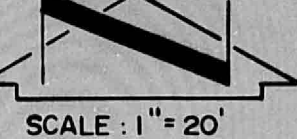


VICINITY MAP
SCALE: 1" = 800'

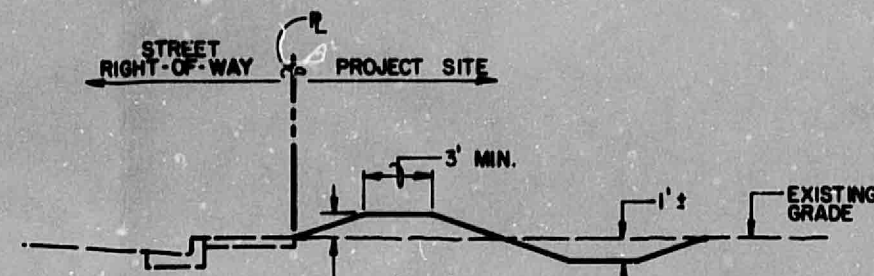
PROJECT BENCHMARK
A SQUARE, CHISELED ON TOP OF CONCRETE CURB AT THE NOSE OF THE MEDIAN WEST SIDE OF INTERSECTION BENCHMARK IS JUST LOCATED AT THE INTERSECTION OF PENNSYLVANIA ST. N.E. & MOUNTAIN BLVD. N.E. ON THE MEDIAN ON CENTER LINE OF MOUNTAIN AND EAST SIDE OF INTERSECTION. ELEVATION = 5333.056 FEET (MOLD)

T.B.M.
TEMPORARY BENCHMARK LOCATED AT THE BASE OF A LIGHT POLE FOUND AT THE SOUTHWEST CORNER OF THE PROPERTY AS SHOWN BELOW ON THE DRAWING. ELEVATION = 5326.21 FEET (MOLD)



- LEGEND**
- ◆ PROPOSED SPOT ELEVATION
 - ◆ EXISTING SPOT ELEVATION
 - 5G — PROPOSED CONTOUR
 - 5550 — EXISTING CONTOUR
 - — — SWALE
 - — — PROPERTY LINE
 - — — CONCRETE
 - — — PROPOSED ASPHALT
 - — — PROPOSED FENCE
 - — — EXISTING FENCE
 - — — TOP OF CURB
 - — — FLOW LINE

DESCRIPTION
LOT 23, ASHCRAFT CENTER



TEMPORARY EROSION CONTROL BERM SECTION
SCALE: 1" = 8'

DRAINAGE PLAN

The following items concerning the Ashcraft Office Development Drainage Plan are contained hereon:

1. Vicinity Map
2. Grading Plan
3. Calculations

The proposed improvements, as shown by the Vicinity Map, are located on the east side of Pennsylvania Avenue between Mountain Road and Pennsylvania Circle N.E. At present the site is undeveloped. The adjoining parcels to the east are developed.

As shown on Plate J-19 of the Albuquerque Master Drainage Study, this site does not lie within a designated Flood Hazard Zone. Further study of this plate reveals that one block to the west a major storm sewer exists. The site and the storm sewer are approximately two to three blocks from the North Diversion Channel in the median of Interstate No. 40. The project site is bordered on the northwest and the south by streets. The site is higher than all three streets. The land to the east of the project site is higher than the project site, but those parcels have been developed and ponds exist on each parcel, therefore, drainage from the east is negligible. Offsite drainage to this site is also negligible.

The Grading Plan shows 1) existing and proposed plans indicated by spot elevations and contours at 1'-0" intervals, 2) continuity between existing and proposed elevations, 3) limit and character of existing improvements, and 4) the limit and character of proposed improvements. As shown by this plan, the proposed improvements consist of the construction of two buildings, landscaping, and an asphalt parking lot. At present, the site drains from east to west. The proposed grading scheme will alter the existing contours slightly. It is proposed that the site will drain from the southeast to the northwest. Stormwater runoff from the parking area will be discharged through a driveway into Mountain Road. Stormwater from the landscaped areas and the buildings will be discharged through an 8-inch PVC drain into the back of an inlet in Mountain Road.

The Calculations which appear below analyze both the existing and developed conditions for the 100-year, 6-hour rainfall event. The Rational Method has been used for this analysis in accordance with the City of Albuquerque Development Process Manual, Volume II. As shown by these calculations, the proposed improvements will result in an increase of runoff discharged from the site. Due to the proximity of the site to major drainage facilities, storm waters are not detained. This pattern of runoff is consistent with the pre-design recap which accompanies this submittal.

