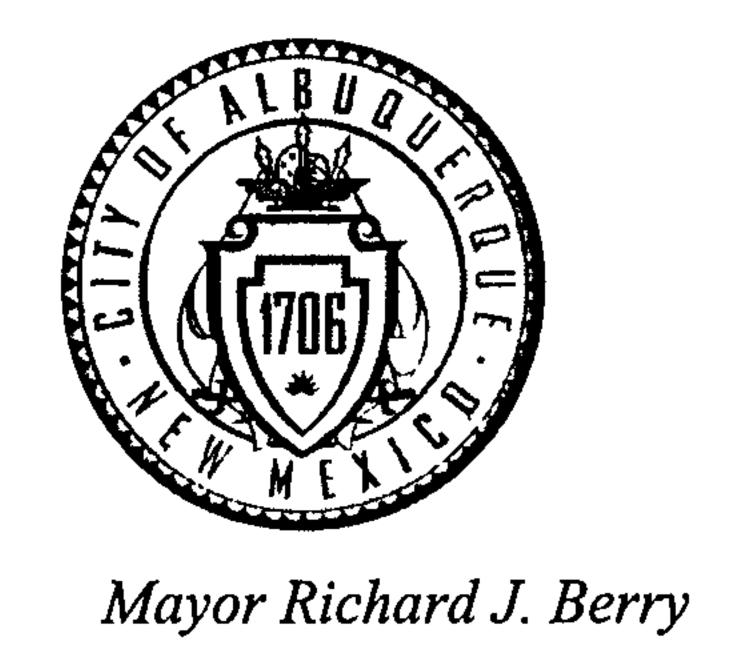
Planning Department Suzanne Lubar, Director



March 18, 2016

Richard Dourte, P.E. RHD Engineering, LLC 4305 Purple Sage Ave NW Albuquerque, NM 87120

RE: Monterey Baptist Church

12501 Lomas Blvd NE

Request Permanent C.O. - Approved

Engineers Stamp Date 6/17/15 (J22D012A)

Certification date: 3-16-16

Dear Mr. Dourte,

PO Box 1293

Based on the Certification received 3/17/2016, the above site is acceptable for release of Certificate of Occupancy by Hydrology.

Albuquerque

If you have any questions, you can contact me at 924-3695 or Totten Elliott at 924-

3982.

New Mexico 87103

www.cabq.gov

Sincerely,

Rita Harmon, P.E.

Senior Engineer, Planning Dept.

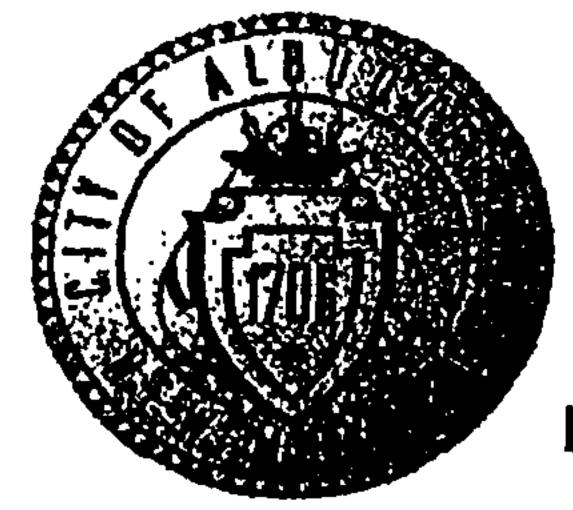
Development Review Services

TE/RH

C: E-Mail

Cordova, Camille C.; Connor, Miranda, Rachel; Sandoval, Darlene M.;

Blocker, Lois



City of Albuquerque

Planning Department

Development & Building Services Division

DRAINAGE AND TRANSPORTATION INFORMATION SHEET

(REV 02/2013)

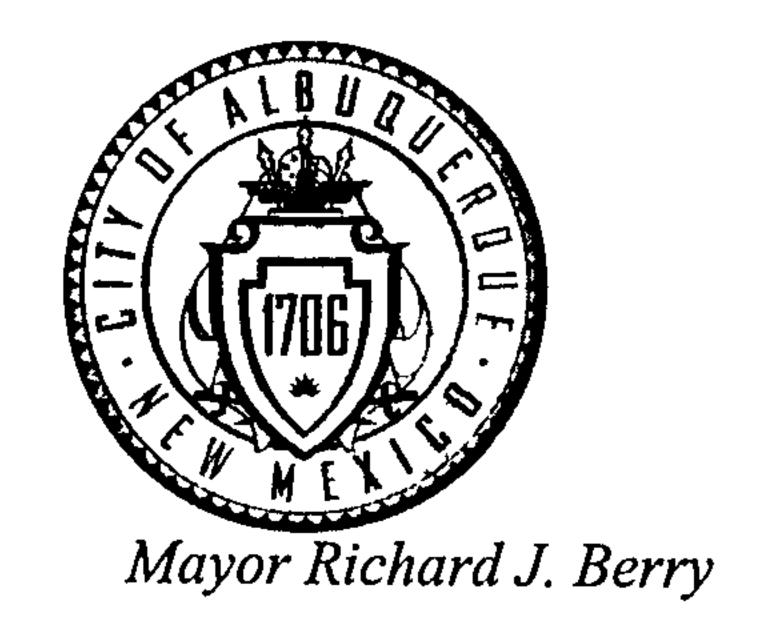
Project Title: Monterey Baptist Church	Building Permit #:	City Drainage #: IZZ D9
DRB#: EPC#:		Work Order#:
Legal Description: Lot 6A, Block D. Monterey Mand	or Subdivision	
City Address: 12501 Lomas-Blvd NE		
Engineering Firm: RHD Engineering, LLC		Contact: Richard Dourte
Address: 4305 Purple Sage Ave. NW. Albuquero	ue. NM. 87120	
Phone#: 505-288-1621 Fax#:		E-mail: rhdengineering@outlook.com
Owner:		Contact:
Address:		
hone#:Fax#:	ودورو وروان المراور والمراور و و و و و و و و و و و و و و و و و و	E-mail:
Architect: Simons Architecture PC		Contact: Joe Simons
Address:		
hone#: 505-480-4796 Fax#:		E-mail: joe@simonsarchitecture.com
urveyor: Harris Surveys		Contact: Tony Harris
\ddress:		
hone#: Fax#:		E-mail:
Contractor;		Contact:
\ddress:		
hone#:		E-mail:
YPE OF SUBMITTAL:	CHECK TYPE OF APPROVA	LIACCEPTANCE SOUGHT:
DRAINAGE REPORT	SIA/FINANCIAL GUARANT	
· DRAINAGE PLAN 1st SUBMITTAL	PRELIMINARY PLAT APPR	OVAL
DRAINAGE PLAN RESUBMITTAL	8. DEV. PLAN FOR SUB'D	APPROVAL
CONCEPTUAL G & D PLAN	S. DEV. POR BLDG. PERMI	T APPROVAL
GRADING PLAN	SECTOR PLAN APPROYAL	
EROSION & SEDIMENT CONTROL PLAN (ESC)	FINAL PLAT APPROVALD	
ENGINEER'S CERT (HYDROLOGY)	CERTIFICATE OF OCCUPA	
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TRAFFIC CIRCULATION LAYOUT (TCL)	FOUNDATION PERMIT ARE	1 i - 1
ENGINEER'S CERT (TCL)	X BUILDING PERMIT APPRO	NO DEVELOPMENT SECTION
ENGINEER'S CERT (DRB SITE PLAN)	GRADING PERMIT APPROV	ADDEVELOPMENT SECTION.
ENGINEER'S CERT (ESC)	TWAIMO LEKWII WALKOAN	ESC PERMIT APPROVAL
SO-19	WORK ORDER APPROVAL	ESC CERT. ACCEPTANCE
OTHER (SPECIFY)	ORADING CERTIFICATION	OTHER (SPECIFY)
WAS A PRE-DESIGN CONFERENCE ATTENDED:		py Provided
DATE SUBMITTED: MARCH 16,2016	By: De St	

Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a drainage submittal. The particular nature, location, and scope to the proposed development defines the degree of drainage detail. One or more of the following levels of submittal may be required based on the following 1. Conceptual Grading and Drainage Plan: Required for approval of Site Development Plans greater than five (5) acres and Sector Plans

2. Drainage Plans: Required for building permits, grading permits, paving permits and site plans less than five (5) acres
3. Drainage Report: Required for subdivision containing more than ten (10) lots or constituting five (5) acres or more

4. Erosion and Sediment Control Plan: Required for any new development and redevelopment sits with 1-acre or more of land disturbing area, including project less than 1-acre than any part of a larger common plan of development

Planning Department
Suzanne Lubar, Director



February 11, 2016

Richard Dourte, P.E. RHD Engineering, LLC 4305 Purple Sage Ave NW Albuquerque, NM 87120

RE: Monterey Baptist Church
12501 Lomas Blvd NE
Request 90-Day Temporary C.O. - Approved
Engineers Stamp Date 6/17/15 (J22D012A)

Dear Mr. Dourte,

PO Box 1293

Based on the e-mail received today 2/11/2016, the Monterey Baptist Church is acceptable for 90-Day Temporary release of Certificate of Occupancy by Hydrology. The permanent Certificate of Occupancy cannot be issued until the following comments are addressed:

Albuquerque

 A portion of the existing cobble swale that connects to the new pond has been removed and is missing.

New Mexico 87103

If you have any questions, you can contact me at 924-3695 or Totten Elliott at 924-3982.

www.cabq.gov

Sincerely,

Rita Harmon, P.E.

Senior Engineer, Planning Dept. Development Review Services

TE/RH

C: e-mail, Cordova, Camille C.; Connor, Miranda, Rachel; Sandoval, Darlene M.; Blocker, Lois

February 11, 2016

Richard Dourte, P.E. RHD Engineering, LLC 4305 Purple Sage Ave NW Albuquerque, NM 87120

RE: Monterey Baptist Church
12501 Lomas Blvd NE
Request 90-Day Temporary C.O. - Approved
Engineers Stamp Date 6/17/15 (J22D012A)

Dear Mr. Dourte,

Based on the e-mail received today 2/11/2016, the Monterey Baptist Church is acceptable for 90-Day Temporary release of Certificate of Occupancy by Hydrology. The permanent Certificate of Occupancy cannot be issued until the following comments are addressed:

 A portion of the existing cobble swale that connects to the new pond has been removed and is missing.

If you have any questions, you can contact me at 924-3695 or Totten Elliott at 924-3982.

Sincerely,

Rita Harmon, P.E. Senior Engineer, Planning Dept. Development Review Services

TE/RH

C: e-mail, Cordova, Camille C.; Connor, Miranda, Rachel; Sandoval, Darlene M.; Blocker, Lois

Elliott, Stanice

From:

Richard Dourte <rhdengineering@outlook.com>

Sent:

Thursday, February 11, 2016 10:57 AM

To:

Elliott, Stanice; Carrasco, Martin N.

Cc:

Joe Simons; Chris Romero; Patrick Joseph

Subject:

Monterey Baptist Church

Ms. Elliot,

After a field visit to the site yesterday, I can verify that there is a swale on the north side of the sidewalk along Lomas. The swale is not lined or has there been stone place in this swale.

Please issue a temp. CO for this building until the swale has been lined and stone has been placed. Some regrading of the swale entrance to the pond may be needed...

Mr. Carrasco,

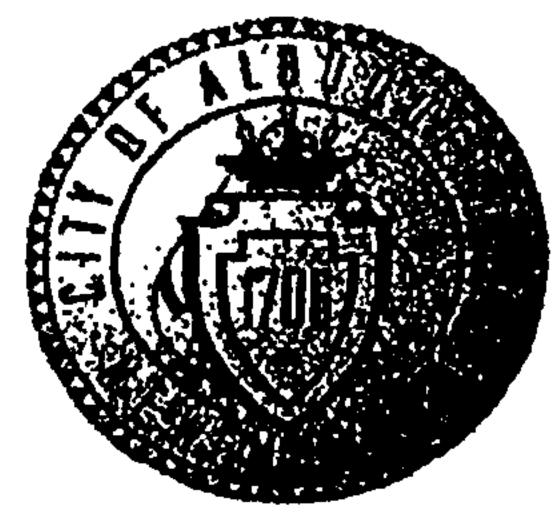
After our discussing this morning it is my understanding that there has not been a resolution on who removed the sidewalk adjacent to the missing stone swale (50ft +/-). You have contacted street maintenance to see if they replaced this section of sidewalk. You will contact us when you receive a response from street maintenance. You also plan on contacting the ABCWUA and see if they removed and replaced this sidewalk.

Another item I would like shed some light on is the sidewalk ordinance. It is my understanding that the adjacent property owner is responsible for the maintenance of their sidewalk. If the sidewalk was replaced due to disrepair of this sidewalk by the City, the church did benefit from this sidewalk replacement. It is not unusual that the cost of doing such work is borne by the adjacent property owner.

Thank you for everyone's help in this matter.

Richard

Sent from Windows Mail



City of Albuquerque

Planning Department

Development & Building Services Division

DRAINAGE AND TRANSPORTATION INFORMATION SHEET

(REV 02/2013)

Project Title: Monterey Baptist Church	Building Permit #:	City Drainage #: 12001 2
DRB#: EPC#:		Work Order#:
Legal Description: Lot 6A, Block D. Monterey Mano	r Subdivision	
City Address: 12501 Lomas Blvd NE		
Engineering Firm: RHD Engineering, LLC		Contact: Richard Dourte
Address: 4305 Purple Sage Ave. NW. Albuquerg	ue. NM. 87120	
Phone#: 505-288-1621 Fax#:		E-mail: rhdengineering@outlook.com
Owner:		Contact:
Address: Phone#: Fax#:		17 22.
Phone#: Fax#:		E-mail:
Architect: Simons Architecture PC	ده چرچ چرچ پر در	Contact: Joe Simons
Address:		
Phone#: 505-480-4796 Fax#:		E-mail: joe@simonsarchitecture.com
Surveyor: Harris Surveys		Contact: Tony Harris
Address:		
Phone#: Fax#:		E-mail:
Contractor: Address:		Contact:
Phone#:		E-mail:
TYPE OF SUBMITTAL:	CHECK TYPE OF APPROVA	WACCEPTANCE SOUGHT!
DRAINAGE REPORT	SIA/FINANCIAL GUARANT	BERELRASE COENTY PINT
· DRAINAGE PLAN 1st SUBMITTAL	PRELIMINARY PLAT APPR	
DRAINAGE PLAN RESUBMITTAL	S. DEV. PLAN FOR SUB'D A	PPROVAL JAN 28 2016
CONCEPTUAL G & D PLAN	S. DEV. POR BLDG. PERMIT	CAPPROVAL JAN Z
GRADING PLAN	SECTOR PLAN APPROVAL	· DEVITE OF MENT SECTION
EROSION & SEDIMENT CONTROL PLAN (ESC)	FINAL PLAT APPROVAL	LAND DEVELOPMENT SECTION
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ENGINEER'S CERT (DRB SITE PLAN)	GRADING PERMIT APPROV	
ENGINEER'S CERT (ESC)	PAVING PERMIT APPROVA	
SO-19	WORK ORDER APPROVAL	ESC CERT. ACCEPTANCE
OTHER (SPECIFY)	ORADING CERTIFICATION	OTHER (SPECIFY)
WAS A PRE-DESIGN CONFERENCE ATTENDED:	Van Y Ma — Ca	me. Dans Jaca
DATE SUBMITTED:/23-/6		py Provided Cl
	By: 14 (1) 0-	

Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a drainage submittal. The particular nature, location, and scope to the proposed development defines the degree of drainage detail. One or more of the following levels of submittal may be required based on the following

1. Conceptual Grading and Drainage Plan: Required for approval of Site Development Plans greater than five (5) acres and Sector Plans
2. Drainage Plans: Required for huilding nermits, grading permits, proving permits and site plans less than five (5) acres and Sector Plans

2. Drainage Plans: Required for building permits, grading permits, paving permits and site plans less than five (5) acres
3. Drainage Report: Required for subdivision containing more than ten (10) lots or constituting five (5) acres or more

4. Erosion and Sediment Control Plan: Required for any new development and redevelopment site with 1-acre or more of land disturbing area, including project less than 1-acre than any part of a larger common plan of development

Harmon Rita T.

To: rhdengineering@outlook.com

Cc: Elliott, Stanice

Subject: J22D012A - Monterey Baptist Church (Stamp Date: 6-17-15)

Mr. Dourte,

This email is being sent in lieu of an attached comment letter in order to expedite the response for initial reviews. Responses to comments should continue to be included in the re-submittal. A reply to this email with responses to comments will not be considered a re-submittal.

Based upon the information provided in your submittal received 1-28-2016, the above referenced plan cannot be approved for Certification of Occupancy by Hydrology until the following comments are addressed:

• A portion of the existing cobble swale that connects to the new pond has been removed and is missing.

If you have any question please do not hesitate to contact me.

Rita Harmon, P.E.

Senior Enginneer, Planning Department Development Review Services 505-924-3695

June 22, 2015



Richard Dourte, P.E. RHD Engineering, LLC 4305 Purple Sage Ave NW Albuquerque, NM 87120

RE: Monterey Baptist Church
12501 Lomas Blvd NE
Grading and Drainage Plan
Engineers Stamp Date 6/17/15 (J22D012A)

Dear Mr. Dourte,

Based upon the information provided in your submittal received 6/12/15, this plan is approved for Building Permit.

Please attach a copy of this approved plan dated 6/17/15 to the construction sets in the permitting process prior to sign-off by Hydrology.

Prior to Certificate of Occupancy release, Engineer Certification per the DPM checklist will be required.

PO Box 1293

If you have any questions, please contact me at 924-3695 or Rudy Rael at 924-3977.

Albuquerque

New Mexico 87103

www.cabq.gov

Sincerely,

Rita Harmon, P.E.

