

January 11, 2011

Jeffery G. Mortensen, P.E. High Mesa Consulting Group 6010-B Midway Park Blvd. NE Albuquerque, NM 87109

Re: Chelwood Elementary School Handicap Parking Spaces

**Grading Plan** 

Engineer's Stamp dated 1-07-11 (J22/D005)

Dear Mr. Mortensen,

PO Box 1293

NM 87103

www.cabq.gov

Based upon the information provided in your submittal dated 1-07-11, the above referenced plan is approved for Grading Permit and Paving Permit. Please attach a copy of this approved plan to the construction sets prior to sign-off by Hydrology.

Hydrology is requesting that proposed landscape area be depressed to retain/infiltrate the rain that falls in this area.

Upon completion of the project, please provide an Engineer Certification for our \_\_\_\_\_ files.

If you have any questions, you can contact me at 924-3695.

Sincerely,

Curtis A. Cherne, P.E.

Senior Engineer, Planning Dept.

Development and Building Services

C: file

CITY OF ALBUQUE	RQUE
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July 12, 2011	
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Jeff Mortensen, P.E.  High Mesa Consulting Group	
6010-B Midway Park Blvd. NE. Albuquerque, NM 87109	
Albuquelque, Ivivi 6/109	
Re: Chelwood Elementary School	Handicap Parking Spaces
Engineer's Stamp dated: 3-13	A Property of the second of th
Certification dated: 7-5-2011	
	A STATE OF THE STA
Dear Mr. Mortensen,	
	Hydrology department with the Engineers
PO Box 1293 Certification of Grading dated 7/5/11	
Albuquerque	Sincoroly:
	Sincerely,
NM 87103	
	Rudy E. Rael, CE Engineer Assistant, Hydrology Section
www.cabq.gov	Development and Building Services
C:File	
	Albuquerque - Making History 1706-2006
	Thought que - Indicing 1115101 y 1700-2000

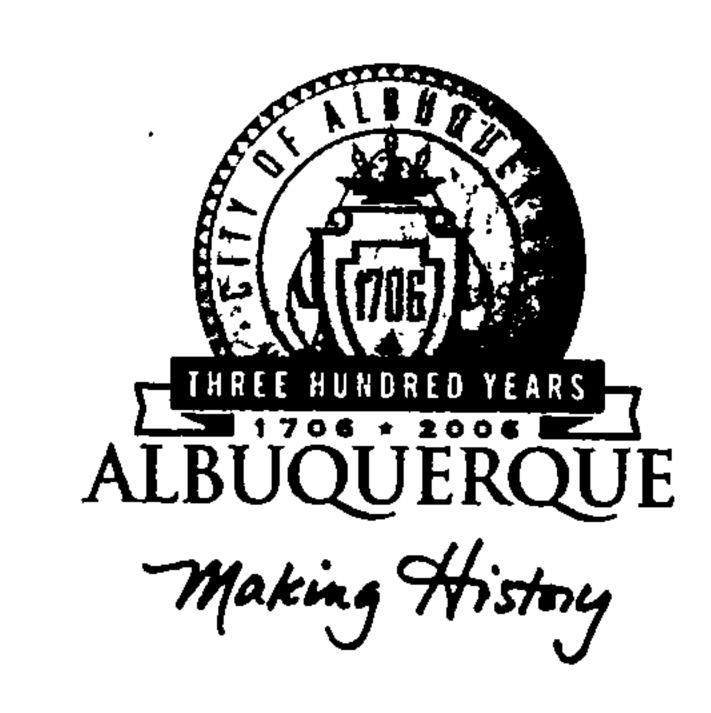
### DRAINAGE AND TRANSPORTATION INFORMATION SHEET

(REV. 1/28/2003rd)

PROJECT TITLE: DRB #:	CHELWOOD HANDICAP PARKIN EPC #:		ONE ATLAS/DRNG. FILE #: J-22/D005 VORK ORDER #:		
LEGAL DESCRIPTION:	TRACT A, CHELWOOD ELEME	NTARY SCHOOL			
CITY ADDRESS:	12701 CONSTITUTION NE	ITTAITI GOLIOGE	<u></u>		
ENGINEERING FIRM: ADDRESS: CITY, STATE:	HIGH MESA CONSULTING GRO 6010-B MIDWAY PARK BLVD. N ALBUQUERQUE, NM		JEFF MORTENSEN, NMPE 8547 (505) 345-4250 87109		
OWNER:  ADDRESS: CITY, STATE:	ALBUQUERQUE PUBLIC SCHO 915 OAK STREET SE ALBUQUERQUE, NM	CONTACT: PHONE: ZIP CODE:	TYLER MASON (505) 848-8822 87106		
ARCHITECT: ADDRESS: CITY, STATE:	N/A	CONTACT: PHONE: ZIP CODE:			
SURVEYOR: ADDRESS: CITY, STATE:	HIGH MESA CONSULTING GR 6010-B MIDWAY PARK BLVD. I ALBUQUERQUE, NM		CHARLES CALA JR, NMPS 11184 (505) 345-4250 87109		
CONTRACTOR: ADDRESS: CITY, STATE:	ALTOR CONSTRUCTION 5215 EDITH BOULEVARD NE ALBUQUERQUE, NM	CONTACT: PHONE: ZIP CODE:	MARIE ALVIDREZ (505) 341-1551 87107-4121		
DRAINAGE PLAN CONCEPTUAL GR GRADING PLAN EROSION CONTR X ENGINEER'S CER CLOMR/LOMR TRAFFIC CIRCUL ENGINEER'S CER	1 <sup>st</sup> SUBMITTAL, <i>REQUIRES TCL of</i> RESUBMITTAL RADING & DRAINAGE PLAN	SIA/FINANC PRELIMINA S. DEV. PLA S. DEV. PLA SECTOR PI FINAL PLAT FOUNDATION BUILDING FOUNDATION CERTIFICA CERTIFICA GRADING FOUNDATION PAVING PE WORK ORE	APPROVAL SOUGHT: CIAL GUARANTEE RELEASE RY PLAT APPROVAL AN FOR SUB'D APPROVAL AN FOR BLDG. PERMIT APPROVAL LAN APPROVAL ON PERMIT APPROVAL PERMIT APPROVAL TE OF OCCUPANCY (PERM.) TE OF OCCUPANCY (TEMP.) PERMIT APPROVAL CRAMIT APPROVAL CRAMIT APPROVAL CONDITION OF APPROVAL DECIFY): CONDITION OF APPROVAL		
WAS A PRE-DESIGN C YES X NO	ONFERENCE ATTENDED:		JUL 6 2011		
COPY PROVIDED			HYDROLOGY SECTION		
DATE SUBMITTED:	07-06-2011 BY:	BRIAN E. EVEMEYER			

Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a drainage submittal. The particular nature, location and scope of the proposed development defines the degree of drainage detail. One or more of the following levels of submittal may be required based upon the following:

- 1. Conceptual Grading and Drainage Plan: Required for approval of Site Development Plans greater than five (5) acres and Sector Plans.
- 2. Drainage Plans: Required for building permits, grading permits, paving permits and site plans less than five (5) acres.
- 3. Drainage Report: Required for subdivisions containing more than ten (10) lots or constituting five (5) acres or more.



October 20, 2004

John Tellez, P.E. Wilson & Company 2600 The American Rd. SE, Suite 100 Rio Rancho, NM 87124

Re: Chelwood Elementary School Drop Off Lane, 12701 Constitution Ave NE, Grading and Drainage Plan

Engineer's Stamp dated 10-18-04 (J22-D5)

Dear Mr. Tellez,

Based upon the information provided in your submittal received 10-18-04, the above referenced plan is approved for Paving Permit and Grading Permit. Please attach a copy of this approved plan to the construction sets prior to sign-off by Hydrology. Upon completion of the project, please provide an Engineer Certification for our files.

Albuquerque

P.O. Box 1293

This project requires a National Pollutant Discharge Elimination System (NPDES) permit. If you have any questions regarding this permit please feel free to call the DMD Storm Drainage Design section at 768-3654 (Charles Caruso).

New Mexico 87103

If you have any questions, you can contact me at 924-3981.

www.cabq.gov

Kristal D. Metro

Sincerely,

Engineering Associate, Planning Dept. Development and Building Services

C: Charles Caruso, DMD Storm Drainage Design File



March 10, 2009

Lawerance D. Read, P.E.

Larry Read & Associates, Inc.

2430 Midtown Place, NE Ste. C

Albuquerque, NM 87107

Re: Chelwood Elementary School, 12701 Constitution Ave. NE,

(J-22/D005)

Approval of Permanent Certificate of Occupancy,

Engineer's Stamp Dated: 10-22-07

Engineer's Certification Date: 3-07-09

Dear Mr. Read,

PO Box 1293

Based upon the information provided by our visual inspection on 3/09/09, the above referenced certification is approved for release of Permanent Certificate of Occupancy by Hydrology.

Albuquerque

If you have any questions, you can contact me at 924-3982.

NM 87103

www.cabq.gov

Timothy E. Sims

Sinterely,

Plan Checker-Hydrology, Planning Dept Development and Building Services

C: CO Clerk—Katrina Sigala file

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### DRAINAGE AND TRANSPORTATION INFORMATION SHEET

(REV. 1/28/2003rd)

PROJE DRB #:	CT TITLE: <u>CHELWOOD ELEME</u>	NTARY SCHOOL EPC#:			E MAP/DRG. F RK ORDER#:		<u>-D005</u>	
	DESCRIPTION: <u>TRACT A, CHELWC</u> DRESS: <u>12701 CONSTITUTION AV</u>		<u>OL</u>					
	ERING FIRM: LARRY READ & ASS ADDRESS: 2430 MIDTOWN PLACE CITY, STATE: ALBUQUERQUE, NE	E, NE SUITE C	,	PHO	NTACT: <u>LARRY</u> ONE: <u>237-8421</u> CODE: <u>87107</u>	•	•	
<u>OWNER</u>	ALBUQUERQUE PUBLI ADDRESS: CITY, STATE: _	IC SCHOOLS	, · · · · · · · · · · · · · · · · · · ·	55 % PH0	NTACT:	•	•	
·	ECT: RMKM ADDRESS: CITY, STATE: _	•	· · · ·	PH0	NTACT: ONE: CODE:	•	•	
SURVEY	<u>OR:</u> ADDRESS: CITY, STATE:		•	PHO	NTACT:ONE:ODE:			
	ACTOR: ADDRESS: CITY, STATE:			: '`' PH(	NTACT: ONE: CODE:	•		•
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DATE SI	JBMITTED:March 9, 2009		BY:LAF	RRY D. RE		SECTIO		

Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a drainage submittal. The particular nature, location and scope of the proposed development defines the degree of drainage detail. One or more of the following levels of submittal may be required based on the following:

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- 2. **Drainage Plans**: Required for building permits, grading permits, paving permits and site plans less than five (5) acres.
- 3. Drainage Report: Required for subdivisions containing more than ten (10) lots or constituting five (5) acres or more.



## City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

November 20, 2003

Bruce Stidworthy, P.E.
Bohannan Huston, Inc.
7500 Jefferson NE – Courtyard 1
Albuquerque, NM 87109

Re: Chelwood School Kindergarten Addition, Corner of Chelwood Park Blvd.

and Constitution Ave., Certificate of Occupancy

Engineer's Stamp dated 2-28-03 (J22/D5)

Certification dated 11-19-03

Dear Mr. Stidworthy,

Based upon the information provided in your submittal received 11-20-03, , the above referenced certification is approved for release of permanent Certificate of Occupancy by Hydrology.

If you have any questions, you can contact me at 924-3981.

Sincerely,

Kristal D. Metro

Engineering Associate, Planning Dept.

Development and Building Services

C: Phyllis Villanueva

file

#### DRAINAGE AND TRANSPORTATION INFORMATION SHEET

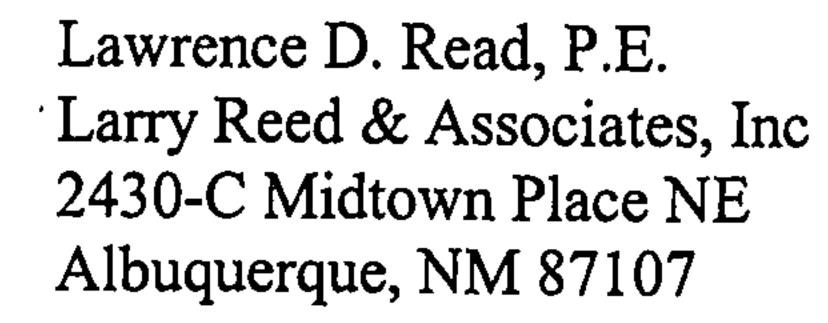
(REV. 1/11/2002)

PROJECT TITLE: Chelwood School Kindergarten Ade		
DRB #:EPC#:		WORK ORDER#:
LEGAL DESCRIPTION: Tract B as shown and designated on Pla CITY ADDRESS: N.E> Corner of Chelwood Park Blvd. & C		
ENGINEERING FIRM: Bohannan Huston, Inc.  ADDRESS: 7500 Jefferson NE – Courtyard I CITY, STATE: Albuquerque, NM  OWNER:  ADDRESS: CITY STATE:		CONTACT: Bruce Stidworthy PHONE: (505) 823-1000 ZIP CODE: 87109  CONTACT: PHONE: ZIP CODE:
CITY, STATE:  ARCHITECT: ADDRESS: CITY, STATE:		CONTACT: PHONE: ZIP CODE:
SURVEYOR: ADDRESS: CITY, STATE:		CONTACT:
CONTRACTOR:  ADDRESS:  CITY, STATE:		CONTACT:PHONE:ZIP CODE:
CHECK TYPE OF SUBMITTAL:  DRAINAGE REPORT  DRAINAGE PLAN  CONCEPTUAL GRADING & DRAINAGE PLAN  X GRADING PLAN  EROSION CONTROL PLAN  ENGINEER'S CERTIFICATION (HYDROLOGY)  CLOMR/LOMR  TRAFFIC CIRCULATION LAYOUT (TCL)  ENGINEERS CERTIFICATION (TCL)  ENGINEERS CERTIFICATION (DRB APPR. SITE PLAN)  OTHER		SK TYPE OF APPROVAL SOUGHT:  SIA / FINANCIAL GUARANTEE RELEASE  PRELIMINARY PLAT APPROVAL  S. DEV. PLAN FOR SUB'D. APPROVAL  S. DEV. PLAN FOR BLDG. PERMIT APPROVAL  SECTOR PLAN APPROVAL  FINAL PLAT APPROVAL  FOUNDATION PERMIT APPROVAL  BUILDING PERMIT APPROVAL  CERTIFICATE OF OCCUPANCY (PERM.)  CERTIFICATE OF OCCUPANCY (TEMP.)  GRADING PERMIT APPROVAL  PAVING PERMIT APPROVAL  WORK ORDER APPROVAL  OTHER (SPECIFY)
WAS A PRE-DESIGN CONFERENCE ATTENDED:  YES  NO COPY PROVIDED		DECEIVE NOV 2 0 2003
DATE SUBMITTED: 11-20-03	BY:	HYDROLOGY SECTION  Bruce Stidworthy

Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a drainage submittal. The particular nature, location and scope of the proposed development defines the degree of drainage detail. One or more of the following levels of submittal may be required based on the following:

- Conceptual Grading and Drainage Plan: Required for approval of Site Development Plans greater than five (5) acres and Sector Plans.
- 2. Drainage Plans: Required for building permits, grading permits, paving permits and site plans less than five (5) acres.
- 3. Drainage Report: Required for subdivisions containing more than ten (10) lots or constituting five (5) acres or more.

October 25, 2007



Re:

Chelwood Park Elementary – New Classroom Building

Engineer's Stamp dated 10-22-07 (J22-D5)

Dear Mr. Read,

Based on the information contained in your submittal received on October 22, 2007, the above referenced plan is approved for Building Permit, Grading Permit, and SO-19 Permit.

Please attach a copy of this letter and the approved plan to the construction sets prior to sign-off by Hydrology. A copy of this approval letter must be on hand when applying for the excavation permit. Prior to Certificate of Occupancy release, Engineer Certification per the DPM checklist will be required.

Be advised that no Certificate of Occupancy, temporary or permanent, will be released prior to inspection and approval of the storm drain connection by the Storm Drain Maintenance department. Contact Duane Schmitz at 235-8016 to schedule an inspection.

This project will also require a National Pollutant Discharge Elimination System (NPDES) permit. Inquiries regarding this permit should be directed to Sertil Kandar at 768-3645. In addition to submitting an NOI to the EPA and preparing a SWPPP, please send a copy of the SWPPP on a CD in .pdf format to Kathy Verhage with the Department of Municipal Development Storm Drainage Division at the following address.

www.cabq.gov

New Mexico 87103

Department of Municipal Development Storm Drainage Division P.O. Box 1293, One Civic Plaza, Rm. 301

Attn: Kathy Verhage Albuquerque, NM 87103

If you have any questions or need additional information, feel free to contact me at 924-3990.

Sincerely,

eremy Hoover/P.M., C.F.I

Senior Engineer/
Hydrology Section

Development and Building Services

cc:

file (J22-D5)

Antoinette Baldonado, Construction Services Duane Schmitz, DMD Street / Storm Maintenance Kathy Verhage, DMD Storm Drainage Division

P.O. Box 1293

Albuquerque



### Planning Department Transportation Development Services Section

March 27, 2009

Don Howard May, Registered Architect Rohde May Keller Architecture 400 Gold Ave. SW Sims Towers, Ste. 1100 Albuquerque, NM 87102

Re: Certification Submittal for Final Building Certificate of Occupancy for

APS Chelwood Elementary School, [J-22 / D005]

12701 Constitution Blvd NE

Architect's Stamp Dated 03/26/09

PO Box 1293

Dear Mr. May:

Sincetely,

Albuquerque

The TCL / Letter of Certification submitted on March 26, 2009 is sufficient for acceptance by this office for final Certificate of Occupancy (C.O.). Notification has been made to the Building and Safety Section.

NM 87103

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www.cabq.gov

Nilo E. Salgado-Fernandez, P.E.

Senior Traffix Engineer

Development and Building Services

Planning Department

C:

Engineer
Hydrology file
CO Clerk

a professional corporation

26 March 2009

City of Albuquerque Traffic and Hydrology

attn: Mr. Nilo Salgado (hand delivered)

re: APS Chelwood Elementary School

12701 Constitution Blvd., NE

(J-22/D005)

Mr. Salgado:

We are corresponding with regard to the recently completed site improvements and classroom building addition referenced above (APS Project No. 0236.1002.40105), and more specifically, the provision of additional accessible handicap parking stalls and ramp in the north parking lot as required.

۲,

Per your request, and for the purpose of obtaining the final Certification of Occupancy, as the Architect-of-Record I am providing you with this letter certifying that these improvements have been completed by T.A. Cole & Sons (General Contractor) in accordance with the documents prepared by my firm and my Consulting Civil Engineer, Larry Read and Associates, and that the finished construction is in compliance with the jurisdictional requirements. Please see the attached reference drawings and photos, and please don't hesitate to contact us if you have any questions.

Thank you in advance for your immediate attention, we look forward to the release of the final Certificate of Occupancy.

Of

Sincerely,

Don H. May, AIA, NCARB

Principal, Architect

copy T.A.Cole & Sons

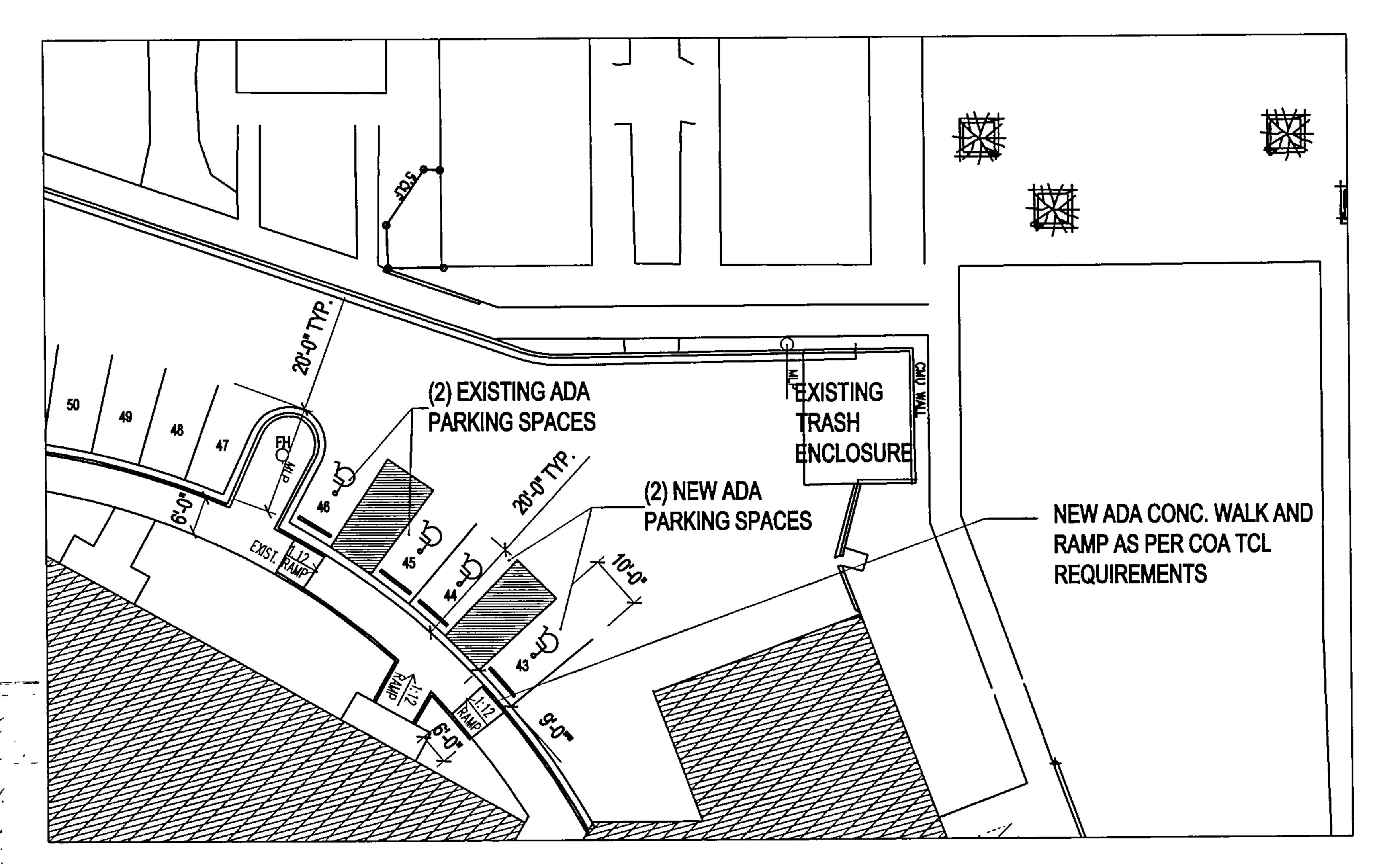
T. Mason, APS

L. Read, LRA

file, RMKMA

MAR 26 2009
HYDPOLOGY
STOWARD

SIMMS TOWER . STUDIO 1100 . 400 Gold Avenue SW . Albuquerque . NM . 87102-3265 p: 505.243.5454 f: 505.243.5858 www.rmkmarch.com



### NOTE:

THIS TCL REVISION TO THE TWO NEW ADA PARKING SPACES AS SHOWN IS IN ORDER TO COMPLY WITH COA TRAFFIC AND HYDROLOGY REQUIREMENTS FOR PERMANENT CERTIFICATE OF OCCUPANCY.

