MASTER DRAINAGE PLAN

for the

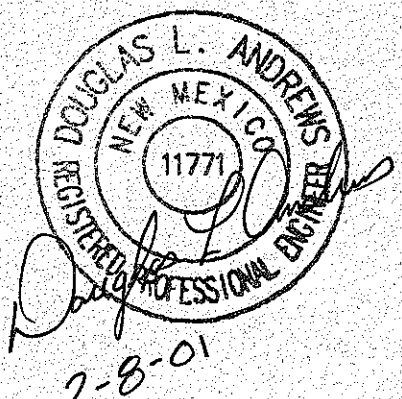
WEST SIDE TRANSIT FACILITY

CITY OF ALBUQUERQUE, NEW MEXICO

Prepared for:

CITY OF ALBUQUERQUE
TRANSIT DEPARTMENT

February, 2001



2-8-01

Prepared by:

Smith Engineering Company
A Full-Service Engineering Company

TABLE OF CONTENTS

TABLES

FIGURES

TABLE OF CONTENTS - continued

APPENDICIES

MAPS – in back pockets of Report

I. INTRODUCTION

The City of Albuquerque Transit Department is planning to construct a transit facility on the city's west side to maintain and operate a fleet of 125 buses and 35 vans. The proposed facility will be an integral part of the Transit Department's promotion of alternative transportation services. The proposed site consists of approximately 20 acres located south and east of the Interstate 40 and Unser Boulevard interchange. A vicinity map showing the location of the proposed facility is shown in Figure 1.

Smith Engineering Company (SEC) has been retained by DWL Architects to provide civil engineering services for the proposed West Side Transit Facility. Included in the civil engineering scope of work is to provide drainage engineering services for the development of the West Side Transit Facility.

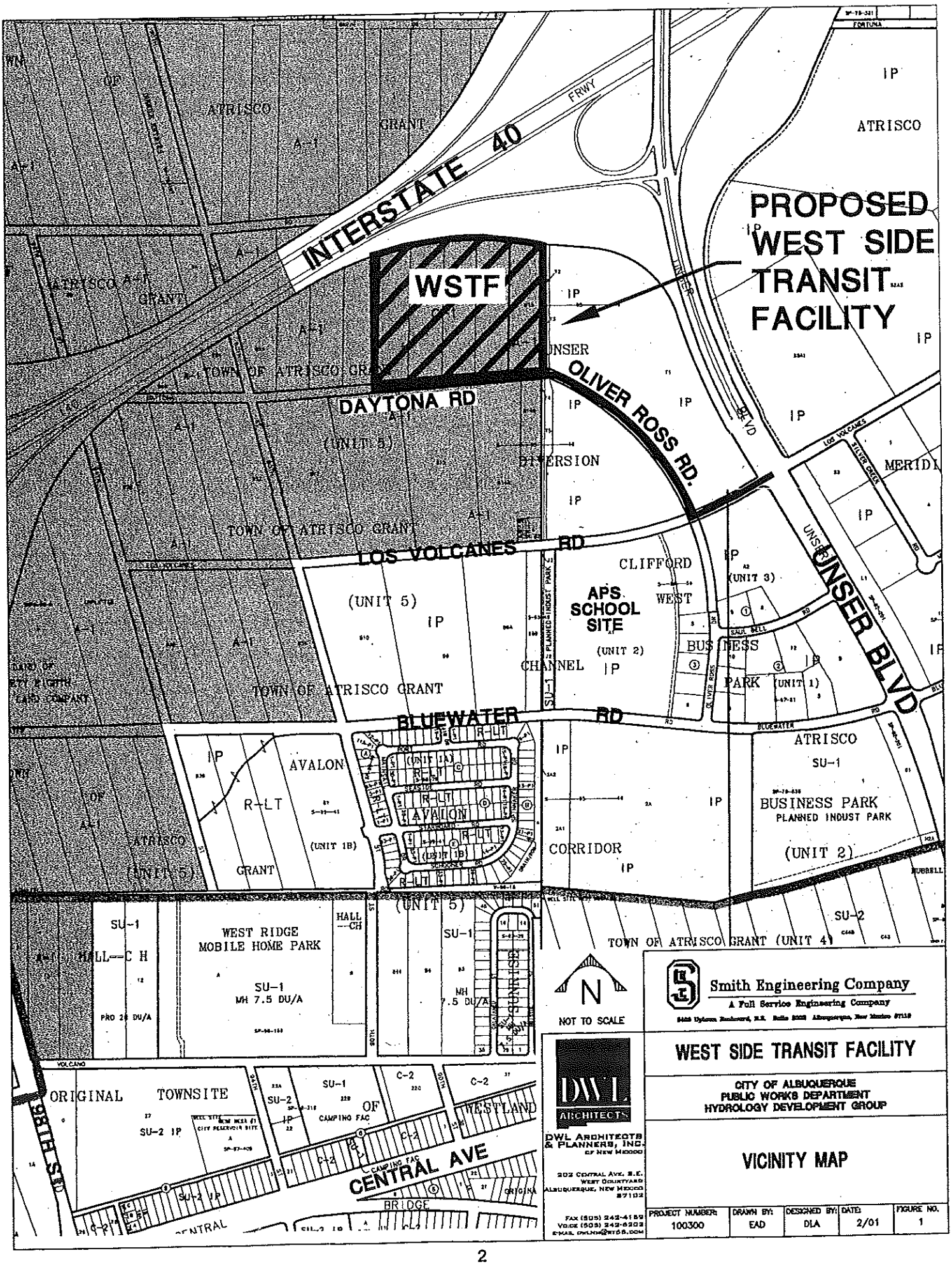
The proposed site lies within a drainage basin that contributes to the Unser Diversion. The Unser Diversion is a series of six detention ponds connected in series by storm drain culvert pipes. Initially, SEC met with the City of Albuquerque Hydrology Development Section to determine the requirements for proper drainage of the proposed West Side Transit Facility. SEC entered the meeting proposing free discharge from the fully developed West Side Transit Facility to the Unser Diversion Pond No. 6. The city's obvious response was "What effect will this have on the Unser Diversion system as a whole?" At the same meeting, the City mentioned that a master plan for the Unser Diversion drainage basin could be useful for future development.

In order to accommodate the city's requirement (of analyzing the entire Unser Diversion), SEC decided to prepare this master plan not only to obtain preliminary approval of the drainage plan for the Transit Facility, but to also provide the city with a plan for future development within the Unser Diversion drainage basin.

This master drainage plan will analyze the hydraulic capacity of the Unser Diversion for existing, interim, and fully developed conditions. This master plan will also provide a hydraulic analysis/design of storm drain required within roadways impacted by the development of the Transit Facility. These roads included Daytona Road, Oliver Ross Road and Los Volcanes Road.

II. DRAINAGE AREA BOUNDARIES

The drainage basin boundary for the area north of Interstate 40 (considered off-site basins in this report) is bounded on the north by the Ladera Diversion and Detention Facilities, the south by Interstate 40 and east and west by existing topographic features. The drainage basin boundary for the area south of Interstate 40 (considered on-site basins in this report) is generally bounded on the north by Interstate 40, the south by the Avalon Subdivision Unit I, the west by property lines and topographic features between 94th Street and 98th Street and the east by the Unser Diversion. Drainage Area Maps 1 through 4 located in the back pockets of this report show the drainage area boundaries.