Senior Engineer, Hydrology Planning Department

RR/RH C: File



City of Albuquerque

Planning Department

Development & Building Services Division

DRAINAGE AND TRANSPORTATION INFORMATION SHEET

(REV 02/2013)

Project Title: Monterey Baptist Church	Building Permit #:	City Drainage #: 2200
DRB#:		Work Order#:
Legal Description: Lot 6A. Block D. Monterey Manor	Subdivision	
City Address: 12501 Lomas.Blvd NE		
Engineering Firm: RHD Engineering, LLC		Contact: Richard Dourte
Address: 4305 Purple Sage Ave. NW. Albuquerqu	e. NM. 8/120	
Phone#: 505-288-1621 Fax#:		E-mail: rhdengineering@outlook.com
Owner:		Contact:
Address:		
Phone#:		E-mail:
Architecta Simons Architecture PC		Contact: Joe Simons
Address:		R
Phone#: 505-480-4796 Fax#:		E-mail: joe@simonsarchitecture.com
Surveyor: Harris Surveys		Contact: Tony Harris
Address:	الارداد المراجع المراج - المراجع	
Phone#: Fax#:		E-mail:
Centractor:		Contact:
Address:		
Phone#:		E-mail:
TYPE OF SUBMITTAL:	CHECK TYPE OF APPROVA	LUACCEPTANCE SOUGHT:
DRAINAGE REPORT	SIA/FINANCIAL GUARANT	
DRAINAGE PLAN 1st SUBMITTAL	PRELIMINARY PLAT APPR	1/11/ [P 11 15 11 \\// 1P 1 \
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CONCEPTUAL G & D PLAN	S. DEV. POR BLDG. PERMI	TAPPROVAL JUN 1/2015
GRADING PLAN	SECTOR PLAN APPROVAL	. [00]
EROSION & SEDIMENT CONTROL PLAN (ESC)	FINAL PLAT APPROVAL	NCY (PERM) LAND DEVELOPMENT SECTION
ENGINEER'S CERT (HYDROLOGY)	CERTIFICATE OF OCCUPA	
CLOMR/LOMR	CERTIFICATE OF OCCUPATION REPLACE A RE	
TRAFFIC CIRCULATION LAYOUT (TCL)	POUNDATION PERMIT APPRO	
ENGINEER'S CERT (TCL)	GRADING PERMIT APPROV	
ENGINEER'S CERT (DRB/SITE PLAN) ENGINEER'S CERT (ESC)	PAVING PERMIT APPROVA	
SO-19	WORK ORDER APPROVAL	and the second s
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Requests for approvals of Site Development Plans and/or Subdivision		a submittel. The certicular nature incetion and

scope to the proposed development defines the degree of drainage detail. One or more of the following levels of submittal may be required based on the following

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Erosion and Sediment Control Plant Required for any new development and redevelopment sits with 1-acre or more of land disturbing area, including project less than i-sare than any part of a larger common plan of development

Drainage Report

For

Monterey Baptist Church 12501 Lomas Blvd NE Albuquerque, New Mexico

Prepared by

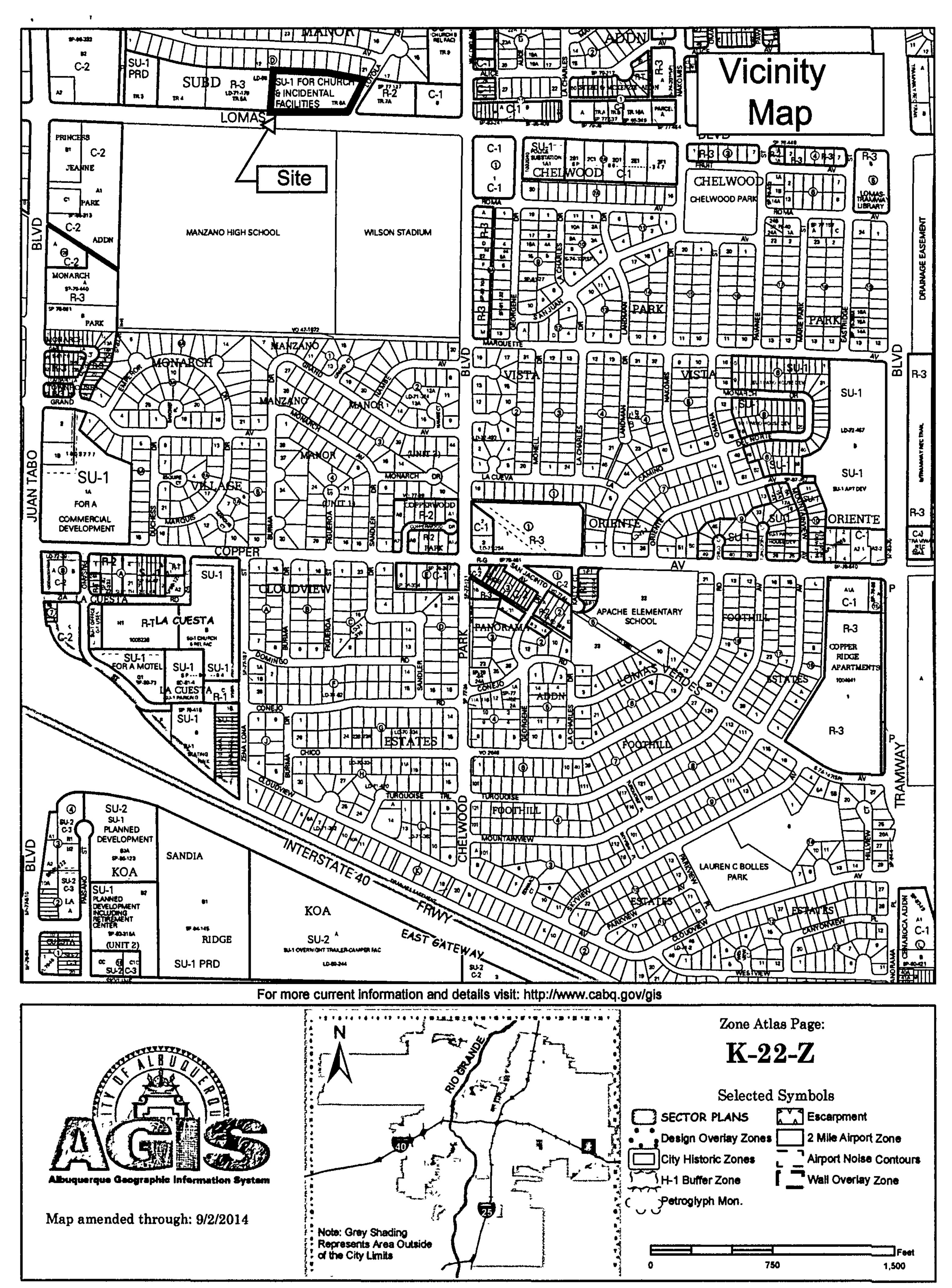
RHD Engineering, LLC Albuquerque, New Mexico

June 2015



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Existing Conditions	page 2
Proposed Conditions	page 2
Summary	page 3
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Nyloplast 12" Pedestrian Grate Inlet Capacity Chart Append	dix B
Site Grading Plan	1:
Die Oraging i am Append	



Page 1

Purpose:

The purpose of this drainage report is to provide a drainage management plan for the proposed changes to the Monterey Baptist Church. The site is approximately 2.92 acres in size. The proposed changes include increasing the square footage of the existing building by approximately 1100 sf. This drainage report and plan is prepared utilizing the City of Albuquerque Development Process Manual.

Introduction:

This site is located at 12501 Lomas Boulevard NE on zone atlas K-22.

The Monterey Baptist Church is presently developed, the proposed new improvements include the addition of approximately 1,100 sf of new building on the east side of the existing structure. The entrance into this church will also be reconfigured. No changing or altering of the parking lot is anticipated, thus the drainage pattern for this parking lot will remain the same.

FEMA Firm map 35001C0359G identifies that this site is located in zone x. Please refer to the drainage plan.

Existing Drainage Conditions:

Approximately half of this site drains (sheet flows) into an existing pond located at the southwest corner of this site discharges that has a controlled outlet into Lomas Boulevard. The majority of the remaining portion of this site free discharges into an existing cobble stone swale that runs along the north side of the sidewalk that is adjacent to Lomas Boulevard and empties into the existing pond located at the southwest corner of this site, and it discharges as mentioned above.

Proposed Conditions:

The proposed conditions for the majority of the site will remain as they are today.

The runoff from drainage basin A will discharge into two inlets, located within the adjoining sidewalk area of the new addition. The pipe from these catch basins will daylight onsite and discharge into the cobble stone swale that empties into the pond at the southwest corner of the site.

The runoff from drainage basin B will sheet flow toward the southeast corner of this basin and discharge into Lomas blvd into the cobble stone swale that empties into the pond at the southwest corner of the site.

The runoff from drainage basin C will flow onto drainage basin A and discharge and discharge into the inlets.

The required first flush volume of stormwater generated by the new impervious areas (1,100 sf addition) is 31cf.

Summary:

This site will discharge the stormwater flows in the same manner as it is today.

For Basin A

The peak flows will increase from .34 cfs to .39 cfs or by 0.05 cfs for the 100 yr 6 hr. event.

The excess precipitation will increase from 529 cf to 642 cf or by 113 cf for the 100 yr 6 hr event.

For Basin B

The peak flows will increase from .45 cfs to .49 cfs or by 0.04 cfs for the 100 yr 6 hr. event.

The excess precipitation will increase from 748 cf to 843 cf or by 95 cf for the 100 yr 6 hr event.

For Basin C

The peak flows will remain the same at 0.32 cfs.

The excess precipitation will remain the same at 594 cf.

The City of Albuquerque's first flush requirements will be adhered to for the construction of the proposed improvements. The first flush for the proposed improvements is 31 cf. The existing pond is going to be increased by 140cf, thus the increased volume of the pond is greater than the first flush requirements.

APPENDIX A

Drainage Calculations for Basin D

Zone 4 (100yr, 6hr)

Land Treatment	Peak discharge	Excess Precipitation
Type A -	2.20 cfs/ac	0.80 inches
Type B -	2.92 cfs/ac	1.08 inches
Type C -	3.73 cfs/ac	1.46 inches
Type D -	5.25 cfs/ac	2.64 inches

Existing Conditions for Basin A

Impervious area (type D)= 800 sf = 0.018 ac

Area other than Impervious (type C)= 3,700 sf - 800 sf = 2900 sf = .067 ac

Peak Flow generated

Excess Precipitation

Existing Conditions for Basin B

Impervious area (type D)= 2160 sf = 0.050 ac

Area other than Impervious (type C)= 4400 sf - 2160 sf = 2240 sf = .051 ac

Peak Flow generated

Excess Precipitation

$$2160 \text{ sf x } 2.64 \text{ in}/12 = 475 \text{ cf}$$

 $2240 \text{ sf x } 1.46 \text{ in}/12 = 273 \text{ cf}$
Total = 748 cf

Existing Conditions for Basin C

Impervious area (type D)= 2700 sf = 0.06 ac

Peak Flow generated

$$.060 \text{ ac } x 5.25 \text{ cfs} = .32 \text{ cfs}$$

Total = .32 cfs

Excess Precipitation

2700 sf x 2.64 in/12 = 594 cf

Total = 594 cf

Proposed Conditions for Basin A

Impervious area (type D)= 1950 sf = 0.045 ac

Area other than Impervious (type C)= 3,700 sf - 1950 sf = 1750 sf = .040 ac

Peak Flow generated

$$.045 \text{ ac } x 5.25 \text{ cfs} = .24 \text{ cfs}$$

 $.040 \text{ ac } x 3.73 \text{ cfs} = .15 \text{ cfs}$
Total = .39 cfs

Excess Precipitation

Proposed Conditions for Basin B

Impervious area (type D)= 3125 sf = 0.072 ac

Area other than Impervious (type C)= 4400 sf - 3125 sf = 1275 sf = .029 ac

Peak Flow generated

$$.072 \text{ ac } x 5.25 \text{ cfs} = .38 \text{cfs}$$

 $.029 \text{ ac } x 3.73 \text{ cfs} = .11 \text{ cfs}$
Total = .49 cfs

Excess Precipitation

$$3125 \text{ sf x } 2.64 \text{ in}/12 = 688 \text{ cf}$$

 $1275 \text{ sf x } 1.46 \text{ in}/12 = 155 \text{ cf}$
Total = 843 cf

First flush requirements (for new impervious area)

$1,100 \text{ sf } \times 0.34 \text{ inches}/12 = 31 \text{ cf.}$

Increase in pond volume

The area of pond at contour 5623 is 280 sf, by increasing the depth of the pond by 0.5 ft, the increased volume is $280/2 \times .5 = 140 \text{ cf}$.

Inlet Grate Capacity (see Appendix B)

Each grate @ 0.1 ft depth has a capacity of .40 cfs, with a clogging factor of 50%, each grate will have a capacity of 0.20cfs. Thus four inlet at 0.20 cfs capacity each for a total capacity of 0.80 cfs >0.71cfs (0.39 cfs + 0.32cfs) needed for drainage Basin A.

6 inch PVC capacity

$$Q = 1.49 \times A \times R^{5}.67 \times S^{5}.5$$

Area =
$$0.20 \text{ sf}$$

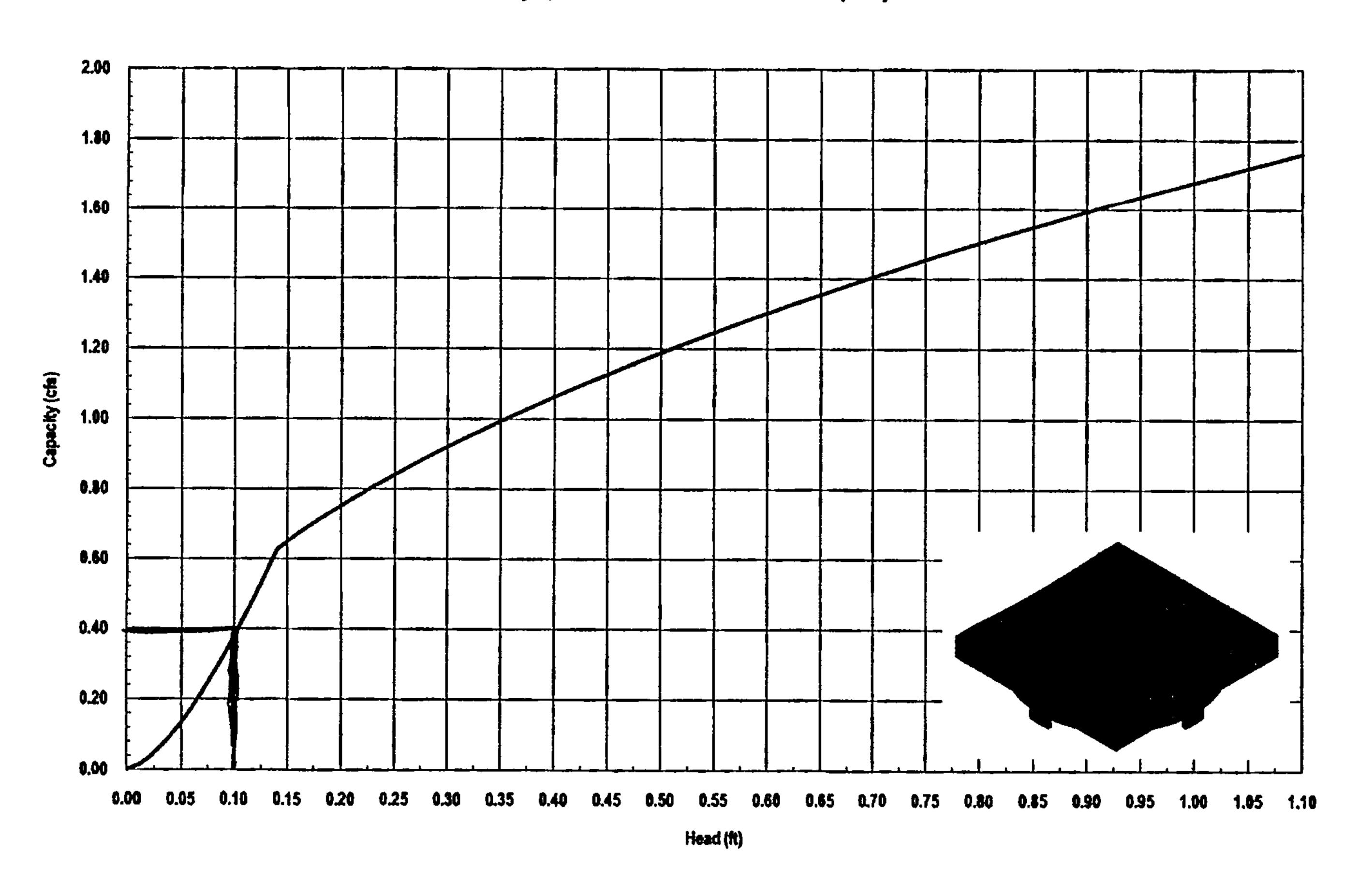
R = A/P = $0.20/1.57 = 0.13$
S = 0.01
n = $.009$

$$Q = 1.49(0.20)((0.13)^{.67})((.01)^{.5})$$
.009

$$Q = 0.84 cfs$$

Thus the capacity of the 6 inch pvc pipe, 0.84 cfs > 0.71 cfs required for Basin A. Use an 8 inch pvc pipe

Nyloplast 12" Pedestrian Grate Inlet Capacity Chart



NYIODIAST
3130 Verona Avenue • Buford, GA 30518
(866) 888-8479 / (770) 932-2443 • Fax: (770) 932-2490
© Nyloplast Inlet Capacity Charis June 2012

Appendix B



July 10, 2013

David Soule, PE

Rio Grande Engineering
PO Box 93924
Albuquerque, NM 87199

Re: Monterey Baptist Church

12501 Lomas Blvd NE

Request for Permanent C.O. -Accepted

Engineer's Stamp dated: 1-23-12, (J22/D012A)

Certification dated: 7-3-13

Dear Mr. Soule,

Based upon the information provided in the Certification received 7-03-13, the above referenced Certification is acceptable for a release of a Permanent Certificate of Occupancy by Hydrology.

PO Box 1293

Hydrology is asking for an electronic copy, in .pdf format, of this certification for our records. This certification can be e-mailed to me at: ccherne@cabq.gov or rrael@cabq.gov.

If you have any questions, you can contact me at 924-3986 or Rudy Rael at 924-3977.

Albuquerque

New Mexico 87103

New Mexico 8/103

www.cabq.gov

Sincerely,

Curtis A. Cherne, P.E.

Principal Engineer—Hydrology Section Development and Building Services

RR/CC

C: CO Clerk—Katrina Sigala

e-mail

DRAINAGE AND TRANSPORTATION INFORMATION SHEET

(REV. 01/28/2003rd)

PROJECT TITLE: DRB #:	Monterey Baptist Church EPC #:		ONE MAP/D	PRG. FILE #: <u>J22/ D012A</u> ER #:
LEGAL DESCRIPTION:	tract 6 monterrey Mannor			
CITY ADDRESS:	12501 Lomas NE			
ENGINEERING FIRM:	Rio Grande Engineering	C	ONTACT:	David Soule, PE
ADDRESS:	po box 93924		HONE:	(505)321-9099
CITY, STATE:	Albuquerque, New Mexico		P CODE:	87199
			ONTA OT	
OWNER:	Montery baptist church		ONTACT:	
ADDRESS:	12501 Lomas ne		HONE:	OT440
CITY, STATE:	Albuquerque, NM 87102		P CODE:	87112
ARCHITECT:	Joe simmons.	C	ONTACT:	
ADDRESS:		Pł	HONE:	·
CITY, STATE:		ZI	P CODE:	
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SURVEYOR:	Geo surv co		ONTACT:	
ADDRESS:			HONE:	
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DRAINAGE PLA	PORT AN 1st SUBMITTAL, <i>REQUIRES TCL or equal</i> AN RESUBMITTAL GRADING & DRAINAGE PLAN		SIA / FINANA PRELIMINAR S. DEV. PLA S. DEV. PLA SECTOR PL	PROVAL SOUGHT: ACIAL GUARANTEE RELEASE RY PLAT APPROVAL IN FOR SUB'D. APPROVAL IN FOR BLDG. PERMIT APPROVAL AN APPROVAL AN APPROVAL
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CLOMR/LOMR				ERMIT APPROVAL
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DATE SUBMITTED:	6/6/2013	BY:	4	David Soule

Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a dranage submittal.

The particular nature, location and scope of the proposed development defines the degree of drainage detail.

One or more of the following levels of sumbittal may be required based on the following:

- 1. Conceptual Grading and Drainage Plans: Required for approval of Site Development Plans greater than five (5) acres and Sector Plans.
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- 3. Drainage Report: Required for subdivisions containing more than ten (10) lots or constituting five (5) acres or more.



Planning Department Transportation Development Services Section

July 3, 2013

Joseph F. Simons, R.A. Simons Architecture.com PO Box 67408 Cedar Crest, NM 87193-7408

Re:

Request for Certificate of Occupancy for

Monterey Baptist Church (J-22/D012A)

12501 Lomas NE

Architect's Stamp dated 05-30-13

Dear Mr. Simons,

Based upon the information provided in your submittal received 07-03-13, Transportation Development has no objection to the issuance of a <u>Permanent Certificate of Occupancy</u>. This letter serves as a "green tag" from Transportation Development for a <u>Permanent Certificate of Occupancy</u> to be issued by the Building and Safety Division.

PO Box 1293

If you have any questions, please contact me at (505)924-3630.

Albuquerque

www.cabq.gov

New Mexico 87103

Jilo E. Salgado-Fernandez, P.E.

Senior Traffic Engineer

Development Review Services

Planning Department

C.

