

*Return with plans (review)*

RESUBMITTED  
11-13-78

932 CHELWOOD PARK BLVD N.E.  
STANDARD REQUIREMENTS FOR DRAINAGE PLANS  
GRANDVIEW HEIGHTS LOT 3 BLK 10

**PURPOSE:** The increasing volume of drainage plans submitted to this office makes it mandatory that such plans be standardized as much as possible in order to expedite reviews. This standardization is as much to the advantage of the developer and engineer as it is to the Hydrology Section which enforces the AMAFCA RES. 72-2. For parcels of land less than 20,000 sq. ft. in surface area no formal drainage report is required; the construction plans need only to include the standard form attached herein and the site drainage plan. Developers for larger parcels of land will have to submit a formal drainage report as specified in the Resolution.

*Approved  
because  
ponding was  
mentioned  
adequately*

**RUNOFF PONDING:** In most instances on site ponding is mandatory, with disposal in the ground of the excess runoff arising from newly created impervious surfaces. The only exception allowed, is for those properties adjacent to a diversion channel which was designed for higher standard than 100 years frequency storm (existing conditions). For detailed computations of the runoff before and after development the assumed runoff coefficient recommended are  $C = 0.4$  for undeveloped, landscaped or similar open areas and  $C = 0.9$  for all other impervious surfaces, including areas in southwestern type landscaping with underlying polyethylene film and gravel covered parking areas where vehicular traffic will compact the soil and render it impervious. Due to the inadequacy of the existing drainage facilities in the valley area and to the limited capabilities of the City for providing relief, ponding requirements in the valley are higher than elsewhere.

COMPUTATION OF VOLUME OF RETENTION:

Valley Area =  $1.0 \times \frac{2.2"}{12"} \times \text{Area (ft.)} = 0.18 \times A$

East and West Mesa =  $(0.9 - 0.4) \times \frac{2.4"}{12"} \times \text{Area (ft.)} = 0.1 \times A$

In order to facilitate the design of drainage facilities, a checklist that will be followed in the review process is listed below:

CHECK LIST

1 - Flooding potential - adjacent water courses

*NO* Is property located in the flood plain? *J22 FROM 1825*

If so, is the finished floor above the 100 yrs. flood level?

*NO* Is property adjacent to a natural or artificial water course?

If so, what are the specific AMAFCA or City requirements?

I NEED THE PROPOSED GRADES FOR THE ALLEY.

REQUIRE TWO COPIES OF SITE GRADING PLAN - FOR MY FILES

STANDARD REQUIREMENTS FOR  
DRAINAGE PLANS

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~~NO~~ Are drainage R.O.W or easements shown on, or in the proximity of property? If so, are there drainage problems?

2 - Relation of property to surroundings

~~YES~~ Per topo map, does property intercept other drainage upstream?

? If so, how is runoff conveyed across property?

May there be erosion associated with offsite runoff conveyance?

May erosion or siltation result from proposed construction activities?

Does development block drainage from adjacent property?

3 - Site grading

~~NO~~ Does site plan show contours before development (extending a minimum of 25 ft. beyond property lines)?

~~NO~~ Does site plan show proposed grading with adequate swale definition to convey water to ponds?

~~NO~~ Is all runoff conveyed to ponding areas before it overflows to public facilities?

? Does the proposed grading plan indicate that under cutting or back-filling adjacent to property lines may require retention walls?

? Is there continuity between proposed new contours and old contours offsite?

~~YES~~ Is elevation of property line at least 0.3 ft. above top of curb?

4 - Storm water retention

Is ponding volume adequate (supply detailed computation)? *3:1 SLOPE REQUIRED*

Are ponds balanced with areas they drain (can area draining to each pond be easily identified and will actually water flow there)? The plot plan must outline each drainage area. *SHOW VOLUME DRAINING TO EACH POND*

~~NO~~ Can pond volume be computed and verified?

Are ponds practical, can they be built as shown?

5 - Safety

Do the drainage provisions constitute an attractive nuisance, or safety hazard?

STANDARD REQUIREMENTS FOR  
DRAINAGE PLANS

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*LESS THAN 18"* If the pond depth is greater than 18", are safety provisions supplied?  
(Minimum 3.0 ft. high chain link fence or similar physical barrier  
of ponding areas are adjacent to public R.O.W.?)

In general, ponds of depth greater than 18" will not be accepted for both safety consideration and for long term effectiveness of the facilities. In those cases where limited space is available for ponding, the use of gravel pits under the parking area is suggested. It must be pointed out that mainstream and effectiveness of these facilities is necessary and is the responsibility of the owner. Existing or planned City facilities (streets, channels, storm sewers) can accommodate the natural runoff volumes. Greater discharges would cause flooding downstream and need to be limited at the source.