PROJECT

 $\sim$  -

CHELWOOD ELEMENTARY SCHOOL CLASSROOM BUILDING ADDITION AND CAMPUS IMPROVEMENTS 12701 CONSTITUTION BLVD. NE ALBUQUERQUE, NM .

RMKMc PROJECT NO

0608

TCL REVISION TO REQUIRED ADA PARKING SPACES AND RAMP

DRAWING FILE NO. TCL-1 REVISION 3.26 2009 SHEET NUMBER

SHEET TITLE

TCL 1-A

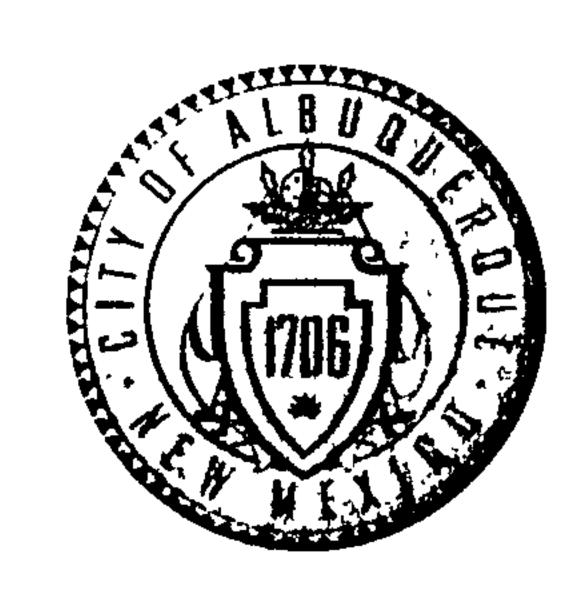
SCALE 1" = 30'-0"

## ROHDE MAY KELLER McNAMARA

DATE 3.26.2009 DRAWN BY DAE

PROFESSIONAL CORPORATION

400 GOLD AVE. S.W STUDIO 1100 SIMMS TOWER Albuquerque New Mexico 87102 USA tale 505 243 5454



### Planning Department Transportation Development Services Section

March 6, 2009

Don Howard, Registered Architect, Rohde May Keller McNamara Architecture Professional Corporation 400 Gold Avenue, SW, Ste. 1100 Simms Tower Albuquerque, NM 87102

Re: Approval of Temporary Certificate of Occupancy (C.O.) for

Chelwood Elementary School Classroom Addition, [J-22 / D005]

12701 Constitution Avenue NE Architect's Stamp Dated 03/05/09

Dear Mr. Howard:

PO Box 1293

Based on the information provided on your submittal dated March 6, 2009, the above referenced project is approved for a 120-day Temporary C.O.

Albuquerque

A Temporary C.O. has been issued allowing the H.C. ramp design (need to provide an alternative for ADA access from stall to sidewalk connection) issues to be completed within this time period. When these remaining issues have been fully completed, are in substantial compliance, and a final Certification for Transportation has been resubmitted to the City's Hydrology office for approval, a Permanent C.O. will be issued.

NM 87103

www.cabq.gov

The Certification package for Final C.O. must include an <u>exact</u> copy of the approved TCL, or signed off D.R.B. Site Plan, which is in each of the two City Permit Plan Sets—the contractor's City field set and the City's plan set in the basement of the Plaza Del Sol building. Package also must include a letter of certification on designer's letterhead-stamped with his seal, signed, and dated. Submit package along with fully completed Drainage Information Sheet to front counter personnel for log in and evaluation by Transportation.

If you have any questions, please call me at 924-3630.

Sincerely

Ny 6'E. Salgado-Fernandez, P.E.

Senior Traffic Engineer

Development/and Building Services

Planning Department

Engineer
Hydrology file
CO Clerk

#### DRAINAGE AND TRANSPORTATION INFORMATION SHEET

(REV 12/2005)

PROJECT TITLE: CHELLYON FENESTAR	24 SCHOOL ZONE MAP: J-72/DO05
DRB#: EPC#:	WORK ORDER#:
LEGAL DESCRIPTION:	······································
CITY ADDRESS:	
ENGINEERING FIRM:	CONTACT:
ADDRESS:	PHONE:
CITY, STATE:	ZIP CODE:
OLLIS DIZKID.	
OWNER:	CONTACT:
ADDRESS:	PHONE:
CITY, STATE:	ZIP CODE:
2	Declaration CONTRACTION TRAINED 168
ARCHITECT: WHOE MY KELLER MCHENGER	CONTACT RECEIVED
ADDRESS: 400 GOLD AVE. SILLY	100, Suns Jones PHONE: 243.5454
CITY, STATE: ABUQUERQUE, J	ZIP CODE: 8402
SURVEYOR:	CONTACT:
ADDRESS:	PHONE:
CITY, STATE:	ZIP CODE:
CONTRACTOR:	CONTACT:
ADDRESS:CITY, STATE:	PHONE: ZIP CODE:
CIII, BIAIC.	Zif CODE:
TYPE OF SUBMITTAL:	CHECK TYPE OF APPROVAL SOUGHT:
DRAINAGE REPORT	SIA/FINANCIAL GUARANTEE RELEASE
DRAINAGE PLAN 1 <sup>st</sup> SUBMITTAL	PRELIMINARY PLAT APPROVAL
DRAINAGE PLAN RESUBMITTAL	S. DEV. PLAN FOR SUB'D APPROVAL
CONCEPTUAL G & D PLAN	S. DEV. FOR BLDG. PERMIT APPROVAL
GRADING PLAN	SECTOR PLAN APPROVAL
EROSION CONTROL PLAN	FINAL PLAT APPROVAL
ENGINEER'S CERT (HYDROLOGY) CLOMR/LOMR	FOUNDATION PERMIT APPROVAL
TRAFFIC CIRCULATION LAYOUT	BUILDING PERMIT APPROVAL.  CERTIFICATE OF OCCUPANCY (PERM)
ENGINEER'S CERT (TCL)	CERTIFICATE OF OCCUPANCY (TEMP)
ENGINEER'S CERT (DRB SITE PLAN)	GRADING PERMIT APPROVAL
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Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a drainage submittal. The particular nature, location, and scope to the proposed development defines the degree of drainage detail. One or more of the following levels of submittal may be required based on the following:

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- 2. Drainage Plans: Required for building permits, grading permits, paving permits and site plans less than five (5) acres.
- 3. Drainage Report: Required for subdivision containing more than ten (10) lots or constituting five (5) acres or more.



August 15, 2008

Don May, R.A. Rohde May Keller McNamara Architects 400 Gold Ave SW Studio 1100, Sims Tower Albuquerque NM 87102

Re: Chelwood Elementary School Classroom Addition, 12701 Constitution Avenue NE

Traffic Circulation Layout

Engineer's Stamp dated 8-07-08 (J-22/D005)

Dear Mr. May,

The TCL submittal received 8-07-08 is approved for Building Permit. The plan is stamped and signed as approved. A copy of this plan will be needed for each of the building permit plans. Please keep the original to be used for certification of the site for final C.O. for Transportation. Public infrastructure or work done within City Right-of-Way shown on these plans is for information only and is not part of approval. A separate DRC and/or other appropriate permits are required to construct these items.

PO Box 1293

If a temporary CO is needed, a copy of the original TCL that was stamped as approved by the City will be needed. This plan must include a statement that identifies the outstanding items that need to be constructed or the items that have not been built in "substantial compliance," as well as the signed and dated stamp of a NM registered architect or engineer. Submit this TCL with a completed <u>Drainage and Transportation Information Sheet</u> to Hydrology at the Development Services Center of Plaza Del Sol Building.

Albuquerque

NM 87103

When the site is completed and a final C.O. is requested, use the original City stamped approved TCL for certification. A NM registered architect or engineer must stamp, sign, and date the certification TCL along with indicating that the development was built in "substantial compliance" with the TCL. Submit this certification TCL with a completed <u>Drainage and Transportation Information Sheet</u> to Hydrology at the Development Services Center of Plaza Del Sol Building.

www.cabq.gov

Once verification of certification is completed and approved, notification will be made to Building Safety to issue Final C.O. To confirm that a final C.O. has been issued, call Building Safety at 924-3306.

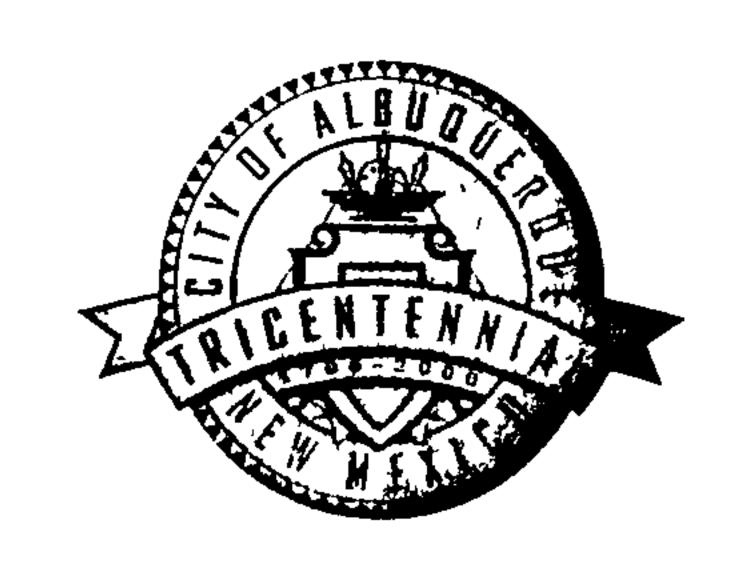
Sincerely,

Kristal D. Metro, P.E.

Senior Engineer, Planning Dept.

Development and Building Services

C: File



October 25, 2007

Don May, R.A. Rohde May Keller McNamara Architects 400 Gold Ave SW Studio 1100, Sims Tower Albuquerque NM 87102

Re:

Chelwood Elementary School Classroom Addition, 12701 Constitution Avenue NE Traffic Circulation Layout Engineer's Stamp dated 10-25-07 (J-22/D005)

Dear Mr. May,

The TCL submittal received 10-25-07 is approved for Building Permit. The plan is stamped and signed as approved. A copy of this plan will be needed for each of the building permit plans. Please keep the original to be used for certification of the site for final C.O. for Transportation. Public infrastructure or work done within City Right-of-Way shown on these plans is for information only and is not part of approval. A separate DRC and/or other appropriate permits are required to construct these items.

P.O. Box 1293

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Albuquerque

New Mexico 87103

www.cabq.gov

When the site is completed and a final C.O. is requested, use the original City stamped approved TCL for certification. A NM registered architect or engineer must stamp, sign, and date the certification TCL along with indicating that the development was built in "substantial compliance" with the TCL. Submit this certification TCL with a completed Drainage and Transportation Information Sheet to Hydrology at the Development Services Center of Plaza Del Sol Building.

Once verification of certification is completed and approved, notification will be made to Building Safety to issue Final C.O. To confirm that a final C.O. has been issued, call Building Safety at 924-3306.

Sincerely,

Kristal D. Metro, P.E.

Senior Engineer, Planning Dept. Development and Building Services

1/2/

File



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October 10, 2008

Ramon Sarason, R.A.
Rohde May Keller McNamara Architects
400 Gold Ave SW Studio 1100, Sims Tower
Albuquerque NM 87102

Re: Chelwood Elementary School Classroom Addition, 12701 Constitution Avenue NE,

Certificate of Occupancy – Transportation Development

Certification dated 10-09-08

Dear Mr. Sarason,

Based upon the information provided in your submittal received 10-09-08, the above referenced certification is approved for release of 90-day temporary Certificate of Occupancy by Transportation Development.

PO Box 1293

If you have any questions, you can contact me at 924-3991.

Albuquerque

NM 87103

Sincerely,

Kristal D. Metro, P.E.

Traffic Engineer, Planning Dept.

Development and Building Services

www.cabq.gov

C:

CO Clerk

File

## ARCHITECTURE ROHDE MAY KELLER MCNAMARA

9 October 2008

Kristal Metro
City of Albuquerque
Senior Engineer
600 2nd St. NW
Albuquerque, NM 87103

Re: Temporary Certificate of Occupancy for APS Chelwood Elementary Schools New Classroom Building

Dear Kristal,

This letter is in response to your request for a letter outlining the parking situation at the above referenced school. As indicated on the attached Phase 2 Demo Overlay Grading Plan (sheet C-7A) two existing classroom buildings and a portion of the existing parking lot will be demolished in order to make room for the construction of the new parking lot and turn-around. Parking Calculations currently for the school call for (1) space for each employee thus 45 employees = 45 parking spaces. In the unused portion of the campus to the west of the north curb on Eastridge Drive NE a temporary parking lot will provide the location for (31) employee parking spaces. These (31) spaces along with the undisturbed (15) spaces will provide a total of (46) spaces including (4) handicap accessible spaces while construction on the permanent lot proceeds. Also during construction some overflow parking can be accommodated curb side along Eastridge Drive. Once Phase II construction is complete the total permanent parking capacity will be (57) spaces.

If there are any questions regarding this issue please do not hesitate to contact me or project manager, David Edwards at 243-5454.

Sincerely,

Ramón J. Sarason, A.I.A., LEED

Associate-In-Charge RMKM Architecture OCT 0 9 2008

HYDROLOGY
SECTION

SIMMS TOWER

STUDIO 1100

400 Gold Avenue SW

Albuquerque

New Mexico 87102 USA

tel (505)243-5454

fax (505)243-5858

email rmkmarch.com



## City of Albuquerque P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

December 9, 2003

Bruce Stidworthy, P.E.
Bohannan Huston, Inc.
7500 Jefferson NE – Courtyard 1
Albuquerque, NM 87109

Re: Chelwood School Phase 1, Corner of Chelwood Park Blvd. and Constitution Ave., Certificate of Occupancy

Phase 1 Grading and Drainage Plan Engineer's Stamp dated 5-28-02

Phase 1 Miscellaneous Details Engineer's Stamp dated 6-21-02 (J22/D5)

Certification dated 12-08-03

Dear Mr. Stidworthy,

Based upon the information provided in your submittal received 12-09-03, the above referenced certification is approved for release of permanent Certificate of Occupancy by Hydrology.

If you have any questions, you can contact me at 924-3981.

Sincerely,

Kristal D. Metro

Engineering Associate, Planning Dept. Development and Building Services

C: Phyllis Villanueva file

## Bohannan A Hustong

December 8, 2003

Courtyard I 7500 Jefferson NE Albuquerque, NM 87109-4335 www.bhinc.com

voice: 505.823.1000 fax: 505.798.7988 toll free: 800.877.5332

Brad Bingham, P.E., Senior Engineer
Planning Dept/Hydrology Development Section
City of Albuquerque
P O Box 1293
Albuquerque, NM 87103

(Phase 1)

Re:

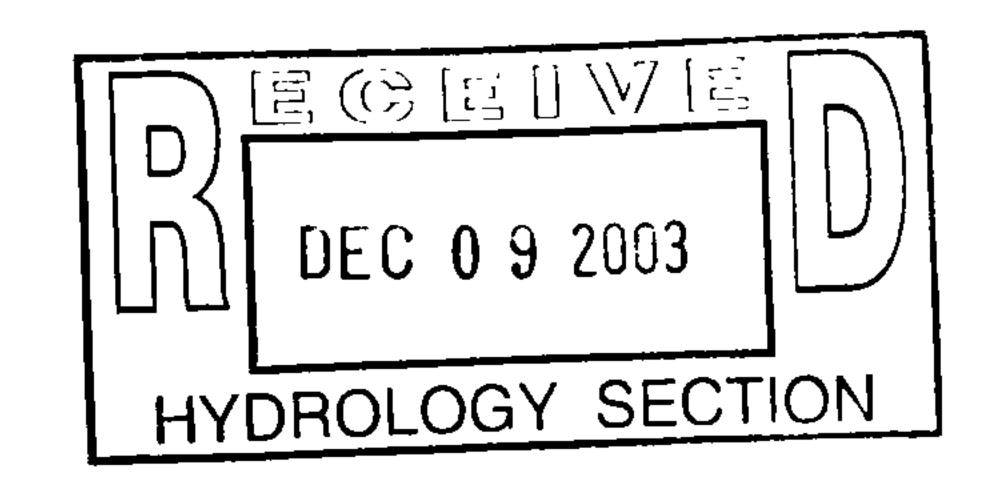
Chelwood Elementary School Classroom Addition Drainage Certification (J22/D5)

Dear Mr. Bingham:

The purpose of this letter is to address the comments received from Carlos Montoya in a letter addressed to me dated November 14, 2003. After I received that letter, I had a meeting with Carlos to review his comments. For your reference, I have attached a copy of an e-mail that I sent to the architect and copied to you and Carlos, after my meeting with Carlos. I have also attached a copy of Carlos' letter.

The responses to the comments provided below are based on the above referenced meeting with Carlos, as well as a field meeting with the contractor, and a follow-up inspection of the site.

- 1. The correct plans, matching the previously approved plans, are included with this submittal.
- 2. The only fill material placed in the pond was a very small quantity (approximately 2CY). The fill material was placed for aesthetic reasons in order to cover the remaining portion of the previous pond outlet structure. This quantity of fill material is insignificant compared to the overall pond volume and will not have a measurable effect on the hydrologic function of the pond.
- 3. During my meeting with Carlos, he explained that there was one specific area near the rip-rap rundown into the pond that he had observed rip-rap depth less than 8 inches. The contractor has installed additional rip-rap in this area.
- 4. The portion of 24" storm drain with the site is HDPE; the portion with the Right-of-Way is RCP.
- 5. The rip-rap was installed per plan except that it was not wire-tied. The wire-tie was not necessary for hydraulic reasons; it was suggested in order to minimize the potential for the rip-rap rock to be thrown by the students. Carlos acknowledged that this is not a drainage issue.
- 6. Additional rip-rap has been installed at the tow of the rundown slope, in order to further stabilize the slope and flatten it somewhat. It is now close to a 3:1 slope, and it has extra rip-rap. The design intent of providing a stable rundown has been achieved.
- 7. This rip-rap was also installed per plan except that it was not wire-tied. Again, the wire-tie is not required for hydraulic reasons. The installation meets the design intent.
- 8. This is the same comment as #6.



ENGINEERING  $\Delta$ 

SPATIAL DATA A

Brad Bingham, P.E., Senior Engineer COA Planning Dept/Hydrology Development December 8, 2003 Page 2

- 9. A "stand-up" asphalt curb was not constructed. An approximately 3" to 4" tall asphalt speed hump was installed. This hump serves the same purpose of diverting runoff into the drop inlet as the asphalt curb. Should the water overflow the speed hump, it will run overland to the same location; namely, the pond. Again, the design intent was achieved.
- 10. The contractor's material supplier did not have D50=4" rip-rap in stock. They had either 3" or 6" available; therefore, 3" was selected and used for the project. The 3" material was placed to the specified depth and will function adequately.

With this submittal, we are requesting permanent certificate of occupancy approval for this project. Should you have any questions, please contact me.

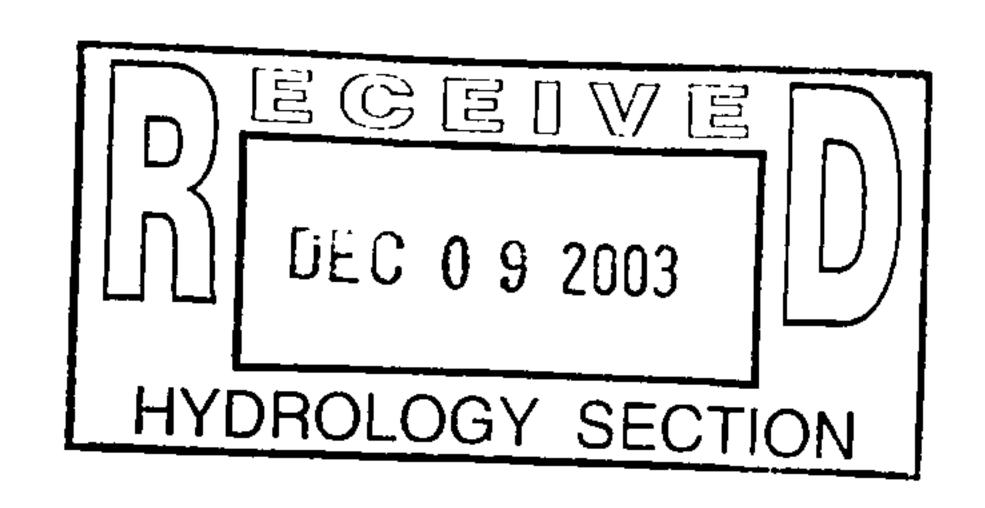
Sincerely,

Bruce J. Stidworthy, P.E.