File

Since Ret

Hydrology

CO Clerk



City of Albuquerque

Planning Department

Development & Building Services Division

DRAINAGE AND TRANS		TION SHEET # 22/10/12
•	(REV 02/2013)	
Project Title: MONTEPELL BAPTIST CH	Wett Building Permit#:	20129062 City Drainage #:
DRB#: 1006921 EPC#:		Work Order#:
Legal Description: TRACT G-A BLOCK	-D, MONTEPE	MANDES SUBPINISION
City Address: 12501 Lowas AUF A	BO. HM	
Engineering Firm: PLO GPANOE ENG	NEFINE	Contact: DAVID SOULE
Address: 1606 CENTRUL AVIE		BQ 97106
Phone#: 321-9099 Fax#:		E-mail: david @ riogrande
Owner: MONTEPEN BAPTIST CHI	JA-CH	Contact: John Lystpan
Address: 12501 LONAS BLVD. NE		
Phone#: 291-7679 Fax#:		E-mail: monterey be equestoffice.
Architect: SIMONS APCHITECTU	2 E PC	Contact: USE SIMONIS NET
Address: P.O. 120x 67408 ARD.	and the second s	
Phone#: 480-4796 Fax#:		E-mail: 160 Simons
Surveyor:;		drohitecture.com
Address:	······································	Contact:
Phone#:		· E-mail:
Contact 12 Contact Con	•	
Contractor: BP117012 C0135TP12CTTC		Contact: FEVIN BRITTON
Phone#: 268-2626 Fax#:		E-mail: Kbritton@britton
TYPE OF SUBMITTAL:	CHECK TUDE OF ADDROY	E-mail: Kbritton@britton Construction.com AL/ACCEPTANCE SOUGHTOTE
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GRADING PLAN		
EROSION & SEDIMENT CONTROL PLAN (ESC)	FINAL PLAT APPROVAL	THE VELOPMENT SECTION
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ENGINEER'S CERT (TCL)	BUILDING PERMIT APPRO	
ENGINEER'S CERT (DRB SITE PLAN)	GRADING PERMIT APPRO	
ENGINEER'S CERT (ESC)	PAVING PERMIT APPROVA	
SO-19	WORK ORDER APPROVAL	
OTHER (SPECIFY)	GRADING CERTIFICATION	
WAS A PRE-DESIGN CONFERENCE ATTENDED:		py Provided
DATE SUBMITTED: 6.5.13 By	: LOE SIMONS	JAIA

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PO Box 67408 ALBUQUERQUE, NM 87193-7408 JFS @ SIMONSARCHITECTURE.COM

505.480.4796 VOICE

May 30, 2013

TRAFFIC CERTIFICATION

I, JOE SIMONS JR., NMRA 002890, OF THE FIRM SIMONS ARCHITECTURE PC, HEREBY CERTIFY THAT THIS PROJECT IS IN SUBSTANTIAL COMPLIANCE WITH AND IN ACCORDANCE WITH THE DESIGN INTENT OF THE DRB, AA OR TCL APPROVED PLAN DATED 1/27/2012. THE RECORD INFORMATION EDITED ONTO THE ORIGINAL DESIGN DOCUMENT HAS BEEN OBTAINED BY JOE SIMONS JR., OF THE FIRM SIMONS ARCHITECTURE. I FURTHER CERTIFY THAT I HAVE PERSONALLY VISITED THE PROJECT SITE ON 5/29/13 AND HAVE DETERMINED BY VISUAL INSPECTION THAT THE SURVEY DATA PROVIDED IS REPRESENTATIVE OF ACTUAL SITE CONDITIONS AND IS TRUE AND CORRECT TO THE BEST OF MY KNOWLEDGE AND BELIEF. THIS CERTIFICATION IS SUBMITTED IN SUPPORT OF A REQUEST FOR CERTIFICATE OF OCCUPANCY (PERMANENT). ANY ALTERATIONS TO THE APPROVED TCL IS NOTED IN RED ON THE ATTACHED PLAN AND IS NOTED AS FOLLOWS:

THE RECORD INFORMATION PRESENTED HERON IS NOT NECESSARILY COMPLETE AND INTENDED ONLY TO VERIFY SUBSTANTIAL COMPLIANCE OF THE TRAFFIC ASPECTS OF THIS PROJECT. THOSE RELYING ON THE RECORD DOCUMENT ARE ADVISED TO OBTAIN INDEPENDENT VERIFICATION OF ITS ACCURACY BEFORE USING IT FOR ANY OTHER PURPOSE.

Joseph F.
Simons, Jr.
No. 2890
RG, 30-13-140

REC. STERED ARCHITECT

Signature of Engineer or Architect

5.30.2013

Date

Salgado-Fernandez, Nilo E.

From: Salgado-Fernandez, Nilo E.

Sent: Tuesday, July 02, 2013 3:49 PM

To: 'Simons Architecture'

Subject: RE: Permanent CO for 12501 Lomas Blvd. NE

Mr. Simons, in accordance with the Drainage and Transportation Information Sheet a Temporary C.O. was

requested and checked on this sheet. So a temporary CO was approved by our office.

Nilo Salgado-Fernandez

From: Simons Architecture [mailto:jfs@simonsarchitecture.com]

Sent: Tuesday, July 02, 2013 3:35 PM

To: Salgado-Fernandez, Nilo E.

Subject: Permanent CO for 12501 Lomas Blvd. NE

Hello Nilo

We talked on the phone earlier today regarding the final C.O. for this property. The address is 12501 Lomas Blvd. NE. The file number is 11-10136. The project number is 1006921.

Last month I provided a the letter request for the permanent certificate of occupancy. You granted a temporary 120 day which is totally fine but we now need the permanent C.O. letter.

The only traffic room that was done was paving a fire turnaround and striping stating that there was no parking in that turnaround. That was completed at the time of the Certificate of Occupancy letter.

Attached is another copy of the letter I submitted to you last month and a copy of the approved site plan.

We are looking to get the permanent letter tomorrow if that is possible. I can come by before lunch or after lunch whichever is convenient for you.

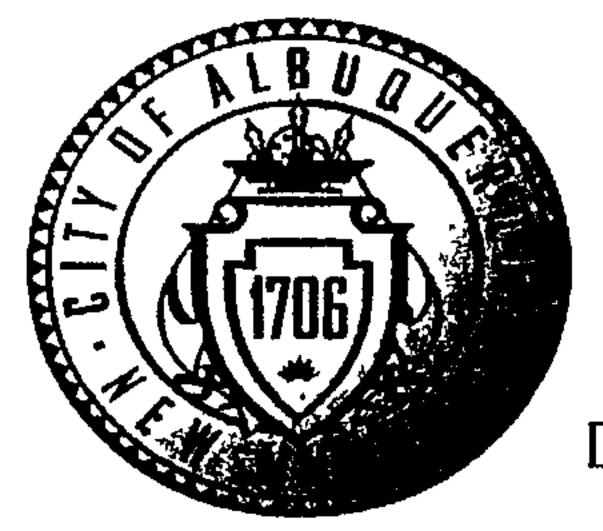
Thanks so much for the help you always provide on these projects.

Joe Simons, AIA



Building Design With Integrity

Mr. Joe Simons Jr.
Simons Architecture
architect aia
(505) 480-4796 Work
(505) 480-4796 Mobile
jfs@simonsarchitecture.com
P.O. Box 67408
Albuquerque, NM 87193-7408
http://www.simonsarchitecture.com



City of Albuquerque

Planning Department

Development & Building Services Division

DRAINAGE AND TRANSPORTATION INFORMATION SHEET

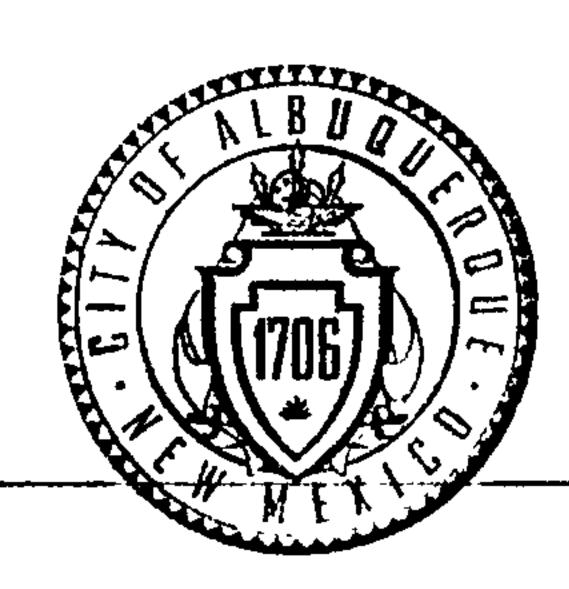
J-22/D012A

(REV 02/2013)

Project Title: Monterey Baptist Church	Building Permit #: 201229062: City Drainage #:
DRB#: 1006921 EPC#:	Work Order#:
Legal Description: Tract 6-A, block D, Monterey	Manor Subdivision
City Address: 12501 Lomas, NE, Abq., NM	
Engineering Firm: Rio Grande Engineering	Contact: David Soule
Address: 1606 Central Ave., SE, #201, Abq., 8	
Phone#: 321-9099 Fax#:	E-mail: david@riograndeengineerin
Owner: Monterey Baptist Church	Contact:
Address: 12501 Lomas Blvd., NE, Abq., NM	
Phone#: 294-7679 Fax#:	E-mail: montereybc@qwestoffice.ne
Architect: Simons Architecture PC	Contact:
Address: P.O. Box 67408, Abq., NM 87103	
Phone#: 480-4796 Fax#:	E-mail: ifs@simonsarchitecture.con
Surveyor:	Contact:
Address:	Contact.
Phone#:	E-mail:
Contractor: Britton Construction	Contact: Kevin Britton
Address:	E mail. Ichnittan@hnittanaanatmaatia
Phone#: 268-2626 Fax#:	E-mail: kbritton@brittonconstructio
TYPE OF SUBMITTAL:	CHECK TYPE OF APPROVAL/ACCEPTANCE SOUGHT:
DRAINAGE REPORT	SIA/FINANCIAL GUARANTEE RELEASE
DRAINAGE PLAN 1st SUBMITTAL	PRELIMINARY PLAT APPROVAL
DRAINAGE PLAN RESUBMITTAL	S. DEV. PLAN FOR SUB'D APPROVAL
CONCEPTUAL G & D PLAN	S. DEV. FOR BLDG. PERMIT APPROVAL
GRADING PLAN	SECTOR PLAN APPROVAL
EROSION & SEDIMENT CONTROL PLAN (ESC)	FINAL PLAT APPROVAL
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WAS A PRE-DESIGN CONFERENCE ATTENDED:	Yes No Copy Provided
DATE SUBMITTED: 7.3.13	By: Joe Simons Jr., AIA

Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a drainage submittal. The particular nature, location, and scope to the proposed development defines the degree of drainage detail. One or more of the following levels of submittal may be required based on the following

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- 4. Erosion and Sediment Control Plan: Required for any new development and redevelopment site with 1-acre or more of land disturbing area, including project less than 1-acre than are part of a larger common plan of development



Planning Department Transportation Development Services Section

June 6, 2013

Joseph F. Simons, R.A. Simons Architecture.com PO Box 67408 Cedar Crest, NM 87193-7408

Re:

Request for Certificate of Occupancy for Monterey Baptist Church (J-22/D012A)

12501 Lomas NE

Architect's Stamp dated 05-30-13

Dear Mr. Simons,

The issuance of a <u>120-day Temporary Certificate of Occupancy</u> has been provided until all minor modification has been completed. This letter serves as a "green tag" from Transportation Development for a <u>120-day Temporary Certificate</u> <u>of Occupancy</u> to be issued by the Building and Safety Division.

PO Box 1293

Once all modifications have been completed, please resubmit an acceptable package along with fully completed Drainage Information Sheet to front counter personnel for log in and evaluation by Transportation for a Permanent C.O. submittal.

Albuquerque

If you have any questions, please contact me at (505)924-3630.

New Mexico 87103

www.cabq.gov

Wild E. Salgado-Fernandez, P.E.

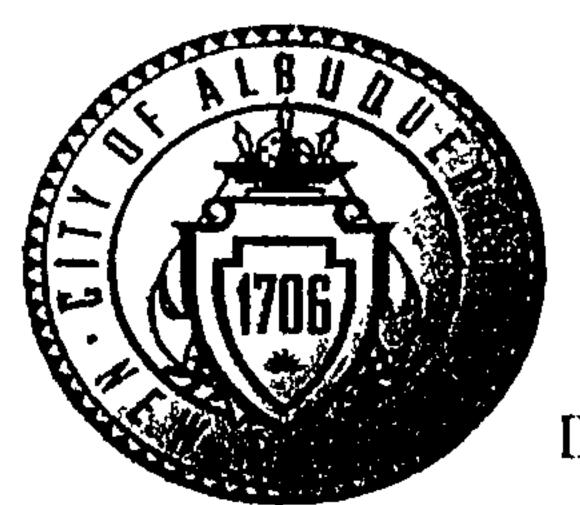
Symor Traffic Engineer

Bévelopment Révièw Services

/Planning Department

Singerelly

File, Hydrology, and CO Clerk



DATE SUBMITTED:

City of Albuquerque

Planning Department

Development & Building Services Division

DRAINAGE AND TRANSPORTATION INFORMATION SHEET (REV 02/2013) HAPCH Building Permit #: 20129062 City Drainage Project Title: MONTEP EPC#: Work Order#: Legal Description: Engineering Firm: PLO GPANCE ENGINEEPING Contact: ひかいの らのにし ほ 1606 CENTRAL AVE SE 87106 Phone#: 321-9099 E-mail: davide riogrande Fax#: engineering, com BAPTIST CHUPCH Contact: JOHN NYSTPON Address: 12501 LONIAS BLVD. NE ABO NH E-mail: monterente equestoffice. Phone#: 294-7679 Fax#: net SIMON'S APCHITECTURE Contact: UOF SIMONS Architect: P.O. 120x 67408 ARD. NM 480-4796 Phone#: Fax#: Surveyor: Contact: Address: Phone#: Fax#: • E-mail: Contractor: Contact: KEVIN BP1110N Address: E-mail: Kbritton@britton Phone#: 268-2626 Fax#: construction.com TYPE OF SUBMITTAL: CHECK TYPE OF APPROVAL/ACCEPTANCE SOUGHT: DRAINAGE REPORT SIA/FINANCIAL GUARANTEE RELEASE DRAINAGE PLAN 1st SUBMITTAL PRELIMINARY PLAT APPROVAL DRAINAGE PLAN RESUBMITTAL S. DEV. PLAN FOR SUB'D APPROVAL CONCEPTUAL G & D PLAN S. DEV. FOR BLDG. PERMIT APPROVAL GRADING PLAN SECTOR PLAN APPROVAL EROSION & SEDIMENT CONTROL PLAN (ESC) FINAL PLAT APPROVAL ENGINEER'S CERT (HYDROLOGY) CERTIFICATE OF OCCUPANCY (PERM) CLOMR/LOMR CERTIFICATE OF OCCUPANCY (TCL TEMP) TRAFFIC CIRCULATION LAYOUT (TCL) FOUNDATION PERMIT APPROVAL ENGINEER'S CERT (TCL) BUILDING PERMIT APPROVAL ENGINEER'S CERT (DRB SITE PLAN) GRADING PERMIT APPROVAL SO-19 APPROVAL ENGINEER'S CERT (ESC) PAVING PERMIT APPROVAL ESC PERMIT APPROVAL SO-19 WORK ORDER APPROVAL ESC CERT. ACCEPTANCE OTHER (SPECIFY) **GRADING CERTIFICATION** OTHER (SPECIFY) WAS A PRE-DESIGN CONFERENCE ATTENDED: Yes No Copy Provided

Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a drainage submittal. The particular nature, location, and scope to the proposed development defines the degree of drainage detail. One or more of the following levels of submittal may be required based on the following

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By:

- 2. Drainage Plans. Required for building permits, grading permits, paving permits and site plans less than five (5) acres
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P.O. Box 67408 **Albuquerque, NM 87193-7408** 505-480-4796 voice

> is@simonsarchitecture.com www.simonsarchitecture.com

September 30, 2011

City of Albuquerque Planning Department **Environmental Planning Commission** Design Review Board 600 2nd St. NW Albuquerque, NM 87103

Re:

Letter of Authorization Monterey Baptist Church 12501 Lomas Blvd. NE

Albuquerque, NM 87112

To Whom It May Concern:

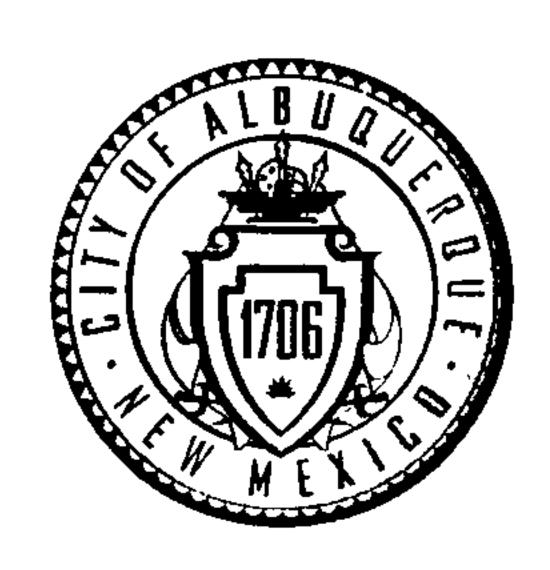
This letter authorizes David Soule of Rio Grande Engineering and/or Joe Simons of Simons Architecture to act as our representative during the City of Albuquerque Planning Department EPC / DRB process for our church located at 12501 Lomas Blvd. Ne, Albuquerque, New Mexico 87112.

Sincerely,

We Cone

Senior Pastor

Monterey Baptist Church



January 27, 2012

David Soule, P.E. Rio Grande Engineering P.O. Box 93924 Albuquerque, NM 87199

Re: Monterey Baptist Church Grading and Drainage Plan, Engineer's Stamp Date 1/27/2012 (J22/D012A)

Dear Mr. Soule,

Based upon the information provided in your submittal received 1-27-12, the Grading and Drainage Plan is approved for Building Permit, Grading Permit, and SO-19 Permit.

PO Box 1293

Albuquerque

A separate SO-19 permit is required for construction within City ROW. A copy of this approval letter must be on hand when applying for the excavation/barricading permit. The work in the City ROW must be inspected and accepted. Contractor must contact Martin Pacheco at 235-8016 and Construction Coordination at 924-3416 to schedule an

inspection.

NM 87103

Please attach a copy of this approved plan to the construction sets prior to sign-off by Hydrology. Prior to Certificate of Occupancy release, Engineer Certification per the DPM checklist and a Green Tag for the SO-19 permit will be required

Sincerely,

www.cabq.gov

If you have any questions, you can contact me at 924-3695.

Shahab Biazar, P.E.

Senior Engineer, Planning Dept.

Development and Building Services

C: File

DRAINAGE AND TRANSPORTATION INFORMATION SHEET

(REV. 01/28/2003rd)

PROJECT TITLE:	Monterey Baptist Churc	<u>h</u>		DRG. FILE #: <u>J22/ D012A</u>
DRB #:	EPC #:		WORK ORD	E <u>#:</u>
LEGAL DESCRIPTION: CITY ADDRESS:	tract 6 monterrey Manne 12501 Lomas NE)r		
OH I ADDICEOU.	12301 LUMAS ML			
ENGINEERING FIRM:	Rio Grande Engineering		CONTACT:	David Soule, PE
ADDRESS:	po box 93924	· · · · · · · · · · · · · · · · · · ·	PHONE:	(505)321-9099
CITY, STATE:	Albuquerque, New Mexico		ZIP CODE:	87199
			00NTA0T	
<u>OWNER:</u> ADDRESS:	Montery baptist church		CONTACT:	
CITY, STATE:	12501 Lomas ne Albuquerque, NM 87102	——————————————————————————————————————	PHONE: ZIP CODE:	87112
O111, O17112.	Moudaerdae, Militor Toz	· · · · · · · · · · · · · · · · · · ·	ZII CODE.	07 112
ARCHITECT:	Joe simmons.		CONTACT:	
ADDRESS:			PHONE:	-
CITY, STATE:			ZIP CODE:	
SURVEYOR:	Geo surv co		CONTACT:	
ADDRESS:			PHONE:	- · · · · · · · · · · · · · · · · · · ·
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DATE OUDSAITTED	410010040	HYDROLOGY SECTION		
DATE SUBMITTED:	1/26/2012	BY:		David Soule

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The particular nature, location and scope of the proposed development defines the degree of drainage detail.

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RIO GRANDE ENGINEERING OF NEW MEXICO, LLC

January 26, 2012

Mr. Shahab Biazar, PE Senior Engineer Planning Department City of Albuquerque

RE: Grading and Drainage Plan

Monterey Baptist Church (J22/D012A)

Dear Shahab:

The purpose of this letter is to accompany the enclosed grading plan for the referenced project. This plan has been modified to address your verbal comments. The following is a summary of your comments with the annotation as to how the plans were modified to address the comments.

