ n = 0.013 $A = 0.667 \text{ FT} * 2 \text{ FT} = 1.33 \text{ FT}^2$ P = 0.667 FT + 0.667 FT + 2 FT = 3.33 FT

R = A/P = 0.4 FTS = 0.02 FT/FT

 $Q_{CAP} = 11.73 \text{ CFS}$

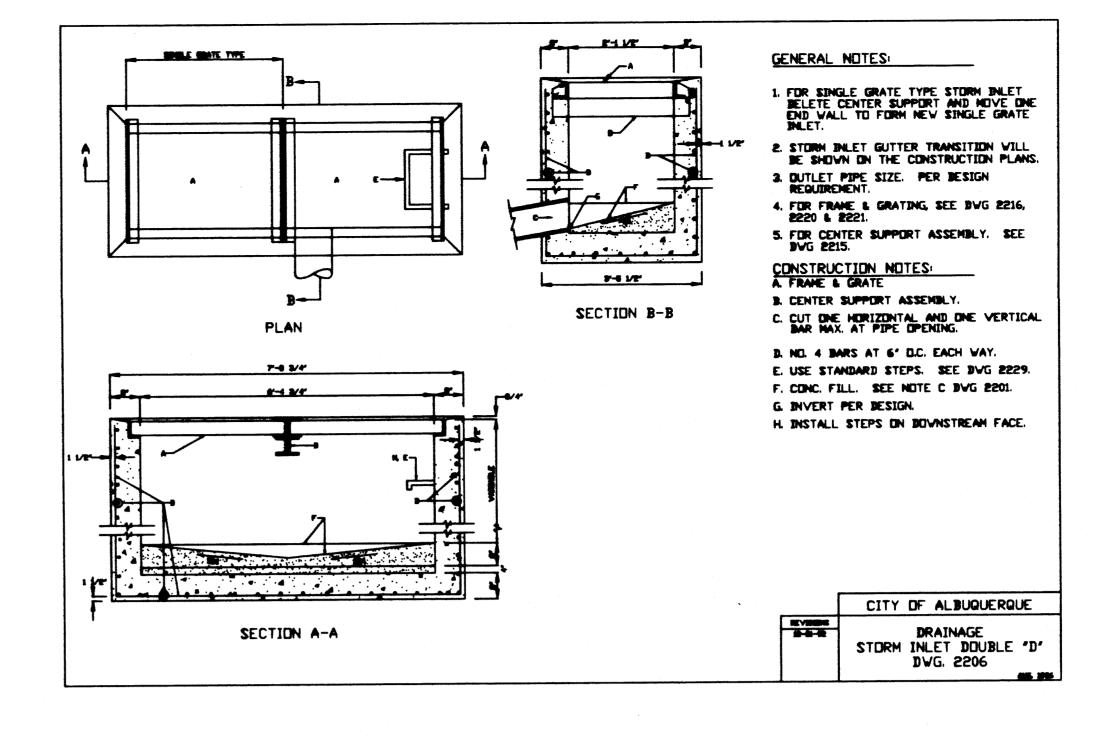
B. CULVERT DESIGN TO HOLD 10-YEAR, 6-HOUR STORM Q10,0N-SITE = 9.95 CFS Q10,0007708 = 5.45 CFS $Q_{10,REQ7D} = 9.95 + 5.45 = 15.4 CFS$ Q10,0000 = 15.4 CFS>QCAP = 11.73 CFS

