

CONSTRUCTION NOTES:

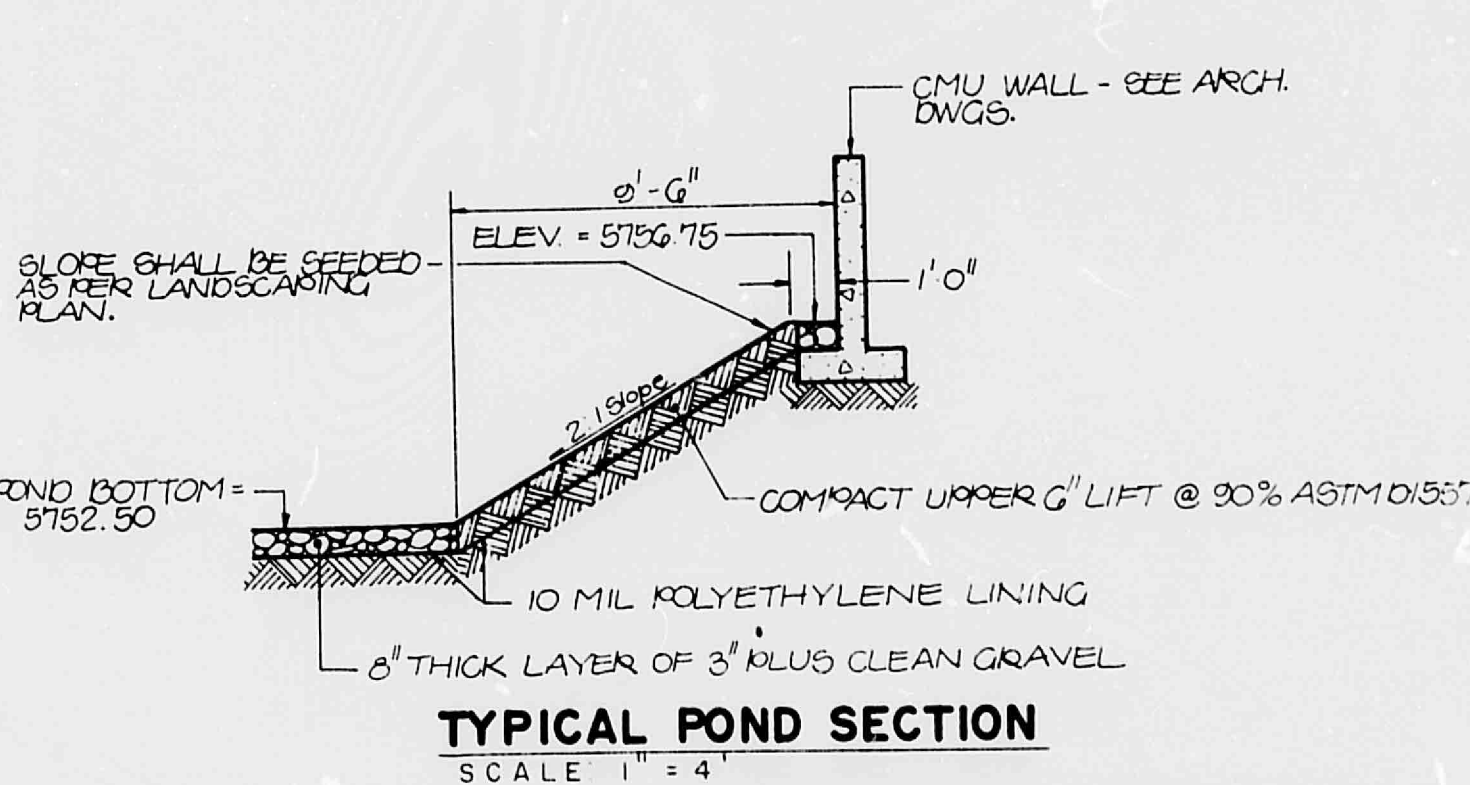
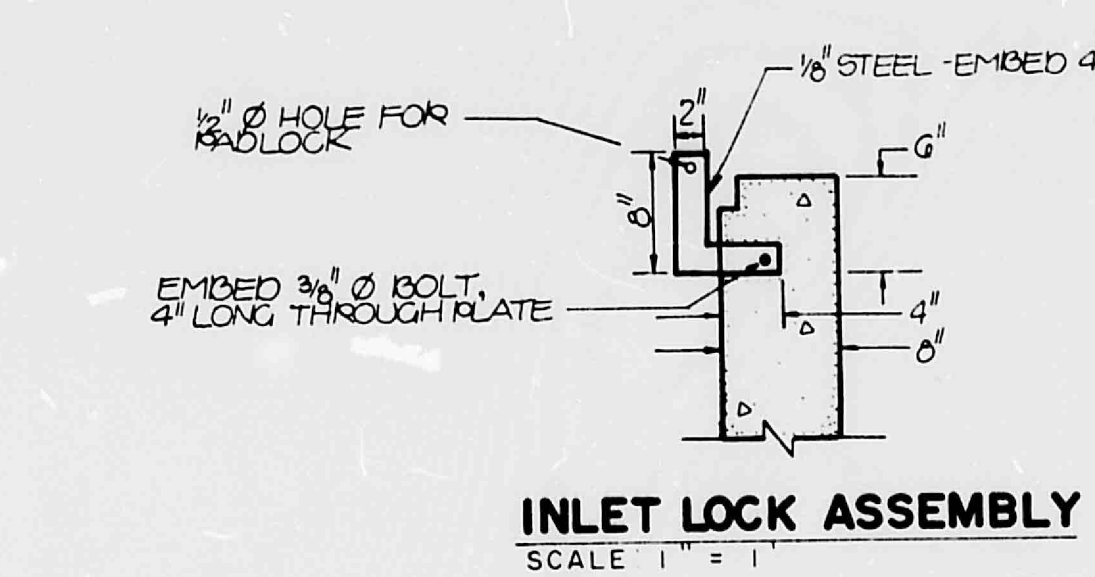
- TWO (2) WORKING DAYS PRIOR TO ANY EXCAVATION, CONTRACTOR MUST CONTACT LINE LOCATING SERVICE 765-1234, FOR LOCATION OF EXISTING UTILITIES.
- PRIOR TO CONSTRUCTION, THE CONTRACTOR SHALL EXCAVATE AND VERIFY THE HORIZONTAL AND VERTICAL LOCATION OF ALL POTENTIAL OBSTRUCTIONS. SHOULD A CONFLICT EXIST, THE CONTRACTOR SHALL NOTIFY THE ENGINEER SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM AMOUNT OF DELAY.
- ALL WORK ON THIS PROJECT SHALL BE PERFORMED IN ACCORDANCE WITH APPLICABLE FEDERAL, STATE AND LOCAL LAWS, RULES AND REGULATIONS CONCERNING CONSTRUCTION SAFETY AND HEALTH.
- ALL CONSTRUCTION WITHIN PUBLIC RIGHT-OF-WAY SHALL BE PERFORMED IN ACCORDANCE WITH APPLICABLE CITY OF ALBUQUERQUE STANDARDS AND PROCEDURES.
- MANHOLES SHALL BE CONSTRUCTED IN ACCORDANCE WITH CITY OF ALBUQUERQUE STANDARD DRAWING S-2-3. MANHOLES SHALL BE 4' DIAMETER TYPE 'E'.
- CATCH BASINS SHALL BE SINGLE 'D' CONSTRUCTED IN ACCORDANCE WITH CITY OF ALBUQUERQUE STANDARD DRAWING K-6-2.
- SIDEWALK CULVERTS SHALL BE CONSTRUCTED IN ACCORDANCE WITH CITY OF ALBUQUERQUE STANDARD DRAWING K-16-1.