Smith Engineering Company
A Full Service Engineering Company

DWL ARCHITECTS & PLANNERS, INC.
OF NEW MEXICO
202 CENTRAL AVE. S.E.
WEST COURTYARD
ALBUQUERQUE, NEW MEXICO
87102
FAX (505) 242-4159
VOICE (505) 242-6202
E-MAIL DWLNM@MTS6.COM

WEST SIDE TRANSIT FACILITY

CITY OF ALBUQUERQUE
PUBLIC WORKS DEPARTMENT
HYDROLOGY DEVELOPMENT GROUP

VICINITY MAP

PROJECT NUMBER:	DRAWN BY:	DESIGNED BY:	DATE:	FIGURE NO.
100300	EAD	DLA	2/01	1

III. DRAINAGE CHARACTERISTICS

A. Existing Topography

The contributing drainage area north of Interstate 40 slopes from the northwest to the southeast with a surface gradient varying from 2.5 percent to 3.75 percent. Natural arroyos exist within this northern off-site basin, which convey storm water runoff to culverts under Interstate 40 and eventually to the Unser Diversion.

The contributing drainage area south of Interstate 40 also slopes from the northwest to the southeast with surface gradients varying from 2.5 percent to 6 percent. Natural arroyos as well as gradually sloping lands (causing sheet flow) exist within this southern on-site basin, which convey storm water runoff to the Unser Diversion detention pond facilities. The topography of both the off-site basins and the on-site basins are shown on Maps 1 through 4 located in the back pockets of this report.

B. Existing Vegetation

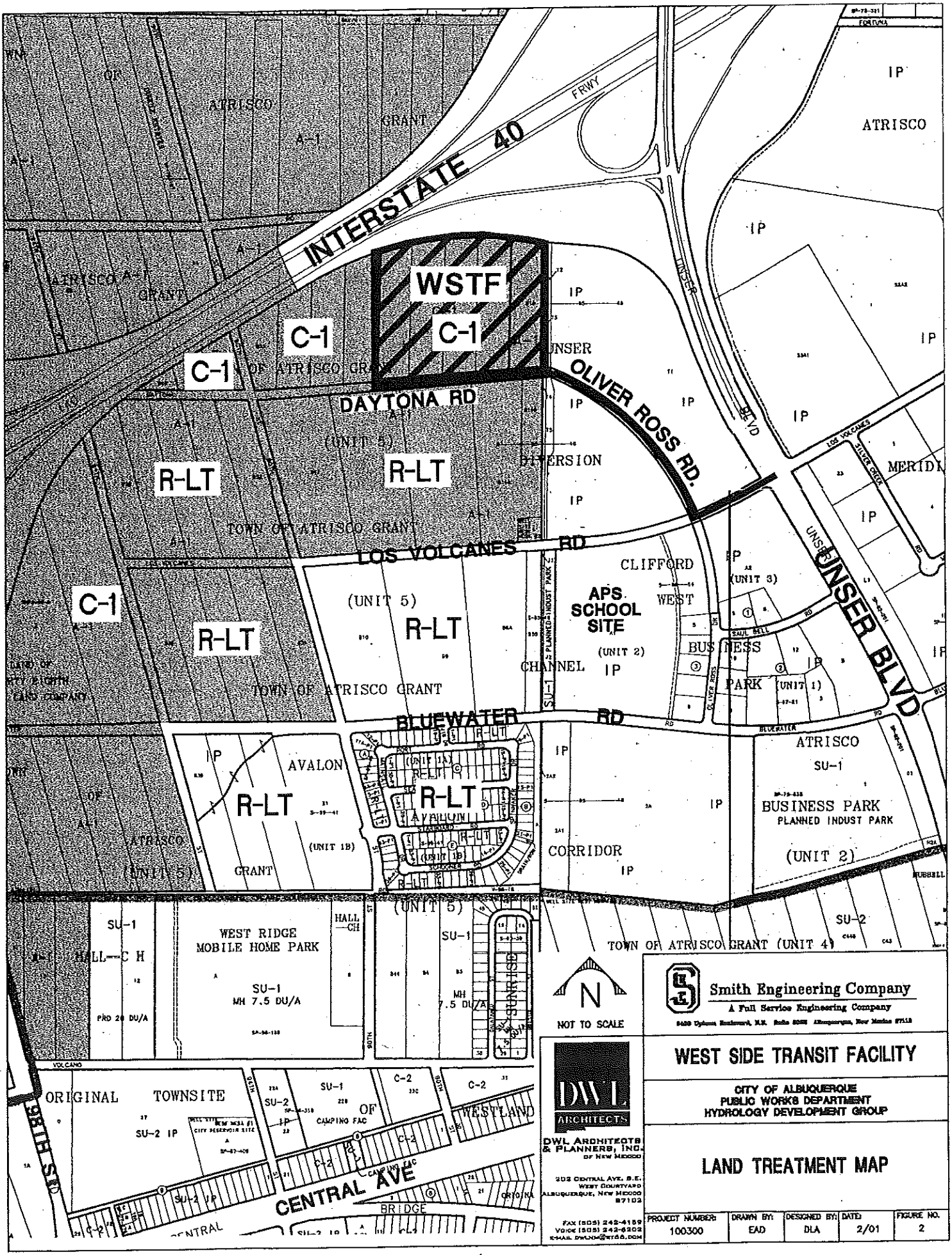
Existing vegetation is relatively sparse throughout the area and consists of open-type desert grassland. Generally, vegetative cover is estimated at less that 40 percent. Under developed conditions, it is anticipated that the area will be covered with hard impervious surfaces and various types of landscaping.

C. Land Use

Under existing conditions approximately 95 percent of the drainage basin is undeveloped. Some residential development has recently occurred in the southern portion of the area south of Interstate 40. Developed condition land uses were determined from City of Albuquerque Zone Atlas Maps. According to the Zone Atlas Maps, some of the land south of Interstate 40 is zoned A-1. It is difficult to predict future zoning for land currently designated A-1. After visiting the drainage basin several times, and reviewing the Bernalillo County mapping mentioned above, educated assumptions were made in determining future zoning of these parcels. Figure No. 2 shows zoning used for this drainage master plan.

IV. EXISTING DRAINAGE FACILITIES

Four existing culverts under Interstate 40 currently convey storm water runoff generated from the north side of Interstate 40 to the south. The culverts range in size from a single 30 inch CMP to triple 6'x4' CBCs. Once the runoff exits these culverts, natural channels convey the storm water to the Unser Diversion. The Unser Diversion is located west of Unser Blvd. and is comprised of six detention ponds (in series) connected by culvert type pipes. The storm water runoff from the Unser Diversion is then discharged east through a storm drain pipe and eventually discharges to the Unser Blvd. storm drain system. Maps 1 through 4 located in the back pocket of the report show the existing drainage facilities as they currently exist.



Smith Engineering Company
A Full Service Engineering Company

DWL ARCHITECTS
DWL ARCHITECTS & PLANNERS, INC. OF NEW MEXICO
202 CENTRAL AVE. S.E. WEST COURTYARD ALBUQUERQUE, NEW MEXICO 87102
FAX (505) 242-4159 VOICE (505) 242-6202 E-MAIL DWLNM@NTSS.COM

WEST SIDE TRANSIT FACILITY

CITY OF ALBUQUERQUE
PUBLIC WORKS DEPARTMENT
HYDROLOGY DEVELOPMENT GROUP

LAND TREATMENT MAP

PROJECT NUMBER:	DRAWN BY:	DESIGNED BY:	DATE:	FIGURE NO.
100300	EAD	DLA	2/01	2

V. FLOOD PLAINS

After reviewing the Flood Insurance Rate Map for Bernalillo County and Incorporated Areas (Panel 328 of 825 Map No. 35001C0328 D) dated September 20, 1996, flood plains do exist on properties in the drainage area. Figure 3 shows the existing flood plains within the drainage basin. Since the construction of the Unser Diversion in 1997, the Federal Emergency Management Agency (FEMA) has issued a Letter of Map Revision, removing downstream flood plains.