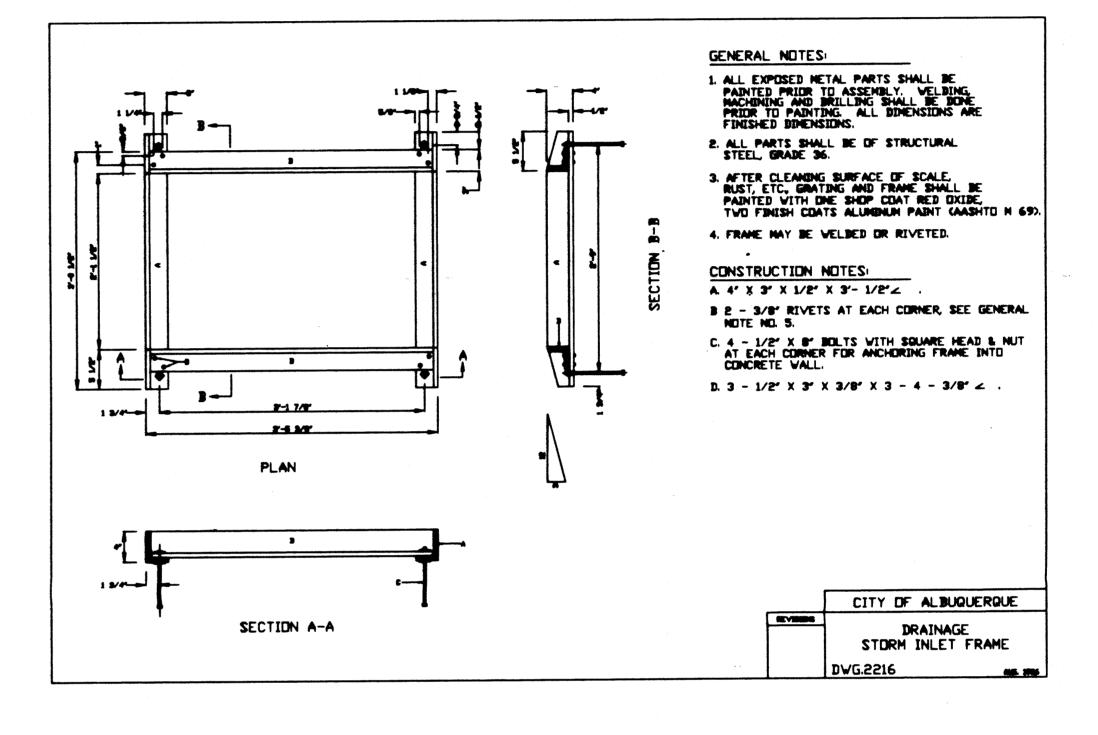
Q10,REDTD = 15.4 CFS<2+Qcap = 23.46 CFS

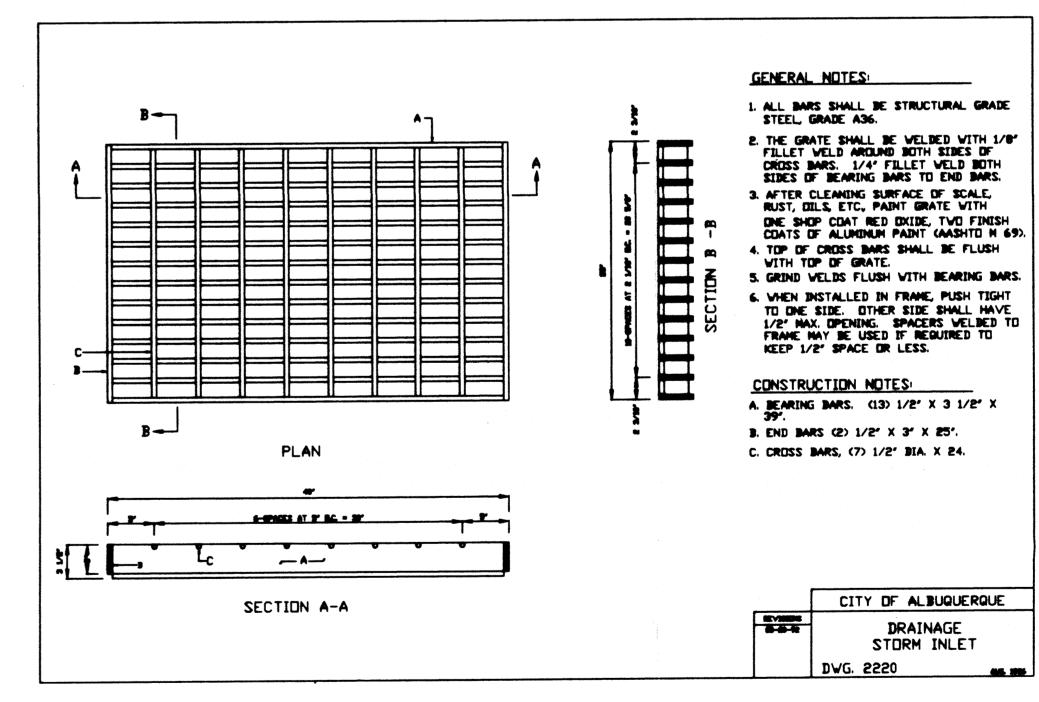
DESIGN REQUIRES TWO CULVERTS 8. COMPARISON