Project Manger

Community Development and Planning

BJS/am



## DRAINAGE AND TRANSPORTATION INFORMATION SHEET J-22/D5

(REV. 1/11/2002)

Mannam (Phasel)

	Sindergarten Addition #1_ZONE MAP/DRG. FILE # J22/D5
DRB #:EPC;	#:WORK ORDER#:
LEGAL DESCRIPTION: Tract B as shown and	designated on Plat of Panorama Heights Addition
CITY ADDRESS: N.E> Corner of Chelwo	od Park Blvd. & Constitution Ave.
ENGINEERING FIRM: Bohannan Huston, In	c. CONTACT: Bruce Stidworthy
ADDRESS: 7500 Jefferson NE –	
CITY, STATE: Albuquerque, NM	ZIP CODE: 87109
	$\triangle \triangle A = A = A = A = A = A = A = A = A = $
OWNER: ADDRESS:	CONTACT: PHONE:
CITY, STATE:	ZIP CODE:
O111, O1711 L.	
ARCHITECT:	CONTACT:
ADDRESS:	PHONE:
CITY, STATE:	ZIP CODE:
SURVEYOR:	CONTACT:
ADDRESS:	PHONE:
CITY, STATE:	ZIP CODE:
CONTRACTOR:	CONTACT:
ADDRESS:	PHONE:
CITY, STATE:	ZIP CODE:
CHECK TYPE OF SUBMITTAL:  DRAINAGE REPORT  DRAINAGE PLAN  CONCEPTUAL GRADING & DRAINAGE  X GRADING PLAN  EROSION CONTROL PLAN  ENGINEER'S CERTIFICATION (HYDRO CLOMR/LOMR  TRAFFIC CIRCULATION LAYOUT (TCL)  ENGINEERS CERTIFICATION (DRB API OTHER	S. DEV. PLAN FOR BLDG. PERMIT APPROVAL SECTOR PLAN APPROVAL FINAL PLAT APPROVAL FOUNDATION PERMIT APPROVAL BUILDING PERMIT APPROVAL X CERTIFICATE OF OCCUPANCY (PERM.)
WAS A PRE-DESIGN CONFERENCE ATTENDED  YES  NO COPY PROVIDED  DATE SUBMITTED: 12-9-03	DEC 0 9 2003  HYDROLOGY SECTION  BY: Bruce Stidworthy

Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a drainage submittal. The particular nature, location and scope of the proposed development defines the degree of drainage detail. One or more of the following levels of submittal may be required based on the following:

- Conceptual Grading and Drainage Plan: Required for approval of Site Development Plans greater than five (5) acres and Sector Plans.
- 2. Drainage Plans: Required for building permits, grading permits, paving permits and site plans less than five (5) acres.
- 3. Drainage Report: Required for subdivisions containing more than ten (10) lots or constituting five (5) acres or more.

### **Bruce Stidworthy**

From: Allison Abraham [caa@smpcarch.com]

Sent: Thursday, November 20, 2003 1:37 PM

To: Richard Beall

Cc: Karen Alarid; Bruce Stidworthy

Subject: Fw: Chelwood Elementary School

Richard -- when can you meet at Chelwood with Bruce and me? Monday morning and Wednesday afternoon are my best times.

#### Allison

---- Original Message ----From: Bruce Stidworthy

To: Allison Abraham; Karen Alarid

Cc: Carlos Montoya, CoA Hydrology; Brad Bingham, CoA Hydrology

Sent: Thursday, November 20, 2003 12:58 PM

Subject: Chelwood Elementary School

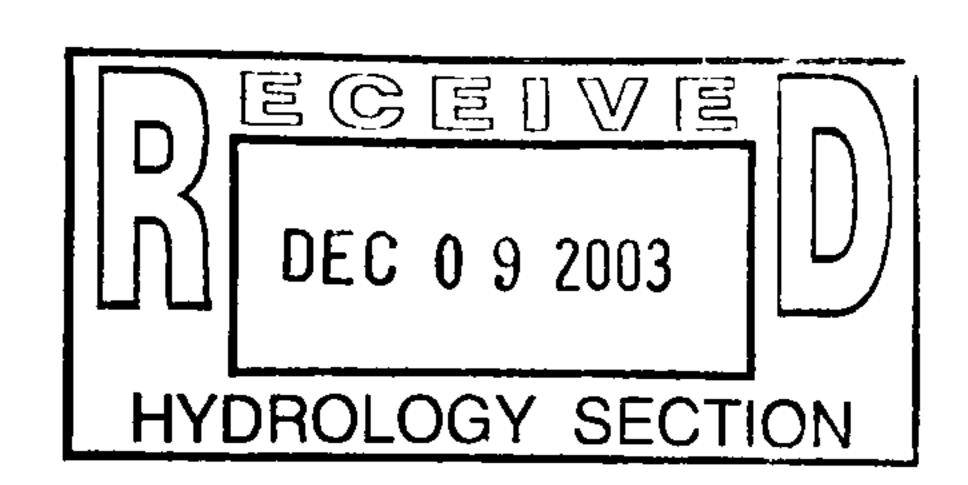
Allison: I met with Carlos Montoya from City Hydrology this morning. City Hydrology has approved the Kindergarten for permanent C.O. I then checked with the folks who issue the CO's, and they said that the Kindergarten has been completely approved for temp. CO (I know there is an issue with the Fire Marshall that must be resolved prior to permanent CO - there may be others as well).

The City does have remaining concerns relative to the Classroom/Cafeteria project. Carlos made a site inspection himself and he will require that the following issues must be corrected before hydrology will grant CO approval:

- 1. Additional Rip Rap must be placed at the rundown into the east side of the pond. The drawings call for the rip-rap to be 8" thick, however there are places where the rock is only one layer thick, and you can see the rip-rap through it.
- 2. The grading and berm along the east edge of the pond, south of the rundown is not graded correctly. As a result, the drainage is directed to the south, instead of to the north and into the pond. Some re-work will be necessary.

I think I should meet at the site with Richard Beall one more time to get this worked out. Please let me know when you'd like to do that. Thanks, Bruce.

Bruce Stidworthy, P.E.
Bohannan Huston Inc.
7500 Jefferson NE
Albuquerque NM 87109
505 823 1000





## City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

November 14, 2003

Bruce Stidworthy, P.E.
Bohannan Huston, Inc.
7500 Jefferson NE – Courtyard 1
Albuquerque, NM 87109

Re: Chelwood School Phase 1, Corner of Chelwood Park Blvd. and Constitution Ave., Certificate of Occupancy

Engineer's Stamp dated 2-28-03 (J22/D5)

Certification dated 11-07-03

DEC 0 9 2003
HYDROLOGY SECTION

Dear Mr. Stidworthy,

Based upon the information provided in your submittal received 11-10-03, the above referenced certification cannot be approved until the following comments are addressed:

- 1. The plans included in your submittal do not match the plans previously approved. Please include the Phase 1 Grading and Drainage Plan (sheet C201) stamped by Bruce Stidworthy on May 28, 2002 and the Phase 1 Miscellaneous Details (sheet C302) stamped by Glenn Broughton on June 21, 2002.
- 2. Fill material was placed in the detention pond. What is its purpose and how will it affect the volume of the pond?
- 3. The detail sheet indicated that all rip-rap should be 8" thick. However, a site inspection indicates the depth is less than this.
- 4. Is the 24" storm drain pipe running from the pond to the street HDPE or RCP?
- 5. The outlet cross section shows wire-enclosed rip-rap. The wire tied rip-rap was omitted.
- 6. The detention pond rundown was steeper than 3:1.
- 7. Sheet C201 indicates that the rip-rap rundown under grading keyed note "N" should be wire tied.
- 8. The grading keyed note "O," located east of the detention pond, is incorrectly sloped.

- 9. Grading keyed note "B" indicates a 6" asphalt concrete curb. This curb was not constructed.
- 10. Please confirm that a D50 of 4" was used for the rip-rap.

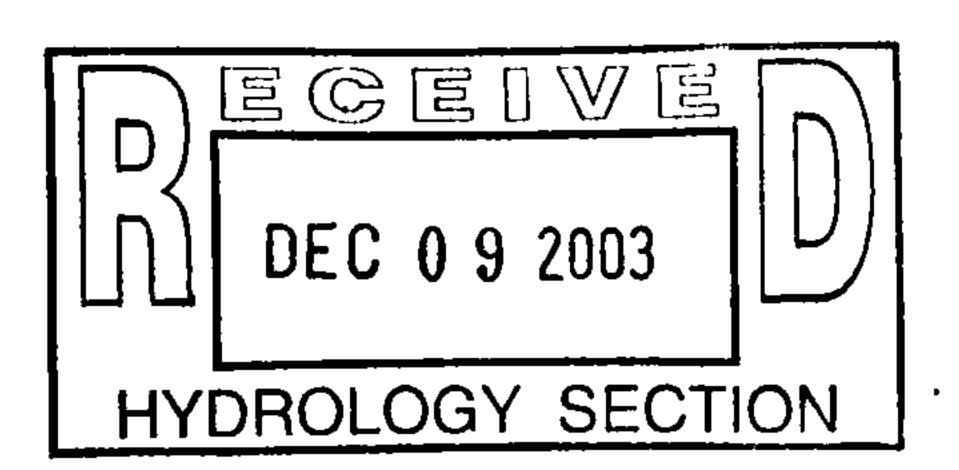
If you have any questions, you can contact me at 924-3982.

Sincerely,

Carlos A. Montoya, PE

City Floodplain Administrator

C: file



## Bohannan A Hustong-

REC'D BY:

### CLIENT/COURIER TRANSMITTAL

Courtyard I 7500 Jefferson St. NE Albuquerque, NM 87109-4335

www.bhinc.com

voice: 505.823.1000 facsimile: 505.798.7988 toll free: 800.877.5332

			toll free: 800.877
To:	Brad Bingham	Requested by:	Kelly Warren
	City of Albuquerque 600 Second Street NW, 2ndh Floor Albuquerque, NM 87103	Date:	December 9, 2003
		Time Due:	This A.M. This P.M. Rush By Tomorrow
Phone:	924-3986		
Job No.:	020138	Job Name:	Chelwood-School Kindergarten Add.
<u>DE</u>	LIVERY VIA	PICK	
Cour	ier Federal Express	Item:	
Mail	UPS	<del></del>	
Othe			
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TEM NO.	QUANTITY DESCRIPTION		Workon
1	1 Drainage Info Sheet		this
1	1 Grading & Drainage Plan		
			ASAP BUS
COMMENTS	/ INSTRUCTIONS		<u> </u>
			DEC 0 9 2003  YDROLOGY SECTION

DATE:

TIME:

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# City of Albuquerque P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

November 14, 2003

Bruce Stidworthy, P.E.
Bohannan Huston, Inc.
7500 Jefferson NE – Courtyard 1
Albuquerque, NM 87109

Re: Chelwood School Phase 1, Corner of Chelwood Park Blvd. and Constitution Ave., Certificate of Occupancy

Engineer's Stamp dated 2-28-03 (J22/D5)

Certification dated 11-07-03

Dear Mr. Stidworthy,

Based upon the information provided in your submittal received 11-10-03, the above referenced certification cannot be approved until the following comments are addressed:

- 1. The plans included in your submittal do not match the plans previously approved. Please include the Phase 1 Grading and Drainage Plan (sheet C201) stamped by Bruce Stidworthy on May 28, 2002 and the Phase 1 Miscellaneous Details (sheet C302) stamped by Glenn Broughton on June 21, 2002.
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- 6. The detention pond rundown was steeper than 3:1.
- 7. Sheet C201 indicates that the rip-rap rundown under grading keyed note "N" should be wire tied.
- 8. The grading keyed note "O," located east of the detention pond, is incorrectly sloped.

- 9. Grading keyed note "B" indicates a 6" asphalt concrete curb. This curb was not constructed.
- 10. Please confirm that a D50 of 4" was used for the rip-rap.

If you have any questions, you can contact me at 924-3982.

Sincerely,

Carlos A. Montoya, PE

City Floodplain Administrator

C: file

44. . . . . Sand

### DRAINAGE AND TRANSPORTATION INFORMATION SHEET

(REV. 1/11/2002)

### Phase 1

PROJECT TITLE:	Chelwood School		ZONE MAP/DRG. FILE #	J22/D5
DRB #:	EPC#:		_WORK ORDER#:	
LEGAL DESCRIPTION:_				
CITY ADDRESS:	N.E> Corner of Chelwood Park B	vd. & Constitutio	n Ave.	
ENGINEERING FIRM:	Pohannan Huston Inc		CONTACT: D.	una Ctialuna milana
ADDRESS:	Bohannan Huston, Inc. 7500 Jefferson NE – Courtyard	<u> </u>		uce Stidworthy 23-1000
CITY, STATE:	Albuquerque, NM	<u> </u>	ZIP CODE: 87109	23-1000
OITT, OITTL	7 libuquei que, i titi		ZII OODL	263-5772
OWNER:			CONTACT:	
ADDRESS:			PHONE:	
CITY, STATE:	——————————————————————————————————————	· · · · · · · · · · · · · · · · · · ·	ZIP CODE:	
ARCHITECT:			CONTACT:	
ADDRESS:			PHONE:	
CITY, STATE:		<del>,                                    </del>	ZIP CODE:	
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SURVEYOR:			CONTACT:	<u> </u>
ADDRESS:	•	<del> </del>	PHONE:	
CITY, STATE:		——————————————————————————————————————	ZIP CODE:	
CONTRACTOR:		•	CONTACT:	
ADDRESS:			PHONE:	· <del></del>
CITY, STATE:	· · · · · · · · · · · · · · · · · ·		ZIP CODE:	
GRADING PLAN EROSION CONT ENGINEER'S CI CLOMR/LOMR TRAFFIC CIRCU ENGINEERS CE	PORT IN GRADING & DRAINAGE PLAN		CK TYPE OF APPROVAL SOUG  SIA / FINANCIAL GUARANTE PRELIMINARY PLAT APPRO S. DEV. PLAN FOR SUB'D. A S. DEV. PLAN FOR BLDG. P SECTOR PLAN APPROVAL FINAL PLAT APPROVAL FOUNDATION PERMIT APPROV CERTIFICATE OF OCCUPAN CERTIFICATE OF OCCUPAN GRADING PERMIT APPROVAL PAVING PERMIT APPROVAL PAVING PERMIT APPROVAL CONTROPPIOSE CONTROPPIO	E RELEASE VAL PPROVAL ERMIT APPROVAL  ROVAL AL ICY (PERM.) ICY (TEMP.) AL
YES NO COPY PROVIDE		HAYDRO HAYDRO	OV 1 0 2003 LOGY SECTION	
DATE SUBMITTED:	11-7-03	BY:Bruce	<u>e Stidworthy</u>	<del></del>

Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a drainage submittal. The particular nature, location and scope of the proposed development defines the degree of drainage detail. One or more of the following levels of submittal may be required based on the following:

- 1. Conceptual Grading and Drainage Plan: Required for approval of Site Development Plans greater than five (5) acres and Sector Plans.
- 2. Drainage Plans: Required for building permits, grading permits, paving permits and site plans less than five (5) acres.
- 3. Drainage Report: Required for subdivisions containing more than ten (10) lots or constituting five (5) acres or more.



## City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

July 28, 1994

Mike Walla, P.E. Chavez-Grieves Engineering 5639 Jefferson NE Albuquerque, NM 87109

RE: ENGINEER'S CERTIFICATION FOR CHELWOOD ELEMENTARY SCHOOL (J22-D5)
RECEIVED JULY, 25, 1994 FOR CERTIFICATE OF OCCUPANCY APPROVAL
ENGINEER'S STAMP DATED 7/22/94.

Dear Mr. Walla:

Based on the information included in the submittal referenced above, City Hydrology accepts the Engineer's Certification of grading & drainage and approves a Permanent Certificate of Occupancy for the Kindergarten Addition at 12701 Constitution NE.

If I can be of further assistance, You may contact me at 768-2727.

Sincerely,

John P. Curtin, P.E.

Civil Engineer/Hydrology

WPHYD/7956/jpc

c: - Andrew Garcia



## City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

September 2, 1993

Mike Walla Chavez-Grieves Engineering 5639 Jefferson NE Albuquerque, NM 87109

RE: REVISED DRAINAGE PLAN FOR CHELWOOD ELEMENTARY KINDERGARDEN ADDITION (J22-D5) ENGINEER'S STAMP DATED 8/24/93.

Dear Mr. Walla:

Based on the information provided on your August 27, 1993 resubmittal, the above referenced site is approved for Building Permit.

Please attach a copy of the approved plan (Sheet CC2 & CC2A) to the construction sets prior to sign-off by Hydrology.

Also, prior to Certificate of Occupancy release, Engineer Certification per the D.P.M. checklist will be required. Along with the Certification submittal, you will need to provide a compaction report for the proposed l'foot high earthen berm.

If I can be of further assistance, please feel free to contact me at 768-2667.

Sincerely,

Bernie J. Montoya, CE

Engineer Associate

BJM/d1/WPHYD/7956

xc: Alan Martinez

File

City of Albuquerque DPM Section 22.2 (January, 1993)

### PRECIPITATION ZONE "4"

### Site Information:

Project Name: Chelwood Elementary School Site Project Location: Chelwood / Indian School Road

Analysis Condition Pre and Post-Construction - Drainage Basin 2 - 2.94 Acres

Analysis Date: 4/24/93

	والمراق والمراق والأربي والتناب والمراق			
Summary Table of Land Treatments				
Treatment	Area (Ac.)			
Α	0.0000			
В	0.0000			
С	2.9400			
D	0.0000			

0.00% Impervious Area

Total Area = 2.9400 Acres

Peak Discharge Rate:

<del></del>		
D	5.25	0.00
C	3.73	10.97
В	2.92	0.00
Α	2.20	0.00
Treatment	Peak Discharge Rate (cfs/Acre)	QxA

Total Qp = 10.97 cfs

Utilizing the Rational Method: Peak Intensity = 5.61 IN/HR. at Tc=0.2 Hr.

Rational Method

 Treatment
 Coefficient, C
 Q x A

 A
 0.39
 0.00

 B
 0.52
 0.00

 C
 0.66
 10.89

 D
 0.94
 0.00

Total Qp = 10.89 cfs

### Volumetric Runoff:

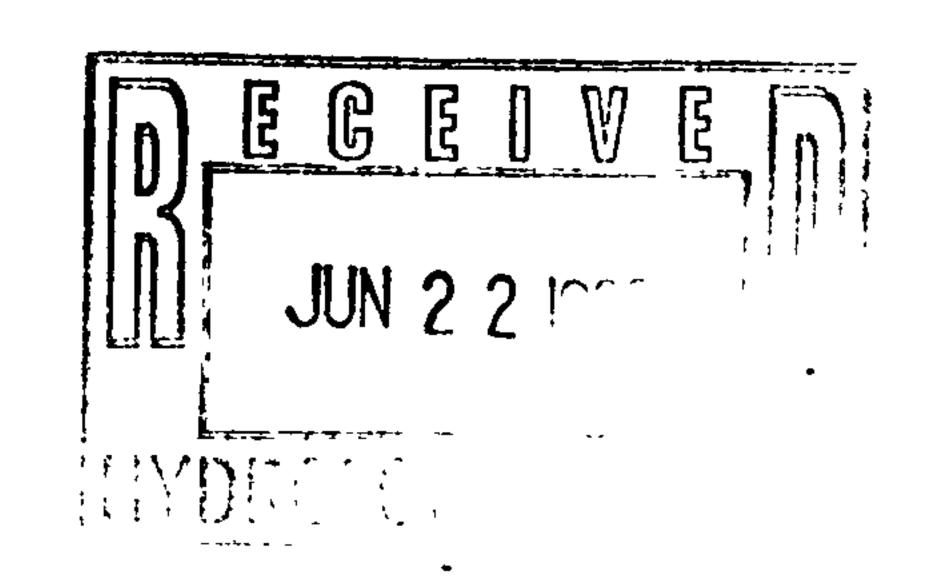
	Excess Precipitation, E	
Treatment	(Inches)	ExA
Α	0.8	0.00
В	1.08	0.00
C	1.46	4.29
D	2.64	0.00
	Total E x A =	4.29

Weighted E =

1.460 inches

Volume (360) =

0.36 Acre-Ft.



### Chelwood Elementary School Grading / Drainage Information

		S.F.	Acres	
Area Drainage Basin	18	130, 432	2.9943	
Area Drainage Basin	1B	210,512	4.8327	
Area Drainage Basin				
Total Area Drainage				•
•				
				~
DDATNACE ADEA 13				

DRAINAGE AREA 1A		
EXISTING STRUCTURES		
Building Area	17,677	0. <b>4</b> 058
Sidewalk Area	5,990	0.1375
Student Dropoff Area	5,200	0.1194
Main Parking Area	30,140	0.6919
	59,007	1.3546 SUBTOTAL EXISTING STRUCTURES
PROPOSED NEW STRUCTURES		
Building Area	5,081	0.1166
Sidewalk Area	14,117	0.3241

SUMMARY

PERCENT IMPERVIOUS BEFORE NEW CONSTRUCTION 45.24%

PERCENT IMPERVIOUS AFTER NEW CONSTRUCTION 59.96%

DRAINAGE AREA 1B EXISTING STRUCTURES

Building Area 26,810 0.6155 Concrete / Asphalt Play 9,350 0.2146 Sidewalk Area 12,004 0.2756

48, 164 1.1057 SUBTOTAL EXISTING STRUCTURES

PROPOSED NEW STRUCTURES

0 0.0000 SUBTOTAL PROPOSED NEW STRUCTURES

19, 198 0.4407 SUBTOTAL PROPOSED NEW STRUCTURES

JUN 4 2 1993
YDROLOGY DIVIS:

Page 1

City of Albuquerque DPM Section 22.2 (January, 1993)

### PRECIPITATION ZONE "4"

### Site Information:

Project Name: Chelwood Elementary School Site Project Location: Chelwood / Indian School Road

Analysis Condition Pre-Construction - Drainage Basin 1A - 2.9943 Acres

Analysis Date: 4/24/93

Summary Table of Land Treatments		
Treatment	Area (Ac.)	
A	0.0000	
В	0.0000	
С	1.6397	
D	1.3546	

45.24% Impervious Area

Total Area = 2.9943 Acres

Peak Discharge Rate:

	Total Qp =	13.23 cf
D	5.25	- 7.11
C	3.73	6.12
В	2.92	0.00
Α	2.20	0.00
Treatment	Rate (cfs/Acre)	QxA
	Peak Discharge	

Utilizing the Rational Method: Peak Intensity = 5.61 IN/HR. at Tc=0.2 Hr.

			. •
	Rational Method i		
Treatment	Coefficient, C	QxA	
Α	0.39	0.00	1
В	0.52	0.00	
С	0.66	6.07	
D	0.94	7.14	
	7		

Total Qp = 13.21 cfs

### Volumetric Runoff:

	Excess Precipitation, E	
Treatment	(Inches)	ExA
Α	0.8	0.00
В	1.08	0.00
C	1.46	2.39
D	2.64	3.58
	Total E x A =	5.97

Weighted E = 1.994 inches

Volume (360) = 0.50 Acre-Ft.

JUN 4 2 1933

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City of Albuquerque DPM Section 22.2 (January, 1993)

### PRECIPITATION ZONE "4"

### Site Information:

Project Name: Chelwood Elementary School Site Project Location: Chelwood / Indian School Road

Analysis Condition Post-Construction - Drainage Basin 1A - 2.9943 Acres

Analysis Date: 4/24/93

Summary Table of Land Treatments		
Treatment	Area (Ac.)	
Α	0.0000	
В	0.0000	
C	1.1989	
D	1.7954	

59.96% Impervious Area

Total Area = 2.9943 Acres

### Peak Discharge Rate:

	Total Qp =	13.90 c	fs
D	5.25	9.43	
С	3.73	4.47	
В	2.92	0.00	
Α	2.20	0.00	
Treatment	Peak Discharge Rate (cfs/Acre)	QxA	

Utilizing the Rational Method: Peak Intensity = 5.61 IN/HR. at Tc=0.2 Hr.