1. Hydrology recommends both basin A and B discharge into detention pond.

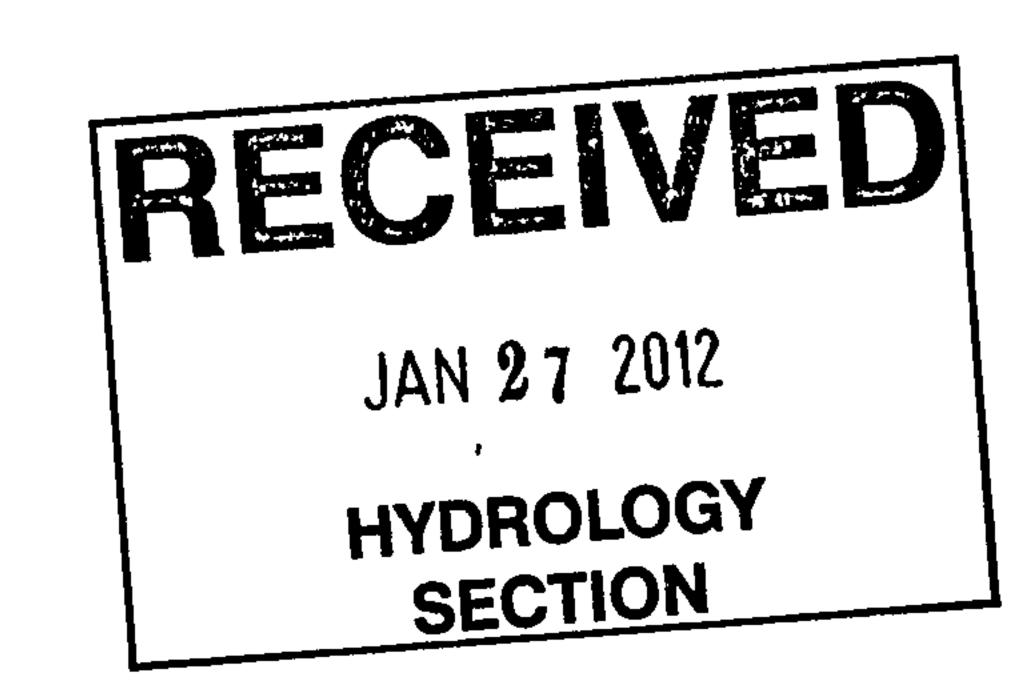
The site grading has been modified so both A and B are route through pond. The outfall has been increased from 8" to a 10". The provided peak discharge rate of 6.28 which is less than if native (7.08). Due to increase in pipe size and flow rate two sidewalk culverts are being proposed

Should you have any questions regarding this resubmittal, please do not hesitate to call me.

Sincerely,

David Soule, PE

Enclosures



VOLUME CALCULATIONS

	ACTUAL ELEV.	DEPTH (FT)	CONTOUR AREA	VOLUME cf	VOLUME AC-FT	Q (CFS)	
		(above outlet)					BOTH PIPES
	5621	0		0		0.000	
	5622.00	0.00	0.00	0.0000	0.0000	0.000	
inv=23.50	5623.00	0.00	1350.00	0.0000	0.0000	0.000	
	5624.00	0.67	2550.00	1950.0000	0.0448	1.322	2.643612141
	5625.00	1.67	4100.00	5275.0000	0.1211	2.940	5.88010206
	5626.00	2.67	5500.00	10075.0000	0.2313	3.942	7.884320853

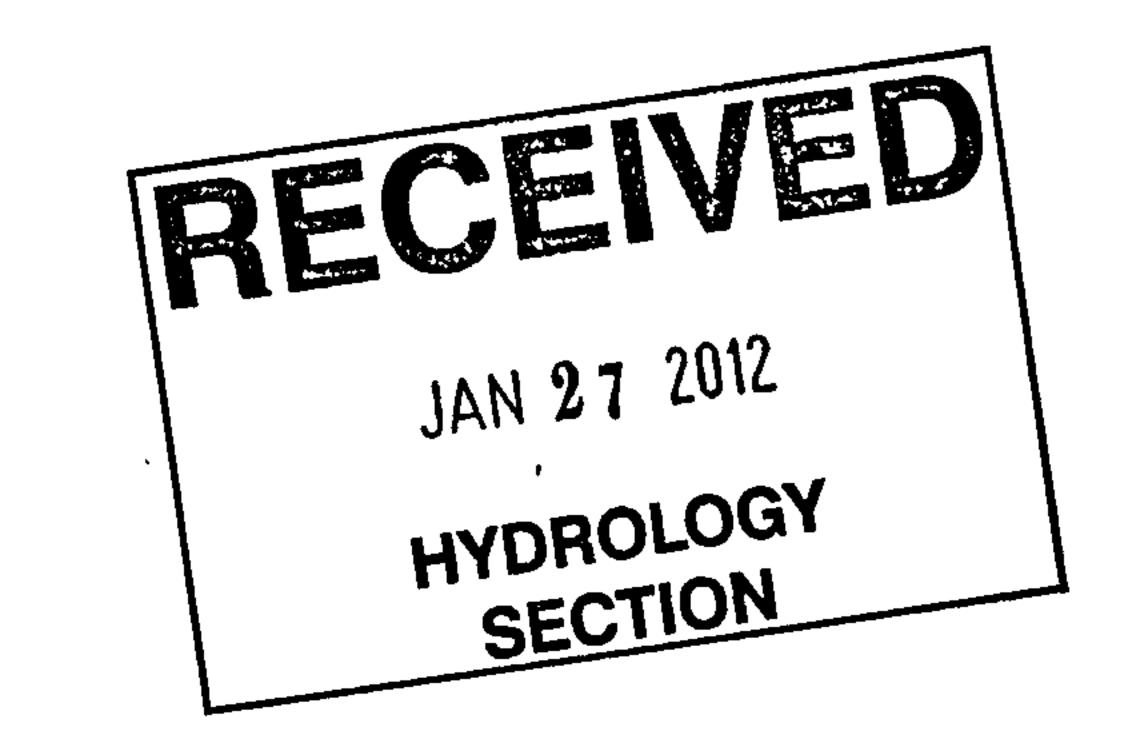
Orifice Equation

Q = CA SQRT(2gH)

C = 0.6Diameter (in) 10
Area (ft^2)= 0.545415391 g = 32.2

H (Ft) = Depth of water above center of orifice

Q(CFS)= Flow



POND 022612.txt

*****S AHYMO - MONTEREY

*****S POND ROUTING

START

TIME=0.0 PUNCH CODE=0

RAINFALL

TYPE=2

QUARTER=0.0 ONE= 2.60 IN

SIX= 3.10 INDAY = 3.95 IN DT = 0.05 HR

COMPUTE NM HYD

ID=1 HYD NO=101 DA= .004647 SQ MI PER A=30 PER B=21 PER C=19 PER D=30

TP=-.20 MASSRAIN=-1

PRINT HYD

ID=1 CODE=3

* ROUTE THE TOTAL FLOW THROUGH THE PROPOSED RESERVOIR

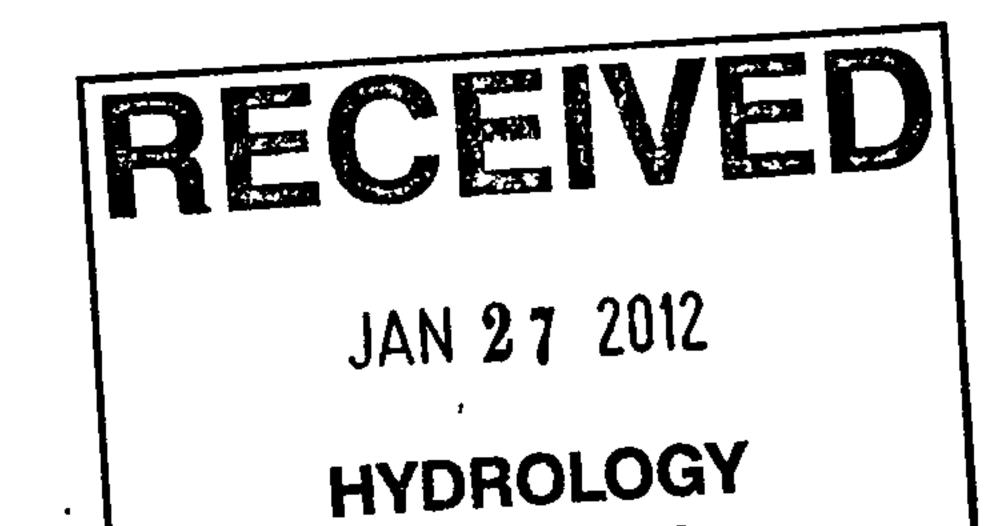
ID=2 HYD NO=102 INFLOW=1 ROUTE RESERVOIR OUTFLOW(CFS) STORAGE(AC-FT) ELEV(FT)

0.0	0.000	23.00	
2.64 5.88	0.045 0.121	24.00 25.00	
7.88	0.231	26.00	

PRINT HYD

ID=2CODE=3

FINISH



SECTION

AHYMO.OUT AHYMO PROGRAM (AHYMO-S4) - Version: S4.01a - Rel: 01a RUN DATE (MON/DAY/YR) = 01/26/2012START TIME (HR:MIN:SEC) = 14:44:14USER NO.= RioGrandeSingleA41963517 and Settings\Owner\Desktop\2011jobs\1162-monterey baptist churc\POND 022612.txt *S AHYMO MONTEREY *****S POND ROUTING TIME=0.0START PUNCH CODE=0 TYPE=2 RAINFALL QUARTER=0.0 ONE= 2.60 INSIX= 3.10 INDAY=3.95 IN DT = 0.05 HR24-HOUR RAINFALL DIST. - BASED ON NOAA ATLAS 14 FOR CONVECTIVE AREAS (NM & AZ) -D1 0.050000 HOURS 24.000002 HOURS END TIME = 0.00000.0033 0.0105 0.0068 0.0145 0.0186 0.0233 0.0299 0.0400 0.0510 0.0624 0.0749 0.0876 0.1007 0.1142 0.1280 0.1434 0.1595 0.1769 0.2078 0.2454 0.2958 0.3529 0.4234 0.5175 0.6233 0.8060 1.0901 1.5769 1.9190 2.1889 2.3245 2.4433 2.5287 2.5966 2.6559 2.6992 2.7390 2.7718 2.7937 2.8101 2.8246 2.8382 2.8502 2.8614 2.8724 2.8830 2.8917 2.8967 2.9157 2.9066 2.9111 2.9017 2.9201 2.9244 2.9286 2.9327 2.9366 2.9405 2.9444 2.9482 2.9518 2.9553 2.9588 2.9657 2.9623 2.9690 2.9722 2.9754 2.9785 2.9816 2.9847 2.9877 2.9907 2.9936 2.9965 2.9994 3.0022 3.0050 3.0078 3.0105 3.0132 3.0159 3.0185 3.0211 3.0237 3.0263 3.0288 3.0313 3.0338 3.0362 3.0387 3.0411 3.0434 3.0458 3.0481 3.0504 3.0527 3.0550 3.0572 3.0594 3.0616 3.0638 3.0660 3.0681 3.0702 3.0723 3.0744 3.0765 3.0785 3.0806 3.0826 3.0846 3.0866 3.0885 3.0905 3.0924 3.0943 3.0962 3.0981 3.1000 3.1024 3.1047 3.1071 3.1094 3.1118

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JAN 27 2012
HYDROLOGY
SECTION

3.1472 3.1496 3.1519 3.1543 3.1567 3.1590 3.1614 3.1661 3.1637 3.1685 3.1708 3.1732 3.1756 3.1779 3.1803 3.1826 3.1850 3.1874 3.1897 3.1921 3.1944 3.1968 3.1992 3.2015 3.2039 3.2062 3.2086 3.2110 3.2157 3.2133 3.2181 3.2204 3.2228 3.2251 3.2275 3.2299 3.2322 3.2346 3.2369 3.2393 3.2417 3.2440 3.2464 3.2487 3.2511 3.2535 3.2558 3.2582 3.2606 3.2629 3.2653 3.2676 3.2700 3.2724 3.2747 3.2771 3.2818 3.2794 3.2842 3.2865 3.2889 3.2912 3.2936 3.2960 3.2983 3.3007 3.3031 3.3054 3.3078 3.3101 3.3149 3.3125 3.3172 3.3196 3.3219 3.3243 3.3267 3.3290 3.3314 3.3337 3.3361 3.3385 3.3408 3.3432 3.3456 3.3479 3.3503 3.3526 3.3550 3.3574 3.3597 3.3621 3.3644 3.3668 3.3692 3.3715 3.3739 3.3762 3.3786 3.3810 3.3833 3.3857 3.3880 3.3904 3.3928 3.3951 3.3975 3.3999 3.4022 3.4046 3.4069 3.4093 3.4117 3.4140 3.4164 3.4187 3.4211 3.4235 3.4258 Page 1

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AHYMO.OUT
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COMPUTE NM HYD ID=1 HYD NO=101 DA= .004647 SQ MI PER A=30 PER B=21 PER C=19 PER D=30 TP=-.20 MASSRAIN=-1

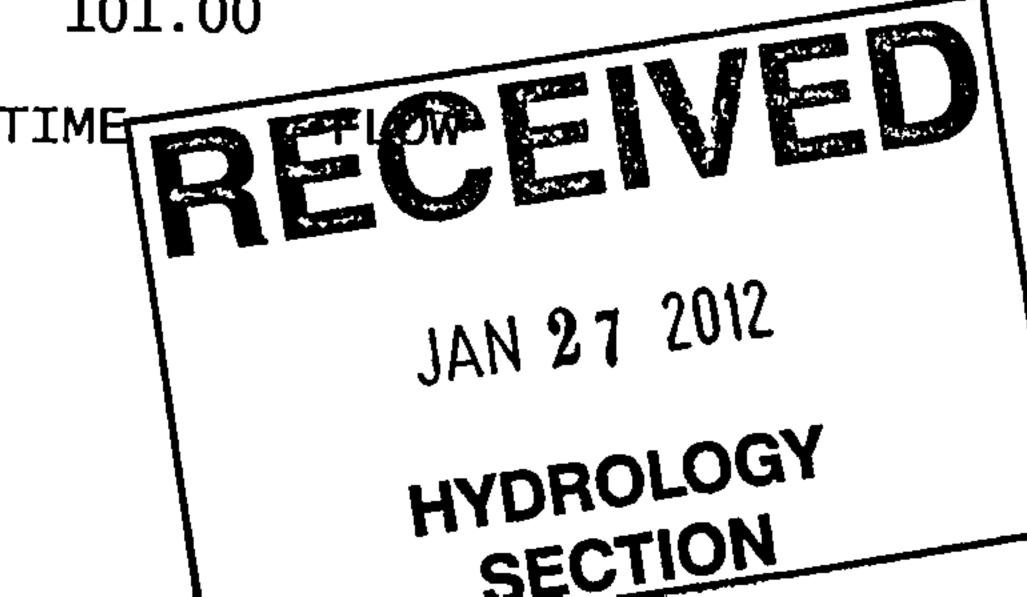
K = 0.109000HR TP = 0.200000HR K/TP RATIO = 0.545000 SHAPE CONSTANT, N = 7.106428 UNIT PEAK = 3.6684 CFS UNIT VOLUME = 0.9960 B = 526.28 P60 = 2.6000 AREA = 0.001394 SQ MI IA = 0.10000 INCHES INF = 0.04000 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = 0.050000

K = 0.214779HR TP = 0.200000HR K/TP RATIO = 1.073894 SHAPE CONSTANT, N = 3.288961 UNIT PEAK = 4.9541 CFS UNIT VOLUME = 0.9972 B = 304.60 P60 = 2.6000 AREA = 0.003253 SQ MI IA = 0.52357 INCHES INF = 1.31600 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD ~ DT = 0.050000

PRINT HYD ID=1 CODE=3

PARTIAL HYDROGRAPH 101.00

TIME FLOW TIME FLOW Page 2



TTME	ET (A)44	TT TO A A IT	AHYMO	O.OUT		
TIME	FLOW HRS	CFS	FLOW HRS	CFS	HRS	CFS
HRS	0.000	0.0	CFS 5.100	0.0	10.200	. 0.0
15.300	0.0 0.150	0.0	0.0 5.250	0.0	10.350	0.0
15.450	0.0	0.0	0.0 5.400	0.0	10.500	0.0
15.600	0.0 0.450	0.0	0.0 5.550	0.0	10.650	0.0
15.750	0.0 0.600	20.850 0.0	0.0 5.700	0.0	10.800	0.0
15.900	0.0 0.750	0.0 0.0	0.0 5.850	0.0	10.950	0.0
16.050	$0.0 \\ 0.900$	21.150 0.1	0.0 6.000	0.0	11.100	0.0
16.200	0.0 1.050	21.300 0.3	0.0 6.150	0.0	11.250	0.0
16.350	0.0 1.200	21.450 0.6	0.0 6.300	0.0	11.400	0.0
16.500	0.0 1.350	21.600 2.1	0.0 6.450	0.0	11.550	0.0
16.650	0.0 1.500	21.750 8.7	0.0 6.600	0.0	11.700	0.0
16.800	0.0 1.650	21.900 9.8	0.0 6.750	0.0	11.850	0.0
16.950	0.0 1.800	22.050 5.7	0.0 6.900	0.0	12.000	0.0
17.100	0.0 1.950	22.200 3.1	0.0 7.050	0.0	12.150	0.0
17.250	0.0 2.100	22.350 1.7	0.0 7.200	0.0	12.300	0.0
17.400	0.0 2.250	22.500 1.1	0.0 7.350	0.0	12.450	0.0
17.550	0.0 2.400	0.8	0.0 7.500	0.0	12.600	0.0
17.700	0.0 2.550	22.800 0.6	0.0 7.650	0.0	12.750	0.0
17.850	0.0 2.700	22.950 0.4	7.800	0.0	12.900	0.0
18.000	0.0 2.850	23.100 0.3	0.0 7.950	0.0	13.050	0.0
18.150	0.0 3.000	0.2	0.0 8.100	0.0	13.200	0.0
18.300	0.0 3.150	0.2	0.0 8.250	0.0	13.350	0.0
18.450	3.300	0.1	0.0 8.400	0.0	13.500	0.0
18.600	0.0 3.450	23.700 0.1	0.0 8.550	0.0	13.650	0.0
18.750	0.0 3.600	23.850 0.1	0.0 8.700	0.0	13.800	0.0
18.900	0.0 3.750	24.000 0.1	0.0 8.850	0.0	13.950	0.0
19.050	0.0 3.900	24.150 0.1	9.000	0.0	14.100	0.0
19.200	0.0 4.050	24.300 0.1	0.0 9.150	0.0	14.250	
19.350	4.200 4.200	24.450 0.1	9.130	0.0	14.250	0.0
19.500	4.250 4.350	24.600	9.300 9.450			0.0
19.650	0.0	24.750	0.0	0.0	14.550	0.0
			Page	:)		

PECENTO

JAN 27 2012

HYDROLOGY

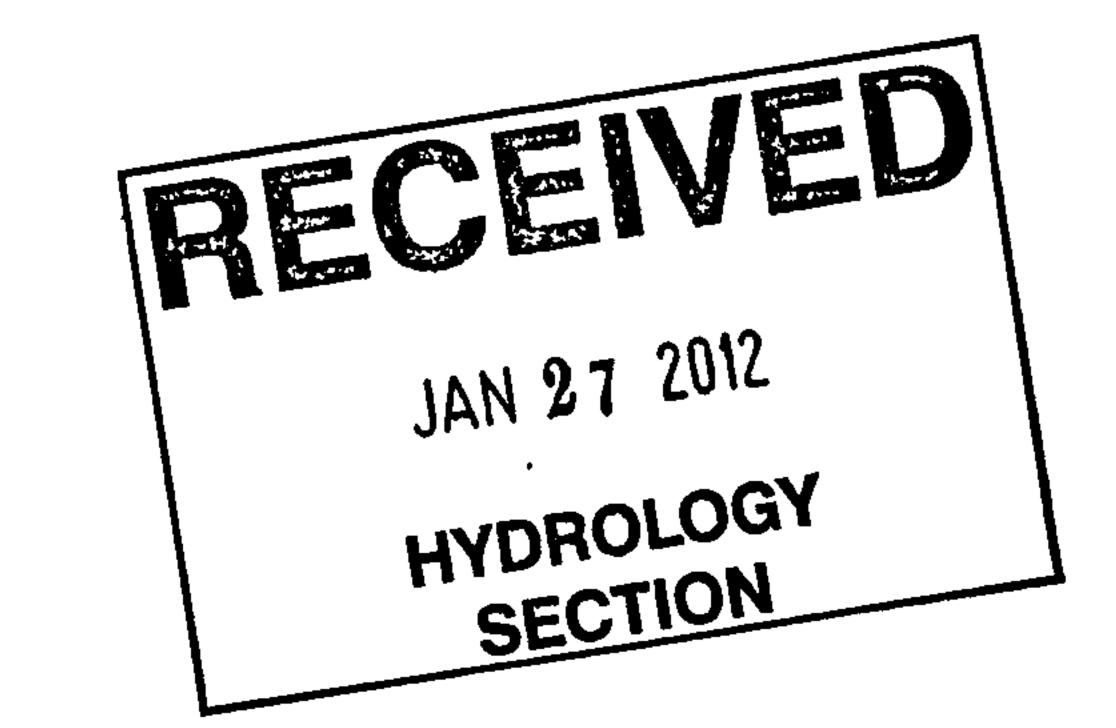
			AHYMO	. OUT		
	4.500	0.0	9.600	0.0	14.700	0.0
19.800	0.0	24.900	0.0			
	4.650	0.0	9.750	0.0	14.850	0.0
19.950	0.0					
	4.800	0.0	9.900	0.0	15.000	0.0
20.100	0.0					
	4.950	0.0	10.050	0.0	15.150	0.0
20.250	0.0					

RUNOFF VOLUME = 2.10272 INCHES = 0.5211 ACRE-FEET PEAK DISCHARGE RATE = 10.64 CFS AT 1.600 HOURS BASIN AREA = 0.0046 SQ. MI.