VI. HYDROLOGY

Frequency flows were quantified using the AHYMO computer program according to "Section 22.2 Hydrology of the Development Process Manual, Design Criteria for the City of Albuquerque, New Mexico". Mapping for this study utilized the orthophotography and vector contour composite images (part of the Bernalillo County Digital Mapping). The mapping was obtained from the Albuquerque Metropolitan Arroyo and Flood Control Authority (AMAFCA).

Rainfall amounts for the frequency events were derived from the NOAA Atlas show in "Section 22.2 Hydrology of the Development Process Manual", figures C-1, C-2 and C-3. Developed condition sub-basins were generally broken down into platted lands while topographic features were utilized to determine existing condition drainage sub-basins.

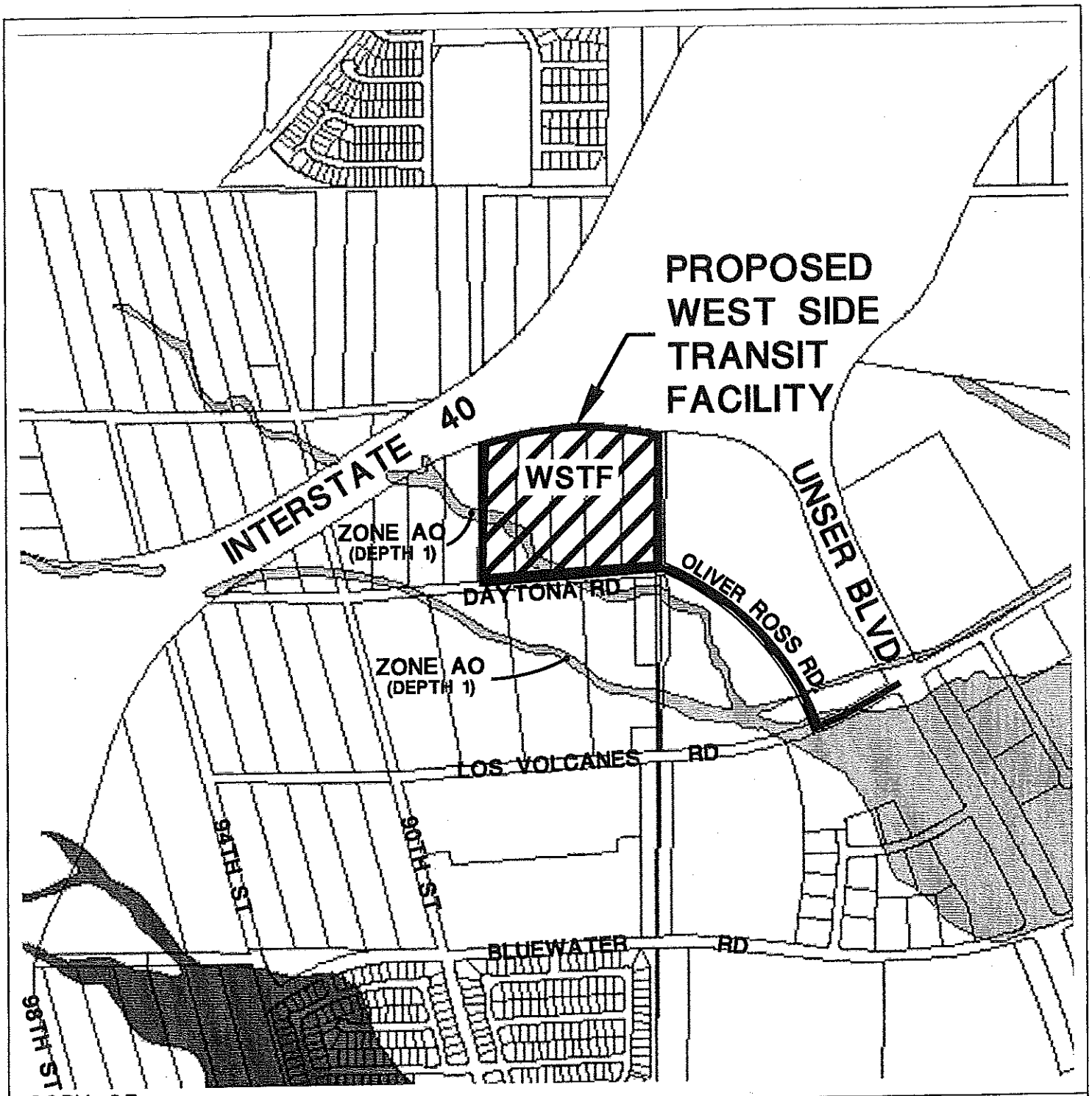
Existing condition land treatments were determined utilizing the Bernalillo County mapping mentioned above. Developed condition land treatments were determined by correlating the proposed zoning mentioned above to the appropriate land treatments as shown in "Section 22.2 Hydrology of the Development Process Manual", tables A-4 and A-5. Appendix "A" links the zoning used for developed conditions to the appropriate land treatments used in the AHYMO computer models.

Time of concentration calculations utilized the SCS Upland Method as described in "Section 22.2 Hydrology of the Development Process Manual". Appendix "B" contains AHYMO basin parameter worksheets together with peak flows and volumes for each sub-basin. AHYMO output summary tables are included in Appendix "C". Detailed AHYMO computer model input and output files are available for observation at the offices of Smith Engineering Company. 10 year and 100 year peak flows and volumes at designated analysis points are shown on Maps 1 through 4 located in the back pockets of this report.

VII. HYDRAULICS

The hydraulics of the proposed storm drain system are based on utilizing the following three basic system parts:

1. Streets – which act as open channels to convey storm water runoff the drop inlets.



COPY OF:

FIRM
FLOOD INSURANCE RATE MAP

BERNALILLO COUNTY,
NEW MEXICO AND INCORPORATED AREAS

PANEL 328 OF 825

MAP NUMBER 35001C0328 D

EFFECTIVE DATE:
SEPTEMBER 20, 1996

FEDERAL EMERGENCY MANAGEMENT AGENCY

N
NOT TO SCALE

Smith Engineering Company
A Full Service Engineering Company
5428 Uptown Boulevard, N.E. Suite 100B Albuquerque, New Mexico 87110

WEST SIDE TRANSIT FACILITY

CITY OF ALBUQUERQUE
PUBLIC WORKS DEPARTMENT
HYDROLOGY DEVELOPMENT GROUP

FLOOD PLAIN MAP

DWL ARCHITECTS
DWL ARCHITECTS & PLANNERS, INC.
OF NEW MEXICO
202 CENTRAL AVE. S.E.
WEST COURTYARD
ALBUQUERQUE, NEW MEXICO
87102
FAX (505) 242-4159
VOICE (505) 242-6202
E-MAIL DWLNM@MSN.COM

PROJECT NUMBER:	DRAWN BY:	DESIGNED BY:	DATE:	FIGURE NO.
100300	EAD	DLA	2/01	3

2. Underground piping – with convey storm water runoff from the drop inlets to the Unser Diversion.
3. Unser Diversion – which detains storm water runoff through six detention ponds and ultimately discharge it at a controlled rate to the Unser Blvd storm drain system.