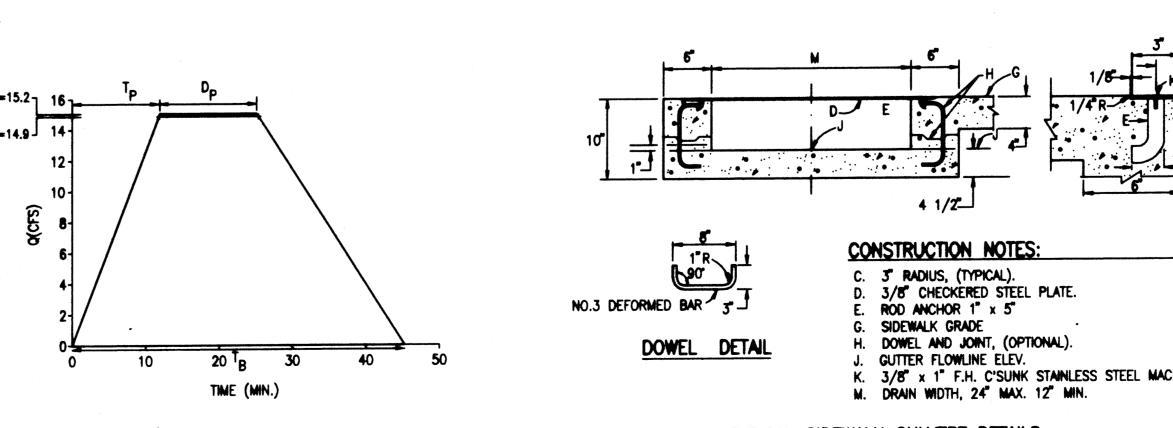
 $\triangle V_{100} = 25.315-20.628 = 4.687 \text{ CF (INCREASE)}$

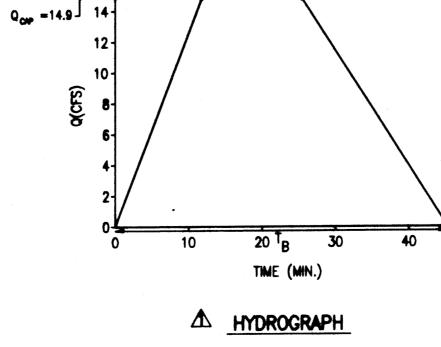
B. PEAK DISCHARGE $\triangle_{Q100} = 15.18-13.33 = 1.85 \text{ CFS (INCREASE)}$