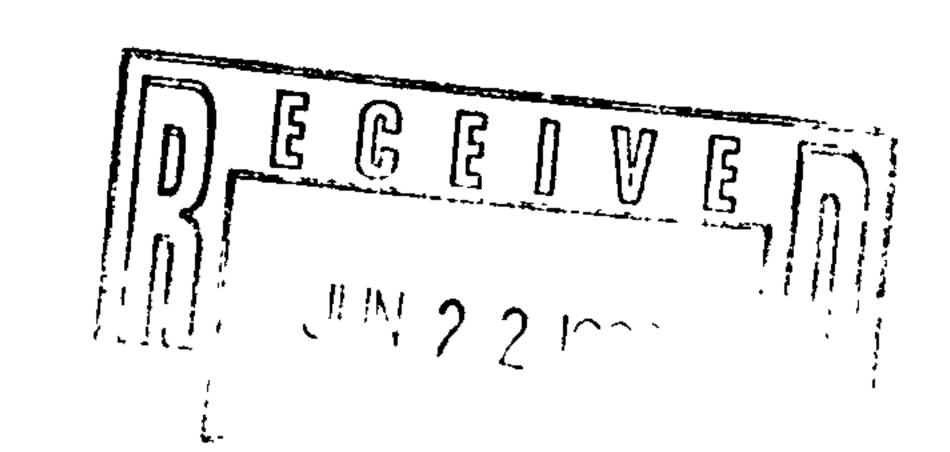
			_ • •
	Rational Method		
Treatment	Coefficient, C	QxA	
Α	0.39	0.00	
В	0.52	0.00	
С	0.66	4.44	! !
D	0.94	9.47	
	Total Qp =	13.91	ci

#### Volumetric Runoff:

	Excess Precipitation, E	
Treatment:	(Inches)	ExA
A	0.81	0.00
В	1.08	0.00
C	1.46	1.75
D	2.64	4.74
	Total E x A =	6.49

Weighted E = 2.168

Volume (360) = 0.54 Acre-Ft.



City of Albuquerque DPM Section 22.2 (January, 1993)

### PRECIPITATION ZONE "4"

### Site Information:

Project Name: Chelwood Elementary School Site Project Location: Chelwood / Indian School Road

Analysis Condition Pre and Post-Construction - Drainage Basin 1B - 4.8327 Acres

Analysis Date:

4/24/93

Summary Table of Land Treatments		
Treatment Area (Ac.)		
Α	0.0000	
В	0.0000	
C	3.7270	
D	1.1057	

22.88% Impervious Area 4.8327 Acres

Total Area =

### Peak Discharge Rate:

	Peak Discharge	
Treatment	Rate (cfs/Acre)	QxA
Α	2.20	0.00
В	2.92	0.00
С	3.73	13.90
D	5.25	5.80
<del></del>	Total Qp =	19.71 c

Utilizing the Rational Method: Peak Intensity = 5.61

IN/HR. at Tc=0.2 Hr.

	Total On	40.00
D	0.94	5.83
C	0.66	13.80
В	0.52	0.00
A	0.39	0.00
Treatment	Coefficient, C	QxA
	Rational Method i	
Canaling and reduction intellige.		1 Cak Interiory -

19.63 cfs Total Qp =

### Volumetric Runoff:

	Excess	
	Precipitation, E	
Treatment	(Inches)	ExA
A :	0.8	0.00
В	1.08	0.00
C	1.46	5.44
D	2.64	2.92
**************************************		

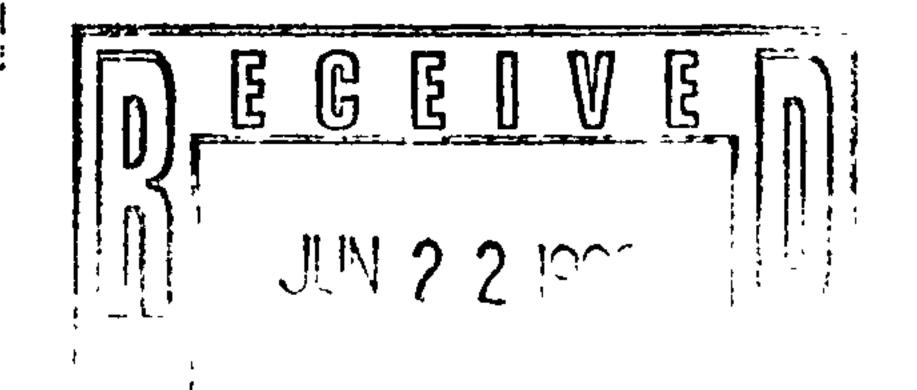
8.36 Total  $E \times A =$ 

Weighted E =

1.730 inches

Volume (360) =

0.70 Acre-Ft.





# City of Albuquerque P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

April 9, 2003

Bruce Stidworthy
Bohannan-Huston, Inc.
7500 Jefferson NE
Courtyard One
Albuquerque New Mexico 87109

RE: Grading and Drainage Plan for Chelwood School Kindergarten Addition #1 (J22-D5) Dated February 28, 2003

Dear Mr. Stidworthy:

The above referenced drainage plan is approved for building permit. Upon completion of the project please certify the drainage plan per the DPM.

Please contact the contractor because runoff and sediment is being allowed to enter Chelwood Boulevard. Sediment is being deposited on the public street. Please ask the contractor to address this issue. Also, my June 25, 2002 letter addressed how the connection from the detention pond to the storm drain needs to be submitted to DRC for review and approval.

If you have any questions please call me at 924-3982.

This site requires a National Pollution Discharge Elimination System (NPDES) permit. Refer to the attachment that is provided with this letter for details. If you have any questions please feel free to call the Public works Hydrology section at 768-3654 (Charles Caruso) or 768-3645 (Brian Wolf).

Sincerely,

Carlos A. Montoya

City Floodplain Administrator

C: Charles Caruso, Hydrology Public Works



# City of Albuquerque P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

April 8, 2003

Bruce Stidworthy
Bohannan-Huston, Inc.
7500 Jefferson NE
Courtyard One
Albuquerque New Mexico 87109

RE: Grading and Drainage Plan for Chelwood School Kindergarten Addition #1 (J22-D5) Dated February 28, 2003

Dear Mr. Stidworthy:

The above referenced drainage plan is approved for building permit. Upon completion of the project please certify the drainage plan per the DPM.

Please contact the contractor because runoff and sediment is being allowed to enter Chelwood Boulevard. Sediment is being deposited on the public street. Please ask the contractor to address this issue. Also, my June 25, 2002 letter addressed how the connection from the detention pond to the storm drain needs to be submitted to DRC for review and approval. If you have any questions please call me at 924-3982.

Sincerely,

Carlos A. Montoya

City Floodplain Administrator



# City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

June 25, 2002

Glenn Broughton, PE Bohannan Huston, Inc. 7500 Jefferson NE Albuquerque, NM 87109

RE: Chelwood Elementary School (J22-D5)

Dear Mr. Broughton:

The below referenced grading plans, drainage plans, and drainage report are approved for Building Permit for Phase 1 and Drainage Master Plan.

- Master Drainage Plan stamped by Glenn Broughton on May 30, 2002, received May 31, 2002
- Ultimate Development Drainage Basin Map stamped by Glenn Broughton on March 16, 2002, received May 31, 2002
- Portable Park Grading, Drainage and Utility Plan stamped by Bruce Stidworthy on March 14, 2002, received May 31, 2002
- Phase 1 Grading and Drainage Plan stamped by Bruce Stidworthy on May 28, 2002, received May 31, 2002
- Phase 1 Miscellaneous Details stamped by Glenn Broughton on June 21, 2002, received June 21, 2002

Prior to Certificate of Occupancy release, the work order needs to be processed through DRC. The construction needs to be accepted by the City of Albuquerque. Please notify the contractor of this situation.

If you have any questions please call me at 924-3982

Sincerely,

Carlos A. Montoya, PE

City Floodplain Administrator

# DRAINAGE AND TRANSPORTATION INFORMATION SHEET

(REV. 1/11/2002)

PROJECT TITLE: Che DRB #:	lwood Elementary School Master Drain EPC#:	age Report ZONE MAP/DRG. FILE # J22-D5 WORK ORDER#:
LEGAL DESCRIPTION:		
CITY ADDRESS:		
ENGINEERING FIRM:  ADDRESS:  CITY, STATE:  OWNER:  ADDRESS:	Bohannan Huston, Inc. 7500 Jefferson NE – Courtyard I Albuquerque, NM	CONTACT: Glenn Broughton PHONE: (505) 823-1000 ZIP CODE: 87109  CONTACT: PHONE:
CITY, STATE:		ZIP CODE:
ARCHITECT:  ADDRESS: CITY, STATE: SURVEYOR:	SMPC Architects 115 Amherst Drive SE Albuquerque, NM	CONTACT: Allison Abraham  PHONE: (505) 255-8668  ZIP CODE: 87106  CONTACT:
ADDRESS: CITY, STATE:		PHONE: ZIP CODE:
CONTRACTOR:  ADDRESS: CITY, STATE:  CHECK TYPE OF SUBMI  X DRAINAGE REPO DRAINAGE PLAN CONCEPTUAL GONCEPTUAL GONCEPTUAL GONCEPTUAL GONCEPTUAL GONCEPTUAL GONCEPTUAL GONCEPTUAL GONCEPTUAL GONCENGINEER'S CENGINEER'S CENGINEER	ORT N RADING & DRAINAGE PLAN	CONTACT: PHONE: ZIP CODE:  CHECK TYPE OF APPROVAL SOUGHT: SIA / FINANCIAL GUARANTEE RELEASE PRELIMINARY PLAT APPROVAL. S. DEV. PLAN FOR SUB'D. APPROVAL S. DEV. PLAN FOR BLDG. PERMIT APPROVAL SECTOR PLAN APPROVAL FINAL PLAT APPROVAL FOUNDATION PERMIT APPROVAL X BUILDING PERMIT APPROVAL CERTIFICATE OF OCCUPANCY (PERM.) CERTIFICATE OF OCCUPANCY (TEMP.) GRADING PERMIT APPROVAL PAVING PERMIT APPROVAL
WAS A PRE-DESIGN CO YES NO COPY PROVIDE  DATE SUBMITTED:	D	WORK ORDER APPROVAL OTHER (SPECIFY) Drainage Master Plan Approval  JUN 3 1 2002  HYDROLOGY SECTION

Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a drainage submittal. The particular nature, location and scope of the proposed development defines the degree of drainage detail. One or more of the following levels of submittal may be required based on the following:

- Conceptual Grading and Drainage Plan: Required for approval of Site Development Plans greater than five (5) acres and Sector Plans.
- 2. Drainage Plans: Required for building permits, grading permits, paving permits and site plans less than five (5) acres.
- 3. Drainage Report: Required for subdivisions containing more than ten (10) lots or constituting five (5) acres or more.



# City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

June 10, 2002

Glenn Broughton
Bohannan Huston, Inc.
7500 Jefferson NE – Courtyard 1
Albuquerque, NM 87109

RE: Chelwood Elementary School Master Drainage Report (J22-D5) Dated May 30, 2002

Dear Mr. Broughton:

I have reviewed the referenced drainage report received May 31, 2002 and forward the following comments:

- 1. Plan C302, the riprap rundown detail and plan view have different widths for the same rundown, respectively 4 ft. and 15 ft. Please correct this on consistency.

  2. Plan C302, section 'A-A" of the detention pond has a varied length of 9 ft. to
- Plan C302, section 'A-A" of the detention pond has a varied length of 9 ft. to 15 ft. from the top of the embankment to the bottom. However, it appears on plan C201 that the length does not vary. The plan view of the emergency spillway dimensions are not to scale on plan C302.
- 3. Plan C302 engineer stamp is not signed and dated.
- 4. The use of riprap should be discussed with APS. It appears that the use of riprap could develop into a problem with the school children. We strongly request that riprap be wire-tired or grouted.
  - 5.) The drainage report does not state whether the drainage pond improvements will be built in phase 1. Please correct this.
- 6. Prior to Certificate of Occupancy release, the work order needs to be processed through DRC. The construction needs to be accepted by the City of Albuquerque. Please notify the contractor of this situation.

If you have any questions please call me at 924-3982

Sincerely,

Carlos A. Montoya

City Floodplain Administrator

# DRAINAGE AND TRANSPORTATION INFORMATION SHEET

(REV. 1/11/2002)

PROJECT TITLE: Chelwood Elementary School Master Drainage Report DRB #:EPC#:	ZONE MAP/DRG. FILE #J22-D5 WORK ORDER#:
LEGAL DESCRIPTION:	
CITY ADDRESS:	
ENGINEERING FIRM: Bohannan Huston, Inc.  ADDRESS: 7500 Jefferson NE – Courtyard I  CITY, STATE: Albuquerque, NM	CONTACT: Glenn Broughton PHONE: (505) 823-1000 ZIP CODE: 87109
CITT, STATE. <u>Albuquerque, Ivivi</u>	ZIF CODE. <u>07 103</u>
OWNER:	CONTACT:
ADDRESS: CITY, STATE:	PHONE:ZIP CODE:
ARCHITECT: SMPC Architects	CONTACT: Allison Abraham
ADDRESS: 115 Amherst Drive SE	PHONE: (505) 255-8668
CITY, STATE: <u>Albuquerque, NM</u>	ZIP CODE: 87106
SURVEYOR:	CONTACT:
ADDRESS:	PHONE:
CITY, STATE:	ZIP CODE:
CONTRACTOR:	CONTACT:
ADDRESS:	PHONE:
CITY, STATE:	ZIP CODE:
DRAINAGE REPORT	PE OF APPROVAL SOUGHT: A / FINANCIAL GUARANTEE RELEASE RELIMINARY PLAT APPROVAL DEV. PLAN FOR SUB'D. APPROVAL DEV. PLAN FOR BLDG. PERMIT APPROVAL ECTOR PLAN APPROVAL NAL PLAT APPROVAL DUNDATION PERMIT APPROVAL ERTIFICATE OF OCCUPANCY (PERM.) ERTIFICATE OF OCCUPANCY (TEMP.) RADING PERMIT APPROVAL AVING PERMIT APPROVAL ORK ORDER APPROVAL THER (SPECIFY) Drainage Master Plan Approval
WAS A PRE-DESIGN CONFERENCE ATTENDED: HYDROLOGY SECTION YES NO COPY PROVIDED	)N_
DATE SUBMITTED:BY:	

Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a drainage submittal. The particular nature, location and scope of the proposed development defines the degree of drainage detail. One or more of the following levels of submittal may be required based on the following:

- Conceptual Grading and Drainage Plan: Required for approval of Site Development Plans greater than five (5) acres and Sector Plans.
- 2. Drainage Plans: Required for building permits, grading permits, paving permits and site plans less than five (5) acres.
- 3. Drainage Report: Required for subdivisions containing more than ten (10) lots or constituting five (5) acres or more.

# CLIENT/COURIER TRANSMITTAL

CHANNAN HUSTON Ourtyard One 500 JEFFERSON NE	City F City of 600 2	S Montoya loodplain Admini f Albuquerque – nd Street NW - 2 e: 924-3982	Hydrology	Date			enn Broughto 30, 2002 This A.M. Rush	2 M.		
buquerque EW MEXICO 87109 ice 505.823.1000	Job No.: 02 (	0166 001		Job	Name:	Chelw	ood Elementa	ary Sc	hool	
x 505.798.7988		Federal UPS	Express		ltem:	PICK	PICK UP			
	ITEM NO.  1	QUANTITY  1	DESCRIPTION  Revised "Master Dated May 30, 200  Copy of Wilson & Playground Improve	)2. Co. "Dr	ainage R	eport fo	or Chelwood	•		
			ordance	e with you	ur com	nents. A wo				
	REC'D BY:				DATE:		TIME:			

ENGINEERS PLANNERS PHOTOGRAMMETRISTS SURVEYORS SOFTWARE DEVELOPERS

# MASTER DRAINAGE PLAN FOR CHELWOOD ELEMENTARY SCHOOL

**BOHANNAN HUSTON** 

Courtyard One

7500 JEFFERSON NE

Albuquerque

**NEW MEXICO 87109** 

voice 505.823.1000

fax 505.821.0892

MAY 30, 2002

PREPARED FOR:

ALBUQUERQUE PUBLIC SCHOOLS
725 UNIVERSITY BOULEVARD SE
ALBUQUERQUE, NM 87125

PREPARED BY:

BOHANNAN HUSTON, INC.
COURTYARD I
7500 JEFFERSON STREET NE
ALBUQUERQUE, NM 87109

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# MASTER DRAINAGE PLAN **FOR** CHELWOOD ELEMENTARY SCHOOL

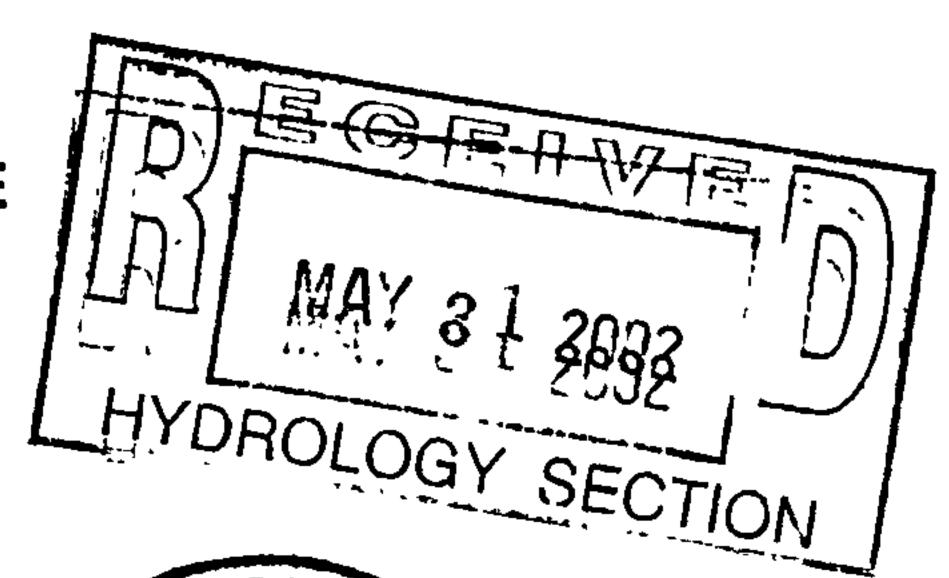
MAY 30, 2002

Prepared for:

ALBUQUERQUE PUBLIC SCHOOLS 725 UNIVERSITY BOULEVARD SE **ALBUQUERQUE, NM 87125** 

Prepared by:

BOHANNAN HUSTON, INC. **COURTYARD I** 7500 JEFFERSON STREET NE **ALBUQUERQUE, NM 87109** 



PREPARED BY:

Glenn Broughton, P.E. Date

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# I. PURPOSE AND BACKGROUND

The purpose of this report is to present the existing and future drainage conditions of Chelwood Elementary School. Improvements to Chelwood Elementary School campus are planned to occur in two phases. The interim, or Phase I, improvements include construction of new buildings and a portable park, and parking lot improvements. Phase I improvements will be staged to provide adequate facilities during construction. A portable park will be set up for classrooms adjacent to the playground on the north side of the site. Once this is complete, construction within the main campus can commence. See Exhibits 3 and 4 for Portable Park and Phase I Grading and Drainage Plans.

The ultimate development plans for the campus include an additional building and parking lot improvements. The drainage basin map (see Exhibit 2) shows the ultimate developed condition. This plan is conceptual, and some reconfiguration may occur, but no significant changes to the plan are anticipated.

The Master Drainage Plan is submitted in support of building permit approval for the portable park and Phase I improvements, as well as master plan approval for the ultimate developed condition.

# II. METHODOLOGIES

Site conditions were analyzed for a 100-year, 6-hour storm event in accordance with the City of Albuquerque Drainage Ordinance and the Development Process Manual (DPM) Volume 2, Design Criteria, Section 22.2, Hydrology, for the City of Albuquerque, January 1993.

Part A of the DPM, Section 22.2, Hydrology, January 1993, provides a simplified procedure for projects with sub-basins smaller than 40 acres. The site as described in the "Site Location and Characteristics" section below has sub-basins significantly less than 40 acres. No upland drainage basins enter the site.

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Information regarding existing on-site storm drains was obtained from the Grading and Drainage Plan and Drainage Report for Chelwood Elementary School prepared by Wilson & Co., May 2000.



This plan and report were prepared to support development of the playground and playing field area.

# III. SITE LOCATION AND CHARACTERISTICS

Chelwood Elementary School is located on the northeast corner of Constitution Avenue and Chelwood Park Boulevard. The site is bounded on the north by Eastridge Drive, and on the east by commercial development; Constitution Avenue is to the south, and Chelwood Park Boulevard is along the west side of Chelwood Elementary School. Legal description for the site is Tract K of Panorama Heights. The City of Albuquerque Zone Atlas page is J-22. The site is located within Rainfall Zone 4 as defined in the DPM Section 22.2. The combined acreage of the site described above is approximately 16.4 acres. The property is currently almost completely developed and slopes from east to west and north to south. Average slope across the developed portion of the site is approximately 4.5%.

According to the latest FEMA floodplain maps, there are no existing floodplains within the site boundaries. The closest FEMA floodplain is the Embudito Arroyo north of the site.

# IV. EXISTING DRAINAGE CONDITIONS AND HYDROLOGIC / HYDRAULIC ANALYSIS

Please refer to Exhibit 1, Drainage Basin Map – Existing Conditions, and Appendix A, Drainage Report prepared by Wilson & Co.

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The total flow generated by the site is 72.3cfs. Flows from the site are discharged to two locations. Basins 201 through 207 discharge to a detention/retention pond on the northwest corner of the campus. Runoff from these basins is conveyed to the detention basin by a storm drain and overland flow. The basin ponds to a depth of approximately 3 feet before overflowing into an outlet control structure. Flow is metered to Chelwood Park Boulevard through an 8" orifice. The onsite storm drain consists of 18" and 24" pipe. Basin 208 sheet-flows between existing buildings and discharges directly onto Constitution Avenue and Chelwood Park Boulevard. The basin data table and the basin map show the hydrologic calculations for the basins and are shown in Appendix A. A discussion of the basins follows. Drainage basins and flow calculations for existing conditions are based on the drainage report prepared by Wilson & Co. (Please note that all flows given in the text below are for the 100-year, 6-hour storm).

# Basins 201 Through 207

Basins 201 through 207 roughly include the east half of the campus, playground, play field and undeveloped area, and the northwest quarter of the campus (see Appendix A and Exhibit 1). These basins have a total area of 10.6 acres and generate a  $Q_{100}$  of 42.9cfs.