Street capacities were determined using "Flow Master", a computer program that utilizes Manning's Equation to determine hydraulic capacities of the individual roadways. Hydraulic grade lines for storm drain pipes were calculated using "Storm Cad", a computer program used to perform hydraulic analyses on storm drain pipe systems. "Storm Cad" utilizes Manning's Equation for open channel flow as well as pressurized flow incorporating friction head loss and other minor head losses within the storm drain system. Culvert pipes were analyzed utilizing "Culvert Master", a computer program specializing in the design and analysis of culvert pipes. Hydraulic calculations relative to pond routing were performed using the AHYMO computer program.

VIII. ANALYSIS

This section of the report provides a brief description of the Unser Diversion facility followed by four separate analyses of the detention pond capacities for the following conditions:

- Existing Condition
- Existing Condition with the Transit Facility fully developed
- Developed Interim Condition
- Developed Condition

It should be mentioned that all of the analyses below assume free discharge from each basin. In other words, no basins were assumed to contain detention ponds for individual developments.

A. Unser Diversion

Easterling & Associates Inc. performed the design of the Unser Diversion. The construction of the project was complete in May of 1997. AMAFCA funded the construction of the project. AMAFCA transferred ownership of the Unser Diversion shortly thereafter to the City of Albuquerque who currently maintains the facility. Reinforced concrete outlet structures exist in each of the six detention ponds. Culvert pipe connecting the ponds in series range in size from 36 inch to 48 inch. Currently three eight-inch holes are cored through the reinforced concrete outlet structures to drain low flows from each pond. The volumes of the six detention ponds range in volumes from approximately 4.75 acre-feet to 11.50 acre-feet.

Existing pond stage/storage and discharge curves utilized in the AHYMO computer models are tabulated in Table 1. The existing pond storage volumes were taken from the "Master Drainage Plan for the Atrisco Business Park, October 1993, prepared by Easterling & Associates, Inc. Discharge rating curves for each pond were calculated

from the existing reinforced outlet structures and incorporating downstream tailwater elevations shown on the Unser Diversion as-built construction drawing plan set.

Drainage easements exist west of each of the Unser Diversion detention pond with the exception of Pond No. 3 and Pond No. 4. These two easements were vacated in the last 18 months as part of the development of a residential subdivision. The drainage easements exist for possible expansion of the ponds if additional volume is required to adequately handle future developed runoff volume if required. According to the "Master Drainage Plan for the Atrisco Business Park" the existing reinforced concrete outlet structures could also be modified to discharge fully developed flows by coring additional 10 inch holes in the remaining three sides of the square outlet structures.

According to the "Master Drainage Plan for the Atrisco Business Park", Pond No. 5 is not utilized in fully developed conditions. Since Pond No. 5 is considered only a temporary facility, existing outlet works along with existing pond grading are utilized in all of the analyses mentioned below relative to Pond No. 5. Discharge curves for the proposed outlet structures were calculated similar to that of the existing outlet structure mentioned above. Table 1 also shows the proposed condition stage/storage and discharge curves for the Unser Diversion used in the AHYMO computer models.

B. Existing Condition Analysis

Map No. 1 located in the back pocket of this report shows the contributing drainage basin used in the analysis of existing conditions. Four individual analyses were performed to determine freeboard in each of the six detention ponds.

Analysis 1
Existing pond outlet works were utilized for each pond in the AHYMO model.
Existing pond grading was utilized for each pond in the AHYMO model.

Analysis 2
Proposed pond outlet works were utilized for each pond in the AHYMO model.
Existing pond grading was utilized for each pond in the AHYMO model.

Analysis 3
Existing pond outlet works were utilized for each pond in the AHYMO model.
Proposed pond grading was utilized for each pond in the AHYMO model.

Analysis 4
Proposed pond outlet works were utilized for each pond in the AHYMO model.
Proposed pond grading was utilized for each pond in the AHYMO model.

Freeboard results for each of the analyses are shown in Table 2. A glance at Table 2 shows that by modifying the pond outlet works and increasing pond volumes provides additional freeboard. But for obvious reasons, using existing pond outlet works and existing pond grading for existing conditions provides adequate freeboard.

C. Existing Condition with the Transit Facility Fully Developed Analysis

Map No. 2 located in the back pocket of this report shows the contributing drainage basin used in the analysis of existing condition with the transit facility fully developed. This analysis is similar to that of existing conditions with the exception of the West Side Transit Facility is considered fully developed. Four individual analyses were performed to determine freeboard in each of the six detention ponds.

Analysis 1
Existing pond outlet works were utilized for each pond in the AHYMO model.
Existing pond grading was utilized for each pond in the AHYMO model.

Analysis 2
Proposed pond outlet works were utilized for each pond in the AHYMO model.
Existing pond grading was utilized for each pond in the AHYMO model.

Analysis 3
Existing pond outlet works were utilized for each pond in the AHYMO model.
Proposed pond grading was utilized for each pond in the AHYMO model.

Analysis 4
Proposed pond outlet works were utilized for each pond in the AHYMO model.
Proposed pond grading was utilized for each pond in the AHYMO model.

Freeboard results for each of the analyses are shown in Table 3. Table 3 shows that by modifying the pond volumes to the proposed condition while keeping the outlet works in existing condition provides maximum freeboard for the system as a whole. Keeping the existing outlet works along with the existing pond volumes provides adequate freeboard within all the ponds. Freeboard for Analysis 1 exceeds two feet in all detention ponds with the exception of Pond No. 6, which has 1.8 feet of freeboard.

D. Developed Interim Condition Analysis

Map No. 3 located in the back pocket of this report shows the contributing drainage basin used in the analysis of developed interim conditions. Developed interim condition assumes the land within the contributing drainage basin south of Interstate 40 is fully developed while the contributing basin north of Interstate 40 remains in existing condition. Four individual analyses were performed to determine freeboard in each of the six detention ponds.

Analysis 1
Existing pond outlet works were utilized for each pond in the AHYMO model.
Existing pond grading was utilized for each pond in the AHYMO model.

Analysis 2
Proposed pond outlet works were utilized for each pond in the AHYMO model.
Existing pond grading was utilized for each pond in the AHYMO model.

Analysis 3
Existing pond outlet works were utilized for each pond in the AHYMO model.
Proposed pond grading was utilized for each pond in the AHYMO model.

Analysis 4
Proposed pond outlet works were utilized for each pond in the AHYMO model.
Proposed pond grading was utilized for each pond in the AHYMO model.

Freeboard results for each of the analyses are shown in Table 4. Table 4 shows runoff in Pond No. 4 overtops the emergency spillway in all of the analyses with the exception of Analysis 4. Analysis 4 results show that Pond No. 4 contains the 100 year event with no freeboard (HWL = Emergency Spillway Elevation) while Pond No. 3 has a freeboard less than one foot. Even though Analysis 4 shows the system works, this is not a desirable option. Some of the properties south of Interstate 40 may be required to detain runoff until the I-40 Interceptor is constructed to divert the off-site flow north of Interstate 40 to the east.

E. Developed Condition Analysis

Map No. 4 located in the back pocket of this report shows the contributing drainage basin used in the analysis of fully developed condition. The developed condition analysis assumes the I-40 Interceptor has been constructed and will divert all off-site flows north of Interstate 40 to the east. The developed condition model assumes all land within the contributing drainage basin south of Interstate 40 is fully developed. Three individual analyses were performed to determine freeboard in each of the six detention ponds.

Analysis 1
Existing pond outlet works were utilized for each pond in the AHYMO model.
Existing pond grading was utilized for each pond in the AHYMO model.

Analysis 2
Proposed pond outlet works were utilized for each pond in the AHYMO model.
Existing pond grading was utilized for each pond in the AHYMO model.