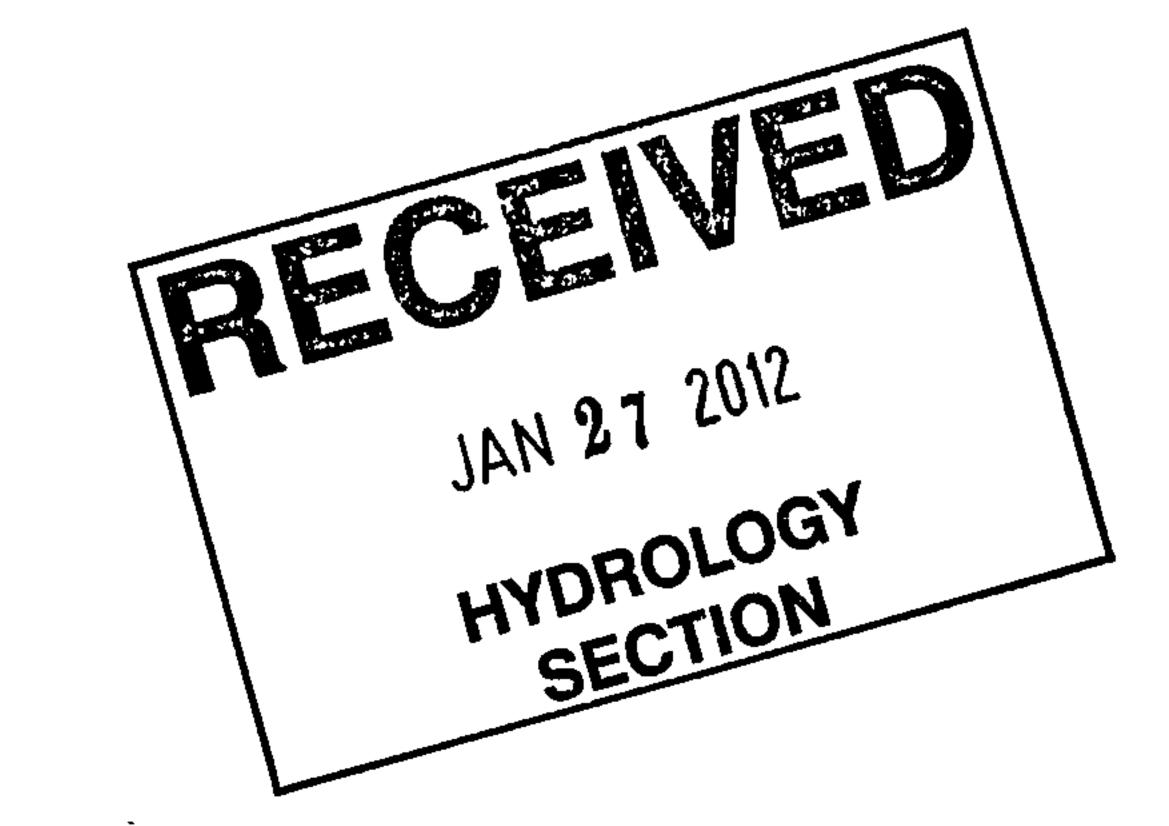
* ROUTE THE TOTAL ROUTE RESERVOIR		PROPOSED RESERVO 102 INFLOW=1 STORAGE(AC-FT) 0.000 0.045 5.88	CODE=3	25.00
	7.88	0.231	26.00	

* * * * * * * * * * * * * *

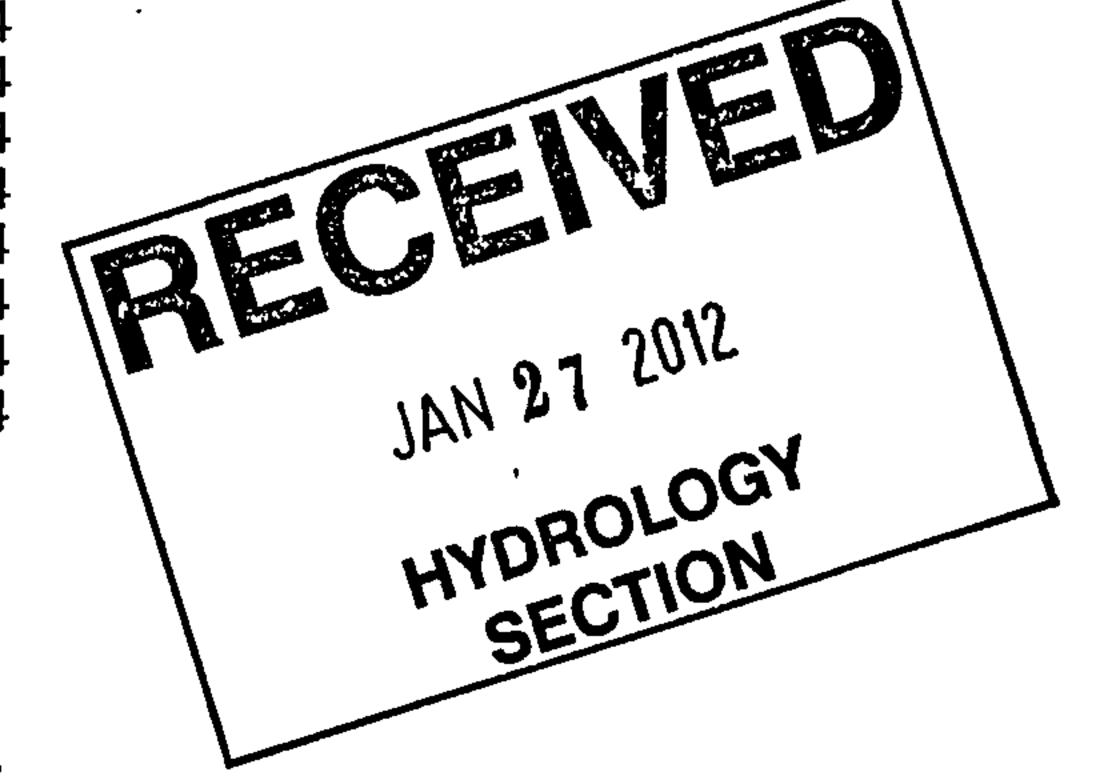
TIME	INFLOW	ELEV	VOLUME	OUTFLOW
(HRS)	(CFS)	(FEET)	(AC-FT)	(CFS)
0.15 0.35 0.45 0.45 0.45 0.65 0.75 0.15 1.85 1.85 1.85 1.85 1.85 1.85 2.25 2.35 3.35 3.35 3.35 3.35 3.35 3.3	0.00 0.00 0.00 0.02 0.14 0.28 0.64 2.12 8.71 9.83 5.71 3.06 1.09 0.80 0.43 0.32 0.19 0.15 0.10 0.08 0.07	23.00 23.00 23.00 23.00 23.02 23.02 23.02 23.12 23.30 24.53 24.53 24.53 24.53 23.79 23.34 23.17 23.12 23.09 23.07 23.04 23.04	0.000 0.000 0.000 0.000 0.001 0.002 0.005 0.123 0.143 0.120 0.086 0.056 0.036 0.023 0.015 0.011 0.008 0.001 0.004 0.004 0.003 0.002 0.002 0.002 0.002	0.00 0.00 0.00 0.00 0.04 0.13 0.79 3.08 5.28 5.84 4.37 3.12 2.08 1.36 0.45 0.45 0.19 0.14 0.12 0.10



4.20 4.35 4.35 4.60 4.95 4.95 5.70 5.70 5.70 5.70 5.70 5.70 5.70 5.7	0.05 0.05 0.03 0.03 0.03 0.03 0.03 0.04 0.04 0.04	23.03 23.02 23.02 23.02 23.01 23.01 23.01 23.01 23.01 23.01 23.01 23.02 23.02 23.02 23.02 23.02 23.02 23.02 23.02 23.02 23.02 23.02 23.02 23.02	AHYMO.OUT 0.001	0.08 0.07 0.06 0.05 0.04 0.04 0.03 0.03 0.03 0.03 0.03 0.04 0.04
TIME (HRS)	INFLOW (CFS)	ELEV (FEET)	VOLUME (AC-FT)	OUTFLOW (CFS)
8.40 8.55 8.70 8.80 9.15 9.30 9.45 9.75 10.20 10.35 10.65 10.95 11.25 11.40 11.25 11.40 11.25 12.30 12.45 12.30 12.45 12.60 12.75	0.04 0.04 0.04 0.04 0.04 0.04 0.04 0.04	23.02 23.02	0.001 0.001 0.001 0.001 0.001 0.001 0.001 0.001 0.001 0.001 0.001 0.001 0.001 0.001 0.001 0.001 0.001 0.001 0.001 0.001	0.04 0.04 0.04 0.04 0.04 0.04 0.04 0.04



12.90 13.20 13.35 13.35 13.65 13.65 13.95 14.25 14.40 14.55 14.85 15.45 15.30 15.45 15.45 15.90 16.20 16.35 16.65	0.04 0.04 0.04 0.04 0.04 0.04 0.04 0.04	23.02 23.02	AHYMO.OUT 0.001	0.04 0.04 0.04 0.04 0.04 0.04 0.04 0.04
TIME (HRS)	INFLOW (CFS)	ELEV (FEET)	VOLUME (AC-FT)	OUTFLOW (CFS)
16.80 16.95 17.10 17.25 17.40 17.55 17.70 17.85 18.30 18.30 18.30 18.45 18.90 19.20 19.35 19.95 19.95 20.25 20.40 20.55 20.25 20.25 21.30 21.30 21.30 21.30 21.30	0.04 0.04 0.04 0.04 0.04 0.04 0.04 0.04	23.02 23.02	0.001 0.001	0.04 0.04 0.04 0.04 0.04 0.04 0.04 0.04



			AHYMO.OUT			
21.75	0.04	23.02	0.001	0.04		
21.90	0.04	23.02	0.001	0.04		
22.05	0.04	23.02	0.001	0.04		
22.20	0.04	23.02	0.001	0.04		
22.35	0.04	23.02	0.001	0.04		
22.50	0.04	23.02	0.001	0.04		
22.65	0.04	23.02	0.001	0.04		
22.80	0.04	23.02	0.001	0.04		
22.95	0.04	23.02	0.001	0.04		
23.10	0.04	23.02	0.001	0.04		
23.25	0.04	23.02	0.001	0.04		
23.40	0.04	23.02	0.001	0.04		
23.55	0.04	23.02	0.001	0.04		
23.70	0.04	23.02	0.001	0.04		
23.85	0.04	23.02	0.001	0.04		
24.00	0.04	23.02	0.001	0.04		
24.15	0.03	23.02	0.001	0.04		
24.30	0.01	23.01	0.001	0.03		
24.45	0.01	23.01	0.000	0.02		
24.60	0.00	23.00	0.000	0.01		
24.75	0.00	23.00	0.000	0.01		
24.90	0.00	23.00		0.00		
PEAK DISCHARG				CCURS AT HOUR	1.80	
MAXIMUM WATER			-	. 200		
MAXIMUM STORA	AGE =	0.1430	AC-FT	INCREMENTAL TI	ME=	0.050000HRS
		3 3				

PRINT HYD ID=2 CODE=3

PARTIAL HYDROGRAPH 102.00

	TIME	FLOW	TIME	FLOW	TIME	FLOW
. TIME	FLOW HRS	TIME CFS	FLOW HRS	CFS	HRS	CFS
HRS	CFS	HRS	CFS		IIICO	CFS
15 200	0.000	0.0	5.100	0.0	10.200	0.0
15.300	0.0 0.150	0.0	0.0 5.250	0.0	10.350	ΛΛ
15.450	0.10	20.550	0.0	0.0	10.550	0.0
45.000	0.300	0.0	5.400	0.0	10.500	0.0
15.600	0.0 0.450	20.700	0.0	^ ^	10 650	
15.750	0.430	0.0 20.850	5.550 0.0	0.0	10.650	0.0
	0.600	0.0	5.700	0.0	10.800	0.0
15.900	0.0	21.000	0.0	^ ^		
16.050	0.750 0.0	0.0	5.850 0.0	0.0	10.950	0.0
40.030	0.900	0.0	6.000	0.0	11.100	0.0
16.200	0.0	21.300	0.0			
16.350	$\begin{array}{c} 1.050 \\ 0.0 \end{array}$	0.1 21.450	6.150	0.0	11.250	0.0
10.330	1.200	0.3	0.0 6.300	0.0	11.400	0.0
16.500	0.0	21.600	0.0			0.0
16.650	1.350	0.8	6.450	0.0	11.550	0.0
TO.020	0.0 1.500	21.750 3.1	0.0 6.600	0.0	11.700	0.0
16.800	0.0	21.900	0.00	0.0	11.700	0.0
16 050	1.650	5.9	6.750	0.0	11.850	0.0
16.950	$0.0 \\ 1.800$	22.050 6.3	0.0 6.900	Λ Λ	12 000	0 0
17.100	0.0	22.200	0.900	0.0	12.000	0.0
	1.950	5.8	7.050	0.0	12.150	0.0
17.250 E		22.350	7 200	~ ~	42 222	
		# . 4	7.200 Page	0.0	12.300	0.0
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JAN 27 2012

HYDROLOGY SECTION

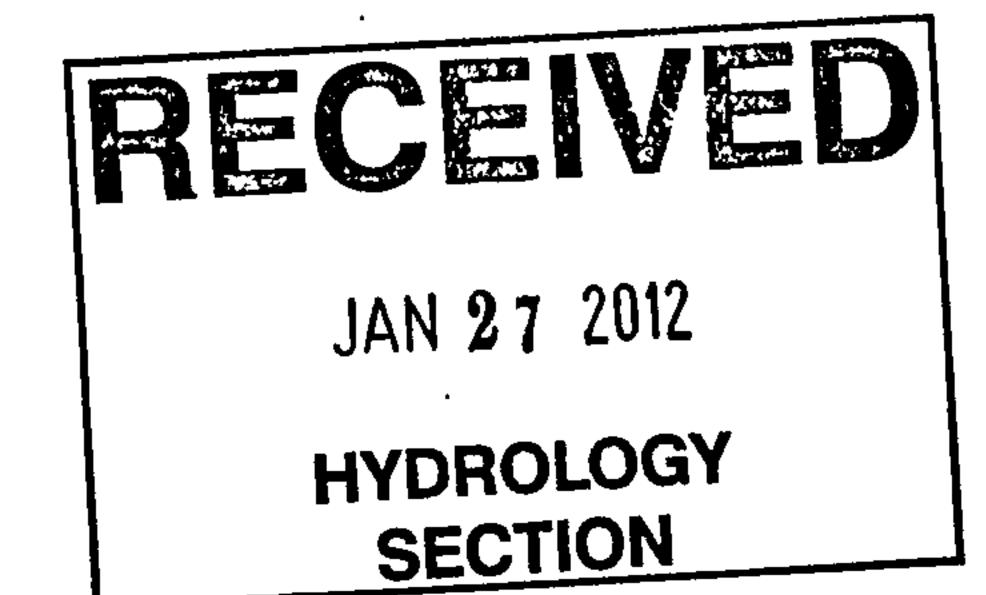
			AHYMO	.OUT		
17.400	0.0	22.500	0.0	^ ^	12 450	^ ^
17.550	2.250 0.0	3.1 22.650	7.350 0.0	0.0	12.450	0.0
17.330	2.400	2.1	7.500	0.0	12.600	0.0
17.700	0.0	22.800	0.0			•
17 050	2.550	1.4	7.650	0.0	12.750	0.0
17.850	0.0 2.700	22.950 0.9	0.0 7.800	0.0	12.900	0.0
18.000	0.0	23.100	0.0			0.0
40 450	2.850	0.6	7.950	0.0	13.050	0.0
18.150	0.0 3.000	23.250 0.4	0.0 8.100	0.0	13.200	0.0
18.300	0.0	23.400	0.0	0.0	13.200	0.0
	3.150	0.3	8.250	0.0	13.350	0.0
18.450	0.0	23.550	0.0	^ ^	12 500	^ ^
18.600	3.300 0.0	0.2 23.700	8.400 0.0	0.0	13.500	0.0
•	3.450	0.2	8.550	0.0	13.650	0.0
18.750	0.0	23.850	0.0		12 000	
18.900	3.600 0.0	0.1 24.000	8.700 0.0	0.0	13.800	0.0
10.500	3.750	0.1	8.850	0.0	13.950	0.0
19.050	0.0	24.150	0.0			
19.200	3.900 0.0	0.1 24.300	9.000 0.0	0.0	14.100	0.0
19.200	4.050	0.1	9.150	0.0	14.250	0.0
19.350	0.0	24.450	0.0			
19.500	4.200	0.1	9.300	0.0	14.400	0.0
T3.300	0.0 4.350	24.600 0.1	0.0 9.450	0.0	14.550	0.0
19.650	0.0	24.750	0.0			•
10 000	4.500	0.1	9.600	0.0	14.700	0.0
19.800	0.0 4.650	0.0 24.900	0.0 9.750	0.0	14.850	. 0.0
19.950	0.0	25.050	0.0	0.0	14.030	0.0
20 100	4.800	0.0	9.900	0.0	15.000	0.0
20.100	0.0 4.950	0.0 25.200	0.0 10.050	0.0	15.150	0.0
20.250	0.0		10.030	0.0	T). T)U	0.0

RUNOFF VOLUME = 2.10265 INCHES = 0.5211 ACRE-FEET
PEAK DISCHARGE RATE = 6.28 CFS AT 1.800 HOURS BASIN AREA = 0.0046 SQ. MI.