**Basin 201.** Basin 201 includes a portion of the play field and undeveloped area. The surface flows in the basin enter an 18" storm drain through a Type 'C' inlet, located along the western edge of the drainage basin. All of the area that drains to this inlet is unpaved play field and undeveloped area. This basin has an area of 0.48 Acres and generates a  $Q_{100}$  of 1.78cfs.

**Basin 202.** Basin 202 includes a portion of the play field and undeveloped area. The surface flows in the basin enter an 18" storm drain through a Type 'C' inlet, located along the western edge of the drainage basin. All of the area that drains to this inlet is unpaved play field and undeveloped area. This basin has an area of 3.34 Acres and generates a  $Q_{100}$  of 12.45cfs.

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**Basin 203.** Basin 203 includes a portion of the playground, play field and undeveloped area. The play field consists of asphalt paved walkways raised planters and raised playground areas. The raised areas detain all of the precipitation that falls in those areas. The surface flows in this basin sheet flows to the west into drainage basin 207 that will be discussed later in this section. This basin has an area of 1.55 Acres and generates a Q<sub>100</sub> of 6.29cfs.

**Basin 204.** Basin 204 includes a portion of the playground and undeveloped area. Approximately half of this drainage basin consists of the playground area. The surface flows in this basin drain flow to a Single 'D' inlet and the 18" storm drain referenced above. This basin has an area of 0.95 Acres and generates a Q<sub>100</sub> of 4.13cfs.

**Basin 205.** Basin 205 includes a portion of the playground and undeveloped area. This basin is primarily in the undeveloped area with a small portion of the playground contributing to the drainage area. The surface flows in this basin drain flow to a Single 'D' inlet and the 18" storm drain referenced above. The storm drain increases to a 24" pipe downstream of this inlet. This basin has an area of 1.78 Acres and generates a Q<sub>100</sub> of 6.68cfs.

Basin 206. Basin 206 is all within the playground area. This basin has asphalt paving over the entire area except several small raised planters. The surface flows in this basin drain flow to a Single 'D' inlet in sump condition and the 24" storm drain referenced above. The 24" storm drain flows west from this inlet to a detention/ retention pond along the western boundary of the property. This basin has an area of 0.42 Acres and generates a Q<sub>100</sub> of 2.19cfs.

**Basin 207**. Basin 207 is all within an area that is presently undeveloped. The surface flows in this basin drain sheet flow to the west into the detention pond with some flows entering Chelwood Park Boulevard directly. This basin has an area of 2.08 acres and generates a  $Q_{100}$  of 9.34cfs.

**Basin 208**. Basin 208 is a separate drainage basin within this site. It is located in the southwestern portion of the campus. This area consists of classroom buildings, parking area and landscaping. Drainage from this basin flows southwest in asphalt lined swales and sheet flows to the corner of Chelwood Park Boulevard and Constitution Avenue. Two inlets are located behind the sidewalk. They appear to be Single 'D' inlets in a shallow sump. This basin has an area of 5.77 Acres and generates a Q<sub>100</sub> of 29.4cfs.

Retention / Detention Pond. The pond mentioned earlier has a concrete outlet control structure. The pond must reach of depth of approximately 3 feet before overflowing into the outlet structure. An 8" orifice plate is used to meter the flow out of the pond. Storm water discharges from the pond into a asphalt swale and to Chelwood Park Boulevard. Chelwood Master Drainage Report prepared by Wilson & Co. noted that the available capacity of the pond is 0.85 Ac-ft and the outlet structure will permit a controlled maximum discharge of 20cfs. A field review of the site showed that there was no emergency overflow provided for this pond.

Impact to Public Facilities. Drainage from Basin 208 enters the storm drain in Chelwood Park Boulevard via existing inlets behind the sidewalk at the northeast corner of Chelwood Park Boulevard and Constitution Avenue and existing inlets in Chelwood Park Boulevard. The inlets behind the sidewalk do not have adequate capacity for the 100-year storm, however in large storm events storm water will bypass these inlets and be captured downstream. Drainage for this basin does not have any obvious detrimental impacts to public facilities.

Storm water discharge from the pond passes through the outlet structure discussed above and then surface-flows into Chelwood Park Boulevard. Inlets downstream convey this drainage to the storm drain in Chelwood Park Boulevard. While this is not an ideal condition it does not significantly impact any existing public facilities. The pond embankment could be a potential hazard if the outlet structure were to become plugged. If the pond embankment were to be breached a surge of water and sediment would enter

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Chelwood Park Boulevard. It is recommended that and emergency spillway be added with future phases of the campus improvements.



# V. INTERIM AND ULTIMATE DEVELOPMENT DRAINAGE CONDITIONS AND HYDROLOGIC ANALYSIS

Please refer to Exhibit 2, Ultimate Development Drainage Basin Map, and Appendices B and C.

For purposes of this analysis, the site has been divided into four drainage basins: A, B, C and D. The drainage basins for the interim and ultimate development conditions are nearly identical, the only difference being that the land treatments will change with future improvements to the site. Basins A, B and C drain to the detention pond located in the northwest corner of the site. Drainage Basin D drains southwest to Chelwood Park Boulevard and Constitution Avenue. A discussion of the basins follows. (Please note that all flows given in the text below are for the 100-year, 6-hour storm).

**Basin A.** Basin A encompasses the southeast quadrant of the campus and includes the playfield and a portion of the undeveloped area of the campus. Approximately 1.65 acres of new sod has recently been placed on the playfield. This change in the land treatment will reduce the peak discharge for this basin. No changes in the land treatment for this basin are anticipated between the interim and ultimate development conditions. This basin has an area of 5.54 acres and generates a Q<sub>100</sub> of 19.32cfs.

**Basin B.** Basin B encompasses the northeast quadrant of the campus and includes the playground and a portion of the undeveloped area of the campus. No changes in the land treatment for this basin are anticipated between the interim and ultimate development conditions. This basin has an area of 4.49 acres and generates a  $Q_{100}$  of 17.93cfs.

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**Basin C.** Basin C includes the north portion of the main campus. The interim (Phase 1) improvements include three new buildings, new and relocated modular buildings, and new paved parking lot. In the ultimate development, additional parking lot improvements are planned. This basin has an area of 3.48 acres and generates a  $Q_{100}$  of 14.07cfs in the interim developed conditions and  $Q_{100} = 15.69$ cfs for the ultimate developed condition.

**Basin D.** Basin D includes the south portion of the main campus. Basin D drains southwest to two existing inlets behind the sidewalk and inlets in Chelwood Park Boulevard. The interim (Phase 1) improvements remain unchanged from the current existing conditions. In the ultimate development, an additional building is proposed, and reconfiguration of the existing parking lot is planned. This basin has an area of 4.30 acres and generates a  $Q_{100}$  of 18.57cfs in the interim developed conditions and  $Q_{100}$  = 19.13cfs for the ultimate developed condition. Basin D freely discharges into Constitution Avenue and Chelwood Park Boulevard and ultimately into the Chelwood Park storm drain.

**Detention Pond.** The Wilson Drainage report referenced stated that the detention pond controlled discharge would be limited to 20cfs and drainage basin 208 freely discharges 29.4cfs, for a total of 49.4cfs.

As described in the existing conditions section of this report, the existing pond has a concrete outlet structure and 8" orifice outlet. This causes the pond to act as a retention / detention pond. An analysis of the existing outlet was performed, and the results show that an outlet flow of 3.6cfs is controlled by the orifice plate. Removal of the orifice plate will increase the discharge to 4.8cfs. This increase is not sufficient to meet the allowable discharge requirements.

Analysis of the interim conditions showed that basin D would freely discharge 18.6cfs onto Chelwood Park Boulevard and Constitution Avenue, which will allow a discharge of 30.8cfs (49.4cfs-18.6cfs) from the detention pond. In the ultimate developed condition,

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Basin D will discharge 19.1cfs, which would reduce the allowable discharge to a discharge of 30.3cfs (49.4cfs-19.1cfs) from the detention pond.

Modification of the existing outlet structure is proposed. The existing outlet structure will be abandoned and replaced with a Double 'D' inlet and a 24" RCP outlet pipe. The inlet will be set 0.5 feet above the pond invert to allow sediment to settle prior to discharge onto Chelwood Park Boulevard. A new manhole will be constructed in Chelwood Park Boulevard to connect the proposed 24" detention pond outlet pipe to the existing 30" storm drain (see exhibits 4 and 5).

An analysis was performed on the proposed outlet structure. The results showed that the outlet pipe controlled the discharge flow rate. This outlet pipe was analyzed as an orifice. In order to reduce pond discharge below the allowable flow rate, a 21"-diameter orifice plate will be installed. A total available head of 5.1 feet (to the center of the orifice plate) was calculated using a maximum high water elevation of 5714. The inlet invert elevation was established by matching the soffit of the existing 30" RCP storm drain in Chelwood Park Boulevard. Maximum discharge was calculated to be 27.0cfs. Based on a maximum allowable discharge of 27.0cfs, the detention pond volume requirements were calculated. For the interim development, the required volume is 0.46 acre-feet, and 0.54 acre-feet is required for the ultimate developed condition. The existing pond volume was calculated to be 0.54 acre-feet. This was based on a recent topographic survey of the site.

Volume of the existing detention pond is adequate to meet the needs of the interim and ultimate improvements. Future parking lot improvements will encroach into the existing pond. Reconfiguration of the detention pond can be completed to accommodate parking lot improvements for the ultimate developed condition.

Discharge / Detention Volume Summary										
	Q allowable (cfs)	Q actual (cfs)	V required * Acre-ft	V provided Acre-ft						
Existing	20.0	3.6	1.33	0.54						
Interim Development (Phase I)	30.8	27.0	0.46	0.54						
Ultimate Development	30.3	27.0	0.54	0.54 **						

Maximum water surface elevation 5714.0.

- \* In accordance with Part A of the DPM Section 22.2 and Wilson & Co. drainage basin calculations.
- \*\* Detention Pond Volume will need to be reconfigured with the ultimate development of the campus.

Other recommended modifications include providing an emergency spillway in the event that the inlet becomes plugged. Elevation of the spillway will be set at 5714.0, which is the high water elevation required to achieve the design detention basin discharge. Width of the weir will need to be approximately 10' to convey the peak flow rate of 52.9cfs (combined peak flow for basins A, B, and C). Erosion protection will be provided by placing riprap over a non-woven filter fabric should the proposed inlet become plugged (see Exhibits 4 and 5 for details of the spillway).

# V. CONCLUSION

The subject site is draining to two primary discharge points: Basins A, B, and C drain to the existing detention basin adjacent to Chelwood Park Boulevard; Basin D discharges onto Chelwood Park Boulevard and Constitution Avenue.

The overall site drainage conditions at Chelwood Elementary School generally appear to be good. There are two areas where erosion of slopes is evident: along the east side of the playground and play field there is a cut slope that has some erosion; and the east slope of the

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detention pond has some erosion. This is shown in Pictures 2 and 10. This erosion could be controlled with slope protection at the detention pond and a drainage swale along the top of the slope east of the play field area (see Exhibits 4 and 5).



# Phase I Improvements

- New detention pond outlet structure; Double 'D' inlet, 24" RCP outlet pipe, 22" orifice plate, and manhole to connect to the existing 30" storm drain.
- Construct emergency spillway.
- Slope protection/erosion control at the east playground cut slope and detention pond.

# <u>Ultimate Development Improvements</u>

Reconfigure and increase volume of detention pond.

# APPENDICES

APPENDIX A - DRAINAGE REPORT PREPARED BY WILSON & CO.

APPENDIX B - PROPOSED HYDROLOGIC CALCULATIONS

APPENDIX C - POND VOLUME AND DISCHARGE CALCULATIONS

# APPENDIXA

DRAINAGE REPORT PREPARED BY WILSON & CO.

### DRAINAGE REPORT

<u>Site Location</u>: Chelwood Middle School is located at the corner of Constitution Ave. and Chelwood Park Blvd. The proposed development includes sandboxes, asphalt paving, addition of basketball courts, a sod playing field and miscellaneous concrete work.

Methodology: Section 22.2 of the City of Albuquerque DPM was used in the hydrology analysis of the site. A principal design storm of 100-yr, 10-day event was used.

Existing Drainage Condition: The site consists of two basins; Basin 101 - South and Basin 102 - North. Basin 101 encompasses 8.0 acres on the southern portion of the site and Basin 102 encompasses 8.4 acres on the northern portion of the site. Basin 101 is developed with buildings, a parking lot on the southwestern end and an undeveloped playground on the East Side. Basin 102 is developed with playground equipment on the western half and undeveloped on the eastern half.

Currently, all flows from Basin 101 are conveyed westward adjacent to the buildings and through the parking lots. Runoff is freely discharged (35.89 cfs) onto Chelwood Park Blvd and Constitution Ave. The flows on basin 102 are also conveyed westward. Approximately 33.82 cfs is freely discharged though the drive pad located on the westside of the site and onto Chelwood Park Blvd. Table 1 provides a breakdown of existing volumetric runoff and peak discharge.

	Table 1 - Exiting Conditions											
	Treatment											
Basin Area (ac) A B C D V <sub>1440</sub> (ac-ft) Q <sub>100</sub>												
101	7.993	0.00%	0.00%	50.00%	50.00%	1.365	35.888					
102	8.383	0.00%	0.00%	80.00%	20.00%	1.185	33.815					
Total	16.376					2.55	69.703					

Table 1: Breakdown of existing land type for each the basin including its respective volumetric runoff and discharge value.

<u>Proposed Conditions</u>: The proposed conditions will separate the site in to eight basins. Basin 201 is located on the southeast corner of the site. The flows are conveyed westward discharging into an inlet. Flows are conveyed via a 24" SD mainline and discharged into a proposed detention pond, located on the northwest side of the school.

Basin 202 and Basin 203 are located north of Basin 201. Flows sheet flow westward, collected and conveyed by a median curb and gutter to the SD mainline and discharged into the proposed pond.

Basin 204 encompasses the northeast portion of the property. Flows sheet flow westward, first over natural ground then over the proposed playground, into the SD mainline and ultimately discharging into the proposed pond.

Basin 205 encompasses the northeast portion of the property. Flows sheet flow westward over natural ground to the edge of the proposed playground, into the SD mainline and ultimately discharging into the proposed pond.

Basin 206, the western portion of the playground, is located west of Basin 204 and Basin 205. Flows sheet flow northwestward into the SD mainline and ultimately discharging into the proposed pond.

Basin 207 encompasses the western portion of the property. Flows sheet flow westward over the basin, discharging directly into the detention pond or onto Chelwood Park Dr.

The allowable capacity of the detention pond is 0.85 acre-ft, which is more than adequate for the required capacity of 0.50 acre-ft. The detention pond will have a headwall that will permit a controlled maximum discharge of 20 cfs on to Chelwood Park Dr.

Basin 208 is located on the southwest corner of the site. The proposed development will not effect this basin. Basin 208 discharges directly to the street and is not detained by the pond.

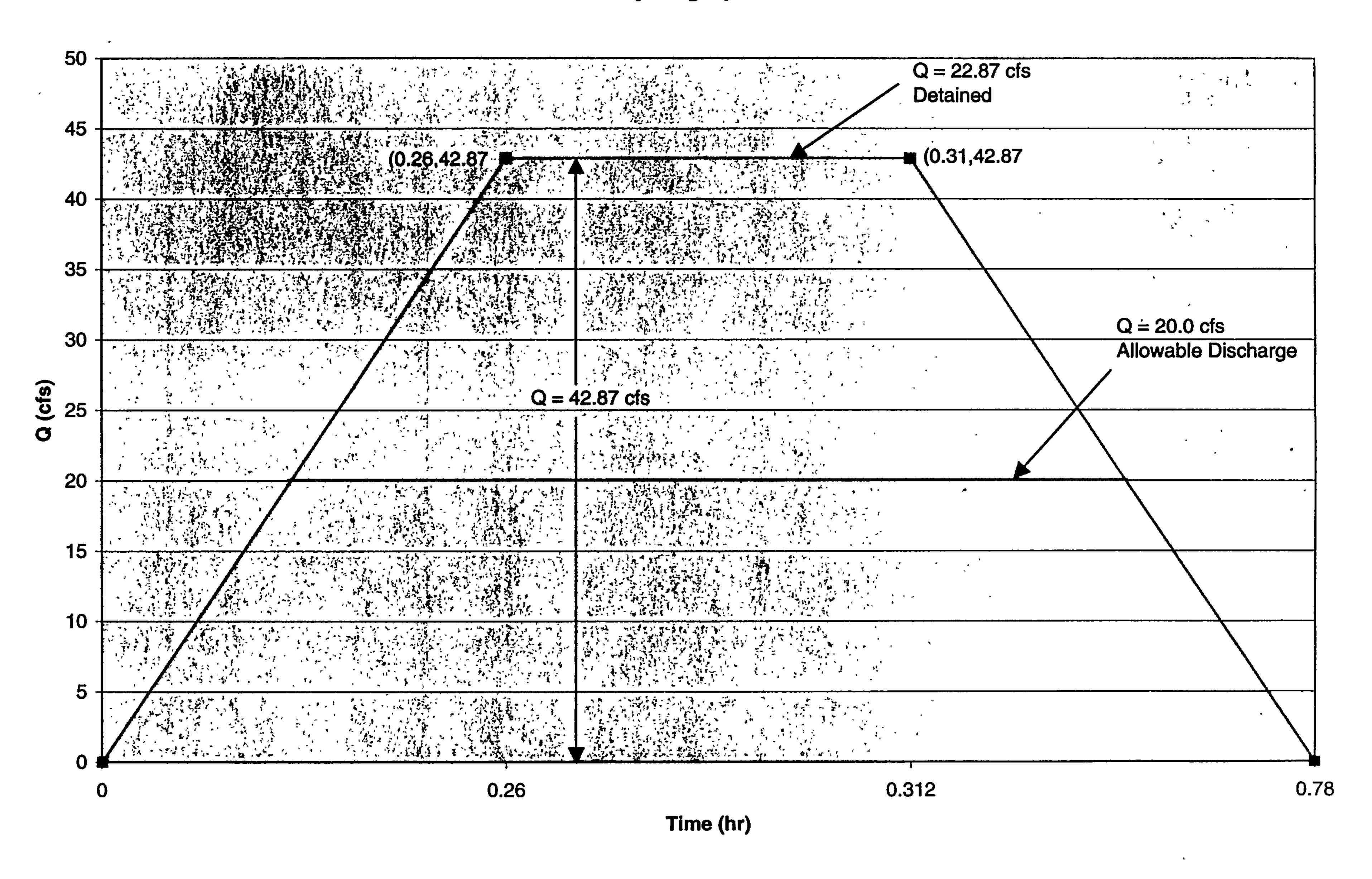
Table 2 provides a breakdown of existing volumetric runoff and peak discharge.

	Table 2 - Proposed Conditions												
	Treatment												
Basin	Area (ac)	Α	В	C	D	V <sub>1440</sub> (ac-ft)	Q <sub>100</sub> (cfs)						
201	0.477	0.00%	0.00%	100.00%	0.00%	0.058	1.779						
202	3.339	0.00%	0.00%	100.00%	0.00%	0.406	12.453						
203	1.552	0.00%	0.00%	78.60%	21.40%	0.222	6.294						
204	0.954	0.00%	0.00%	66.66%	33.33%	0.153	4.134						
205	1.781	0.00%	0.00%	98.00%	2.00%	0.219	6.676						
206	0.418	0.00%	0.00%	0.00%	100.00%	0.092	2.194						
207	2.082	0.00%	0.00%	50.00%	50.00%	0.355	9.338						
208	5.773	0.00%	0.00%	10.00%	90.00%	1.213	29.431						
Total	16.376					2.718	72.299						

Table 2: Breakdown of proposed land types for each basin including their respective volumetric runoff and discharge values.

Conclusions: The proposed development will not have a significant impact on the total volumetric runoff (increase of 0.17 ac-ft) and the total flow rate (increase of 2.6 cfs) when compared to the existing conditions. However, there is an improvement on the allowable discharged onto Chelwood Park Blvd. and Constitution Ave. The allowable discharge onto Constitution Ave. will decrease because the runoff from Basin 101 (specifically Basin 201 and a portion of Basin 202) will be diverted to Basin 102, thus allowing less runoff onto Constitution Ave. The other positive aspect of this proposed development is the controlled discharge onto Chelwood Park Blvd. of 20 cfs due to the headwall within the pond, rather then the uncontrolled 33 cfs. The pond will detain the flows from all basins except for basin 208 which will continue to have free discharge to the adjacent streets.

# Hydrograph



# APPENDIXB

PROPOSED HYDROLOGIC CALCULATIONS

# Chelwood Elementary School Existing Detention Pond Outlet Orifice

# Max water into end of pipe

Using Orifice Equation, Coefficien 0.62

(head measeured center of pipe to water surface)

			P	ipe Size	
Head	8	10	12	18	24
0.25	0.87			-···-	
0.5	1.23	1.92	2.76		
0.75	1.50	2.35	3.38	7.61	
1	1.74	2.71	3.91	8.79	15.63
1.25	1.94	3.03	4.37	9.83	17.48
1.5	2.13	3.32	4.79	10.77	19.14
1.75	2.30	3.59	5.17	11.63	20.68
2	2.46	3.84	5.53	12.43	22.11
2.25	2.61	4.07	5.86	13.19	23.45
2.5	2.75	4.29	6.18	13.90	24.71
2.75	2.88	4.50	6.48	14.58	25.92
3	3.01	4.70	6.77	15.23	27.07
3.25	3.13	4.89	7.04	15.85	28.18
3.5	3.25	5.08	7.31	16.45	29.24
3.75	3.36	5.26	7.57	17.03	30.27
4	3.47	5.43	7.82	17.58	31.26
4.25	3.5	5.59	8.06	18.13	32.22
4.5	3.68	5.76	8.29	18.65	33.16
4.75	3.79	5.91	8.52	19.16	34.07

NOTE: THIS IS AN ANALYSIS OF THE EXISTING 8" ORIFICE PLATE.

# Chelwood Elementary School Existing Detention Pond Outlet Pipe

Drainage Structure Analyzer

Culvert Hydraulic Analysis

Date: Tuesday, March 12, 2002 04:59:53 PM

\_\_\_\_\_\_\_

# Input Data

Shape .	Circular
Material	CMP-12G
Roughness	0.022000
Entrance Edge	Headwall
Number of Barrels	1
Length	60.00 ft
Slope	2.500%
Tailwater	0.00 ft
Inlet Control Equation	Entrance Loss
Size (W x T):	12.00 x 0.1090
Headwater	3.50 ft

# Output Results

Flow Rate	4.8	cfs
Control	Outlet	
Capacity	3.3	cfs
Outlet Velocity	. 6.27	ft/s
Depth At Outlet	0.95	ft
Headwater	3.50	ft
Critical Depth	0.90	ft
Normal Depth	1.00	ft
Size (W x T):	12.00 x 0.1090	

NOTE: THIS IS AN ANALYSIS OF THE EXISTING 12" DETENTION POND OUTLET PIPE. A CULVERT ANALYSIS WAS PERFORMED TO ESTIMATE THE CULVERT CAPACITY.