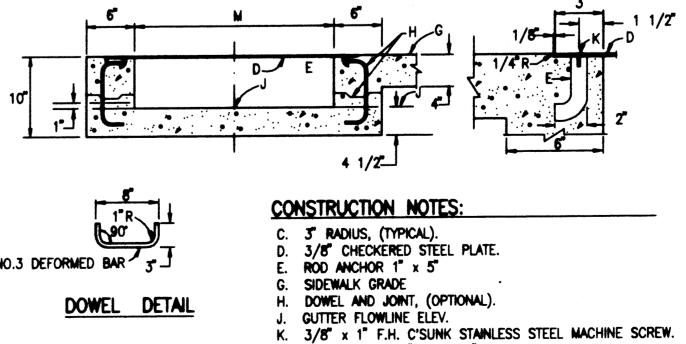












TYPICAL SIDEWALK CULVERT DETAILS NOT TO SCALE

CALCULATIONS, DRAINAGE PLAN AND SECTIONS AND DETAILS Sheet 5 Sheet Title

Preun JMA Checked JGM

Proj. 200437 Date 09/30

Revisions

@ 2005 KEVIN GEORGES & ASSOCIATES, PA

Final Construction Documents

Parking Area A PMG Constitution

8300 Constitution Avenue NE

Albuquerque, New Mexico

Kevin Georges & Associates

214 Trumen Street NE - Albuquerque, New Mexico 87166-1333 565/255-4875

Architecture & Planning

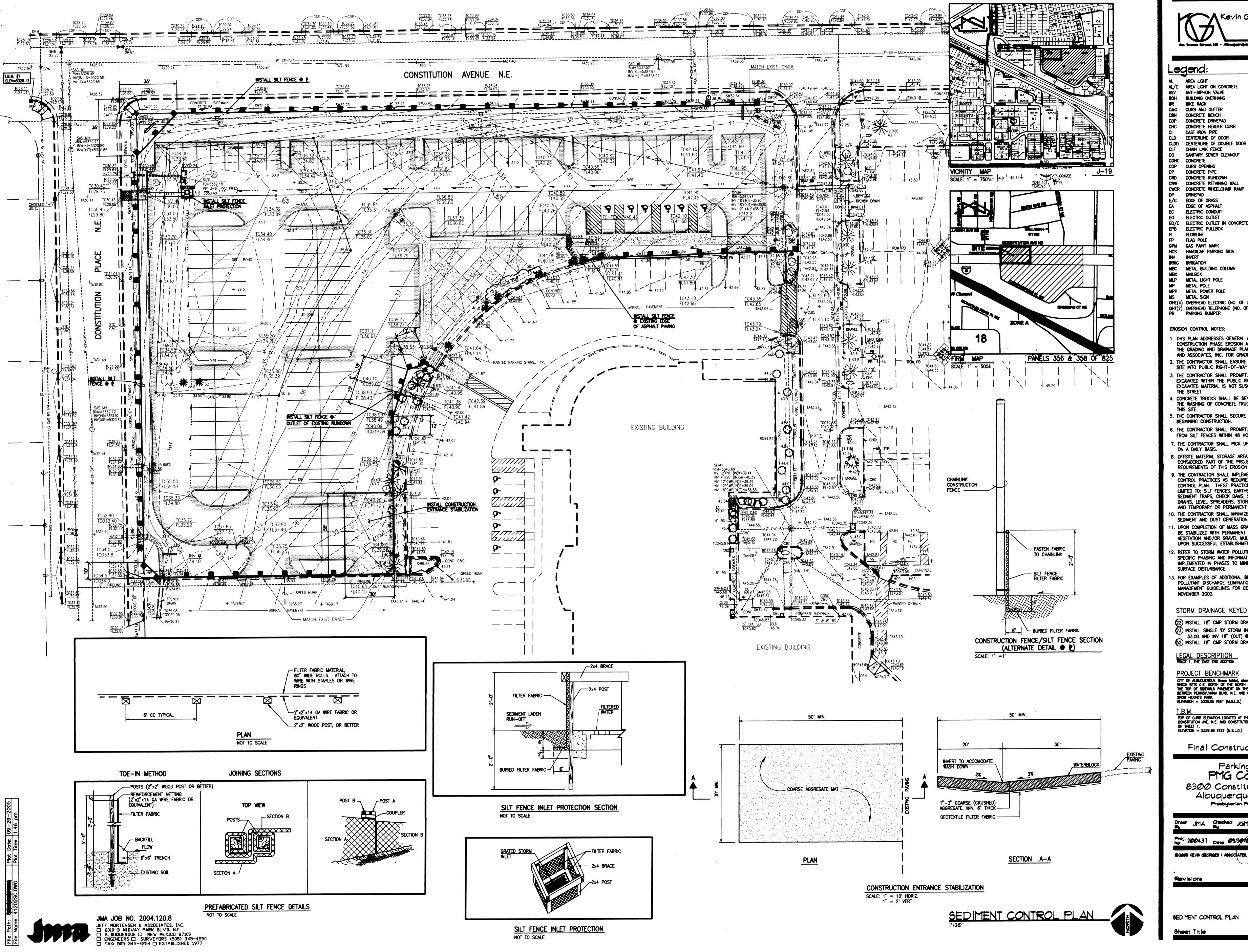
JMA JOB NO. 2004.120.3 JEFF MORTENSEN & ASSOCIATES, INC.

□ 6010-B MIDWAY PARK BLVD. N.E.

□ ALBUQUERQUE □ NEW MEXICO 87109

□ ENGINEERS □ SURVEYORS (505) 345-4250

□ FAX: 505 345-4254 □ ESTABLISHED 1977





AL/C AREA LIGHT ON CONCRETE ASV ANTI-SIPHON VALVE **BUILDING OVERHANG** CBN CONCRETE BENCH CONCRETE DRIVEPAD CONCRETE HEADER CURB

SPRINKLER CONTROL TIMER STORM INLET IN CONCRETE STANDARD CURB AND GUTTER TOP OF SIDEWALK SIDEWALK CULVERT

PLASTIC BENCH

TOP OF ASPHALT TOP OF CURB TOP OF CONCRETE TOP OF GRATE TELEPHONE MANHOLI TELEPHONE PULLBOX TELEPHONE RISER/PULLBOX TRAFFIC SIGN TELEPHONE VAULT

UNDERGROUND VALLEY GUTTER WOOD POLE WATER VALVE BOX CROSSWALK EXISTING CONTOUR EXISTING SPOT ELEVATION

EXISTING BOULDER EXISTING SHRUB EXISTING DECIDUOUS TREE (2.20) (CALIPER SIZE) (2.2°D) (CALIPER SIZE)

OHE(4) OVERHEAD ELECTRIC (NO. OF LINES) OHT(2) OVERHEAD TELEPHONE (NO. OF LINES)
PB PARKING BUMPER

EROSION CONTROL NOTES:

EXCAVATED WITHIN THE PUBLIC RIGHT-OF-WAY SO THAT THE EXCAVATED MATERIAL IS NOT SUSCEPTIBLE TO BEING WASHED DOWN

THE WASHING OF CONCRETE TRUCKS SHALL NOT BE PERMITTED ON

5. THE CONTRACTOR SHALL SECURE ALL REQUIRED PERMITS PRIOR TO BEGINNING CONSTRUCTION.

6. THE CONTRACTOR SHALL PROMPTLY REMOVE SEDIMENT ACCUMULATION FROM SILT FENCES WITHIN 48 HOURS OF A RAINFALL EVENT. 7. THE CONTRACTOR SHALL PICK UP LITTER AND CONSTRUCTION DEBRIS ON A DAILY BASIS.