FINISH

NORMAL PROGRAM FINISH

END TIME (HR:MIN:SEC) = 14:44:14



POND 022612.txt

*S AHYMO -MONTEREY

*S POND ROUTING

START

TIME=0.0 PUNCH CODE=0

RAINFALL

TYPE=2

QUARTER=0.0 ONE= 2.60 IN

SIX= 3.10 INDAY = 3.95 IN DT = 0.05 HR

COMPUTE NM HYD

ID=1 HYD NO=101 DA= .004647 SQ MI

PER B=21 PER C=19 PER D=30 PER A=30

TP = -.20MASSRAIN=-1

PRINT HYD

ID=1 CODE=3

* ROUTE THE TOTAL FLOW THROUGH THE PROPOSED RESERVOIR

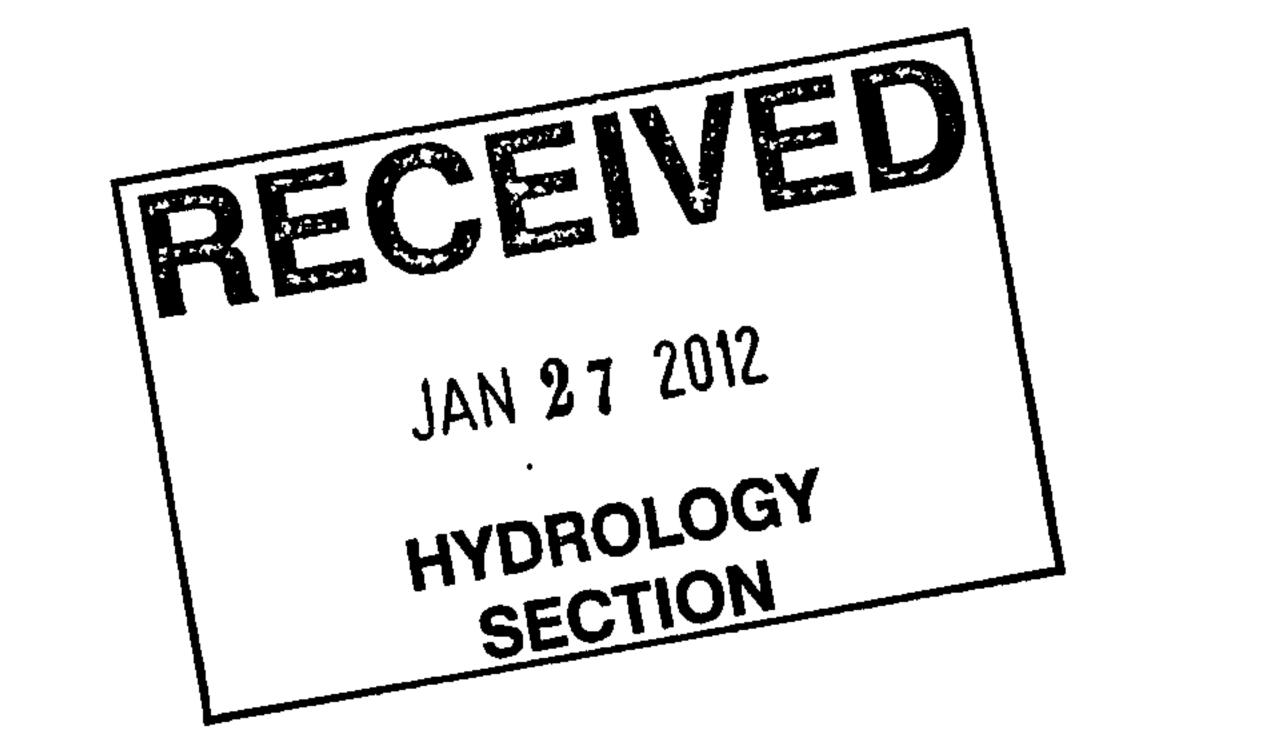
ID=2 HYD NO=102 ROUTE RESERVOIR INFLOW=1 CODE=3OUTFLOW(CFS) STORAGE (AC-FT) ELEV(FT)

0.0 0.000 23.00 2.64 0.045 24.00 5.88 0.121 25.00 7.88 0.231 26.00

PRINT HYD

ID=2CODE=3

FINISH



CITY OF ALBUQUERQUE



January 4, 2012

David Soule, P.E.
Rio Grande Engineering
P.O. Box 93924
Albuquerque, NM 87199

Re: Monterey Baptist Church Grading and Drainage Plan Engineer's Stamp date 12-20-11 (J22/D012A)

Dear Mr. Soule,

Based upon the information provided in your submittal received 12-21-11, the above referenced plan cannot be approved for Building and Grading Permit until the following comments are addressed:

- Hydrology is recommending that both Basin A & Basin B be discharged into the detention pond on site.
- It appears that flows on the northern portion of Basin C will flow into Basin D, will a swale be used to direct these flows to the pond?
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- Details will be needed on the pond's sidewalk culvert illustrating how the 8" HDPE pipe will drain into the 6" culvert.
- Will there be an emergency overflow for the retention pond?
- Please define the hatching adjacent to the existing asphalt paving (at the turnaround area).
- Hydrology requests that proposed landscape areas be depressed to retain/infiltrate the rain that falls on them.

If you have any questions, you can contact me at 924-3695.

Sincerely,

Shahab Biazar, P.E.

Senior Engineer, Planning Dept.

Development and Building Services

C: File CJH\SB

PO Box 1293

Albuquerque

NM 87103

www.cabq.gov

DRAINAGE AND TRANSPORTATION INFORMATION SHEET

(REV. 01/28/2003rd)

PROJECT TITLE: DRB #:	Monterey Baptist Church EPC #:	ZONE MAP/E	DRG. FILE #: <u>J-22/DOL2A</u> ER #:
LEGAL DESCRIPTION: CITY ADDRESS:	tract 6 monterrey Mannor 12501 Lomas NE		
ENGINEERING FIRM: ADDRESS: CITY, STATE:	Rio Grande Engineering po box 93924 Albuquerque, New Mexico	CONTACT: PHONE: ZIP CODE:	David Soule, PE (505)321-9099 87199
OWNER: ADDRESS: CITY, STATE:	Montery baptist church 12501 Lomas ne Albuquerque, NM 87102	CONTACT: PHONE: ZIP CODE:	87112
ARCHITECT: ADDRESS: CITY, STATE:	Joe simmons.	CONTACT: PHONE: ZIP CODE:	
SURVEYOR: ADDRESS: CITY, STATE:	Geo surv co	CONTACT: PHONE: ZIP CODE:	
CONTRACTOR: ADDRESS: CITY, STATE:		CONTACT: PHONE: ZIP CODE:	
DRAINAGE PLA CONCEPTUAL (X GRADING PLAN EROSION CONT ENGINEER'S CE CLOMR/LOMR TRAFFIC CIRCL ENGINEERS CE	PORT IN 1st SUBMITTAL, <i>REQUIRES TCL or equal</i> IN RESUBMITTAL GRADING & DRAINAGE PLAN	PRELIMINAR S. DEV. PLA S. DEV. PLA SECTOR PLA FINAL PLAT FOUNDATIO X BUILDING PLAT CERTIFICAT CERTIFICAT X GRADING PLAT	ACIAL GUARANTEE RELEASE RY PLAT APPROVAL N FOR SUB'D. APPROVAL N FOR BLDG. PERMIT APPROVAL AN APPROVAL APPROVAL N PERMIT APPROVAL ERMIT APPROVAL E OF OCCUPANCY (PERM.) E OF OCCUPANCY (TEMP.) ERMIT APPROVAL MIT APPROVAL
WAS A PRE-DESIGN CONI X YES NO COPY PROVIDE	\$SC	OTHER	DEC 2 1 2011 HYDROLOGY SECTION
DATE SUBMITTED:	12/20/2011	BY:	David Soule

Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a dranage submittal.

The particular nature, location and scope of the proposed development defines the degree of drainage detail.

One or more of the following levels of sumbittal may be required based on the following:

- 1. Conceptual Grading and Drainage Plans: Required for approval of Site Development Plans greater than five (5) acres and Sector Plans.
- 2. Drainage Plans: Required for building permits, grading permits, paving permits and site plans less than five (5) acres.
- 3. Drainage Report: Required for subdivisions containing more than ten (10) lots or constituting five (5) acres or more.

RIO GRANDE ENGINEERING OF NEW MEXICO, LLC

January 17, 2012

Mr. Shahab Biazar, PE Senior Engineer Planning Department City of Albuquerque

RE: Grading and Drainage Plan
Monterey Baptist Church (J22/D012A)

Dear Shahab:

The purpose of this letter is to accompany the enclosed grading plan for the referenced project. This plan has been modified to address your comments dated December 21, 2011. The following is a summary of your comments with the annotation as to how the plans were modified to address the comments.

erosian control/fencing

HYDROLOGY

SECTION

1. Hydrology recommends both basin A and B discharge into detention pond.

The entire site will develop in the future with more impervious area to the west. If all basins drain to the pond the ponds will impact the future development area. An additional culvert has been added to safely get water to street form basin A

2. Flow in basin C will flow into A, will swale be used

There is a building located at the north corner of the site. This building and the existing grading around the building force the water south of the building. Basin C does not drain north of the building. The existing flows that discharge to the adjacent property are significantly reduced form current.

3. Basin C appear to drain across sidewalk, provide swale to pond.

A cobble swale has been added along the south property line. This swale directs all flows to the pond.

4. Provide details for how discharge pipe drains into culvert. .

The 8" discharge pipe was set back 42" from the sidewalk culvert. The transition will be via cobble swale made up of 8" landscape cobbles. The outfall was moved to allow the smooth transition in swale shown

5. Provide emergency overflow. Should be along 5626 Centour REC 1001 17 20

6. Define hatching adjacent to existing asphalt paving

The hatching is new asphalt and has been added to legend.

7. Depress proposed landscape areas for infiltration

The areas around the east side of building are existing. The flat grade of the swale in

Come in 12" with edge of pand

Maried all he but the form

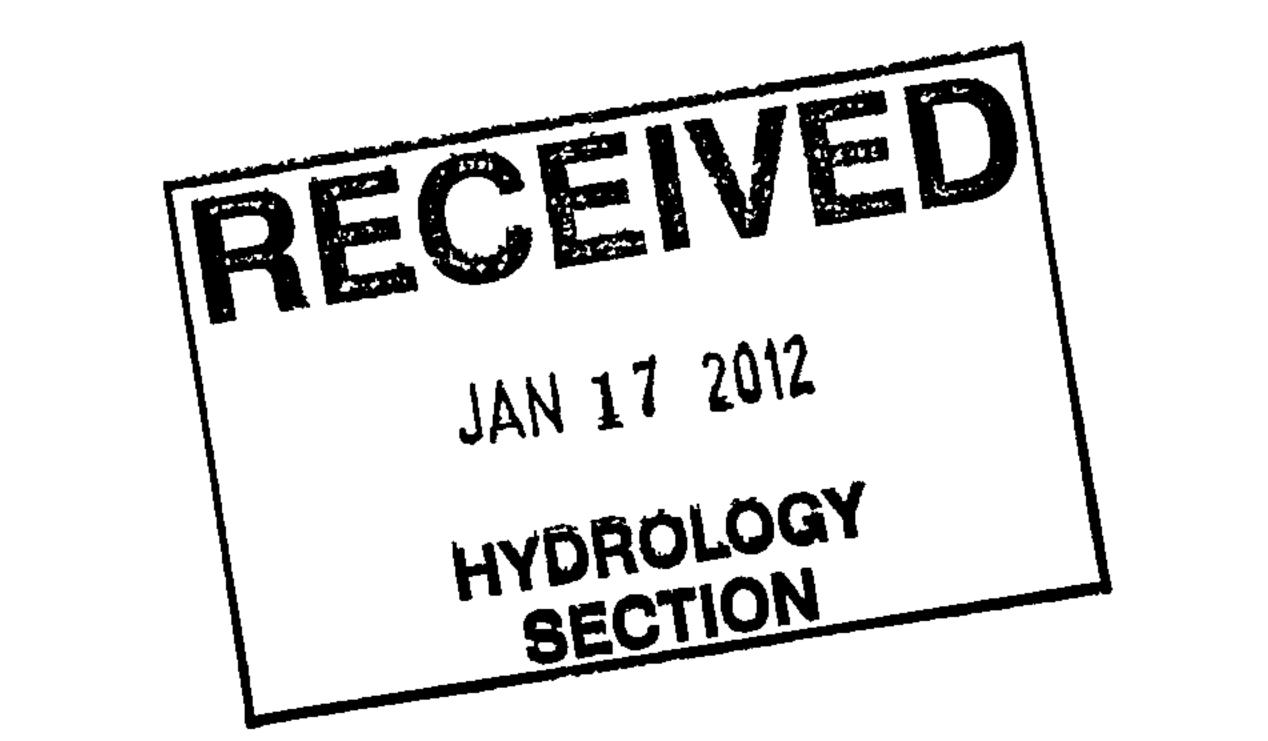
the area provides for infiltration. The swale along the sidewalk is made of cobbles to allow nuisance flows to be absorbed. The pond outlet is 6" above the pond bottom for harvesting.

Should you have any questions regarding this resubmittal, please do not hesitate to call me.

Sincerely,

David Soule, PE

Enclosures



Pond overflow

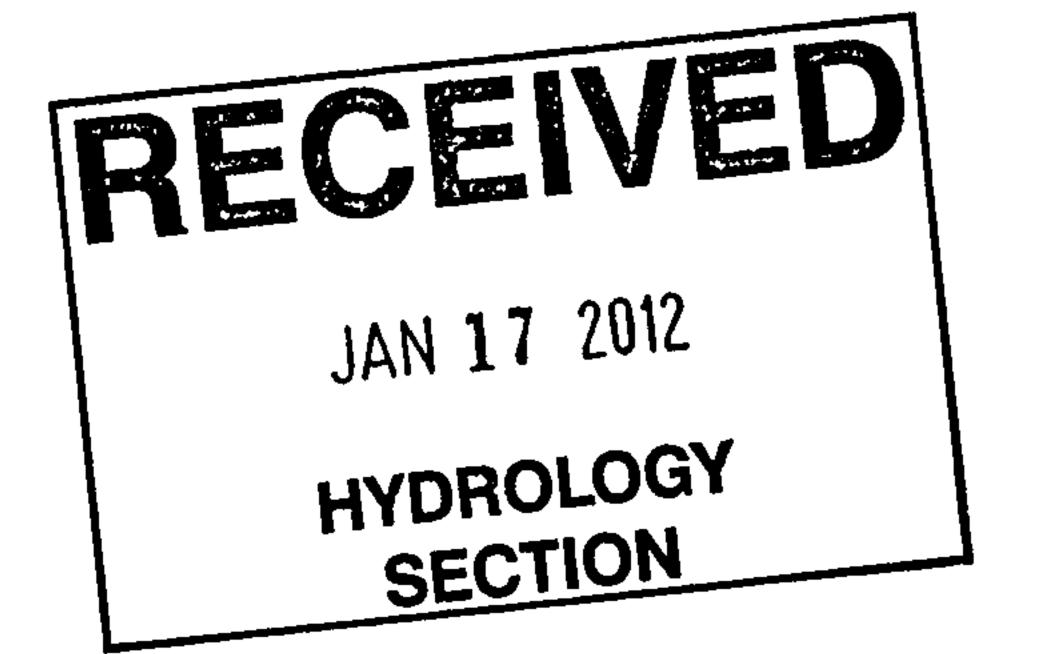
Weir Equation:

$$Q = CLH^{32}$$

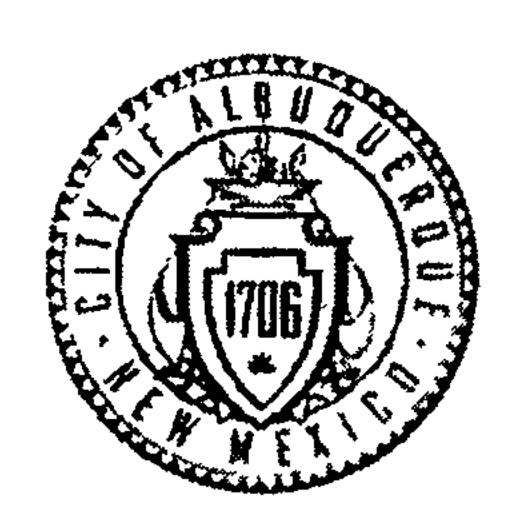
Q=7.1 cfs C = 2.95H = 0.5 ftL = 7

 $=2.95X7X.5^3/2$

=7.3 cfs



CITY OF ALBUQUERQUE



January 4, 2012

David Soule, P.E. Rio Grande Engineering P.O. Box 93924 Albuquerque, NM 87199

Re: Monterey Baptist Church Grading and Drainage Plan Engineer's Stamp date 12-20-11 (J22/D012A)

Dear Mr. Soule,

Based upon the information provided in your submittal received 12-21-11, the above referenced plan cannot be approved for Building and Grading Permit until the following comments are addressed:

- Hydrology is recommending that both Basin A & Basin B be discharged into the detention pond on site.
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- Hydrology requests that proposed landscape areas be depressed to retain/infiltrate the rain that falls on them.

If you have any questions, you can contact me at 924-3695.

PO Box 1293

Albuquerque

NM 87103

www.cabq.gov

RECEIVED

JAN 17 2012

HYDROLOGY SECTION

C: File CJH\SB

Sincerely,

'Shahab Biazar, P.E.

Senior Engineer, Planning Dept.

Development and Building Services

DRAINAGE AND TRANSPORTATION INFORMATION SHEET

(REV. 01/28/2003rd)

PROJECT TITLE: DRB #:	Monterey Baptist Church EPC #:	ZONE MAP/E WORK ORDI	DRG. FILE #: <u>J22/ D012A</u>
			L1\17.
LEGAL DESCRIPTION:		·	
CITY ADDRESS:	12501 Lomas NE		· · · · · · · · · · · · · · · · · · ·
ENGINEERING FIRM:	Rio Grande Engineering	CONTACT:	David Soule, PE
ADDRESS:	po box 93924	PHONE:	(505)321-9099
CITY, STATE:	Albuquerque, New Mexico	ZIP CODE:	87199
OWNER:	Montery baptist church	CONTACT:	
ADDRESS:	12501 Lomas ne	PHONE:	
CITY, STATE:	Albuquerque, NM 87102	ZIP CODE:	<u>87112</u>
ARCHITECT:	Joe simmons.	CONTACT:	
ADDRESS:		PHONE:	
CITY, STATE:		ZIP CODE:	•
SURVEYOR:	Geo surv co	CONTACT:	
ADDRESS:		PHONE:	
CITY, STATE:		ZIP CODE:	
CONTRACTOR:		CONTACT:	
ADDRESS:		PHONE:	
CITY, STATE:		ZIP CODE:	
DRAINAGE PLA CONCEPTUAL X GRADING PLAN EROSION CON' ENGINEER'S CI CLOMR/LOMR TRAFFIC CIRCU ENGINEERS CE	PORT AN 1st SUBMITTAL, <i>REQUIRES TCL or equal</i> AN RESUBMITTAL GRADING & DRAINAGE PLAN	PRELIMINAR S. DEV. PLA S. DEV. PLA SECTOR PL FINAL PLAT FOUNDATIC X BUILDING P CERTIFICAT CERTIFICAT A GRADING PE	ACIAL GUARANTEE RELEASE RY PLAT APPROVAL IN FOR SUB'D. APPROVAL IN FOR BLDG. PERMIT APPROVAL AN APPROVAL
WAS A PRE-DESIGN CON X YES NO COPY PROVIDE			JAN 17 2012 HYDROLOGY SECTION
DATE SUBMITTED:	1/17/2012	BY:	David Soule