CALCULATIONS

Ground Cover Information

From Bernalillo County Soil Survey, Plate 21:
Embudo, EMB, and Tijeras, TgB
Hydrologic Soil Group B

Rational Method

Discharge: $Q = C_i A$
where C varies
 $i = P_6 (6.84) = -0.51 = 5.48 \text{ in./hr}$
 $P_6 = 2.4 \text{ in (DPM Plate 22.2 D-1)}$
 $T_c = 10 \text{ min (minimum)}$
 $A = \text{area, acres}$

Volume: $V = C_p A (1/12)$
where C varies
 $P_6 = 2.4 \text{ in (DPM Plate 22.2 D-1)}$
 $A = \text{area, acres}$

Existing Condition

$A_{\text{total}} = 86,468 \text{ sf} = 1.99 \text{ Ac}$
 $A_{\text{imp}} = 0 \text{ sf; } \& \text{ impervious} = 0\%$
 $C_p = 0.34 \text{ (DPM Plate 22.2 C-1)}$
 $Q_{100} = C_i A = 0.34(5.48)(1.99) = 3.7 \text{ cfs}$
 $V_{100} = C_p A = 0.34(2.4 / 12)(86,468) = 5,880 \text{ cf}$

Developed Condition

$A_{\text{total}} = 86,468 \text{ sf} = 1.99 \text{ Ac}$
 $A_{\text{imp}} = 68,800 \text{ sf; } \& \text{ impervious} = 79\%$
 $C_p = 0.76 \text{ (DPM Plate 22.2 C-1)}$
 $Q_{100} = C_i A = 0.76(5.48)(1.99) = 8.3 \text{ cfs}$
 $V_{100} = C_p A = 0.76(2.4 / 12)(86,468) = 13,143 \text{ cf}$

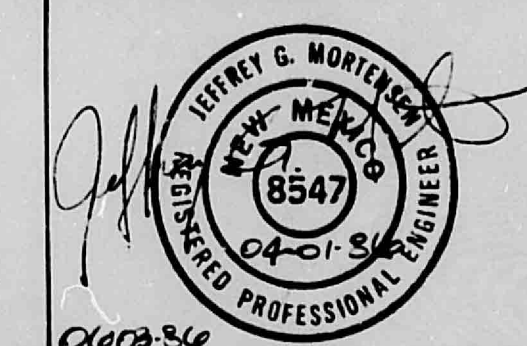
Comparison

$\Delta Q_{100} = 8.3 - 3.7 = 4.6 \text{ cfs (increase)}$
 $\Delta V_{100} = 13,143 - 5,880 = 7,263 \text{ cf (increase)}$

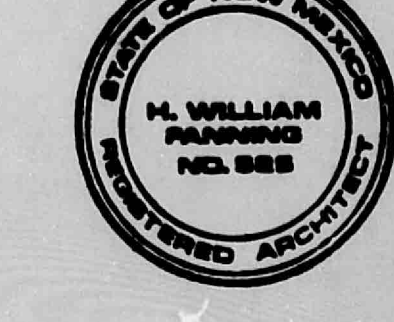
EROSION CONTROL MEASURES

1. THE CONTRACTOR SHALL ENSURE THAT NO SOIL ERODES FROM THE SITE INTO PUBLIC RIGHT-OF-WAY OR ONTO PRIVATE PROPERTY. THIS CAN BE ACHIEVED BY CONSTRUCTING TEMPORARY BERMS AT THE PROPERTY LINES AND WETTING THE SOIL TO KEEP IT FROM BLOWING.
2. THE CONTRACTOR SHALL PROMPTLY CLEAN UP ANY MATERIAL EXCAVATED WITHIN THE PUBLIC RIGHT-OF-WAY SO THAT THE EXCAVATED MATERIAL IS NOT SUSCEPTIBLE TO BEING WASHED DOWN THE STREET.
3. THE CONTRACTOR SHALL SECURE "TOPSOIL DISTURBANCE PERMIT" PRIOR TO BEGINNING CONSTRUCTION.

REVISIONS PREPARED UNDER THE SUPERVISION OF:

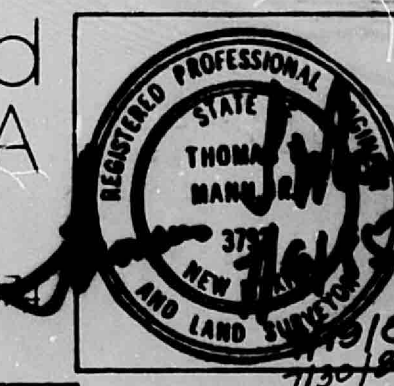


RECEIVED
JUN 09 1986
HYDROLOGICAL SECTION
STATE OF NEW MEXICO



Fanning / Bard
Architects AIA

110 Amherst Drive SE
Albuquerque, New Mexico
87106 (505) 266-5841



Ashcraft
Office Development
1120 Pennsylvania St. NE

August, 1985
DRAINAGE AND DRAINAGE PLAN

3 OF 32



111 DALLAS N.E. - ALBUQUERQUE - NEW MEXICO - 87110
ENGINEERS

NO.	DATE	BY	REVISIONS
1	8/85	JCM	ADD ROOF DRAIN, STEPS & BERM
2	3/86	JCM	REVISE COURTYARD & DRAINAGE (REWORK), REMOVE STEPS
3	5/86	JCM	REVISE DRAINAGE, ADD PARKING LOT, DELETE DRAINAGE

DESIGNED BY: T.T.M.
DRAWN BY: S.G.H.
APPROVED: T.T.M.

JCB NO.
50081
DATE
6-85

J19/DB2