

# City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

Ken Schultz Mayor

UTILITY DEVELOPMENT DIVISION HYDROLOGY SECTION (505) 768-2650

June 15, 1987

George E. Paul, P.E. D.T. Morrison 1020 Texas, NE Albuquerque, New Mexico 87110

RE: REVISED DRAINAGE PLAN FOR BOB MYER'S USED CAR LOT (J-19/D42) REVISION DATE JUNE 8, 1987

#### Dear George:

Based on the information provided on your resubmittal of June 12, 1987, the above referenced plan is approved for Building Permit. Please be advised that a separate permit is required for construction within City right-of-way.

Please attach a copy of this plan to the construction sets prior to sign-off by Hydrology.

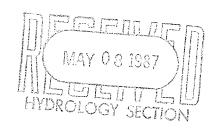
If I can be of further assistance, please feel free to call me at 768-2650.

Cordially,

Bernie J. Montoya, C.E. Engineering Assistant

cc: Becky Sandoval

*BJM/bsj* 



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For Improvements On

LOTS 13 - 19 and TRACT "Y" BLOCK 21, EAST END ADDITION

# DRAINAGE INFORMATION SHEET

GRADING AND PROJECT TITLE: DRAINAGE PLAN	ZONE OTLOC/DONG STUF # 7-19-4 DM
LEGAL DESCRIPTION: Lors 13-19 Find Tr	der Thorne a, Last Ella radition
ENGINEERING FIRM: G.E. PAUL - P.E & L	.,5. CONTACT: Same
ADDRESS: 12717 Viewcrest Pl. NE (	87//Z) PHONE: 299-0295
	CONTACT: Bob Myers or Joe Blythe
ADDRESS: 57/2 Menaul NE (8	7/10 PHONE: <u>884-9785</u>
ARCHITECT:	CONTACT:
ADDRESS:	PHONE:
SURVEYOR: D.T. Morrison Surveyor	. D.T. Morrison or
ADDRESS: 1020 Texas NE (8	7/10) PHONE: 256-7364
CONTRACTOR: Willie Espinosa	CONTACT: 5ame
ADDRESS:	PHONE: 897-2668
PRE-DESIGN MEETING:  YES  NO  HYDROLOGY SECTION  PROJ. NO.  N. A.  SHEET PROVIDED	
	CHECK TYPE OF APPROVAL SOUGHT:
DRAINAGE REPORT  DRAINAGE PLAN	SKETCH PLAT APPROVAL  PRELIMINARY PLAT APPROVAL
CONCEPTUAL GRADING & DRAINAGE PLAN	SITE DEVELOPMENT PLAN APPROVAL
GRADING PLAN	FINAL PLAT APPROVAL

#### DRAINAGE INVESTIGATION



#### Introduction:

The following calculation are intended to show the amount of runoff and the method of handling the stormwater from a 10-year frequency storm in the vicinity of Lots 13-19 and Tract "Y", Block 21 of the East End Addition, located north of Lomas Blvd., between Tennessee and Rhode Island Streets, Northeast.

As this property will be for an automobile sales lot, the area is assumed to be fully paved, with a hot plant-mixed bituminous surface course and accompanied by an office and a shop building, positioned as shown on the Grading and Drainage Plan.

The method of computation is by the "Rational" Formula (Q = A c i), where:

Q = runoff from a 10-year frequency storm in cfs.

A = total drainage area, in acres.

c = coefficient of runoff, or ratio of runoff/total rainfall.

i = Intensity of rainfall (inches/hr.) based on time of concentration, or time for rain falling at most remote point, to reach discharge point (sidewalk opening with plate cover, NW corner of property).

#### Calculations

Q = A c i

A = 1.67 Ac.

c = 0.95 for bituminous pavement with small (4904 s.f.) roofed area.

i = combined flow times of overland (remotest point to drainage channel or "ditch") flow and channel flow, using Seelye Engineering Design Handbook, pages 5-00 and 5-01.

Greatest overland flow distance = 340 ft., southeast to northwest.

Ditch flow distance = 30 ft. along westerly limits of Lot 19.

From Seelye Design Handbook, Nomograph on page 5-00, overland flow time for 340' paved strip, average slope of 1.529% = 6.5 min.

Ditch flow time with a paved swale, 10' bottom, 20' top width and a depth of 0.2', with a slope of 0.0095 and a length of 30 ft., computed by Manning Formula:

Q = a  $\frac{1.486}{n}$  R 2/3 S 1/2, where V =  $\frac{1.486}{n}$  R 2/3 S 1/2

Then, V = velocitv, in ft./sec.

Ditch flow time for 30' ditch length =  $\frac{30}{1.95}$  = 15.4 sec. or  $\frac{0.26 \text{ min} \cdot 1.95 \text{ m$ 

Total Flow Time, or Inlet Concentration Time = 6.5 + 0.26 = 6.76 min.

Then, using Seelye Handbook, pg. 5-00, reference 10-year rainfall from Yarnell curve, figure B; = 1.50 in. for central NM area.