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DRAINAGE REPORT

For

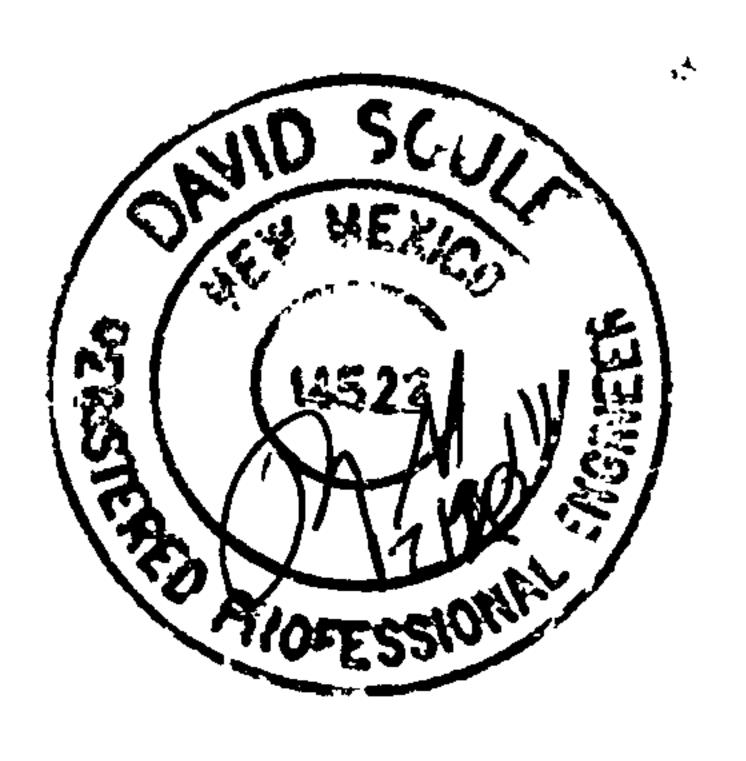
MONTEREY BAPTIST CHURCH 11501 LOMAS NE

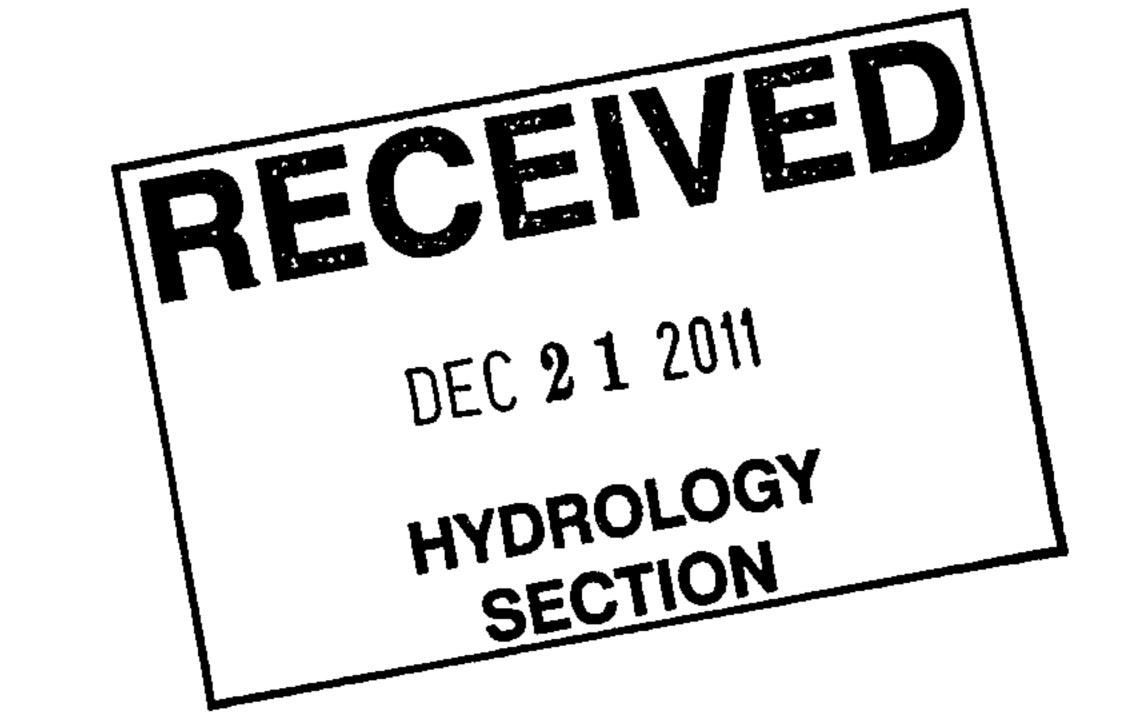
Albuquerque, New Mexico

Prepared by

Rio Grande Engineering PO Box 67305 Albuquerque, New Mexico 87193

DECEMBER 2011





David Soule P.E. No. 14522

TABLE OF CONTENTS

Purpose	
Introduction	
Existing Conditions	
Exhibit A-Vicinity Map	
Proposed Conditions	
Summary	
Appendix Site Hydrology	A
Map Pocket Site Grading and Drainage Plan	

PURPOSE

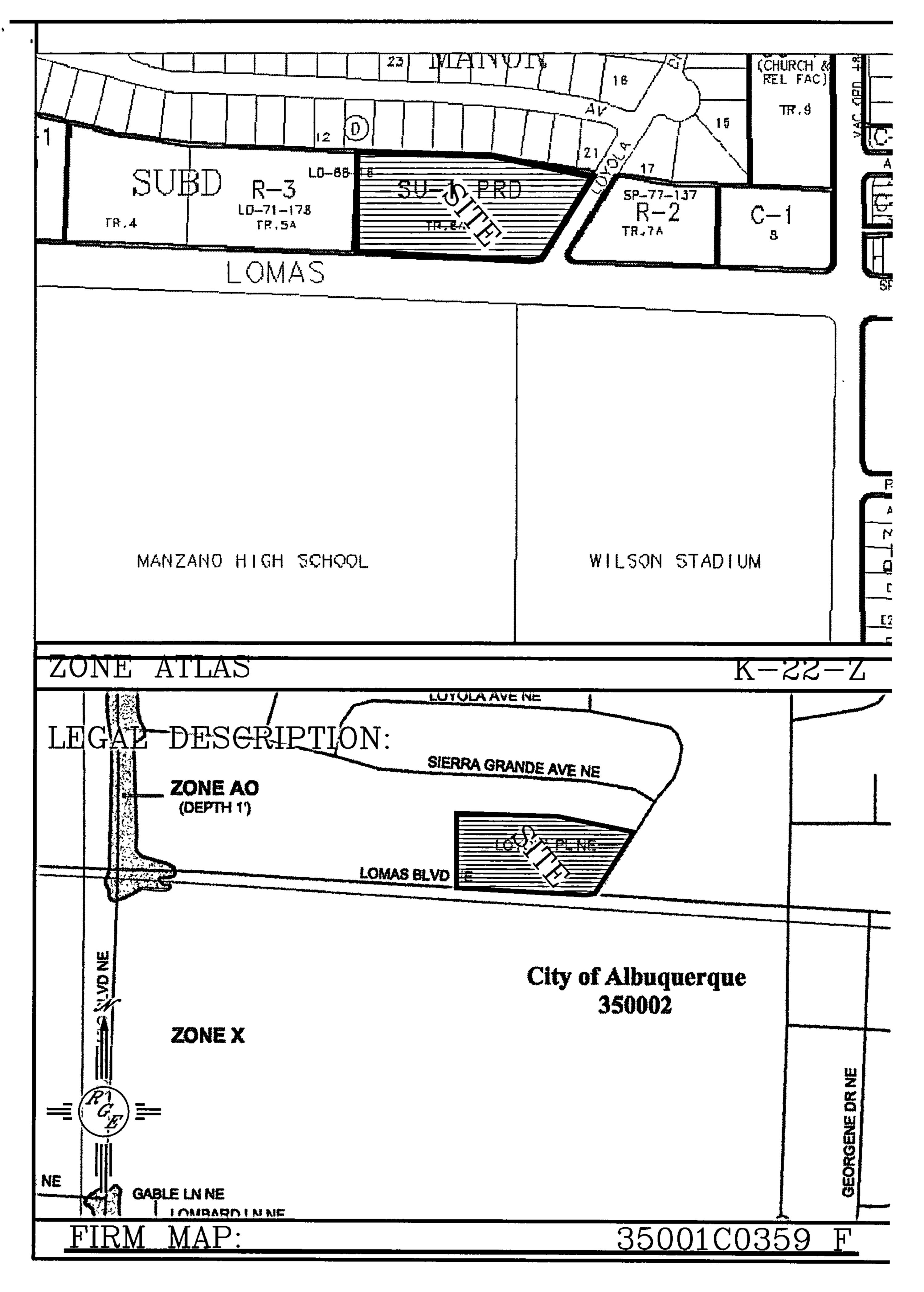
The purpose of this report is to provide the Drainage Management Plan for an approximately 6000 square foot addition and a 600 sf portable unit located on the northwest corner of Lomas and Loyola NE. This plan was prepared in accordance with the City of Albuquerque design regulations, utilizing the City of Albuquerque's Development Process Manual drainage guidelines. This report will demonstrate that the grading does not adversely affect the surrounding properties, nor the upstream or downstream facilities.

INTRODUCTION

The subject of this report, as shown on the Exhibit A, is an existing developed parcel containing an area of 3 acres of land located on the northwest corner of Lomas and Loyola. The legal description of this site is tract 6 Monterey manor Addition. As shown on FIRM map35013C0359E, the entire property is located within Flood Zone X. This site is surrounded by fully developed parcels. This site development will add to an existing church. The development of this site will accommodate the flow from the new addition, correct any onsite drainage issues and allow for future implementation of additional site development and match native conditions as closely as possible.

EXISTING CONDITIONS

The site is currently developed. The site slopes from northeast to southwest. The site currently free discharges into Lomas Boulevard. The site is not impacted by any measurable offsite flows, and is surrounded by developed properties with solid walls and upland adjacent streets. As shown in Appendix A, the native site discharges at a peak rate of 7.09 cfs in a 100-year, 6-hour event.



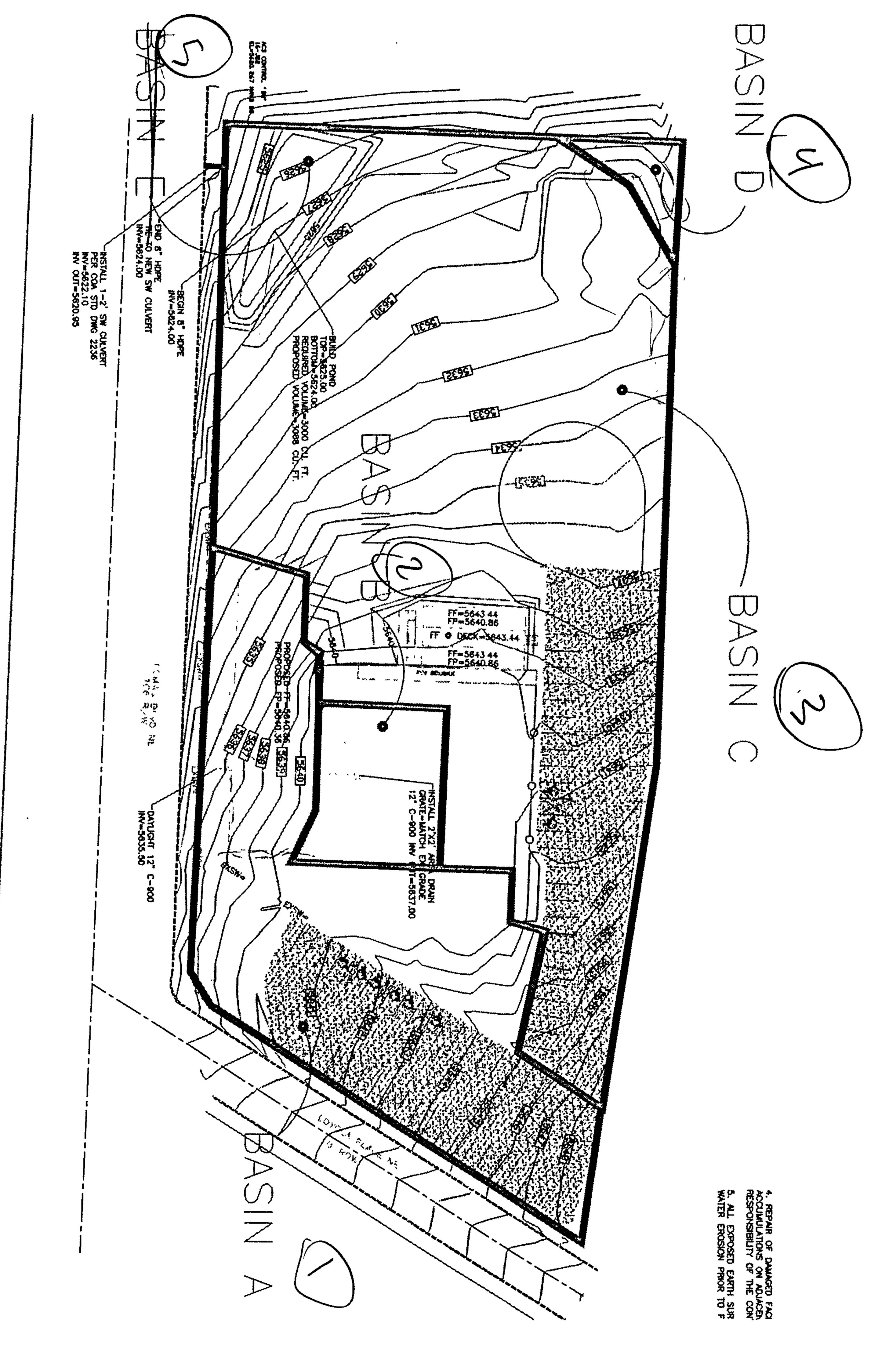
LEGAL DESCRIPTION:

PROPOSED CONDITIONS

The proposed improvements consist of an approximately 5,800 square foot addition to the existing building and a 600 square foot portable. As shown in appendix A, the site will be graded to contain four basins. Basin A contains the eastern parking lot and the area of building addition. Basin, it will free discharge at a peak rate of 3.74 cfs to Lomas. Basin B contains a courtyard area that is created with the development of the addition; it will discharge .65 cfs via a 12" underground pipe that daylights to the landscape are along Lomas. Basin C contains the majority of the building, parking and site; this basin will drain 7.06 cfs to a detention pond built at the southwest corner of the site. As shown in appendix a, the AYHMO pond routing demonstrates the pond will fill to elevation of 5625.4 and discharge at a peak rate of 2.17 cfs. Basin D is a small portion of land that discharges .09 cfs to the adjacent property. As designed the redeveloped site will discharge 6.64 cfs during the 100-year, 6-hour event, which is less than the 7.09 cfs the site would discharge in native conditions. The design of the pond will allow for future development of the site with minor adjustments to volume.

SUMMARY AND RECOMMENDATIONS

This project is a redevelopment project within a completely developed area of northwest Albuquerque. The site discharges will be reduced to less than native. The proposed detention pond was created with a 6" harvesting basin to capture all nuisance flows. Since the disturbed area of the site encompasses less than 1 acre, a NPDES permit should not be required prior to any construction activity.



Should be 4 basins, nots

•

APPENDIX A SITE HYDROLOGY

Weighted E Method

Existing Developed Basins

			· · · · · · · · · · · · · · · · · · ·						. <u>-</u>	· 	100-Year, 6-h	Γ.		10-day
Basin	Area	Area	Treatment A		Treatmen	nt B	Treatm	ent C	Treatme	nt D	Weighted E	Volume	Flow	Volume
	<u>(sf)</u>	(acres)	%	(acres)	%	(acres)	%	(acres)	%	(acres)	(ac-ft)	(ac-ft)	cfs	(ac-ft)
basin a	35320.00	0.811	0%	0	21%	0.170	10%	0.08108	69%	0.559	2.194	0.148	3.74	0.223
basinb	6100.00	0.140	0%	0	20%	0.028	10%	0.014	} 		2.210	0.026	0.65	0.039
basinc	86400.00	1.983	30%	0.595041322	21.0%	0.417	 -	0.37686		0.595	1.536	0.254	7.06	0.333
basind	1742.00	0.040	100%	0.039990817	0.0%	0.000	0.0%			0.000	0.800	0.003	0.09	0.003
native	129562.00	2.974	80%	2.379467401	15.0%	····		0.14872		0.000	0.875	0.003	7.09	

Equations:

Weighted E = Ea*Aa + Eb*Ab + Ec*Ac + Ed*Ad / (Total Area)

Volume = Weighted D * Total Area

Flow = Qa * Aa + Qb * Ab + Qc * Ac + Qd * Ad

Where for 100-year, 6-hour storm

Ea= 0.8	Qa= 2.2
Eb= 1.08	Qb= 2.92
Ec= 1.46	Qc= 3.73
Ed= 2.64	Qd= 5.25

POND VOLUME(6-hour)	0.25	AC-FT	11060.64 CF
UNTHROTTLED	11.53	_	
TOTAL SITE AFTER POND ROUTE	6.64	cfs	
NATIVE	7.09	cfs	
NET DECREASE	0.45	cfs	

Pipe Capacity

Pipe	D	Slope	Area	R	Q Provided	Q Required	Velocity
	<u>(in)</u>	(%)	(ft^2)		(cfs)	(cfs)	(ft/s)
culvert	12	1.9	0.79	0.25	4.27	0.65	0.83

Manning's Equation: Q = 1.49/n * A * R^(2/3) * S^(1/2)

A = Area

R = D/4 S = Slope

n = 0.015

VOLUME CALCULATIONS

- 1						
ļ	ACTUAL	DEPTH	CONTOUR	VOLUME	VOLUME	Q
Ì	ELEV.	(FT)	AREA	cf	AC-FT	(CFS)
		(above outlet)				
	5621	0		0		0.000
	5622.00	0.00	0.00	0.0000	0.0000	0.000
g	5623.00	0.00	1350.00	0.0000	0.0000	0.000
	5624.00	0.67	2550.00	1950.0000	0.0448	0.975
	5625.00	1.67	4100.00	5275.0000	0.1211	1.943
	5626.00	2.67	5500.00	10075.0000	0.2313	2.569

inv=23.50

Orifice Equation

Q = CA SQRT(2gH)

C = 0.6Diameter (in) 8
Area (ft^2)= 0.34906585 g = 32.2

H (Ft) = Depth of water above center of orifice

Q (CFS)= Flow

POND 122011.txt

*S AHYMO - MONTEREY

*S POND ROUTING

START

TIME=0.0 PUNCH CODE=0

RAINFALL

TYPE=2

QUARTER=0.0 ONE= 2.60 IN

SIX= 3.10 IN DAY= 3.95 IN DT = 0.05 HR

COMPUTE NM HYD

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PER A=30 PER B=21 PER C=19 PER D=30

TP=-.20 MASSRAIN=-1

PRINT HYD

ID=1 CODE=3

* ROUTE THE TOTAL FLOW THROUGH THE PROPOSED RESERVOIR

ROUTE RESERVOIR

ID=2 HYD NO=102 INFLOW=1 CODE=3
OUTFLOW(CFS) STORAGE(AC-FT) ELEV(FT)
0.0 0.000 23.00

0.98 0.045 24.00

PRINT HYD

ID=2 CODE=3

FINISH

AHYMO.OUT AHYMO PROGRAM (AHYMO-S4) - Version: S4.01a - Rel: 01a RUN DATE (MON/DAY/YR) = 12/20/2011START TIME (HR:MIN:SEC) = 09:11:29USER NO.≈ RioGrandeSingleA41963517 INPUT FILE = and Settings\Owner\Desktop\2011jobs\1162-monterey baptist churc\POND 122011.txt *****S AHYMO -MONTEREY *S POND ROUTING START TIME=0.0PUNCH CODE=0

RAINFALL TYPE=2

QUARTER=0.0 ONE= 2.60 IN

SIX= 3.10 IN DAY= 3.95 IN DT = 0.05 HR

24-HOUR RAINFALL DIST. - BASED ON NOAA ATLAS 14 FOR CONVECTIVE AREAS (NM & AZ) - D1 0.050000 HOURS DT END TIME = 24.000002 HOURS 0.0000 0.0033 0.0068 0.0105 0.0145 0.0186 0.0233 0.0299 0.0400 0.0510 0.0624 0.0749 0.0876 0.1007 0.1142 0.1280 0.1434 0.1595 0.1769 0.2078 0.2454 0.2958 0.3529 0.4234 0.5175 0.6233 0.8060 1.0901 1.5769 1.9190 2.1889 2.3245 2.4433 2.5287 2.5966 2.6559 2.6992 2.7390 2.7718 2.7937 2.8101 2.8246 2.8382 2.8502 2.8614 2.8724 2.8830 2.8917 2.8967 2.9017 2.9066 2.9111 2.9157 2.9201 2.9244 2.9286 2.9366 2.9327 2.9405 2.9444 2.9482 2.9518 2.9588 2.9623 2.9657 2.9690 2.9722 2.9754 2.9816 2.9847 2.9877 2.9907 2.9936 2.9965 2.9994 3.0022 3.0050 3.0078 3.0105 3.0132 3.0159 3.0185 3.0211 3.0237 3.0263 3.0288 3.0313 3.0338 3.0362 3.0387 3.0411 3.0434 3.0458 3.0481 3.0504 3.0527 3.0550 3.0572 3.0594 3.0616 3.0638 3.0660 3.0681 3.0702 3.0723 3.0744 3.0765 3.0785 3.0806 3.0826 3.0846 3.0866 3.0885 3.0905 3.0924 3.0943 3.0962 3.0981 3.1000 3.1024 3.1047 3.1071 3.1094 3.1118 3.1142 3.1165 3.1212 3.1189 3.1236 3.1260 3.1283 3.1307 3.1331 3.1354 3.1378 3.1401 3.1425 3.1449 3.1472 3.1496 3.1519 3.1543 3.1567 3.1590 3.1614 3.1637 3.1661 3.1685 3.1708 3.1732 3.1756 3.1779 3.1803 3.1826 3.1850 3.1874 3.1897 3.1921 3.1944 3.1968 3.1992 3.2015 3.2039 3.2062 3.2086 3.2110 3.2133 3.2157 3.2181 3.2204 3.2228 3.2251 3.2275 3.2299 3.2322 3.2346 3.2369 3.2393 3.2417 3.2440 3.2464 3.2487 3.2511 3.2535 3.2558 3.2582 3.2606 3.2629 3.2653 3.2676 3.2700 3.2724 3.2747 3.2771 3.2794 3.2818 3.2842 3.2865 3.2889 3.2912 3.2936 3.2960 3.2983 3.3007 3.3031 3.3054 3.3078 3.3101 3.3125 3.3149 3.3196 3.3219 3.3243 3.3267 3.3290 3.3314 3.3337 3.3361 3.3385 3.3408 3.3432 3.3456 3.3479 3.3503 3.3526 3.3550 3.3574 3.3597 3.3621 3.3644 3.3668 3.3692 3.3715 3.3739 3.3762 3.3786 3.3810 3.3833 3.3857 3.3880 3.3904 3.3928 3.3951 3.3975 3.3999 3.4022 3.4046 3.4069 3.4093 3.4117 3.4140 3.4164 3.4187 3.4211 3.4235 3.4258

