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# City of Albuquerque

Planning Department

Development & Building Services Division

DRAINAGE AND TRANSPORTATION INFORMATION SHEET

(REV 02/2013)

Project Title: Monterey Baptist Church	Building Permit #:	City Drainage #:
DRB#: EPC#:		Work Order#:
Legal Description: Lot 6A, Block D, Monterey Mano	r Subdivision	
City Address: 12501 Lomas.Blvd NE		
Engineering Firm: RHD Engineering, LLC		Contact: Richard Dourte
Address: 4305 Purple Sage Ave, NW, Albuquerou	Je. NM. 87120	
Phone#: 505-288-1621 Fax#:		E-mail: rhdengineering@outlook.com
Owner:		Contact;
Address:		
Phone#: Fax#:		E-mail:
Architect: Simons Architecture PC		Contact: Joe Simons
Phone#: 505-480-4796 Fax#:		E-mail: joe@simonsarchitecture.com
Surveyor: Harris Surveys		Contact: Tony Harris
Address:		
Phone#: Paut#:		E-mail:
Contractor:		Contact:
Address:		
Phone#: Fax#:	•	E-mail:
TYPE OF SUBMITTAL:	CHECK TYPE OF APPROV	ALACCEPTANCE SOUGHT:
DRAINAGE REPORT	SIA/FINANCIAL GUARANT	TEE RELEASE
DRAINAGE PLAN 1st SUBMITTAL	PRELIMINARY PLAT APPR	OVAL
X DRAINAGE PLAN RESUBMITTAL	S. DEV. PLAN FOR SUB'D	APPROVAL
CONCEPTUAL G & D PLAN	S. DEV. FOR BLDG. PERMI	TAPPROVAL
GRADING PLAN	SECTOR PLAN APPROVAL	
EROSION & SEDIMENT CONTROL PLAN (ESC)	FINAL PLAT APPROVAL	
ENGINEER'S CERT (HYDROLOGY)	CERTIFICATE OF OCCUPA	NCY (PERM)
CLOMR/LOMR	CERTIFICATE OF OCCUPA	NCY (TCL TEMP)
TRAFFIC CIRCULATION LAYOUT (TCL)	FOUNDATION PERMIT APP	ROVAL
ENGINEER'S CERT (TCL)	X BUILDING PERMIT APPRO	VAL
ENGINEER'S CERT (DRB STE PLAN)	GRADING PERMIT APPROV	AL SO-19 APPROVAL
ENGINEER'S CERT (ESC)	PAVING PERMIT APPROVA	L ESC PERMIT APPROVAL
SO-19	WORK ORDER APPROVAL	ESC CERT. ACCEPTANCE
OTHER (SPECIFY)	ORADING CERTIFICATION	OTHER (SPECIFY)
WAS A PRE-DESIGN CONFERENCE ATTENDED:	By: The Co	py Provided

Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a drainage submittal. The particular nature, location, and scope to the proposed development defines the degree of drainage detail. One or more of the following levels of submittal may be required based on the followin 1. Conceptual Grading and Drainage Plan: Required for approval of Site Development Plans greater than five (5) acres and Sector Plans

 Drainage Plans: Required for building permits, grading permits, paving permits and site plans less than five (5) acres
Drainage Report: Required for subdivision containing more than ten (10) lots or constituting five (5) acres or more 4.

Erosion and Sediment Control Plant Required for any new development and redevelopment site with 1-acre or more of land disturbing area, including project less than 1-acre than any part of a larger common plan of development

Drainage Report

For

Monterey Baptist Church 12501 Lomas Blvd NE Albuquerque, New Mexico

Prepared by

RHD Engineering, LLC Albuquerque, New Mexico

June 2015



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#### Purpose:

The purpose of this drainage report is to provide a drainage management plan for the proposed changes to the Monterey Baptist Church. The site is approximately 2.92 acres in size. The proposed changes include increasing the square footage of the existing building by approximately 1100 sf. This drainage report and plan is prepared utilizing the City of Albuquerque Development Process Manual.