EROSION CONTROL MEASURES

- DURING CONSTRUCTION, THE CONTRACTOR SHALL ENSURE THAT NO SOIL ERODES FROM THE SITE INTO PUBLIC RIGHT-OF-WAY OR ONTO PRIVATE PROPERTY. THIS CAN BE ACHIEVED BY CONSTRUCTING TEMPORARY BERMS AT THE PROPERTY LINES AND WETTING THE SOIL TO KEEP IT FROM BLOWING.
- THE CONTRACTOR SHALL PROMPTLY CLEAN UP ANY MATERIAL EXCAVATED WITHIN THE PUBLIC RIGHT-OF-WAY SO THAT THE EXCAVATED MATERIAL IS NOT SUSCEPTIBLE TO BEING WASHED DOWN THE STREET.

PROJECT BENCHMARK = TM

ACS BRASS CAP STAMPED "1-J23, 1982" LOCATED AT THE TRAMWAY BOULEVARD/INDIAN SCHOOL ROAD INTERSECTION ON THE NOSE OF THE MEDIAN IN TRAMWAY, 182' NORTH OF INDIAN SCHOOL CENTERLINE.
ELEV. = 5840.70 FEET MSLD.



DRAINAGE PLAN

The following items concerning the Estridge Apartment Complex Drainage Plan are contained hereon:

- Vicinity Map
- Grading Plan
- Calculations

The proposed improvements, as shown by the Vicinity Map, are located at the northwest corner of Indian School Road and Estridge Drive. At present, the site is undeveloped. The parcel to the north is currently developed with residential housing. The parcel to the west is currently undeveloped. As shown on the Flood Insurance Maps, this site does not lie in a Flood Hazard Zone. However, downstream flooding does occur in Lomas Boulevard, therefore, onsite ponding is being proposed. The site slopes from east to west and from south to north away from the improved streets, therefore, a pump is proposed to discharge runoff into Indian School Road. No offsite flows enter the site.

The proposed Grading Plan shows 1) existing and proposed grades indicated by spot elevations and contours at 1' 0" intervals, 2) continuity between existing and proposed elevations, and 3) the limit and character of the proposed improvements. As shown by this plan, the proposed improvements consist of construction of two-story apartment buildings with associated parking and entrance ways. Basin 1 will outlet through four 4-inch PVC pipes at the southwest corner of the site. Water will be ponded on the asphalt until discharge into Estridge Drive occurs. Basin 2 will drain to the northwest corner of the site at which point a pond has been provided for storage of the 100-year runoff volume. The water will then be pumped through a 4-inch PVC pipe so that the pond drains within 24 hours.