8. OFFSITE MATERIAL STORAGE AREAS USED BY THIS PROJECT ARE CONSIDERED PART OF THE PROJECT AND ARE SUBJECT TO THE

REQUIREMENTS OF THIS EROSION CONTROL PLAN. 9. THE CONTRACTOR SHALL IMPLEMENT ON-SITE STRUCTURAL EROSION CONTROL PRACTICES AS REQUIRED TO COMPLY WITH THE EROSION CONTROL PLAN. THESE PRACTICES MAY INCLUDE BUT ARE NOT LIMITED TO: SILT FENCES, EARTHEN DIKES, DRAINAGE SWALES, SEDIMENT TRAPS, CHECK DAMS, SUBSURFACE DRAINS, PIPE SLOPE

DRAINS, LEVEL SPREADERS, STORM RETAINING SYSTEM, GABIONS AND TEMPORARY OR PERMANENT SEDIMENT BASINS. 10. THE CONTRACTOR SHALL MINIMIZE OFFSITE VEHICLE TRACKING OF

SEDIMENT AND DUST GENERATION. 11. UPON COMPLETION OF MASS GRADING, ALL DISTURBED AREAS SHALL BE STABILIZED WITH PERMANENT CONSTRUCTION, LANDSCAPING, VEGETATION AND/OR GRAVEL MULCH SILT FENCING CAN BE REMOVED UPON SUCCESSFUL ESTABLISHMENT OF VEGETATION.

12. REFER TO STORM WATER POLLUTION PREVENTION PLAN FOR PROJECT SPECIFIC PHASING AND INFORMATION. THIS PROJECT SHALL BE

IMPLEMENTED IN PHASES TO MINIMIZE THE EXTENT AND DURATION OF SURFACE DISTURBANCE. 13. FOR EXAMPLES OF ADDITIONAL BMP'S, REFER TO THE NATIONAL

POLLUTANT DISCHARGE ELIMINATION SYSTEM MANUAL - STORM WATER MANAGEMENT GUIDELINES FOR CONSTRUCTION AND INDUSTRIAL ACTIVITIES,

STORM DRAINAGE KEYED NOTES:

(22) Install 18" CMP Storm Drain • S = 0.02 (3) INSTALL SINGLE 'D' STORM INLET PER DETAIL, SHEET 4 W/ T.O.G 33.00 AND INV 18" (OUT) • 30.47 (52) INSTALL 18" CMP STORM DRAIN • S = 0.015

PROJECT BENCHMARK

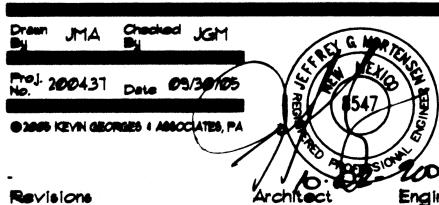
CITY OF ALBUQUERQUE Bross tablet, stemped "1-J19", SET IN TOP OF A CONCRETE POST WHICH SETS 0.8" NORTH OF THE NORTH EDGE OF A CONCRETE SIDEWALK, AND FLUSH WITH THE TOP OF SIDEWALK PAVEMENT ON THE NORTH SIDE OF CONSTITUTION AVE. N.E. AND BETWEEN PENNSYLVANIA BLVD. N.E. AND CONSTITUTION PL. N.E. ON TH SOUTH SIDE OF SNOW HEIGHTS PARK.

ELEVATION = 5320.05 FEET (M.S.L.D.)

TOP OF CURB ELEVATION LOCATED AT THE S.S.E. RETURN OF THE INTERSECTION OF CONSTITUTION AVE. N.E. AND CONSTITUTION PLACE N.E. AS SHOWN ON THE DRAWING ON SHEET 1.
ELEVATION = 5329.86 FEET (M.S.L.D.)

Final Construction Documents

Parking Area A
PMG Constitution 8300 Constitution Avenue NE Albuquerque, New Mexico Presbyterian Project No. MR6562



SEDIMENT CONTROL PLAN