#### Introduction:

This site is located at 12501 Lomas Boulevard NE on zone atlas K-22.

The Monterey Baptist Church is presently developed, the proposed new improvements include the addition of approximately 1,100 sf of new building on the east side of the existing structure. The entrance into this church will also be reconfigured. No changing or altering of the parking lot is anticipated, thus the drainage pattern for this parking lot will remain the same.

FEMA Firm map 35001C0359G identifies that this site is located in zone x. Please refer to the drainage plan.

#### **Existing Drainage Conditions:**

Approximately half of this site drains (sheet flows) into an existing pond located at the southwest corner of this site discharges that has a controlled outlet into Lomas Boulevard. The majority of the remaining portion of this site free discharges into an existing cobble stone swale that runs along the north side of the sidewalk that is adjacent to Lomas Boulevard and empties into the existing pond located at the southwest corner of this site, and it discharges as mentioned above.

### **Proposed Conditions:**

The proposed conditions for the majority of the site will remain as they are today.

The runoff from drainage basin A will discharge into two inlets, located within the adjoining sidewalk area of the new addition. The pipe from these catch basins will daylight onsite and discharge into the cobble stone swale that empties into the pond at the southwest corner of the site.

The runoff from drainage basin B will sheet flow toward the southeast corner of this basin and discharge into Lomas blvd into the cobble stone swale that empties into the pond at the southwest corner of the site.

The runoff from drainage basin C will flow onto drainage basin A and discharge and discharge into the inlets.

The required first flush volume of stormwater generated by the new impervious areas (1,100 sf addition) is 31cf.

#### Summary:

This site will discharge the stormwater flows in the same manner as it is today.

#### For Basin A

The peak flows will increase from .34 cfs to .39 cfs or by 0.05 cfs for the 100 yr 6 hr. event.

The excess precipitation will increase from 529 cf to 642 cf or by 113 cf for the 100 yr 6 hr event.

#### For Basin B

The peak flows will increase from .45 cfs to .49 cfs or by 0.04 cfs for the 100 yr 6 hr. event.

The excess precipitation will increase from 748 cf to 843 cf or by 95 cf for the 100 yr 6 hr event.

#### For Basin C

The peak flows will remain the same at 0.32 cfs.

The excess precipitation will remain the same at 594 cf.

The City of Albuquerque's first flush requirements will be adhered to for the construction of the proposed improvements. The first flush for the proposed improvements is 31 cf. The existing pond is going to be increased by 140cf, thus the increased volume of the pond is greater than the first flush requirements.

#### APPENDIX A

#### Drainage Calculations for Basin D

#### Zone 4 (100yr, 6hr)

Land Treatment	Peak discharge	<b>Excess</b> Precipitation
Type A -	2.20 cfs/ac	0.80 inches
Type B -	2.92 cfs/ac	1.08 inches
Type C -	3.73 cfs/ac	1.46 inches
Type D -	5.25 cfs/ac	2.64 inches