COMPUTATION OF VOLUME OF RETENTION PER  
CITY OF ALBUQUERQUE EQUATION

$$\text{VALLEY AREA } 1.0 \times 2.2 \times \text{AREA (FT.)} = 0.19 \times A$$

$$\text{EAST \& WEST MESA: } \frac{(0.9 - 0.4) \times 2.4 \times \text{AREA (FT.)}}{12} = 0.1 \times A$$

COMPUTATIONS FOR:

LOT # 3

LOT 8280 SQ. FT.

BLK # 10

BUILDINGS 1405 SQ. FT.

ADDITION GRANDVIEW HEIGHTS WALKS \& BLACKTOP 2776 SQ. FT.

TOP SOIL 4099 SQ. FT.

1.  $\frac{0.4 \times 2.4 \times 8280}{12} = 662 \text{ FT}^3 \text{ EXISTING RUNOFF}$
2.  $\frac{0.9 \times 2.4 \times 4181}{12} = 753 \text{ FT}^3 \text{ IMPERVIOUS SURFACE RUNOFF}$
3.  $\frac{0.4 \times 2.4 \times 4099}{12} = 328 \text{ FT}^3 \text{ TOP SOIL \& LANDSCAPED RUNOFF}$
4.  $0.1 \times 4181 = 418 \text{ FT}^3 \text{ TOTAL RETENTION VOLUME PER FORMULA}$

$$2 = 753 \text{ FT}^3$$

$$3 = 328 \text{ FT}^3$$

$$2 = 1081 \text{ FT}^3 \text{ TOTAL RUNOFF AFTER DEVELOPMENT}$$

$$2 = 1081 \text{ FT}^3$$

$$1 = 662 \text{ FT}^3$$

$$419 \text{ FT}^3 \text{ TOTAL RETENTION REQUIRED}$$

P. V. ROBINSON 11/17/78 (REV.)

POND A. SURF. AREA. 840 #

NOT. AREA. 663 #

$$2 \sqrt{1503 \text{ #}} (751 \text{ #} @ .55 \text{ DEPTH} = 413 \text{ FT}^3)$$

ROOF AREA, BLDG. 1338 #

$$\frac{(1.9)(2.4)(1338)}{12} = 240 \text{ FT}^3$$

TOP SOIL AREA, REAR Yd. 1550 #

$$\frac{(1.4)(2.4)(1550)}{12} = 120 \text{ FT}^3$$

360 FT<sup>3</sup> REQ.

$$2400 \times 2.4 \times 4 = 192 \text{ FT}^3$$

$$\text{TOTAL } 240 + 192 = 432 \text{ FT}^3$$

$$\text{VOLUME PROVIDED } 413 < 432$$

POND B SURF AREA 238 M

BOT. AREA 95

$$2 \overline{) 333 A} \left| 166.5 \text{ @ } .55 \text{ DEPTH} = 92 \text{ FT}^3 \right.$$

TOP SOIL @ BLD'G SIDES & FRONT 343 M

$$\frac{(.4)(2.4)(343)}{12} = 27.4 \text{ FT}^3$$

WALKS 250 M

$$\frac{(.9)(2.4)(250)}{12} = 45 \text{ FT}^3$$

72 FT<sup>3</sup> REQ.

POND C

SURF AREA 100.75' 8.5' x 13 = 1105

BOT. AREA 4.76 10' x 6' 160

$$2 \overline{) 142.51} \left| 71.25 \text{ @ } .55 \text{ DEPTH} = 39 \text{ FT}^3 \right. \checkmark$$

TOP SOIL @ N. SIDE PARKING 154 M

$$\frac{(.4)(2.4)(154)}{12} = 12.3 \text{ FT}^3 \text{ REQ.}$$

POND D

SURF AREA 116.25'

BOT. AREA 50.46

$$2 \overline{) 166.71} \left| 83.35 \text{ @ } .55 \text{ DEPTH} = 46 \text{ FT}^3 \right.$$

TOP SOIL @ S. SIDE PARKING 242 M

$$\frac{(.4)(2.4)(242)}{12} = 19.4 \text{ FT}^3 \text{ REQ.}$$

TOTAL BLACKTOP AREA 2244 M

✓ DRIVEWAY 282 M

2526 M

$$\frac{(.9)(2.4)(2526)}{12} = 455 \text{ FT}^3 \text{ REQ.}$$

TOTAL ALLOWABLE RUNOFF 662 FT<sup>3</sup>

TOTAL BLACKTOP DRIVE 455 FT<sup>3</sup>

207 FT<sup>3</sup> RETAINED EXCESS

COULD REDUCE POND DEPTHS. OR ELIMINATE

PONDS C & D