Analysis 3
Proposed pond outlet works were utilized for ponds 6, 2 and 1 in the AHYMO model while ponds 3 and 4 utilize existing outlet works.
Existing pond grading was utilized for each pond in the AHYMO model.

Freeboard results for each of the analyses are shown in Table 5. Analysis 1 provides freeboard in excess of two feet in all of the ponds with the exception of Pond No. 6 and Pond No. 2. Pond No. 6 has a freeboard of 1.2 feet while Pond No. 2 has a freeboard of 1.9 feet. Analysis 2 modifies all detention pond outlet works to proposed condition and produces favorable results. Freeboards in all the ponds exceed two feet in Analysis 2. Analysis 3 looked at modifying the outlet works in Pond No. 6, Pond No. 2 and Pond No. 1. Analysis 3 also produced favorable results relative to freeboard.

TABLE 1

UNSER DIVERSION DETENTION PONDS
STAGE/STORAGE AND DISCHARGE TABLES

Unser Diversion Pond No. 6

Elevation (Ft.)	Existing Storage (Ac.Ft.)	Existing Outflow (cfs)	Possible Future Storage (Ac.Ft.)	Possible Future Outflow (cfs)
70.0	0.00	0.0	0.00	0.0
71.0	0.14	4.0	0.30	4.4
72.0	0.83	6.2	1.16	16.0
73.0	1.77	7.9	2.33	32.0
74.0	2.79	9.2	3.59	45.0
75.0	3.87	10.4	4.93	51.0
76.0	5.04	11.5	6.34	56.0
77.0	6.27	79.5	7.85	79.5
77.9	7.46	86.1	9.43	86.1
79.0	9.01	93.2	11.10	93.2

Unser Diversion Pond No. 5*

Elevation (Ft.)	Existing Storage (Ac.Ft.)	Existing Outflow (cfs)	Possible Future Storage* (Ac.Ft.)	Possible Future Outflow* (cfs)
60.0	0.00	0.0	0.00	0.0
61.0	0.02	4.0	0.02	4.0
62.0	0.37	6.5	0.37	6.5
63.0	0.76	8.2	0.76	8.2
64.0	1.19	9.7	1.19	9.7
64.5	1.42	41.6	1.42	41.6
65.0	1.66	93.0	1.66	93.0
65.5	1.89	106.3	1.89	106.3
66.0	2.12	116.7	2.12	116.7
67.0	2.74	132.6	2.74	132.6
68.0	3.35	146.8	3.35	146.8
68.8	3.87	157.2	3.87	157.2
69.0	4.01	160.0	4.01	160.0
70.0	4.73	172.0	4.73	172.0

* Pond No. 5 is not utilized in fully developed conditions. For this reason, the existing pond outlet works and the existing pond volume were utilized in all AHYMO computer models.

Unser Diversion Pond No. 4

Elevation (Ft.)	Existing Storage (Ac.Ft.)	Existing Outflow (cfs)	Possible Future Storage* (Ac.Ft.)	Possible Future Outflow (cfs)
47.0	0.00	0.0	0.00	0.0
48.0	0.08	0.0	0.08	0.0
49.0	0.51	3.0	0.51	17.8
50.0	1.30	5.7	1.30	36.5
51.0	2.29	7.5	2.29	49.2
51.9	3.26	8.7	3.26	57.7
52.0	3.37	11.7	3.37	61.4
52.5	3.94	50.6	3.94	86.0
53.0	4.52	92.5	4.52	92.5
54.0	5.76	104.3	5.76	104.3
55.1	7.18	115.9	7.18	115.9
56.0	8.51	124.6	8.51	124.6

* Pond No. 4 can not be expanded due to right-of-way constraints

Unser Diversion Pond No. 3

Elevation (Ft.)	Existing Storage (Ac.Ft.)	Existing Outflow (cfs)	Possible Future Storage* (Ac.Ft.)	Possible Future Outflow (cfs)
43.0	0.00	0.0	0.00	0.0
44.0	0.07	0.0	0.07	0.0
45.0	0.50	1.5	0.50	7.5
46.0	1.04	5.1	1.04	24.7
47.0	1.64	7.0	1.64	34.2
47.5	1.97	7.8	1.97	38.0
48.0	2.30	39.8	2.30	72.8
48.5	2.65	85.9	2.65	85.9
49.0	3.01	92.4	3.01	92.4
50.0	3.78	104.3	3.78	104.3
51.0	4.61	114.4	4.61	114.4
51.2	4.79	116.2	4.79	116.2
52.2	5.51	123.4	5.51	123.4

* Pond No. 4 can not be expanded due to right-of-way constraints

Unser Diversion Pond No. 2

Elevation (Ft.)	Existing Storage (Ac.Ft.)	Existing Outflow (cfs)	Possible Future Storage (Ac.Ft.)	Possible Future Outflow (cfs)
37.9	0.00	0.0	0.00	0.0
38.0	0.00	0.0	0.02	0.0
39.0	0.18	0.0	0.27	0.0
40.0	0.73	0.0	1.05	0.0
41.0	1.57	3.4	2.19	16.7
42.0	2.50	5.9	3.49	28.9
43.0	3.56	7.6	4.87	37.3
44.0	4.68	9.0	6.34	44.1
44.8	5.88	10.0	7.58	48.9
45.5	6.52	18.1	8.71	58.0
46.0	7.16	127.2	9.53	142.2
47.0	8.52	154.6	11.25	154.6
47.9	9.83	165.0	12.87	165.0
48.9	11.51	176.8	14.94	176.8

Unser Diversion Pond No. 1

Elevation (Ft.)	Existing Storage (Ac.Ft.)	Existing Outflow (cfs)	Possible Future Storage (Ac.Ft.)	Possible Future Outflow (cfs)
33.5	0.00	0.0	0.00	0.0
34.0	0.00	0.0	0.05	0.0
35.0	0.29	4.0	0.54	10.4
36.0	0.86	6.3	1.34	26.5
37.0	1.54	7.9	2.22	47.1
38.0	2.27	9.3	3.18	61.1
39.0	3.07	10.4	4.22	68.9
40.0	3.94	11.5	5.34	75.8
41.0	4.87	12.5	6.54	82.2
42.0	5.87	101.5	7.83	111.6
43.0	6.94	118.6	9.19	118.6
43.9	7.96	124.6	10.49	124.6
45.0	9.29	131.5	12.16	131.5

TABLE 2

UNSER DIVERSION DETENTION POND RESULTS

EXISTING CONDITIONS

ANALYSIS 1
Pond Outlet Works: Existing
Pond Grading: Existing
100yr. AHYMO Input File: EXT100EE.IN
10yr. AHYMO Input File: EXT10EE.IN

UNSER DIVERSION POND NO.	10 YR. HWL	100 YR. HWL	EMERGENCY SPILLWAY ELEVATION	10 YR. FREEBOARD (FT.)	100 YR. FREEBOARD (FT.)
6	71.4	74.5	77.9	6.5	3.4
5	64.2	66.9	68.8	4.6	1.9
4	50.7	53.1	55.1	4.4	2.0
3	46.3	48.7	51.2	4.9	2.5
2	41.3	45.4	47.9	6.6	2.5
1	35.3	40.5	43.9	8.6	3.4

ANALYSIS 2
Pond Outlet Works: Proposed
Pond Grading: Existing
100yr. AHYMO Input File: EXT100PE.IN
10yr. AHYMO Input File: EXT10PE.IN