#### Existing Conditions for Basin A

Impervious area (type D)= 800 sf = 0.018 ac

Area other than Impervious (type C)= 3,700 sf - 800 sf = 2900 sf = .067 ac

Peak Flow generated

.018 ac x 5.25 cfs = .09 cfs .067 ac x 3.73 cfs = .25 cfs Total = .34 cfs

Excess Precipitation

#### **Existing Conditions for Basin B**

Impervious area (type D)= 2160 sf = 0.050 ac

Area other than Impervious (type C)= 4400 sf - 2160 sf = 2240 sf = .051 ac

Peak Flow generated

.050 ac x 5.25 cfs = .26 cfs .051 ac x 3.73 cfs = .19 cfs Total = .45 cfs

**Excess Precipitation** 

2160 sf x 2.64 in/12 = 475 cf 2240 sf x 1.46 in/12 = 273 cf Total = 748 cf

#### Existing Conditions for Basin C

Impervious area (type D)= 2700 sf = 0.06 ac

Peak Flow generated

.060 ac x 5.25 cfs = .32 cfs

Total = .32 cfs

**Excess Precipitation** 

2700 sf x 2.64 in/12 = 594 cf

Total = 594 cf

#### Proposed Conditions for Basin A

Impervious area (type D)= 1950 sf = 0.045 ac

Area other than Impervious (type C)= 3,700 sf - 1950 sf = 1750 sf = .040 ac

Peak Flow generated

.045 ac x 5.25 cfs = .24 cfs .040 ac x 3.73 cfs = .15 cfs Total = .39 cfs

**Excess Precipitation** 

 $\begin{array}{rl} 1950 \ {\rm sf} \ {\rm x} \ 2.64 \ {\rm in}/12 \ = \ 429 \ {\rm cf} \\ 1750 \ {\rm sf} \ {\rm x} \ 1.46 \ {\rm in}/12 \ = \ 213 \ {\rm cf} \\ {\rm Total} \ \ = \ \ 642 \ {\rm cf} \end{array}$ 

#### **Proposed Conditions for Basin B**

Impervious area (type D)= 3125 sf = 0.072 ac

Area other than Impervious (type C)= 4400 sf - 3125 sf = 1275 sf = .029 ac

Peak Flow generated

.072 ac x 5.25 cfs = .38cfs .029 ac x 3.73 cfs = .11 cfs Total = .49 cfs Excess Precipitation

3125 sf x 2.64 in/12 = 688 cf 1275 sf x 1.46 in/12 = 155 cf Total = 843 cf

First flush requirements (for new impervious area)

#### 1,100 sf x 0.34 inches/12 = 31 cf.

Increase in pond volume

The area of pond at contour 5623 is 280 sf, by increasing the depth of the pond by 0.5 ft, the increased volume is  $280/2 \times .5 = 140$  cf.

#### Inlet Grate Capacity (see Appendix B)

Each grate @ 0.1 ft depth has a capacity of .40 cfs, with a clogging factor of 50%, each grate will have a capacity of 0.20cfs. Thus four inlet at 0.20 cfs capacity each for a total capacity of 0.80 cfs > 0.71cfs (0.39 cfs + 0.32cfs) needed for drainage Basin A.

6 inch PVC capacity

 $Q = \frac{1.49 \text{ x A x R}^{67} \text{ x S}^{5}}{\text{n}}$ Area = 0.20 sf R = A/P = 0.20/1.57 = 0.13 S = 0.01 n = .009  $Q = \frac{1.49(0.20)((0.13)^{67})((.01)^{5})}{.009}$ 

Q = 0.84 cfs

Thus the capacity of the 6 inch pvc pipe, 0.84 cfs >0.71 cfs required for Basin A. Use an 8 inch pvc pipe



#### Nyloplast 12" Pedestrian Grate Inlet Capacity Chart

3130 Vérona Avenue • Buford, GA 30518 (866) 888-8479/ (770) 932-2443 • Fax: (770) 932-2490 © Nyloplast Inlet Capacity Charts June 2012

Appendix B

# **CITY OF ALBUQUERQUE**



June 22, 2015

Richard Dourte, P.E. RHD Engineering, LLC 4305 Purple Sage Ave NW Albuquerque, NM 87120

RE: Monterey Baptist Church 12501 Lomas Blvd NE Grading and Drainage Plan Engineers Stamp Date 6/17/15 (J22D012A)

Dear Mr. Dourte,

Based upon the information provided in your submittal received 6/12/15, this plan is approved for Building Permit.

Please attach a copy of this approved plan dated 6/17/15 to the construction sets in the permitting process prior to sign-off by Hydrology.

Prior to Certificate of Occupancy release, Engineer Certification per the DPM checklist will be required.

If you have any questions, please contact me at 924-3695 or Rudy Rael at 924-3977.

Albuquerque

PO Box 1293

New Mexico 87103

www.cabq.gov

Rite C.H.