P:10201661cdp\hydro\EX 12in outlet.doc Appendix B Sheet 2 of 7

	•	· · · · · · · · · · · · · · · · · · ·	CHI	ELWOOD I	ELEMENT	ARY SCH	OOL			
		DE	ETENTION	POND EX	ISTING OU	JTLET CA	LCULATIC	NS		•
	48" x 48" cc	ncrete inlet	structure	Detention P	ond Depth =	4'				
	Allowable D	ischarge = 3	30.3 cfs	Outlet Struc	ture Height	= 3'				
	Open Area	(for orifice ca	alc in sq. ft.):	16.15625		<u> </u>				
	Length of W	/eir (feet):		17.875						
Llood	l-lood	Mais O	Orifico	Control	<u> </u>	<u> </u>	Coloulation	of open organ	•	
Head	Head	Weir Q	Orifice Q	Control Q	<u> </u>	<del>                                     </del>	Calculation	of open area:		
(ft) 0.05	(in) 0.6	(cfs) 0.54	(cfs) 17.39	(cfs) 0.54			Total Grate	Area	2304	16
0.03 0 1	1.2	1.51	24.60	1.51		<u></u>	Cross Bar A	<del></del>	22.5	0.15625
0.15	1.8	2.78	30.13	2.78	<u> </u>	<u> </u>	Cioss Dai F	u Ca	22.0	0.10020
0.13	2.4	4.28	34.79	4.28					2326.5	16.15625
0.25	3	5.99	38.90	5.99		<u> </u>				
0.3	3.6	7.87	42.61	7.87	<u> </u>				<del></del>	
0.35	4.2	9.92	46.02	9.92					<del></del>	<del></del>
0.4	4.8	12.12	49.20	12.12						
0.45	5.4	14.46	52.18	14.46		<u> </u>	Calculation	of Length of V	Veir:	
0.5	6	16.94	55.01	16.94		<del></del>				
0.55	6.6	19.54	57.69	19.54			Total Perim	eter of Grate	192	16
0.6	7.2	22.26	60.26	22.26			Cross Bars		22.5	1.875
0.65	7.8	25.10	62.72	25.10						
0.7	8.4	28.06	65.09	28.06					214.5	17.875
0.75	9	31.12	67.37	31.12						<del></del>
0.8	9.6	34.28	69.58	34.28	NOTE: THIS	S IS AN ANA	ALYSIS OF T	HE EXISTING	CONCRET	E
0.85	10.2	37.54	71.72	ļ		· <u>· · · · · · · · · · · · · · · · · · </u>		RE. 0.75' OF H		
0.9	10.8	40.90	73.80	40.90	BE DEVELO	OPED TO A	CHIEVE THE	ALLOWABLI	E DISCHAR	GE
0.95	11.4	44.36	75.82	44.36						<del> </del>
1	12	47.91	77.79	47.91				<u> </u>		

				CHELWO	OD ELEME	NTARY BA	ISIN CALCU	LATIONS			
		<del></del>		In	terim Developm	ent Condition	s Basin Data Tab	e		<del></del>	
	•	This table i	is based o	n the DPM Sect	ion 22.2, Zone:	4					
BASIN	Area	Area		Land Treatm	nent Percentag	ges	Q(100)	Q(100)	WTE	V(100) <sub>360</sub>	V(100) <sub>1440</sub>
	(SQ. FT)	(AC.)	Α	В	C	D	(cfs/ac.)	(CFS)	(inches)	(CF)	(CF)
A	241322	5.54	0.0%	0.0%	100.0%	0.0%	3.73	20.66	1.46	29361	29361
В	195584	4.49	0.0%	0.0%	100.0%	0.0%	3.73	16.75	1.46	23796	23796
C	151589	3.48	0.0%	0.0%	78.6%	21.4%	4.06	14.11	1.71	21633	23904
D	187308	4.30	0.0%	0.0%	66.7%	33.3%	4.24	18.22	1.85	28928	33298
•	Total =	17.81					Total =	69.74		Total =	110359

<u></u>		. <u> </u>		Ult	imate Developi	ment Condition	ns Basin Data Tab	le	· · · · · · · · · · · · · · · · · · ·		
		This table	is based or	n the DPM Sect	ion 22.2, Zone:	4					
BASIN	Area	Area		Land Treatm	ent Percenta	ges	Q(100)	Q(100)	WTE	$V(100)_{360}$	V(100) <sub>144</sub>
	(SQ. FT)	(AC.)	Α	В	С	D	(cfs/ac.)	(CFS)	(inches)	(CF)	(CF)
Α	241322	5.54	0.0%	30.0%	70.0%	0.0%	3.49	19.32	1.35	27068	27068
В	195584	4.49	0.0%	22.0%	49.0%	29.0%	3.99	17.93	1.72	28011	31981
C	151589	3.48	0.0%	9.0%	35.0%	56.0%	4.51	15.69	2.09	26359	32301
, D	187308	4.30	0.0%	7.0%	42.0%	51.0%	4.45	19.13	2.04	31767	38454
•	Total =	17.81					Total =	72.06		Total =	129805

CHELWOOD	ELEMENTA	ARY DETEN	ITION PONE	OUTLET	RIFICE
		•			
Max water	into end c	of pipe			
Using Orifice					
(head mease	ured center	of pipe to wa	ater surface)		
			Pipe	Size	
Head	8	12	18	21	24
3.1					
3.35	3.18	7.15			
3.6	3.30	7.41	16.68		
3.85	3.41	7.67	17.25	23.48	
4.1	3.52	7.91	17.80	24.23	31.65
4.35	3.62	8.15	18.34	24.96	32.60
4.6	3.72	8.38	18.86	25.67	33.52
4.85	3.82	8.61	19.36	26.36	34.42
5.1	3.92	8.82	19.86	27.03	35.30
5.35	4.02	9.04	20.34	27.68	36.15
5.6	4.11	9.25	20.81	28.32	36.99
5.85	4.20	9.45	21.27	28.95	37.81
6.1	4.29	9.65	21.72	29.56	38.61
6.35	4.38	9.85	22.16	30.16	39.39
6.6	4.46	10.04	22.59	30.74	40.16
6.85	4.55	10.23	23.01	31.32	40.91
7.1	4.63	10.41	23.43	31.89	41.65
7.35	4.71	10.59	23.84	32.44	42.38
7.6	4.79	10.77	24.24	32.99	43.09

ouble D in	et. in sump	condition wit	h short edge	s adjoined:				
	والمراقب والمراجع والمناسب والمساولة والمراجع والمراجع والمراجع والمراجع والمراجع والمراجع والمراجع والمراجع			7.7977431			- <del></del>	<del></del>
	Length of W	<del></del>		13.645833				<del></del>
		<u> </u>						<del></del>
Head	Head	Weir Q	Orifice Q	Control Q	Calculation	of open area:		, <del></del>
(ft)	(in)	(cfs)	(cfs)	(cfs)				
0.1	1.2	1.16	11.87	1.16	Total Grate	Area	2000	13.8888
0.35	4.2	7.57	22.21	7.57	Cross Bar A	∖rea	-732	-5.0833
0.4	4.8	9.25	23.75	9.25	Supports (e	nds)	-115.625	-0.8029
0.45	5.4	11.04	25.19	11.04		(middle)	-100	-0.6944
0.5	6	12.93	26.55	12.93	Areas Cour	nted Twice	70.5	0.48958
0.55	6.6	14.92	27.84	14.92			1122.875	7.79774
0.6	7.2	17.00	29.08	17.00				
0.65	7.8	19.16	30.27	19.16				
0.7	8.4	21.42	31.41	21.42	Calculation	of Length of \	Veir:	
0.75	9	23.75	32.52	23.75				
0.8	9.6	26.17	33.58	26.17	Total Perim	eter of Grate	210	17
0.85	10.2	28.66	34.62	28.66	Short Cross	Bars	-14	-1.1666
0.9	10.8	31.22	25.62	31,22	Bearing Ba	rs	-13	-1.0833
0.95	11.4	33.86	36.60	33.86	End Suppor	rts	-9.25	-0.7708
1	12	36.57	37.55	36.57	Middle Sup	ports	<u>-10</u>	-0.8333
1.05	12.6	39.35	38.47	38.47			183	13.6458
1.1	13.2	42.19	39.38	39.38				
1.15	13.8	45.10	40.26	40.26				
1.2	14.4	48.07	41.13	41.13				
1.25	15	51.11	41.98	41.98				
1.3	15.6	54.21	42.81	42.81				
1.35	16.2	57.36	43.62	43.62				
1.4	16.8	60.58	44.42	44.42				
1.45	17.4	63.85	45.21	45.21				
OTE: THIS	S IS AN ANA	LYSIS OF A	DOUBLE 'D	'INLET IN A SUM	P CONDITION.			
	<del></del>	· · · · · · · · · · · · · · · · · · ·		ALLOWABLE DIS		<del>   </del>	· · · · · · · · · · · · · · · · · · ·	

# Chelwood Elementary School Detention Pond Spillway

WEIR COEFFICENT=2.6400

POINT	DIST	ELEV	POINT	DIST	ELEV	POINT	DIST	ELEV
1	.00	1.50	3	14.50	.00			
2	4.50	.00	4	19.00	1.50			

WSEL	DEPTH	FLOW AREA	FLOW RATE	FLOW VEL	TOPWID PLUS
FT.	FT.	SQ.FT.	(CFS)	(FPS)	OBSTRUCTIONS
				•	
.10	.10	1.03	.85	.83	10.60
.20	.20	2.12	2.46	1.16	11.20
.30	.30	3.27	4.63	1.42	11.80
.40	.40	4.48	7.29	1.63	12.40
.50	.50	5.75	10.41	1.81	13.00
.60	.60	7.08	13.99	1.98	13.60
.70	.70	8.47	18.00	2.13	14.20
.80	.80	9.92	22.45	2.26	14.80
.90	.90	11.43	27.34	2.39	15.40
1.00	1.00	13.00	32.66	2.51	16.00
1.10	1.10	14.63	38.42	2.63	16.60
1.20	1.20	16.32	44.61	2.73	17.20
1.30	1.30	18.07	51.25	2.84	17.80
1.40	1.40	19.88	58.33	2.93	18.40
1.50	1.50	21.75	65.86	3.03	19.00

NOTE: THIS WEIR ANALYSIS DEMONSTRATES THAT 1.40' OF HEAD IS REQUIRED TO PASS THE TOTAL PEAK FLOW FOR A 100 YEAR, 6-HOUR STORM EVENT. THIS ASSUMES THE OUTLET STRUCTURE BECOMES COMPLETELY CLOGGED.

# APPENDIX C

POND VOLUME AND DISCHARGE CALCULATIONS

# **Chelwood Elementary School**

# <u>Detention Pond Volume Calculations - Existing Conditions</u>

## **ASSUMPTIONS:**

- 1. Area less than 40 acres (simplified hydrograph method).
- 2. 100-year, 6-hour storm event
- 3. Volume Calulation Based on Oulet Structure Actual Allowable Discharge

# Peak Flow per Acre - DPM Section 22.2 Table A-9

Zone	Α	В	С	D
1	1.29	2.03	2.87	4.37
2	1.56	2.28	3.14	4.7
3	1.87	2.6	3.45	5.02
4	2.2	2.92	3.73	5.25

Basin Name: Chelwood Elementary School

Choose Zone (1 - 4) 🔭 🤫 4 🦠

Basin Area = (acres) 10.61

Existing Conditions									
Treatment	Percentage	Area	Q (cfs)						
A	0.0%	0.00	0.00						
В	0.0%	0.00	0.00						
С	79.8%	8.47	31.58						
D	D 20.2% 2.14 <u>11.25</u>								
	Peak Q Developed= 42.83								

\*Values based on Wilson & Co. Drainage Report for Basins 201 through 207

Use my calculated exist cond. flow as the peak controlled discharge (1 = yes, or N)??

If No, what is the maximum allowable discharge?

# Excess Precipitation - DPM Section 22.2 Table A-8

Zone	Α	В	С	D
1	0.44	0.67	0.99	1.97
2	0.53	0.78	1.13	2.12
3	0.66	0.92	1.29	2.36
4	0.8	1.08	1.46	2.64

Determine Developed E (avg excess precipitation for the developed basin)

 $%A \times E =$ 0.00 %B x E = 0.00  $%C \times E =$ 1.17  $D \times E =$ 0.53 Avg E(in) =1.70

Determine Tb (hours)

Tb = 0.836

Determine Tc (Note: Tc is assumed to be 0.2 hours, this should be checked using DPM 22.2.B.2)

Tc = 0.2

Determine Tp and Duration of Peak (hours)

Tp = 0.2565

Peak Duration = 0.0505

Compute the required retention volume using the simple hydrograph, Figure A-3 in DPM Section 22.2

Time to Control Q (hrs)  $\doteq$  0.022 Time to end of Control Q (hrs)= 0.79145 Duration of Control Q (hrs)= 0.770

Required Detention Volume (CF) = 57935.58

# **Chelwood Elementary School**

# Detention Pond Volume Calculations - Interim Developed Conditions

## **ASSUMPTIONS:**

- 1. Area less than 40 acres (simplified hydrograph method).
- 2. 100-year, 6-hour storm event

# Peak Flow per Acre - DPM Section 22.2 Table A-9

Zone	Α	В	Ç	D
1	1.29	2.03	2.87	4.37
2	1.56	2.28	3.14	4.7
3	1.87	2.6	3.45	5.02
4	2.2	2.92	3.73	5.25

Basin Name : Chelwood Elementary School

Choose Zone (1 - 4) 数字 4 3 3 4

Basin Area = (acres) 3.51 BHI

Basin Area = (acres) 10.61 Wilson & Co.

* Exist Conditions				Proposed Conditions				
Treatment	Percentage	Area	Q (cfs)	Treatment	Percentage	Area	Q (cfs)	
Α	0.0%	0.00	0.00	Α	0.0%	0.00	0.00	
В	0.0%	0.00	0.00	В	22.7%	3.07	8.95	
С	79.8%	8.47	31.58	C	60.7%	8.20	30.59	
D	20.2%	2.14	<u>11.25</u>	D	16.6%	2.24	<u>11.77</u>	
	Q Peal	c - exist.=	<u> </u>		Peak Q De	eveloped=	51.32	

<sup>\*</sup>Values based on Wilson & Co. Drainage Report for Basins 201 through 207

Use my calculated exist cond. flow as the peak controlled discharge (1 = yes, or N)??

If No, what is the maximum allowable discharge?

# Excess Precipitation - DPM Section 22.2 Table A-8

		• •	<b>+++++</b>	,,
Zone	Α	В	C	D
1	0.44	0.67	0.99	1.97
2	0.53	0.78	1.13	2.12
3	0.66	0.92	1.29	2.36
4	0.8	1.08	1.46	2.64

# Determine Developed E (avg excess precipitation for the developed basin)

 $%A \times E =$ 0.00  $%B \times E =$ 0.25  $%C \times E =$ 0.89

 $%D \times E =$ 0.44

1.57 Avg E(in) =

# Determine Tb (hours)

Tb = 0.829

Determine Tc (Note: Tc is assumed to be 0.2 hours, this should be checked using DPM 22.2.B.2)

Tc = 0.2

# Determine Tp and Duration of Peak (hours)

Tp = 0.2595

Peak Duration = 0.0415

Compute the required retention volume using the simple hydrograph, Figure A-3 in DPM Section 22.2

Time to Control Q (hrs) = 0.137Time to end of Control Q (hrs)= 0.551277 Duration of Control Q (hrs)= 0.415

Required Detention Volume (CF) = 19970.1

### **Chelwood Elementary School**

### Detention Pond Volume Calculations - Ultimate Developed Conditions

NOTE: Blue shaded cells require user input, all other cells should not be edited. **ASSUMPTIONS:** 

- 1. Area less than 40 acres (simplified hydrograph method).
- 2. 100-year, 6-hour storm event

#### Peak Flow per Acre - DPM Section 22.2 Table A-9

Zone	Α	В	С	D
1	1.29	2.03	2.87	4.37 ·
2	1.56	2.28	3.14	4.7
3	1.87	2.6	3.45	5.02
4	2.2	2.92	3.73	5.25

Basin Name: Chelwood Elementary School

Basin Area = (acres) Basin Area = (acres)

* Exist Co	nditions			Proposed Conditions			
Treatment	Percentage	Area	Q (cfs)	Treatment	Percentage	Area	Q (cfs)
Α	0.0%	0.00	0.00	Α	0.0%	0.00	0.00
В	0.0%	0.00	0.00	В	21.9%	2.96	8.64
C	79.8%	8.47	31.58	С	54.0%	7.30	27.21
D	20,2%	2.14	<u>11.25</u>	D	24.1%	3.26	<u>17.09</u>
	Q Pea	k - exist.=	42.83		Peak Q De	eveloped=	52.94

<sup>\*</sup>Values based on Wilson & Co. Drainage Report for Basins 201 through 207

Use my calculated exist cond. flow as the peak controlled discharge (1 = yes, or N) ?? If No, what is the maximum allowable discharge?

#### Excess Precipitation - DPM Section 22.2 Table A-8

Zone	Α	В	С	D
1	0.44	0.67	0.99	1.97
2	0.53	0.78	1.13	2.12
3	0.66	0.92	1.29	2.36
4	0.8	1.08	1.46	2.64

#### Determine Developed E (avg excess precipitation for the developed basin)

 $%A \times E =$ 0.00  $%B \times E =$ 0.24  $%C \times E =$ 0.79  $D \times E =$ <u>0.64</u> Avg E(in) =1.66

Determine Tb (hours)

Tb = 0.833

Determine Tc (Note: Tc is assumed to be 0.2 hours, this should be checked using DPM 22.2.B.2)

Tc = 0.2

Determine Tp and Duration of Peak (hours)

Tp = 0.25325

Peak Duration = 0.06025

Compute the required retention volume using the simple hydrograph, Figure A-3 in DPM Section 22.2

Time to Control Q (hrs) = 0.129Time to end of Control Q (hrs)= 0.568008 Duration of Control Q (hrs)= 0.439

Required Detention Volume (CF) = 23308.7

EXHIBIT 1 -	DRAINAGE BASIN MAP - EXISTING CONDITIONS
	PREPARED BY WILSON & CO.

- EXHIBIT 2 PROPOSED CONDITIONS: ULTIMATE DEVELOPMENT DRAINAGE BASIN MAP
- EXHIBIT 3 PORTABLE PARK GRADING & DRAINAGE PLAN
- EXHIBIT 4 PHASE I GRADING & DRAINAGE PLAN
- EXHIBIT 5 GRADING DETAILS
- EXHIBIT 6 PHOTOS

DRAINAGE BASIN MAP – EXISTING CONDITIONS,
PREPARED BY WILSON & CO.

# EXHIBIT 2

PROPOSED CONDITIONS:

ULTIMATE DEVELOPMENT DRAINAGE BASIN MAP

PORTABLE PARK GRADING & DRAINAGE PLAN

PHASE I GRADING & DRAINAGE PLAN

GRADING DETAILS

PHOTOS



Photo 1 – Retention/Detention Pond Outlet Structure



Photo 3 – Playground Looking Southwest

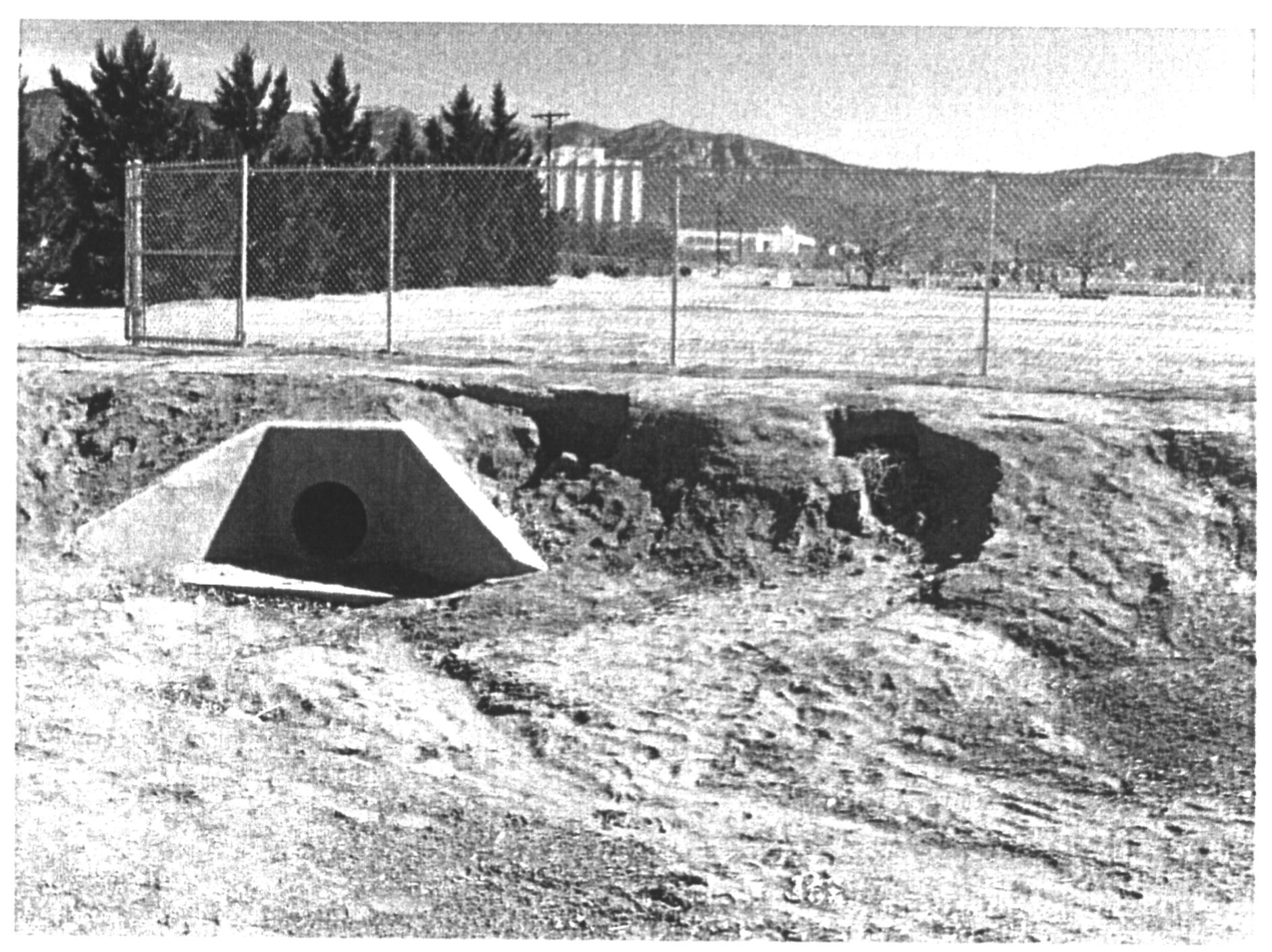


Photo 2 – 24" Storm Drain Outfall to Pond

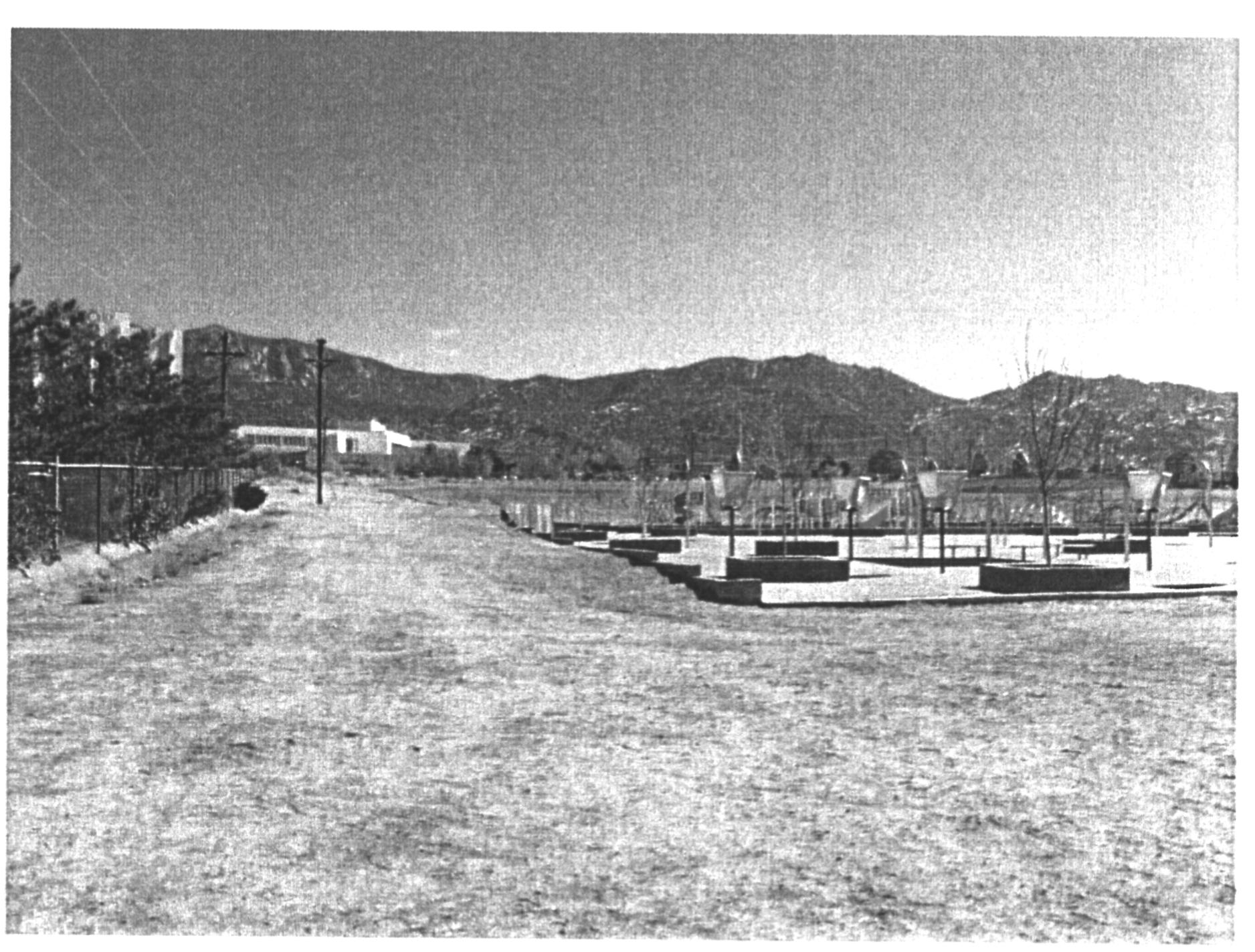


Photo 4 – Playground Looking East



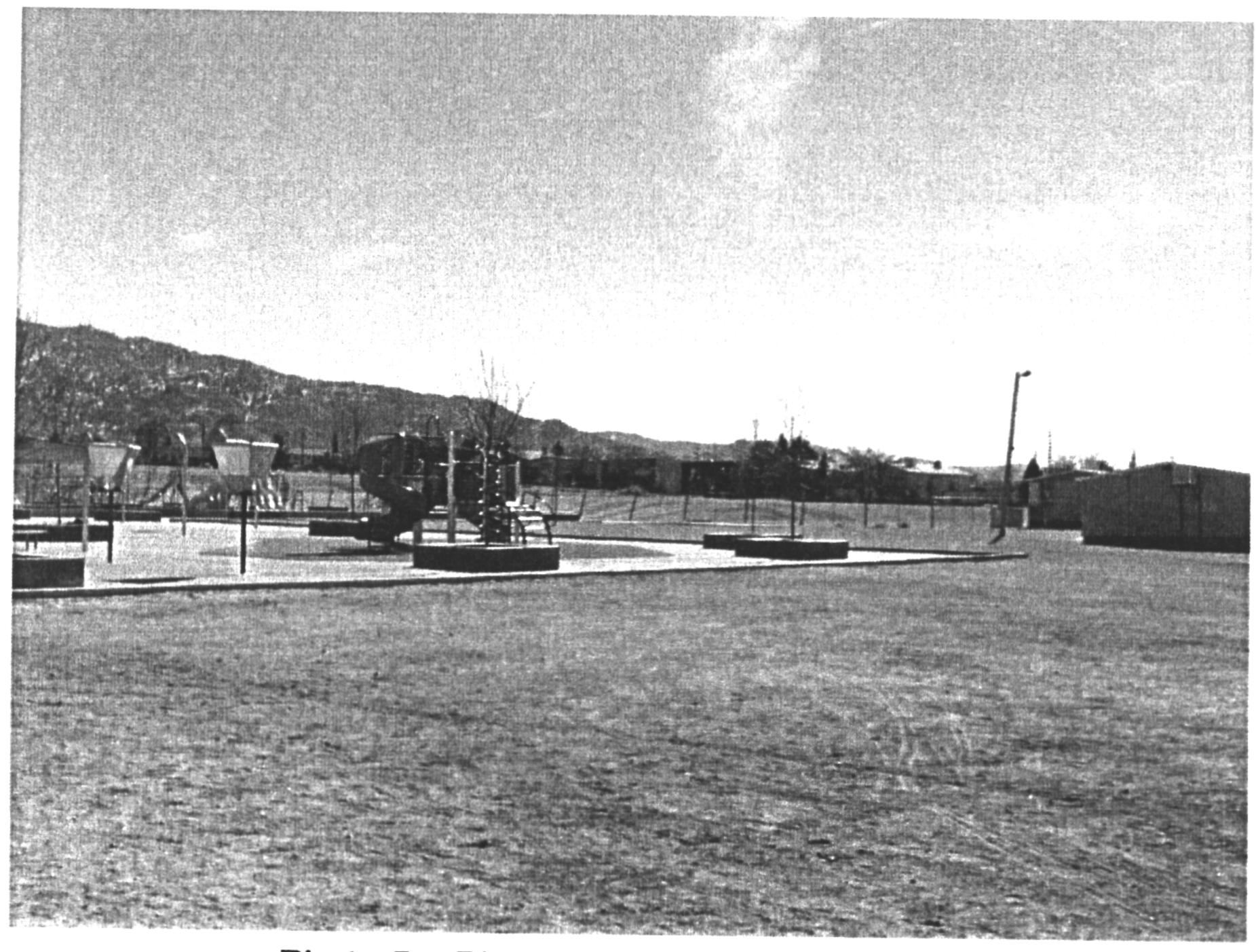


Photo 5 – Playground Looking Southeast

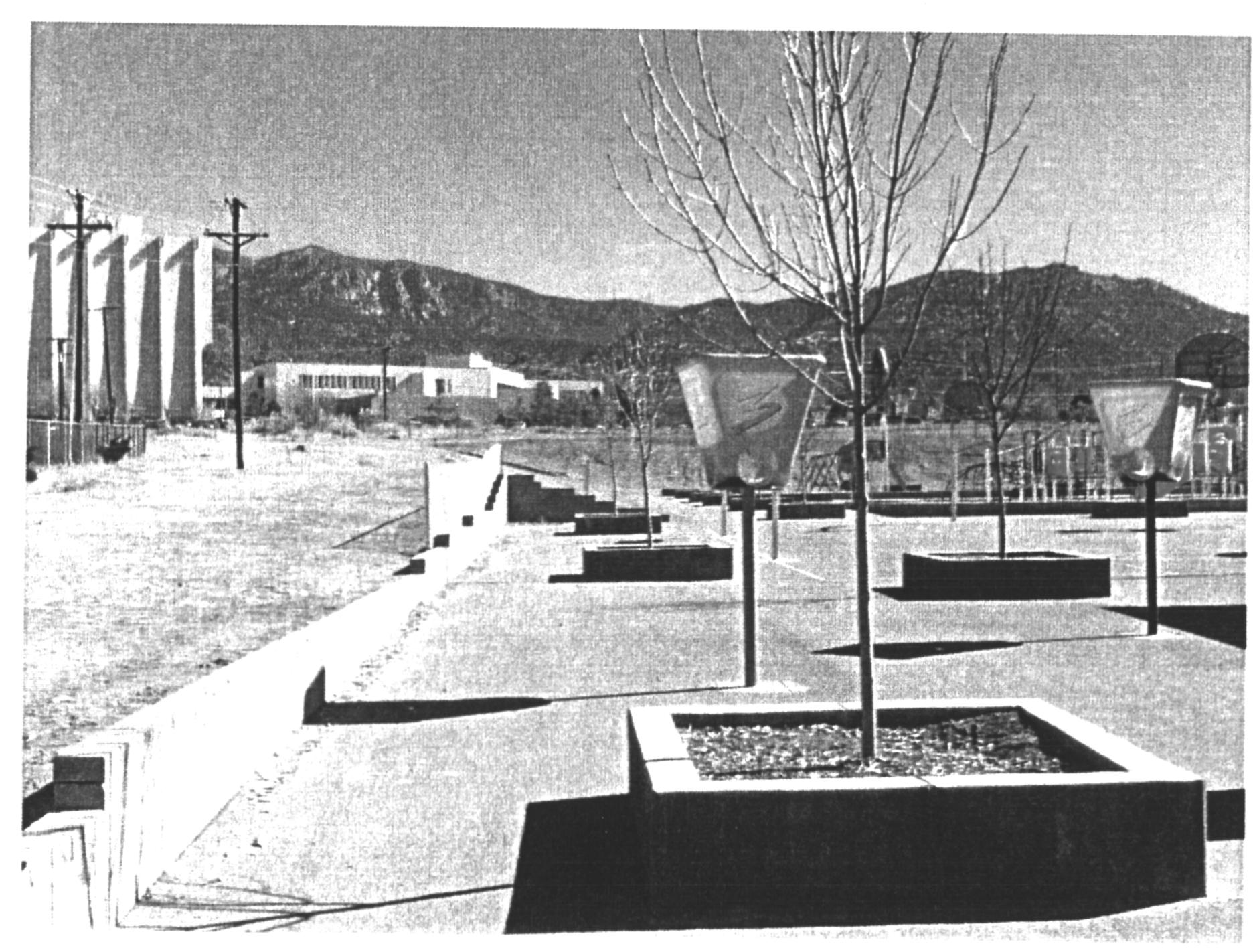


Photo 7 – Playground Looking East

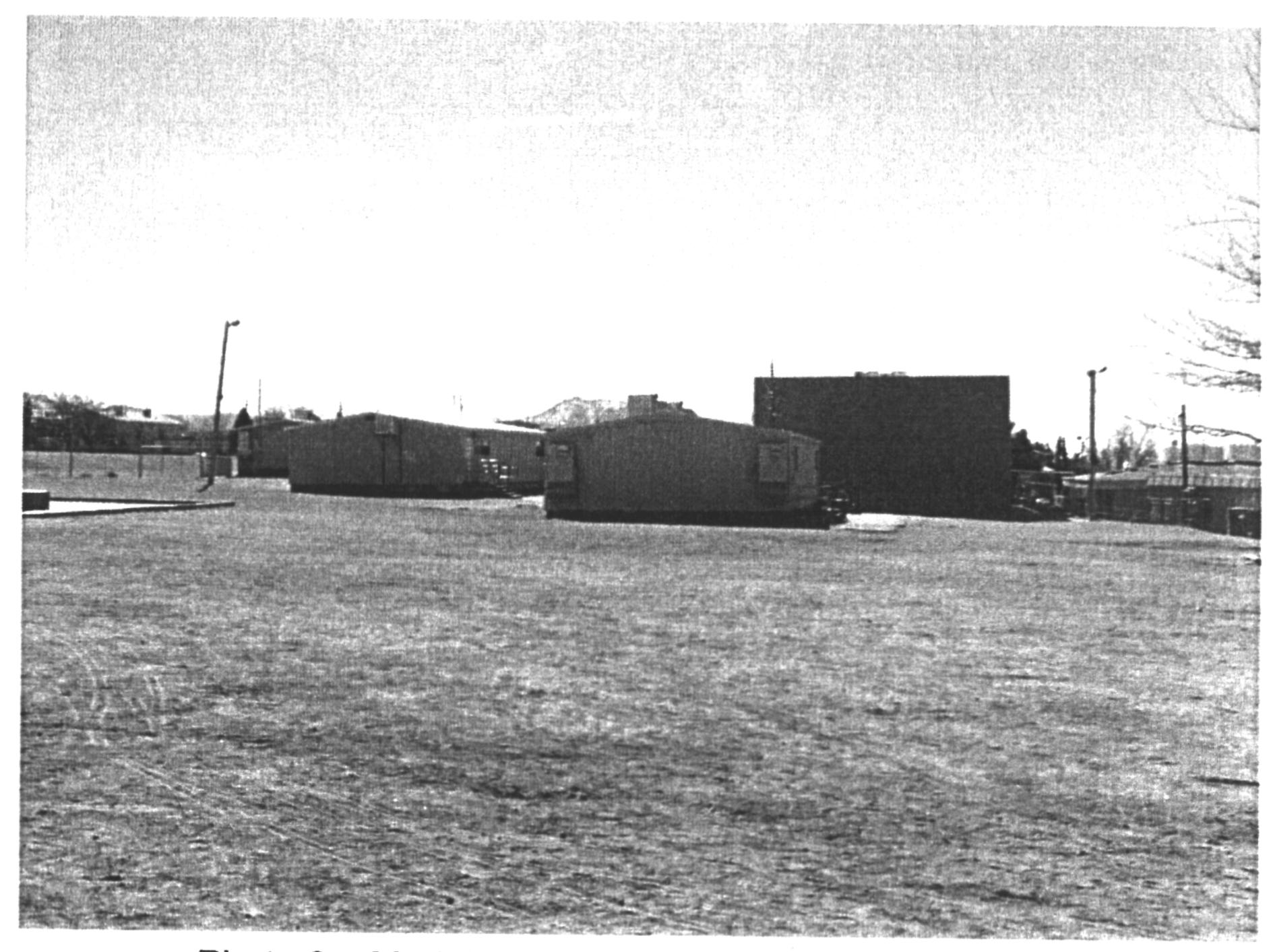


Photo 6 – Modular Building and Gym Looking South

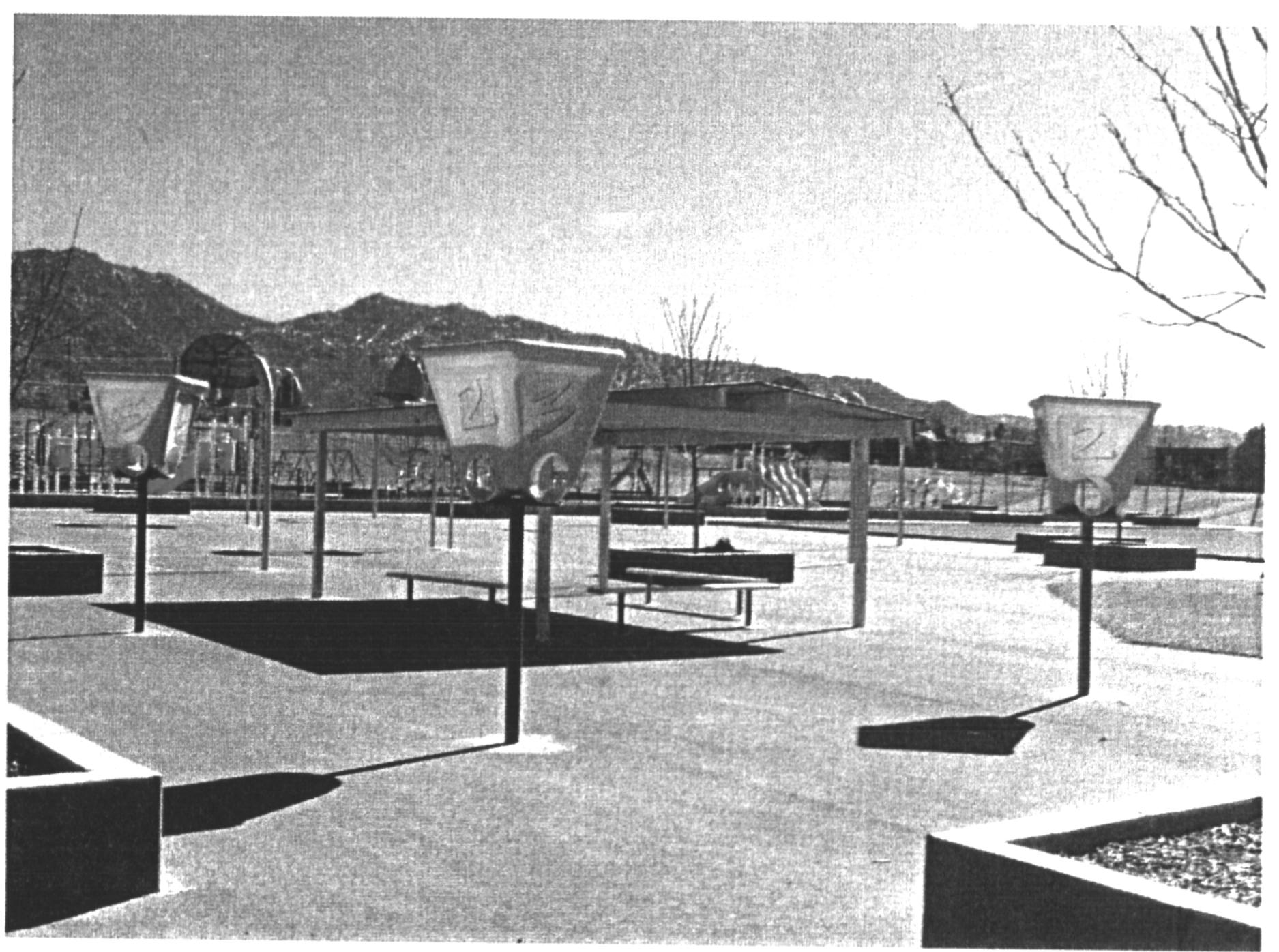


Photo 8 – Playground Looking Southeast





Photo 9 – Playground Looking South



Photo 11 – Asphalt Swale

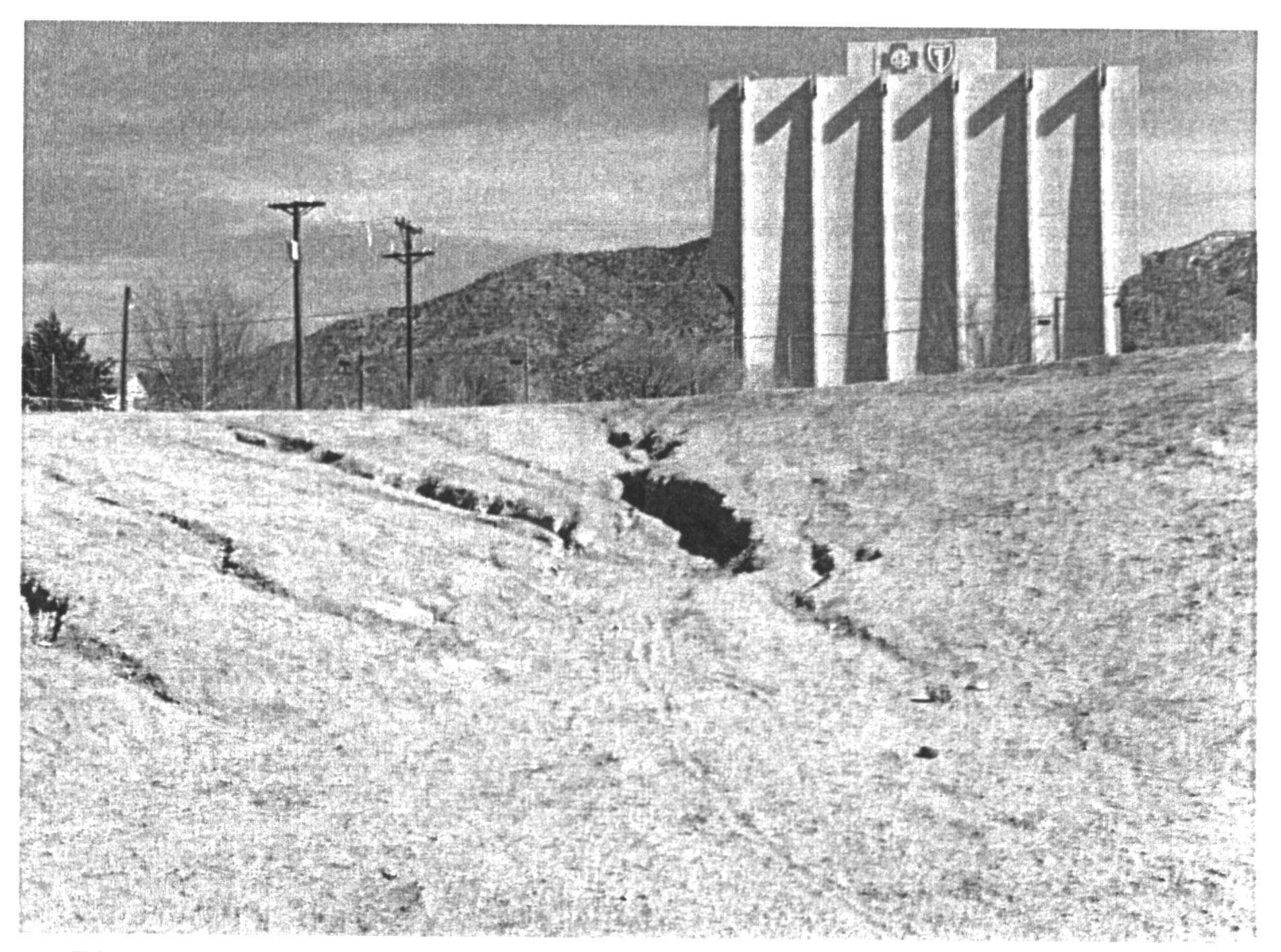


Photo 10 – Erosion Along Cut Slope Along East Side of Playground

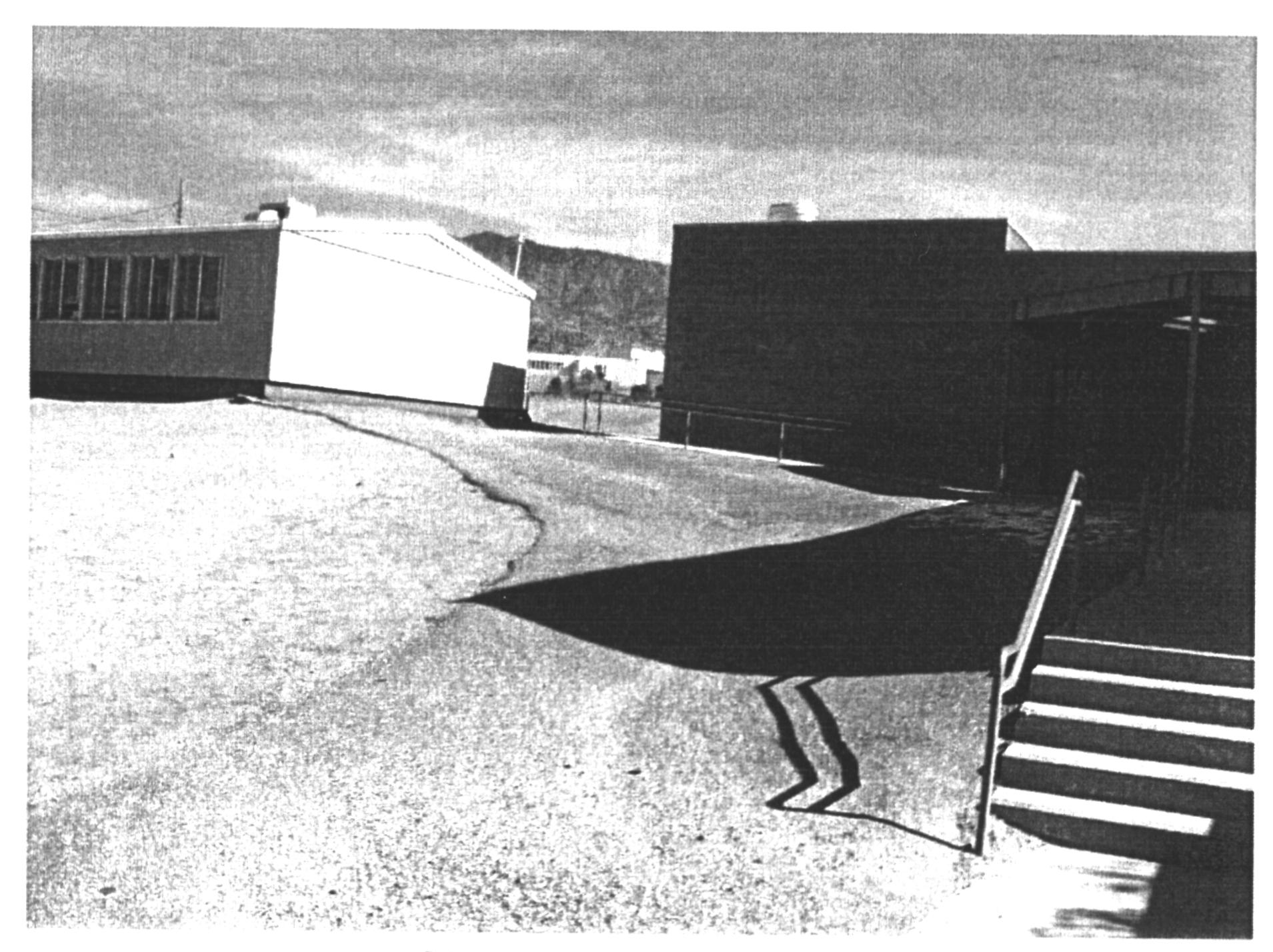
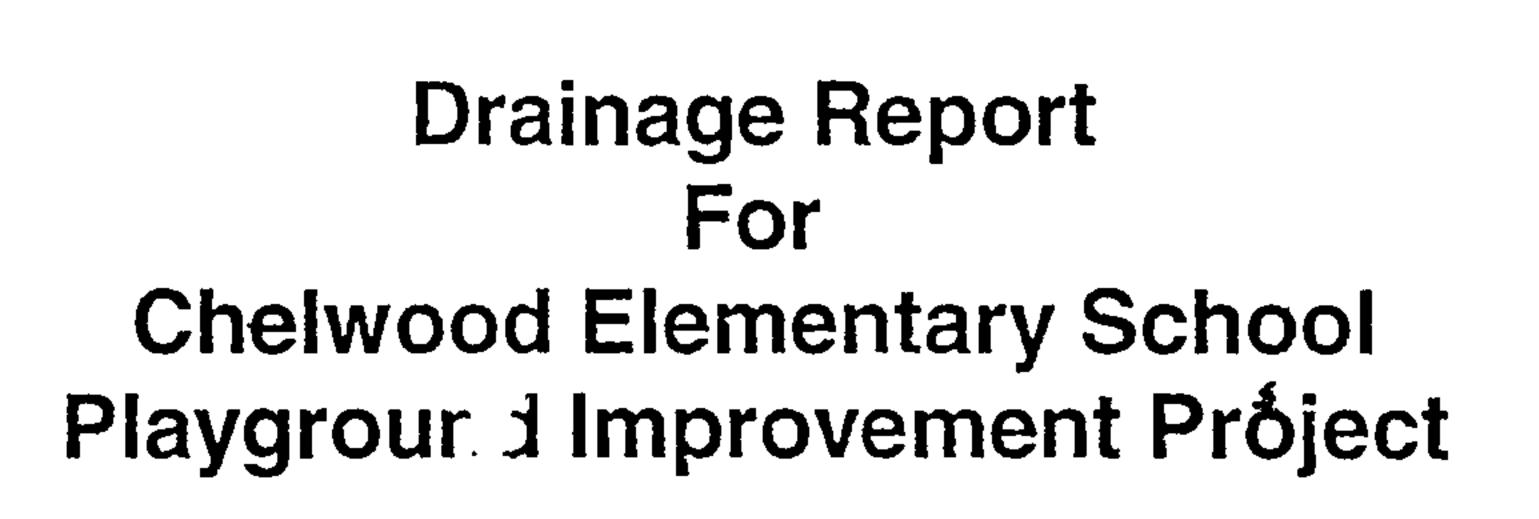
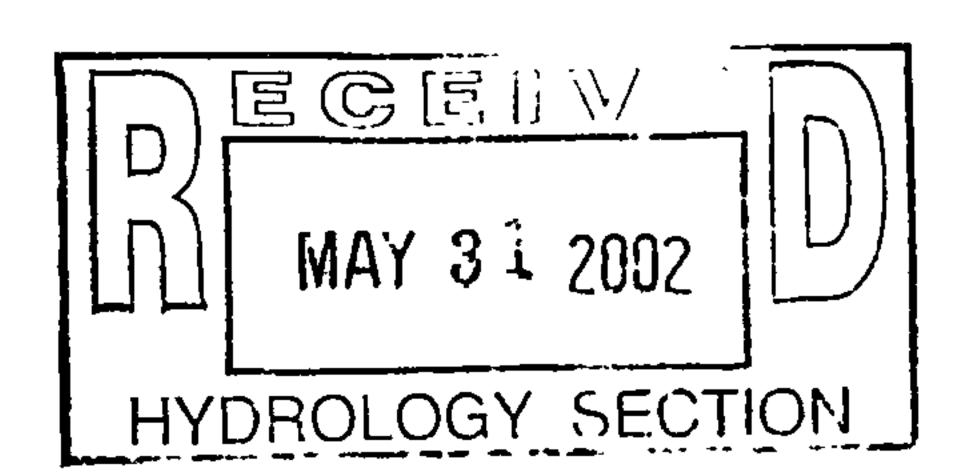


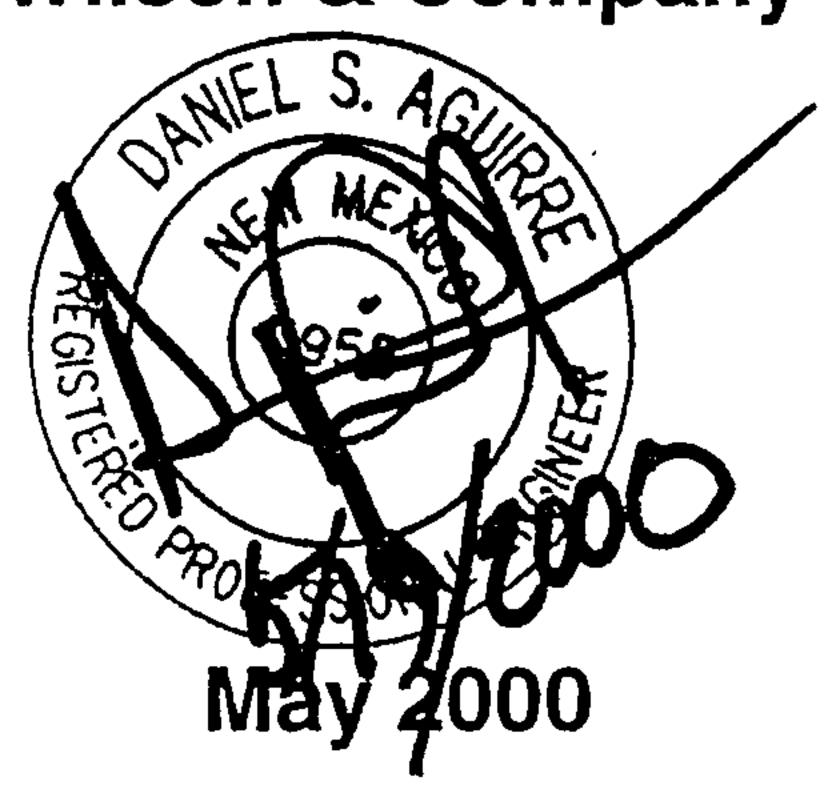
Photo 12 – Asphalt Swale



### Prepared for Albuquerque Public Schools



Prepared By Wilson & Company





#### DRAINAGE REPORT

Site Location: Chelwood Middle School is located at the corner of Constitution Ave. and Chelwood Park Blvd. The proposed development includes sandboxes, asphalt paving, addition of basketball courts, a sod playing field and miscellaneous concrete work.

Methodology: Section 22.2 of the City of Albuquerque DPM was used in the hydrology analysis of the site. A principal design storm of 100-yr, 10-day event was used.

Existing Drainage Condition: The site consists of two basins; Basin 101 - South and Basin 102 - North. Basin 101 encompasses 8.0 acres on the southern portion of the site and Basin 102 encompasses 8.4 acres on the northern portion of the site. Basin 101 is developed with buildings, a parking lot on the southwestern end and an undeveloped playground on the East Side. Basin 102 is developed with playground equipment on the western half and undeveloped property on the eastern half.

Currently, all flows from Basin 101 are conveyed westward adjacent to the buildings and through the parking lots. Runoff is freely discharged (35.89 cfs) onto Chelwood Park Blvd and Constitution Ave. The flows from Basin 102 are also conveyed westward. Approximately 33.82 cfs is freely discharged though the drive pad located on the westside of the site and onto Chelwood Park Blvd. Table 1 provides a breakdown of existing volumetric runoff and peak discharge.

Table 1 - Exiting Conditions							
		Treatment					
Basin	Area (ac)	A	В	C	D	V <sub>1440</sub> (ac-ft)	Q <sub>100</sub> (cfs)
101	7.993	0.00%	0.00%	50.00%	50.00%	1.365	35.888
102	8.383	0.00%	0.00%	80.00%	20.00%	1.185	33.815
Total	16.376					2.55	69.703

Table 1: Breakdown of existing land type for each the basin including its respective volumetric runoff and discharge value.

Proposed Conditions: The proposed conditions will separate the site into eight basins (see Plate 1). Basin 201 is located on the southeast corner of the site. The flows are conveyed westward discharging into an inlet. Flows are conveyed via a 24" SD mainline and discharged into a proposed detention pond, located on the northwest side of the school.

Basin 202 and Basin 203 are located north of Basin 201. Flows sheet flow westward, collected and conveyed by a median curb and gutter to the SD mainline and discharged into the proposed pond.

Basin 204 encompasses the northeast portion of the property. Flows sheet flow westward, first over natural ground then over the proposed playground, into the SD mainline and ultimately discharging into the proposed pond.

Basin 205 encompasses the northeast portion of the property. Flows sheet flow westward over natural ground to the edge of the proposed playground, into the SD mainline and ultimately discharging into the proposed pond.

Basin 206, the western portion of the playground, is located west of Basin 204 and Basin 205. Flows sheet flow northwestward into the SD mainline and ultimately discharging into the proposed pond.

Basin 207 encompasses the western portion of the property. Flows sheet flow westward over the basin, discharging directly into the detention pond or onto Chelwood Park Dr.