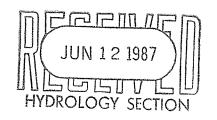
From Intensity - Duration curves (Seelye, pg. 5-00, Figure J.), a duration (concentration time) of 6.76 min. and a rainfall of 1.50 in/hr., gives an "i" value of approximately  $5.0\,$  in./hr.

 $Q = A c i = 1.67 \times 0.95 \times 5 = 7.93 cfs.$ 

#### Recommendation:

(Use double, 24" max. span/section sidewalk culvert with steel plate top, as per City Std. Detail Drawing K-16, located as shown on Grading and Drainage Plan.)

Deorge E. Paul NM DE : L5 # 2544



## SUPPLEMENTAL REPORT

## DRAINAGE INVESTIGATION

For Improvements On

LOTS 13-19 and TRACT "Y"

BLOCK 21, EAST END ADDITION

JUNE 1, 1987

#### Purpose:

The purpose of this report is to supplement the original drainage investigation furnished to the City Public Works Department Engineering Group in mid-May of this year, by addressing certain other items required by the Hydrology Section in its letter of May 18, 1987, addressed to George E. Paul.

#### Actions:

- 1. <u>Vicinity Map</u>: Refer to two attached copies of the Grading/Drainage (Site) Plan, amended to provide this requirement.
- 2. Benchmarks: Refer to two attached copies of site plan, also amended to show T.B.M. on site and Control Survey Vertical Datum.
- 3. Approved filed copy of plat showing referenced lots: Submitted herewith are copies of the recorded plats involving these lots.
- 4. <u>Cross-Lot Drainage</u>: Attached are conveyance statements from the owners of Lots 13-19, Block 21, East End Addition, granting lot use for common access and drainage.
- 5. "Notes" Part from Sign off, No. 19 Format: The notes are now incorporated into the Site Plan as requested. (Refer to two amended copies attached).
- 6. On-Site developed and undeveloped flow volumes: Site is fully paved (bituminous surface course), hence following calculations are for developed flow only:

"C" value = 0.95 (paved area)

Reference curves, plate 22.2 C-4; use "95" value curve

Rainfall (P) inches = 2.4

From the curves, with P = 2.4 & curve no. 95,

Direct Runoff (Q) in inches = 1.9; Area in Acres = 1.67

Volume =  $\frac{1.9 \times 1.67 \times 43,560}{12}$  = 11,518 cu.ft. or 0.264 ac. ft.

7. Minimum Tc is 10 minutes: This parameter has been used in the 100year storm calculations shown below.

#### Actions (cont'd.)

9. Calculations for capacity of proposed 24" sidewalk culvert:

$$Q_T = 2/3 \times L \times \sqrt{2g} \times h_2^{1.5}$$
; where:  $L = 2.00$  ft.  
 $h_1 = 0$ ;  $h_2 = 7.25'' = 0.604'$ . Then,  
 $Q = 2/3 \times 2.00 \sqrt{.64.4} \times .604^{1.5} = 2/3 \times 2.00 \times 8.025 \times .4694$   
 $Q = \underline{5.023}$  cfs capy. ( $\checkmark$  8.048,\*) hence  
Use Two - 24'' sidewalk culverts

- 10. <u>Identify/quantify off site flows, if any</u>: None exist. Refer to Vicinity Map. Abutting streets (Lomas Blvd., Tennesse St., Rhode Island St.,) are fully paved and curbed. Flows are restricted to paved street sections.
- 11. Direction and location of roof drains: None exist. Sheet flow only, off roofs onto pavement. (used 0.95 coeff. for roofs, instead of min. allowable of 0.90)
- 12. <u>100-year frequency calculations</u>: Using the "Rational" Method, (Q=Aci) where:

A = area in acres = 1.67

c = coeff. of runoff for paved surface = 0.95

i = 5.073 in. hr. based on equation, I = 6 hr. rain x 6.84 x  $T_c$  - 51 where 6 hr. rain (Pennsylvania Lomas) = 2.4

 $T_C$  = minimum per DPM = 10 minThen, I = 2.4 x 6.84 x  $10^{-.51}$  = 2.4 x 6.84 x .30903 = 5.073

\* Q =  $1.67 \times 0.95 \times 5.073 = 8.048 \text{ cfs}$  (100-year runoff)

- 13. Encroachment Agreement: Asphalt is not proposed within city right-of-way, hence agreement is not applicable (see Grading/Drainage Plan).
- 14. <u>Cross-section of proposed sidewalk culvert</u>: Refer again to City Standard Drawing No. 2236, with Sec. "A-A" looking east. Spot elevations at flow line of culture (rest and of culture) and cost adds of cidewalk (cost and of culture)

## Conclusion:

The items requested by Mr. Montoya of the Hydrology Section have now been provided; either by this report or on the amended two copies of the site plan and approval to proceed with erection of buildings as shown, is respectfully requested.

George E. Paul NMPE & LS #2544