UNSER DIVERSION POND NO.	10 YR. HWL	100 YR. HWL	EMERGENCY SPILLWAY ELEVATION	10 YR. FREEBOARD (FT.)	100 YR. FREEBOARD (FT.)
6	71.3	73.5	77.9	6.6	4.4
5	64.2	66.9	68.8	4.6	1.9
4	49.2	53.2	55.1	5.9	1.9
3	45.7	48.8	51.2	5.5	2.4
2	40.8	44.9	47.9	7.1	3.0
1	35.2	37.3	43.9	8.7	6.6

ANALYSIS 3
Pond Outlet Works: Existing
Pond Grading: Proposed
100yr. AHYMO Input File: EXT100EP.IN
10yr. AHYMO Input File: EXT10EP.IN

UNSER DIVERSION POND NO.	10 YR. HWL	100 YR. HWL	EMERGENCY SPILLWAY ELEVATION	10 YR. FREEBOARD (FT.)	100 YR. FREEBOARD (FT.)
6	71.2	73.8	77.9	6.7	4.1
5	64.2	66.9	68.8	4.6	1.9
4	50.7	53.1	55.1	4.4	2.0
3	46.3	48.7	51.2	4.9	2.5
2	41.0	44.8	47.9	6.9	3.1
1	34.9	38.3	43.9	9.0	5.6

Notes: Pond 5 was not expanded due to the fact that it is not required in developed conditions.
Ponds 3 and 4 can not be expanded due to Right-of-Way constraints.

ANALYSIS 4
Pond Outlet Works: Proposed
Pond Grading: Proposed
100yr. AHYMO Input File: EXT100PP.IN
10yr. AHYMO Input File: EXT10PP.IN

UNSER DIVERSION POND NO.	10 YR. HWL	100 YR. HWL	EMERGENCY SPILLWAY ELEVATION	10 YR. FREEBOARD (FT.)	100 YR. FREEBOARD (FT.)
6	71.1	73.1	77.9	6.8	4.8
5*	64.2	66.9	68.8	4.6	1.9
4*	49.1	53.1	55.1	6.0	2.0
3	45.6	48.7	51.2	5.6	2.5
2	40.7	44.1	47.9	7.2	3.8
1	35.0	36.9	43.9	8.9	7.0

Notes: Pond 5 was not expanded due to the fact that it is not required in developed conditions.
Ponds 3 and 4 can not be expanded due to Right-of-Way constraints.

TABLE 3

UNSER DIVERSION DETENTION POND RESULTS

EXISTING CONDITIONS WITH THE WEST SIDE TRANSIT FACILITY FULLY DEVELOPED

ANALYSIS 1
Pond Outlet Works: Existing
Pond Grading: Existing
100yr. AHYMO Input File: WTF100EE.IN
10yr. AHYMO Input File: WTF10EE.IN

UNSER DIVERSION POND NO.	10 YR. HWL	100 YR. HWL	EMERGENCY SPILLWAY ELEVATION	10 YR. FREEBOARD (FT.)	100 YR. FREEBOARD (FT.)
6	73.1	76.1	77.9	4.8	1.8
5	64.2	66.8	68.8	4.6	2.0
4	51.6	53.1	55.1	3.5	2.0
3	46.9	48.8	51.2	4.3	2.4
2	41.8	45.5	47.9	6.1	2.4
1	35.5	41.0	43.9	8.4	2.9

ANALYSIS 2
Pond Outlet Works: Proposed
Pond Grading: Existing
100yr. AHYMO Input File: WTF100PE.IN
10yr. AHYMO Input File: WTF10PE.IN

UNSER DIVERSION POND NO.	10 YR. HWL	100 YR. HWL	EMERGENCY SPILLWAY ELEVATION	10 YR. FREEBOARD (FT.)	100 YR. FREEBOARD (FT.)
6	72.4	74.6	77.9	5.5	3.3
5	64.2	66.8	68.8	4.6	2.0
4	49.8	53.6	55.1	5.3	1.5
3	46.3	49.1	51.2	4.9	2.1
2	41.4	45.6	47.9	6.5	2.3
1	35.6	37.8	43.9	8.3	6.1

ANALYSIS 3
Pond Outlet Works: Existing
Pond Grading: Proposed
100yr. AHYMO Input File: WTF100EP.IN
10yr. AHYMO Input File: WTF10EP.IN

UNSER DIVERSION POND NO.	10 YR. HWL	100 YR. HWL	EMERGENCY SPILLWAY ELEVATION	10 YR. FREEBOARD (FT.)	100 YR. FREEBOARD (FT.)
6	72.7	75.3	77.9	5.2	2.6
5	64.2	66.8	68.8	4.6	2.0
4	51.5	53.1	55.1	3.6	2.0
3	46.9	48.7	51.2	4.3	2.5
2	41.5	45.0	47.9	6.4	2.9
1	35.2	38.7	43.9	8.7	5.2

Notes: Pond 5 was not expanded due to the fact that it is not required in developed conditions.
Ponds 3 and 4 can not be expanded due to Right-of-Way constraints.

ANALYSIS 4
Pond Outlet Works: Proposed
Pond Grading: Proposed
100yr. AHYMO Input File: WTF100PP.IN
10yr. AHYMO Input File: WTF10PP.IN

UNSER DIVERSION POND NO.	10 YR. HWL	100 YR. HWL	EMERGENCY SPILLWAY ELEVATION	10 YR. FREEBOARD (FT.)	100 YR. FREEBOARD (FT.)
6	72.2	74.1	77.9	5.7	3.8
5	64.2	66.8	68.8	4.6	2.0
4	49.6	53.5	55.1	5.5	1.6
3	46.2	49.0	51.2	5.0	2.2
2	41.1	44.7	47.9	6.8	3.2
1	35.4	37.1	43.9	8.5	6.8

Notes: Pond 5 was not expanded due to the fact that it is not required in developed conditions.
Ponds 3 and 4 can not be expanded due to Right-of-Way constraints.

TABLE 4

UNSER DIVERSION DETENTION POND RESULTS

DEVELOPED INTERIM CONDITIONS

ANALYSIS 1
Pond Outlet Works: **Existing**
Pond Grading: **Existing**
100yr. AHYMO Input File: **INT100EE.IN**
10yr. AHYMO Input File: **INT10EE.IN**

UNSER DIVERSION POND NO.	10 YR. HWL	100 YR. HWL	EMERGENCY SPILLWAY ELEVATION	10 YR. FREEBOARD (FT.)	100 YR. FREEBOARD (FT.)
6	75.4	77.4	77.9	2.5	0.5
5	64.1	65.8	68.8	4.7	3.0
4*	52.2	55.8	55.1	2.9	-0.7
3	47.8	50.8	51.2	3.4	0.4
2	45.2	46.2	47.9	2.7	1.7
1	38.8	42.7	43.9	5.1	1.2

* 100 Year Storm Event HWL Exceeds Emergency Spillway Elevation

ANALYSIS 2
Pond Outlet Works: **Proposed**
Pond Grading: **Existing**
100yr. AHYMO Input File: **INT100PE.IN**
10yr. AHYMO Input File: **INT10PE.IN**

UNSER DIVERSION POND NO.	10 YR. HWL	100 YR. HWL	EMERGENCY SPILLWAY ELEVATION	10 YR. FREEBOARD (FT.)	100 YR. FREEBOARD (FT.)
6	73.9	76.9	77.9	4.0	1.0
5	64.1	65.8	68.8	4.7	3.0
4*	51.6	55.5	55.1	3.5	-0.4
3	47.8	50.7	51.2	3.4	0.5
2	44.6	46.0	47.9	3.3	1.9
1	37.0	42.0	43.9	6.9	1.9