The proposed capacity of the detention pond is 0.85 acre-ft, which exceeds the required capacity of 0.50 acre-ft. The detention pond will have a controlled outlet based on the orifice equation to limit the discharge to a maximum of 20 cfs onto Chelwood Park Dr. The hydrograph for the site and pond sizing is shown at the end of this report.

Basin 208 is located on the southwest corner of the site. The proposed development will not effect this basin. Basin 208 discharges directly to the street and is not detained by the pond.

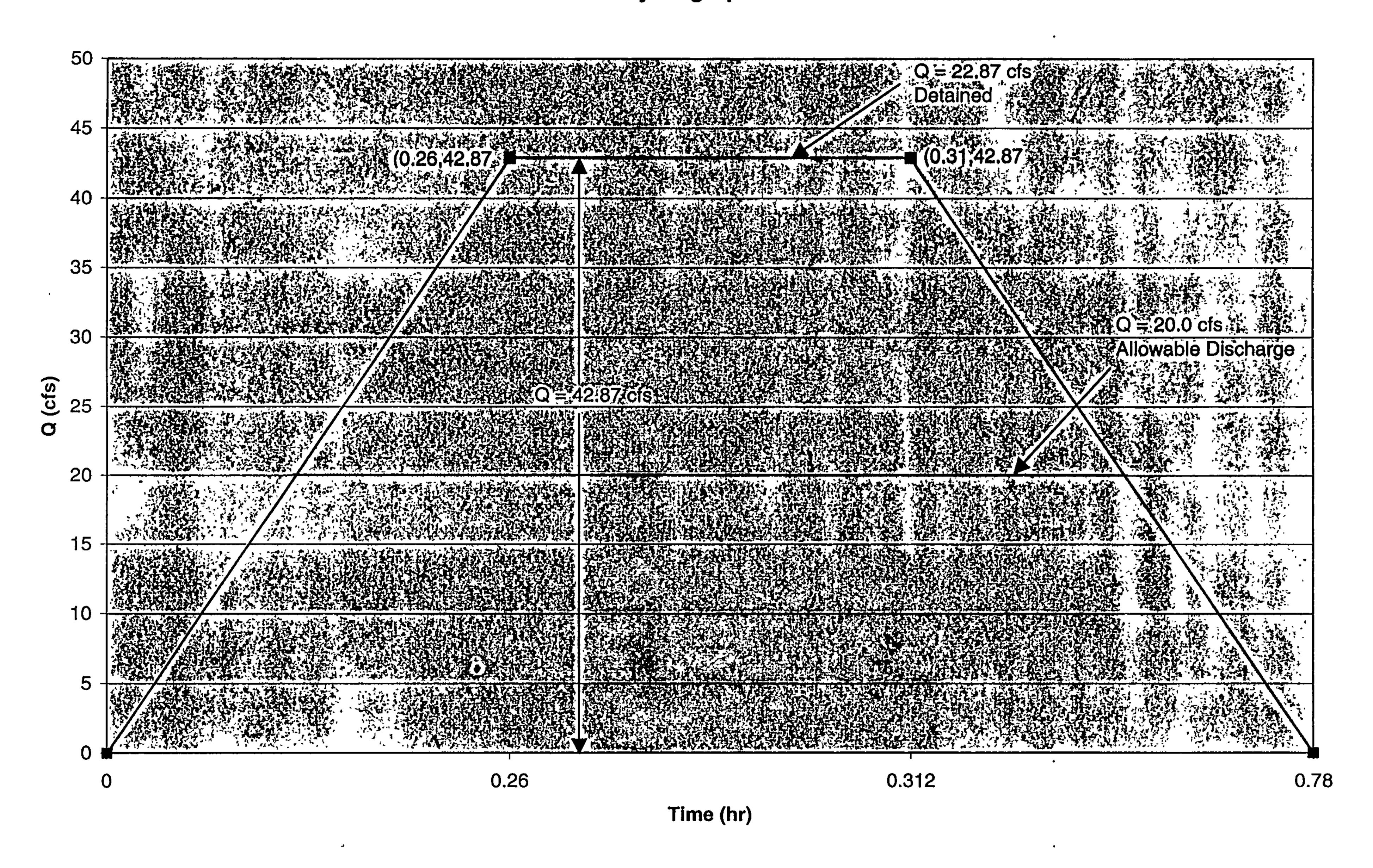
Table 2 provides a breakdown of proposed volumetric runoff and peak discharge.

Table 2 -Proposed Conditions							
		Treatment					
Basin	Area (ac)	A	В	C	D	V <sub>1440</sub> (ac-ft)	Q <sub>100</sub> (cfs)
201	0.477	0.00%	0.00%	100.00 %	0.00%	0.058	1.779
202	3.339	0.00%	0.00%	100.00 %	0.00%	0.406	12.453
203	1.552	0.00%	0.00%	78.60%	21.40%	0.222	6.294
204	0.954	0.00%	0.00%	66.66%	33.33%	0.153	4.134
205	1.781	0.00%	0.00%	98.00%	2.00%	0.219	6.676
206	0.418	0.00%	0.00%	0.00%	100.00 %	0.092	2.194
207	2.082	0.00%	0.00%	50.00%	50.00%	0.355	9.338
208	5.773	0.00%	0.00%	10.00%	90.00%	1.213	29.431
Total	16.376					2.718	72.299

Table 2: Breakdown of proposed land types for each basin including their respective volumetric runoff and discharge values.

Conclusions: The proposed development will not have a significant impact on the total volumetric runoff (increase of 0.17 ac-ft) and the total flow rate (increase of 2.6 cfs) when compared to the existing conditions. However, there is an improvement to the proposed discharged onto Chelwood Park Blvd. and Constitution Ave. The proposed discharge onto Constitution Ave. will decrease because the runoff from Basin 101 (specifically Basin 201 and a portion of Basin 202) will be diverted to Basin 102, thus allowing less runoff onto Constitution Ave. The other positive aspect of these proposed site improvements is the reduction of the discharge onto Chelwood Park Blvd. to a maximum of 20 cfs due to the controlled outlet, rather than the uncontrolled 33 cfs. The pond will detain the flows from all basins except for basin 208 which will continue to have free discharge to the adjacent streets.

#### Hydrograph



### DRAINAGE AND TRANSPORTATION INFORMATION SHEET

(REV 12/2005)

PROJECT TITLE: CHEWAR FLENESTARY	ZHOOL ZONE MAP:
DRB#: EPC#:	WORK ORDER#:
LEGAL DESCRIPTION:	<del></del>
CITY ADDRESS:	······································
ENGINEERING FIRM:	CONTACT:
ADDRESS:	PHONE:
CITY, STATE:	ZIP CODE:
OWNER:	CONTACT:
ADDRESS:	PHONE:
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ARCHITECT: KOHDE WHY KELER MCJAMPAR	ARCHITECTURE CONTACT: DAVID EDUARIS
ADDRESS: 400 (101) No. 31, 31, 410 Up	DELINE TOURS PHONE: 243-5454 Lets-1886
CITY, STATE: MRUQUERQUE, NEW M	ZIP CODE: 534066
SURVEYOR:	CONTACT:
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CITY, STATE:	ZIP CODE:
CILI, DIFILIS.	
TYPE OF SUBMITTAL:	CHECK TYPE OF APPROVAL SOUGHT:
DRAINAGE REPORT	SIA/FINANCIAL GUARANTEE RELEASE
DRAINAGE PLAN 1 <sup>st</sup> SUBMITTAL	PRELIMINARY PLAT APPROVAL
DRAINAGE PLAN RESUBMITTAL	S. DEV. PLAN FOR SUB'D APPROVAL うのアペイ
CONCEPTUAL G & D PLAN	S. DEV. FOR BLDG. PERMIT APPROVAL
GRADING PLAN	SECTOR PLAN APPROVAL
EROSION CONTROL PLAN	FINAL PLAT APPROVAL FOUNDATION PERMIT APPROVAL 「フロアル)」 PLUI DING PERMIT APPROVAL
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TRAFFIC CIRCULATION LAYOUT ENGINEER'S CERT (TCL)	CERTIFICATE OF OCCUPANCY (PERM)
ENGINEER'S CERT (TCL)  ENGINEER'S CERT (DRB SITE PLAN)	CERTIFICATE OF OCCUPANCY (TEMP) TO DIM
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	OTHER (SPECIFY)
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WAS A PRE-DESIGN CONFERENCE ATTEMPED	
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NO	
COPY PROVIDED	····ODOIOGY
	HYDROLOGY SECTION BY: TAULD FRUNCE
DATE SUBMITTED: October 10, Los	SECTION BY: THUN FRUNCS

Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a drainage submittal. The particular nature, location, and scope to the proposed development defines the degree of drainage detail. One or more of the following levels of submittal may be required based on the following:

- 1. Conceptual Grading and Drainage Plan: Required for approval of Site Development Plans greater than five (5) acres and Sector Plans.
- 2. Drainage Plans: Required for building permits, grading permits, paving permits and site plans less than five (5) acres.
- 3. Drainage Report: Required for subdivision containing more than ten (10) lots or constituting five (5) acres or more.