Page 1

```
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                                   3.4376
                                            3.4400
                                                    3.4424
3.4447
         3.4471
                  3.4494
                          3.4518
                                   3.4542
                                            3.4565
                                                    3.4589
3.4612
         3.4636
                  3.4660
                          3.4683
                                   3.4707
                                            3.4730
                                                    3.4754
3.4778
         3.4801
                  3.4825
                          3.4849
                                   3.4872
                                            3.4896
                                                    3.4919
3.4943
         3.4967
                  3.4990
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        3.9429
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                          3.9476
                                  3.9500
         HYD NO=101 DA= .003098 SQ MI
   ID=1
  PER A=30
             PER B=21
                        PER C=19
                                   PER D=30
```

COMPUTE NM HYD TP=-.20 MASSRAIN=-1

```
K = 0.109000HR TP = 0.200000HR K/TP RATIO = 0.545000
                                                                       SHAPE
CONSTANT, N = 7.106428
        UNIT PEAK = 2.4456 CFS UNIT VOLUME = 0.9949
                                                                      526.28
 P60 = 2.6000
        AREA = 0.000929 \text{ SQ MI} IA = 0.10000 \text{ INCHES} INF = 0.04000
INCHES PER HOUR
        RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
0.050000
```

```
K = 0.214779HR TP = 0.200000HR
                                            K/TP RATIO = 1.073894
                                                                       SHAPE
CONSTANT, N = 3.288961
        UNIT PEAK = 3.3028 CFS
                                    UNIT VOLUME = 0.9958
                                                                      304.60
P60 = 2.6000
        AREA = 0.002169 \text{ SQ MI} IA = 0.52357 \text{ INCHES} INF =
                                                                   1.31600
INCHES PER HOUR
        RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
0.050000
```

PRINT HYD

ID=1 CODE=3

PARTIAL HYDROGRAPH 101.00

TIME FLOW TIME FLOW TIME FLOW Page 2

TIME	FLOW	TIME		IO.OUT		
HRS	HRS	CFS	FLOW HRS	CFS	HRS	CFS
15.300	0.000	0.0	CFS 5.100	0.0	10.200	0.0
15.450	$\begin{array}{c} 0.0 \\ 0.150 \\ 0.0 \end{array}$	0.0	0.0 5.250	0.0	10.350	0.0
15.600	0.00	0.0	0.0 5.400	0.0	10.500	0.0
15.750	0.0 0.450	0.0	0.0 5.550	0.0	10.650	0.0
15.900	0.00	0.0	0.0 5.700	0.0	10.800	0.0
	0.00	0.0	0.0 5.850	0.0	10.950	0.0
16.050	0.00	0.1	0.0 6.000	0.0	11.100	0.0
16.200	0.0 1.050	0.2	0.0 6.150	0.0	11.250	0.0
16.350	1.200	0.4	0.0 6.300	0.0	11.400	0.0
16.500	$\frac{0.0}{1.350}$	21.600 1.4	0.0 6.450	0.0	11.550	0.0
16.650	0.0 1.500	5.8 21.750	0.0 6.600	0.0	11.700	0.0
16.800 16.950	0.0 1.650	21.900 6.6	0.0 6.750	0.0	11.850	0.0
17.100	1.800	3.8	0.0 6.900	0.0	12.000	0.0
17.100	$\frac{0.0}{1.950}$	22.200	7.050	0.0	12.150	0.0
17.230	2.100	22.350 1.1	7.200	0.0	12.300	0.0
17.550	2.250	0.7	7.350	0.0	12.450	0.0
17.700	2.400	0.5	7.500	0.0	12.600	0.0
17.750	0.0 2.550 0.0	0.4	7.650	0.0	12.750	0.0
18.000	2.700	0.3	7.800	0.0	12.900	0.0
18.150	2.850	23.100 0.2	7.950 7.950	0.0	13.050	0.0
18.300	3.000	0.2	8.100	0.0	13.200	0.0
18.450	3.150 0.0	23.400 0.1	0.0 8.250	0.0	13.350	0.0
18.600	3.300	23.550 0.1	0.0 8.400	0.0	13.500	0.0
18.750	3.450 0.0	23.700 0.1	0.0 8.550	0.0	13.650	0.0
18.900	3.600	23.850 0.1 24.000	0.0 8.700	0.0	13.800	0.0
19.050	3.750 0.0	0.1	0.0 8.850	0.0	13.950	0.0
19.200	3.900.0 0.0	24.150 0.0 24.300	9.000	0.0	14.100	0.0
19.350	4.050.0	0.0 24.450	0.0 9.150	0.0	14.250	0.0
19.500	4.200	0.0 24.430 24.600	9.300	0.0	14.400	0.0
19.650	4.350	0.0 24.750	9.450	0.0	14.550	0.0
		27./30	0.0 Page	3		

Page 3

			AHYMO	.OUT		
19.800	4.500 0.0	0.0	9.600	0.0	14.700	0.0
19.950	4.650	0.0	9.750	0.0	14.850	0.0
20.100	4.800	0.0	9.900	0.0	15.000	0.0
20.250	4.950	0.0	10.050	0.0	15.150	0.0

RUNOFF VOLUME = 2.10272 INCHES = 0.3474 ACRE-FEET PEAK DISCHARGE RATE = 7.10 CFS AT 1.600 HOURS BASIN AREA = 0.0031 SQ. MI.

* ROUTE THE TOTAL ROUTE RESERVOIR	ID=2 HYD NO=	PROPOSED RESERVE 102 INFLOW=1 STORAGE(AC-FT) 0.000 0.045 1.94	CODE=3	25.00
	2.57	0.231	26.00	

TIME **INFLOW ELEV VOLUME** OUTFLOW (HRS) (CFS) (FEET) (AC-FT) (CFS) 0.00 0.00 23.00 0.000 0.00 0.15 0.00 23.00 0.000 0.00 0.30 0.00 23.00 0.000 0.00 0.45 0.00 23.00 0.000 0.00 0.60 0.00 23.00 0.000 0.00 0.75 0.01 23.00 0.000 0.00 0.90 0.09 23.01 0.001 0.01 1.05 0.19 23.04 0.002 0.04 1.20 0.43 23.11 0.005 0.10 1.35 1.42 23.27 0.012 0.27 1.50 5.82 24.03 0.048 1.01 1.65 6.56 24.90 0.113 1.84 1.80 3.81 25.29 0.153 2.12 1.95 2.04 25.36 0.161 2.17 2.10 1.14 25.30 0.154 2.13 2.25 0.73 25.16 0.139 2.04 2.40 0.54 25.01 0.122 1.95 2.55 0.39 24.79 0.105 1.73 2.70 0.28 24.57 0.089 1.53 2.85 0.21 24.38 0.074 1.35 3.00 0.16 24.21 0.061 1.18 3.15 0.12 24.05 0.049 1.03 3.30 0.10 23.85 0.038 0.84 3.45 0.08 23.67 0.030 0.66 3.60 0.07 23.53 0.024 0.52 3.75 0.06 23.42 0.019 0.41 3.90 0.05 23.33 0.015 0.33 Page 4

4.25 4.35 4.35 4.50 4.50 4.50 5.20 5.70 5.05 6.70 7.70 7.70 7.70 7.80 8.90 7.70 7.70 7.70 7.80 8.90 8.90 8.90 8.90 7.70 7.70 7.70 7.70 7.70 8.90 8.90 8.90 8.90 8.90 8.90 8.90 8.9	0.04 0.03 0.02 0.02 0.02 0.02 0.02 0.03 0.03	23.26 23.21 23.17 23.13 23.03	AHYMO.OUT 0.012 0.009 0.008 0.005 0.004 0.003 0.002 0.002 0.002 0.001 0.001 0.001 0.001 0.001 0.001 0.001 0.001 0.001 0.001 0.001 0.001 0.001 0.001 0.001	0.21 0.21 0.17 0.13 0.03 0.03 0.03 0.03 0.03 0.03 0.03
8.25 TIME (HRS)	0.03 INFLOW (CFS)	23.03 ELEV (FEET)	0.001 VOLUME (AC-FT)	0.03 OUTFLOW (CFS)
8.40 8.55 8.70 8.85 9.00 9.15 9.30 9.45 9.60 10.20 10.35 10.65 10.65 10.80 11.25 11.40 11.25 11.40 11.25 12.30 12.45 12.60 12.75	0.03 0.03 0.03 0.03 0.03 0.03 0.03 0.03	23.03 23.03	0.001 0.001	0.03 0.03 0.03 0.03 0.03 0.03 0.03 0.03

12.90 13.05 13.20 13.35 13.65 13.65 13.65 14.25 14.25 14.25 14.85 15.45 15.45 15.45 15.45 15.45 15.45 15.45 16.20 16.35 16.35 16.65	0.03 0.03 0.03 0.03 0.03 0.03 0.03 0.03	23.03 23.03 23.03 23.03 23.03 23.03 23.03 23.03 23.03 23.03 23.03 23.03 23.03 23.03 23.03 23.03 23.03 23.03	AHYMO.OUT 0.001	0.03 0.03 0.03 0.03 0.03 0.03 0.03 0.03
TIME (HRS)	INFLOW (CFS)	ELEV (FEET)	VOLUME (AC-FT)	OUTFLOW (CFS)
16.80 17.10 17.25 17.40 17.55 17.70 17.85 18.15 18.30 18.45 18.60 19.20 19.35 19.65 19.80 19.80 19.80 19.80 19.80 20.25 20.40 20.25 20.40 20.55 21.45 21.45 21.45 21.45	0.03 0.03 0.03 0.03 0.03 0.03 0.03 0.03	23.03 23.03	0.001 0.001	0.03 0.03 0.03 0.03 0.03 0.03 0.03 0.03

21.75 21.90 22.05 22.35 22.65 22.65 22.80 22.95 23.10 23.25 23.40 23.55 24.00 24.15 24.30 24.45 24.60 24.75 24.90 25.05	0.03 0.03 0.03 0.03 0.03 0.03 0.03 0.03	23.03 23.03 23.03 23.03 23.03 23.03 23.03 23.03 23.03 23.03 23.03 23.03 23.03 23.03 23.03 23.03 23.01 23.01 23.01 23.01	AHYMO.OUT 0.001 0.001 0.001 0.001 0.001 0.001 0.001 0.001 0.001 0.001 0.001 0.001 0.001 0.001 0.001 VOLUME (AC-FT)	0.03 0.002 0.02 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.05 0.05	
25.20 25.35 PEAK DISCHAR MAXIMUM WATE MAXIMUM STOR	R SURFACE	23.01 23.00 2.169 CI ELEVATION 0.1610	= 25	0.01 0.00 CCURS AT HOUR 1. .364 INCREMENTAL TIME=	95 0.050000HRS
PRINT HYD	ID=	2 CODE=3	3		

PARTIAL	HYDROGRAPH	102.00
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TIME	TIME	FLOW	TIME	FLOW	TIME	FLOW
1 TIAIT	FLOW HRS	TIME CFS	FLOW	C=C		-
HRS	CFS	HRS	HRS CFS	CFS	HRS	CFS
_	0.000	0.0	5.250	0.0	10.500	^ ^
15.750	0.0	21.000	0.0	0.0	TO. 200	0.0
	0.150	0.0	5.400	0.0	10.650	0.0
15.900	0.0	21.150	0.0		20.030	0.0
16 050	0.300	0.0	5.550	0.0	10.800	0.0
16.050	0.0	21.300	0.0			
16 200	0.450	0.0	5.700	0.0	10.950	0.0
16.200	0.0	21.450	0.0			
16.350	0.600 0.0	0.0	5.850	0.0	11.100	0.0
10.550	0.750	0.0	6.0	0 0	44 252	
16.500	0.750	21.750	6.000 0.0	0.0	11.250	0.0
	0.900	0.0	6.150	0.0	11 400	^ ^
16.650	0.0	21.900	0.0	0.0	11.400	0.0
	1.050	0.0	6.300	0.0	11.550	0.0
16.800	0.0	22.050	0.0		14. 330	0.0
16 050	1.200	0.1	6.450	0.0	11.700	0.0
16.950	0.0	22.200	0.0			0.0
17 100	1.350	0.3	6.600	0.0	11.850	0.0
17.100	1 500	22.350	0.0			
17.250	1.500 0.0	1.0	6.750	0.0	12.000	0.0
-, . <u>-) (</u>	0.0	22.500	0.0	 -		
			Page	· /		

Page 7

			AHYMO	OUT		
17.400	1.650 0.0	1.8 22.650	6.900	0.0	12.150	0.0
	1.800	2.1	7.050	0.0	12.300	0.0
17.550	0.0 1.950	22.800	0.0 7.200	0.0	12.450	0.0
17.700	0.0 2.100	22.950 2.1	0.0 7.350	0.0	12.600	
17.850	0.0 2.250	23.100	0.0			0.0
18.000	0.0	23.250	7.500 0.0	0.0	12.750	0.0
18.150	2.400	1.9 23.400	7.650 0.0	0.0	12.900	0.0
18.300	2.550 0.0	1.7 23.550	7.800 0.0	0.0	13.050	0.0
18.450	2.700 0.0	1.5 23.700	7.950 0.0	0.0	13.200	0.0
18.600	2.850	1.3	8.100	0.0	13.350	0.0
	3.000	23.850 1.2	0.0 8.250	0.0	13.500	0.0
18.750	0.0 3.150	24.000 1.0	0.0 8.400	0.0	13.650	0.0
18.900	0.0 3.300	24.150 0.8	0.0 8.550	0.0	13.800	
19.050	0.0 3.450	24.300 0.7	0.0			0.0
19.200	0.0	24.450	8.700 0.0	0.0	13.950	0.0
19.350	3.600 0.0	0.5 24.600	8.850 0.0	0.0	14.100	0.0
19.500	3.750 0.0	0.4 24.750	9.000	0.0	14.250	0.0
19.650	3.900 0.0	0.3 24.900	9.150	0.0	14.400	0.0
19.800	4.050	0.3	9.300	0.0	14.550	0.0
	0.0 4.200	0.2	0.0 9.450	0.0	14.700	0.0
19.950	0.0 4.350	25.200 0.2	0.0 9.600	0.0	14.850	0.0
20.100	0.0 4.500	25.350 0.1	0.0 9.750	0.0		
20.250	0.0 4.650	25.500 0.1	0.0		15.000	0.0
20.400	0.0	25.650	9.900	0.0	15.150	0.0
20.550	4.800	0.1 25.800	10.050	0.0	15.300	0.0
20.700	4.950 0.0	0.1 25.950	10.200	0.0	15.450	0.0
20.850	5.100		10.350.0	0.0	15.600	0.0
	RUNOFF VOLU		60 INCHES	=	0.3474 ACRE-FI	FFT
0.0031 5	PEAK DISCHA	RGE RATE =	2.17 CFS	AT 1	_ _	IN AREA =

FINISH

NORMAL PROGRAM FINISH

END TIME (HR:MIN:SEC) = 09:11:29

CITY OF ALBUQUERQUE



February 4, 2009

Jack Tillman, PE
Tillman and Associates
5065 Hunter's Chase
Las Cruces, NM 88011

Re: Monterey Baptist Church Grading and Drainage Plan

Engineer's Stamp dated 8-28-08 (J22/D12A)

Dear Mr. Tillman,

Based upon the information provided in your submittal dated 1-28-09, the above referenced plan is approved for Building Permit. Please attach a copy of this approved plan to the construction sets prior to sign-off by Hydrology.

PO Box 1293

Also, prior to Certificate of Occupancy release, Engineer Certification of the grading plan per the DPM checklist will be required.

Albuquerque

If you have any questions, you can contact me at 924-3986.

NM 87103

Bradley L. Bingham, PE

Sincerely,

www.cabq.gov

Principal Engineer, Planning Dept. Development and Building Services

C: file

DRAINAGE AND TRANSPORTATION INFORMATION SHEET

(REV 12/2005)

PROJECT TITLE: MONTEREY BAPTIST (HURC	<u>ロ</u> ZONE MAP: <u> </u>
· · · · · · · · · · · · · · · · · · ·	WORK ORDER#:
LEGAL DESCRIPTION:	
CITY ADDRESS:	
ENGINEERING FIRM: TILLMAN WID ASSOC	FATES CONTACT: JACK TILLMAN
ADDRESS: 5065 HUNTER'S CHASE	PHONE: 505-379-9715
CITY, STATE: LAS CRUCES NM	
OWNER: MONTEREY BAPTIST CHURCH	CONTACT:
ADDRESS: 12501 Lomas BUD IVE	
CITY, STATE: ALBUQUELQUE, Non	ZIP CODE: 87//2
ARCHITECT:	CONTACT:
ADDRESS:	PHONE:
CITY, STATE:	ZIP CODE:
SURVEYOR:	CONTACT:
ADDRESS:	PHONE:
CITY, STATE:	ZIP CODE:
CONTRACTOR:	CONTACT:
ADDRESS:	PHONE:
CITY, STATE:	ZIP CODE:
TYDE OF CITIES ATTENDED.	CHECK TREE OF A DEPOSITATE COLLEGES
	CHECK TYPE OF APPROVAL SOUGHT:
DRAINAGE REPORT DRAINAGE PLAN 1 st SUBMITTAL	SIA/FINANCIAL GUARANTEE RELEASE
DRAINAGE PLAN I SUBMITTAL DRAINAGE PLAN RESUBMITTAL	PRELIMINARY PLAT APPROVAL
CONCEPTUAL G & D PLAN	S. DEV. PLAN FOR SUB'D APPROVAL S. DEV. FOR DIDG DEDMIT ADDROVATAGE CO.
GRADING PLAN	S. DEV. FOR BLDG. PERMIT APPROYAL) LOGY SECTOR PLAN APPROVAL
EROSION CONTROL PLAN	FINAL PLAT APPROVAL SECTION SECTION FINAL PLAT APPROVAL
ENGINEER'S CERT (HYDROLOGY)	FOUNDATION PERMIT APPROVAL
CLOMR/LOMR	BUILDING PERMIT APPROVAL:
TRAFFIC CIRCULATION LAYOUT	CERTIFICATE OF OCCUPANCY (PERM)
ENGINEER'S CERT (TCL)	CERTIFICATE OF OCCUPANCY (TEMP)
ENGINEER'S CERT (DRB SITE PLAN)	GRADING PERMIT APPROVAL
OTHER (SPECIFY)	PAVING PERMIT APPROVAL
	WORK ORDER APPROVAL
	OTHER (SPECIFY)
-	
WAS A PRE-DESIGN CONFERENCE ATTENDED	•
YES	
NO	
COPY PROVIDED	
	en ame
DATE SUBMITTED: 1/28/09	BY: Beorge Taylor

Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a drainage submittal. The particular nature, location, and scope to the proposed development defines the degree of drainage detail. One or more of the following levels of submittal may be required based on the following:

- 1. Conceptual Grading and Drainage Plan: Required for approval of Site Development Plans greater than five (5) acres and Sector Plans.
- 2. Drainage Plans: Required for building permits, grading permits, paving permits and site plans less than five (5) acres.
- 3. Drainage Report: Required for subdivision containing more than ten (10) lots or constituting five (5) acres or more.