* 100 Year Storm Event HWL Exceeds Emergency Spillway Elevation

ANALYSIS 3
Pond Outlet Works: **Existing**
Pond Grading: **Proposed**
100yr. AHYMO Input File: **INT100EP.IN**
10yr. AHYMO Input File: **INT10EP.IN**

UNSER DIVERSION POND NO.	10 YR. HWL	100 YR. HWL	EMERGENCY SPILLWAY ELEVATION	10 YR. FREEBOARD (FT.)	100 YR. FREEBOARD (FT.)
6	74.6	76.9	77.9	3.3	1.0
5	64.1	65.8	68.8	4.7	3.0
4*	52.1	55.2	55.1	3.0	-0.1
3	47.7	50.4	51.2	3.5	0.8
2	44.4	46.0	47.9	3.5	1.9
1	37.8	42.1	43.9	6.1	1.8

Notes: There is no need to expand Pond 5.
Ponds 3 and 4 can not be expanded due to Right-of-Way constraints.
* 100 Year Storm Event HWL Exceeds Emergency Spillway Elevation

ANALYSIS 4
Pond Outlet Works: **Proposed**
Pond Grading: **Proposed**
100yr. AHYMO Input File: **INT100PP.IN**
10yr. AHYMO Input File: **INT10PP.IN**

UNSER DIVERSION POND NO.	10 YR. HWL	100 YR. HWL	EMERGENCY SPILLWAY ELEVATION	10 YR. FREEBOARD (FT.)	100 YR. FREEBOARD (FT.)
6	73.5	76.3	77.9	4.4	1.6
5	64.1	65.8	68.8	4.7	3.0
4*	51.3	55.1	55.1	3.8	0.0
3	47.8	50.5	51.2	3.4	0.7
2	43.9	45.8	47.9	4.0	2.1
1	36.8	41.4	43.9	7.1	2.5

Notes: There is no need to expand Pond 5.
Ponds 3 and 4 can not be expanded due to Right-of-Way constraints.
* 100 Year Storm Event HWL Equals Emergency Spillway Elevation

TABLE 5

UNSER DIVERSION DETENTION POND RESULTS

DEVELOPED CONDITIONS

ANALYSIS 1

Pond Outlet Works: **Existing**
Pond Grading: **Existing**
100yr. AHYMO Input File: **DEV100EE.IN**
10yr. AHYMO Input File: **DEV10EE.IN**

UNSER DIVERSION POND NO.	10 YR. HWL	100 YR. HWL	EMERGENCY SPILLWAY ELEVATION	10 YR. FREEBOARD (FT.)	100 YR. FREEBOARD (FT.)
6	75.3	76.7	77.9	2.6	1.2
5*	N/A	N/A	N/A	N/A	N/A
4	51.9	52.9	55.1	3.2	2.2
3	47.5	48.7	51.2	3.7	2.5
2	44.1	46.0	47.9	3.8	1.9
1	37.9	41.9	43.9	6.0	2.0

* Unser Diversion Pond No. 5 is not utilized in Developed Conditions.

ANALYSIS 2

Pond Outlet Works: **Proposed**
Pond Grading: **Existing**
100yr. AHYMO Input File: **DEV100PE.IN**
10yr. AHYMO Input File: **DEV10PE.IN**

UNSER DIVERSION POND NO.	10 YR. HWL	100 YR. HWL	EMERGENCY SPILLWAY ELEVATION	10 YR. FREEBOARD (FT.)	100 YR. FREEBOARD (FT.)
6	73.9	75.9	77.9	4.0	2.0
5*	N/A	N/A	N/A	N/A	N/A
4	51.1	52.4	55.1	4.0	2.7
3	47.8	48.9	51.2	3.4	2.3
2	44.3	45.9	47.9	3.6	2.0
1	37.0	41.0	43.9	6.9	2.9

* Unser Diversion Pond No. 5 is not utilized in Developed Conditions.

ANALYSIS 3

Pond Outlet Works: **Existing with the exception of Pond 6, Pond 2 and Pond 1 (Pond 6, Pond 2 and Pond 1 use proposed outlet works)**
Pond Grading: **Existing**
100yr. AHYMO Input File: **DEVP621.IN**
10yr. AHYMO Input File: **No 10 year event modeled**

UNSER DIVERSION POND NO.	10 YR. HWL	100 YR. HWL	EMERGENCY SPILLWAY ELEVATION	10 YR. FREEBOARD (FT.)	100 YR. FREEBOARD (FT.)
6		75.9	77.9		2.0
5*	N/A	N/A	N/A	N/A	N/A
4		53.0	55.1		2.1
3		49.1	51.2		2.1
2		45.9	47.9		2.0
1		40.5	43.9		3.4

* Unser Diversion Pond No. 5 is not utilized in Developed Conditions.

TABLE 6

UNSER DIVERSION POND COMPARISON

100 YEAR - 24 HOUR STORM EVENT

UNSER DIVERSION POND NO.	EXISTING CONDITIONS Existing Pond Volume Existing Pond Outlet Works FREEBOARD (FT.)	EXISTING CONDITIONS WITH WEST SIDE TRANSIT FACILITY FULLY DEVELOPED Existing Pond Volume Existing Pond Outlet Works FREEBOARD (FT.)	DEVELOPED INTERIM CONDITIONS Existing Pond Volume Existing Pond Outlet Works FREEBOARD (FT.)	DEVELOPED INTERIM CONDITIONS Proposed Pond Volume Proposed Pond Outlet Works FREEBOARD (FT.)	FULLY DEVELOPED CONDITIONS Existing Pond Volume Existing Pond Outlet Works FREEBOARD (FT.)
6	3.4	1.8	0.5	1.6	1.2
5	1.9	2.0	3.0	3.0	N/A*
4	2.0	2.0	-0.7	0.0	2.2
3	2.5	2.4	0.4	0.7	2.5
2	2.5	2.4	1.7	2.1	1.9
1	3.4	2.9	1.2	2.5	2.0

* Unser Diversion Pond No. 5 is not utilized in Developed Conditions.

IX. PROPOSED STORM DRAIN

During the initial meetings between SEC and the City of Albuquerque Hydrology Development Section, the city requested that SEC provide a preliminary storm drain design for Daytona Road from the Unser Diversion west to Interstate 40. The storm drain analysis assumes fully developed conditions with the I-40 Interceptor in place with the exception of off-site flows contributing to Daytona Road through the Mirehaven Arroyo "B". Hydraulic analyses relative to Daytona Rd. are shown in Appendices D, E and F. Map No. 5 located in the back pocket of the report shows future storm drain pipe required for Daytona Rd.

SEC also analyzed Oliver Ross Road and Los Volcanes Road to determine storm drain requirements. Appendix G provides the analysis for the Oliver Ross and Los Volcanes roadways. Map No. 5 also shows proposed improvements required for Oliver Ross Road and Los Volcanes Road.

X. CONCLUSIONS

The Existing Condition Analysis shows that all of the Unser Diversion detention ponds (using existing outlet works and existing pond volumes) have calculated freeboard in excess of two feet with the exception of the temporary Pond No. 5. Pond No. 5 has a calculated freeboard of 1.9 ft.

The Existing Condition Analysis with the Transit Facility Fully Developed (using existing outlet works and existing pond volumes) also produced freeboard in excess of two feet with the exception of Pond No. 6. Pond No. 6 has a calculated freeboard = 1.8 ft.