Rita Harmon, P.E. Senior Engineer, Hydrology Planning Department

RR/RH C: File



EROSION CONTROL NOTES 1. CONTRACTOR IS RESPONSIBLE FOR OBTAINING A TOPSOIL DISTURBANCE PERMIT PRIOR TO BEGINNING WORK. 2. CONTRACTOR IS RESPONSIBLE CONSTRUCTION. 3. CONTRACTOR IS RESPONSIBLE FOR CLEANING ALL SEDIMENT THAT GETS INTO EXISTING RIGHT-OF-WAY 5648 4. REPAIR OF DAMAGED FACILITIES AND CLEANUP OF SEDIMENT ACCUMULATIONS ON ADJACENT PROPERTIES AND IN PUBLIC FACILITIES IS THE RESPONSIBILITY OF THE/CONTRACTOR. 5. ALL EXPOSED EARTH SURFACES MUST BE PROTECTED FROM WIND AND WATER EROSION PRIOR TO FINAL ACCEPTANCE OF ANY PROJECT. 5647 5646 Nastall 4 New storm drain inlets with/ Nyloplast 12" pedestrian grates or equivalent-2 inlets at each location XTA=5644.25 XTA=5644.43 -Grate el. = 40.25 pipe invert in 🔫 39.10 f new sdwk=41.00 invert out = 39.00×TA=5643.97 XTA=5643.25 Existing sdwk to remain XTA=5643.31 ×TA=5642.70 XTA=5642.29 XTA=5641.63 -New 8 inch pvc pipe -Existing cobble swale that drains to pond, see sheet 2 -Existing sdwk on Montgomery blvd





## LEGAL DESCRIPTION:

Tract 6—A, Block D, Monterey Manor Subdivision CITY OF ALBUQUERQUE BERNALILLO COUNTY, NEW MEXICO

## NOTES:

1. ALL SPOT ELEVATIONS REPRESENT FLOWLINE ELEVATION UNLESS OTHERWISE NOTED.

2. THE FIRST .34" OF STORM RUNOFF FROM THE NEW ADDITION TO CONFORM TO THE WATER QUALITY REQURIEMENTS.

# LEGEND

46.00	PROPOSED SPOT ELEVATION
TW=44.00 BW=39.00	TOP OF WALL ELEVATION BOTTOM OF WALL ELEVATION
	EXISTING CONTOUR
and a second model model where a second $\sum_{i=1}^{n} \sum_{j=1}^{n} \sum_{i=1}^{n} \sum$	EXISTING INDEX CONTOUR
5601	PROPOSED CONTOUR
5600	PROPOSED INDEX CONTOUR
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	RIGHT-OF-WAY
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	PROPOSED ROCK FACE WALL
	EXISTING CURB AND GUTTER
	PROPOSED EDGE OF CONCRETE
· · · · · · · · · · · · · · · · · · ·	PROPOSED FLOWLINE
	EXISITING WALL
	PROPOSED BASIN BOUNDARY





		AS BUILT INFORMATION	CONTRACTOR WORK DATE	STAKED BY CALL INSPECTOR'S DATE ACCEPTANCE BY DATE FIELD DATE	DRAWINGS DARFICIED RY DATE	MICRO-FILM INFORMATION	RECORDED BY DATE	NO.
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				ARKS BY	CNIDIO	DATE:	DATE:	JOB NO.: DATE:
	RHD Engineering, LLC			DATE REM/	DESIGN	ED BY:	BY:	NAME: ED BY:
	4305 Purple Sage Ave. NW ALBUQUERQUE, NM 87120 (505) 288–1621 CITY OF ALBUQUERQ PUBLIC WORKS DIVIS ENGINEERING GROU TITLE: Monterey Baptist CH POND DETAILS DESIGN REVIEW COMMITTEE CITY ENGINEER APPROVAL	QUE SION JP hurch	M0./[	OZ DAY/YR.		DESIGN	DRAWN	CHECKE
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