The Calculations which appear below analyze both the existing and developed conditions for the 100-year, 6-hour rainfall event. The Rational Method has been used for this analysis in accordance with the City of Albuquerque Development Process Manual, Volume II. As shown by these calculations, the proposed improvements will result in a decrease in the rate of runoff and an increase in the volume of runoff. The development of this site will help to alleviate flooding problems in Lomas Boulevard. The design of this site is consistent with the predesign conference recap which accompanies this submittal.

CALCULATIONS

Ground Cover Information
From SCS Bernalillo County Soil Survey, Plate 32:
Embudo Tijeras Complex
Hydrologic Soil Group B

Rational Method
Discharge: $Q = CiA$
where C varies
 $i = P_2 (6.84) T_c^{-0.51} = 5.50 \text{ in/hr}$
 $P_2 = 2.6 \text{ in (DPM Plate 22.2D-1)}$
 $T_c = 10 \text{ min (minimum)}$
 $A = \text{area, acres}$

Volume: $V = CpA(1/12)$
where c varies
 $P_2 = 2.6 \text{ in (DPM Plate 22.2D-1)}$
 $A = \text{area, sf}$

Existing Condition
 $A_{\text{total}} = 153,750 \text{ sf} = 3.53 \text{ Ac}$
 $A_{\text{imp}} = 0 \text{ sf; } \% \text{ impervious} = 0\%$
 $C_{\text{imp}} = 0.34 \text{ (DPM Plate 22.2C-1)}$
 $Q_{100} = CiA = 0.34 (5.50) 3.53 = 6.60 \text{ cfs}$
 $V_{100} = CpA = 0.34 (2.6/12) 153,750 = 11,325 \text{ cf}$

Developed Condition
Basin 1
 $A_{\text{total}} = 31,205 \text{ sf} = 0.72 \text{ Ac}$
 $A_{\text{imp}} = 23,715 \text{ sf; } \% \text{ impervious} = 76\%$
 $C_{\text{imp}} = 0.74$
 $Q_{100} = CiA = 0.74 (5.50) 0.72 = 4.1$
 $V_{100} = CpA = 0.74 (2.6/12) 31,205 = 5005 \text{ cf}$

Basin 2
 $A_{\text{total}} = 122,545 \text{ sf} = 2.81 \text{ Ac}$
 $A_{\text{imp}} = 84,555 \text{ sf; } \% \text{ imp} = 69\%$
 $C_{\text{imp}} = 0.69$
 $Q_{100} = CiA = 0.69 \times (5.50) 2.81 = 10.7 \text{ cfs}$
 $V_{100} = 18,300 \text{ cf}$

Pond Volume
Basin 1
 $V_{\text{required}} = 465 \text{ cf (by hydrograph analysis)}$
 $V_{\text{pond}} = 160.15 = 1760$
 $A_{59.65} = 676$
 $A_{59.15} = 0$
 $V_{\text{Vol}} = 610$
 $V_{\text{Vol}} = 170$
 $V_{\text{Vol}} = 780 \text{ cf}$

Basin 2
 $V_{\text{required}} = 18,300 \text{ cf}$
 $V_{\text{pond}} = 56.75 = 7125 \text{ sf}$
 $A_{56.75} = 4985 \text{ sf}$
 $A_{52.50} = 3000 \text{ sf}$
 $V_{\text{Vol}} = 4540 \text{ cf}$
 $V_{\text{Vol}} = 13475 \text{ cf}$
 $V_{\text{Vol}} = 18515 \text{ cf}$

Discharge:
Basin 1
4-4" pipes
orifice equation: $Q = CA \sqrt{2gh}$
 $C = 0.7$
 $A = 4 (0.0855) = 0.342 \text{ sf}$
 $h = 32.2 \text{ ft/sec}^2$
 $g = 10^4 = 0.83$
 $Q = 0.7 (0.342) \sqrt{2(32.2)(0.83)} = 1.75 \text{ cfs}$

Basin 2
pump to discharge @ 125 gpm = 0.27 cfs

Comparison
 $Q_{100} = 6.6 - 1.75 - 0.22 = 4.63 \text{ cfs}$
 $V_{100} = 5005 + 18300 - 11325 = 11980 \text{ cf}$

APPROVED
SEP 12 1986
HYDROLOGY SECTION

7/1/86
11-6-85
9.11.86

POND CONSTRUCTION			
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<p>DESIGNED BY: S.K.S.</p> <p>DRAWN BY: J.M.C.</p> <p>APPROVED: T.T.M.</p>	<p>NO. DATE BY REVISIONS</p> <p>1. 11/1/86 S.K.S. SECTION A-A</p>	<p>JOB NO. 40521</p> <p>DATE 6-84</p>	<p>DRAINAGE CERTIFICATION</p> <p>TRACT C & D M.P.E. - EASTRIDGE</p>	<p>FILE NO.</p> <p>J22/D35</p> <p>SHEET 2 OF 2</p>