The Fully Developed Conditions Analysis (using existing outlet works and existing pond volumes and assuming the I-40 interceptor is in place) produced free board is in excess of two feet in all ponds with the exception of Pond No. 6 and Pond No. 2. Pond No. 6 has a calculated freeboard = 1.2 ft. while Pond No. 2 has a freeboard = 1.9 ft. All of these scenarios mentioned thus far work well when utilizing existing pond volumes and pond outlet works.

The Developed Interim Condition Analysis produced different results. Due to the off-site storm water runoff generated from the off-site basin north of Interstate 40 along with the developed flows generated south of Interstate 40, Pond No. 4 overtops the emergency spillway elevation in all cases with the exception of Analysis 4. Analysis 4 utilizes proposed pond volumes along with proposed outlet works, and even then Pond No. 4 has no freeboard (the high water level (HWL) in Pond No. 4 is the same elevation as the emergency spillway elevation). It should also be noted that in Analysis 4, Pond No. 3 has a calculated freeboard of 0.7 feet. All other detention ponds in Analysis 4 have a calculated freeboard in excess of 1.6 ft.

In summary, the additional runoff generated from development of the West Side Transit Facility will have no negative effects on the existing Unser Diversion. Adequate freeboard will still exist within the Unser Diversion by allowing free discharge from the fully developed Transit Facility site.

With the assumption that the I-40 Interceptor will be constructed within the next five to ten years and development of the basin south of Interstate 40 remains at its current rate, the Unser Diversion should operate without any signs of overtopping. If the development south of Interstate 40 begins to accelerate without the construction of the I-40 Interceptor, the City of Albuquerque should look at modifying pond volumes and pond outlet works for the detention ponds within the Unser Diversion or require individual properties to detain storm water runoff generated from their properties. A summary the Unser Diversion detention ponds mentioned in the section of the report is located in Table 6.

XI. REFERENCES

Anderson-Hydro, August 1997, "AHYMO Computer Program User's Manual"

City of Albuquerque, July 1997, "Development Process Manual, Section 22.2, Hydrology".

Easterling & Associates Inc., October 1993, "Master Drainage Plan for the Atrisco Business Park".

Easterling & Associates Inc., September 1993, "Draft Design Analysis Report for the Albuquerque Metropolitan Arroyo Flood Control Authority Unser Diversion".

Easterling & Associates, May 1997, "Unser Diversion As-Built Construction Drawing Plan Set".

Haestad Methods Inc., August 2000, "FlowMaster PE Version 6.0 Computer Software".

Haestad Methods Inc., November 2000, "CulvertMaster Computer Software".

Haestad Methods Inc., December 1999, "StormCad Computer Software".

URS Griener, Inc., March 1998, "Final Design Report Amole Del Norte Storm Diversion Facilities Tierra Bayita Drainage Facilities Phase III".

APPENDIX A

LAND TREATMENT SUMMARY

MASTER DRAINAGE PLAN
WEST SIDE TRANSIT FACILITY

LAND TREATMENT SUMMARY

ZONE DESIGNATION	LAND TREATMENTS			
	"A"	"B"	"C"	"D"
RESIDENTIAL R-LT		43		57
COMMERCIAL C-1		10		90
INDUSTRIAL IP		20		80
COA RIGHT OF WAYS OLIVER ROSS DR. LOS VOLCANES RD.		18 22		82 78
NMSHTD RIGHT OF WAY I40-1, I40-2, I40-3, I40-4	65			35

APPENDIX B

AHYMO BASIN PARAMETER WORKSHEETS, PEAK BASIN FLOWS, AND VOLUMES

MASTER DRAINAGE PLAN
WEST SIDE TRANSIT FACILITY

AHYMO BASIN PARAMETER WORKSHEET
PEAK BASIN FLOWS AND VOLUMES

EXISTING CONDITIONS

BASIN	AREA (sq.mi.)	LENGTH (ft.)	ELEV. DIFF. (ft.)	SLOPE (%)	K	VEL (fps)	T(c) (hr.)	T(p) (hr.)	LAND TREATMENT (%) A	B	C	D	10 YEAR PEAK FLOW (24hr.) (cfs)	10 YEAR RUNOFF VOLUME (24hr.) (ac.ft.)	100 YEAR PEAK FLOW (24hr.) (cfs)	100 YEAR RUNOFF VOLUME (24hr.) (ac.ft.)
O-1E	0.0242	400	13.0	3.25	0.7	1.26	0.09	0.06								
		1350	40.0	2.96	2.0	3.44	0.11	0.07								
		400	9.0	2.25	3.0	4.50	0.02	0.02								
	TOTAL =						0.22	0.15	100				3	0.1	18	0.6
O-2E	0.0422	400	12.0	3.00	0.7	1.21	0.09	0.06								
		1450	50.0	3.45	2.0	3.71	0.11	0.07								
		1600	40.0	2.50	3.0	4.74	0.09	0.06								
	TOTAL =						0.29	0.20	100				4	0.2	24	1.0
O-3E	0.0164	400	14.0	3.50	0.7	1.31	0.08	0.06								
		780	20.0	2.56	2.0	3.20	0.07	0.05								
		900	20.0	2.22	3.0	4.47	0.06	0.04								
	TOTAL =						0.21	0.14	99			1	3	0.1	13	0.4
O-4E	0.0094	400	10.0	2.50	0.7	1.11	0.10	0.07								
		900	22.0	2.44	2.0	3.13	0.08	0.05								
		150	2.0	1.33	3.0	3.46	0.01	0.01								
	TOTAL =						0.19	0.13	98			2	2	0.1	8	0.2
O-5E	0.1097	400	16.0	4.00	0.7	1.40	0.08	0.05								
		1600	51.0	3.19	2.0	3.57	0.12	0.08								
		2100	52.0	2.48	3.0	4.72	0.12	0.08								
	TOTAL =						0.33	0.22	98			2	14	0.7	63	2.8
O-6E	0.0732	400	12.0	3.00	0.7	1.21	0.09	0.06								
		1600	57.0	3.56	2.0	3.77	0.12	0.08								
		650	21.0	3.23	3.0	5.39	0.03	0.02								
		1170	9.0	0.77	3.0	2.63	0.12	0.08								
	TOTAL =						0.37	0.24	100				6	0.3	36	1.7
O-7E	0.0523	400	19.0	4.75	0.7	1.53	0.07	0.05								
		150	12.0	8.00	2.0	5.66	0.01	0.00								
		2950	102.0	3.46	3.0	5.58	0.15	0.10								
	TOTAL =						0.23	0.15	100				7	0.2	39	1.2
O-8E	0.0099	1320	4.0	0.30	3.0	1.65	0.22	0.15								
							0.22	0.15	15			85	15	0.7	23	1.1
O-9E	0.0348	400	18.0	4.50	0.7	1.48	0.07	0.05								
		200	17.0	8.50	2.0	5.83	0.01	0.01								
		1000	43.0	4.30	3.0	6.22	0.04	0.03								
	TOTAL =						0.13	0.09	95			5	8	0.3	32	1.0
I40-1	0.0075	1765	33.0	1.87	3.0	4.10	0.12	0.08								
	TOTAL =						0.12	0.08	65			35	6	0.2	11	0.5
I40-2	0.0031	760	11.0	1.45	3.0	3.61	0.06	0.04								
	TOTAL =						0.06	0.04	65			35	2	0.1	5	0.2

MASTER DRAINAGE PLAN
WEST SIDE TRANSIT FACILITY

AHYMO BASIN PARAMETER WORKSHEET
PEAK BASIN FLOWS AND VOLUMES

EXISTING CONDITIONS

BASIN	AREA (sq.mi.)	LENGTH (ft.)	ELEV. DIFF. (ft.)	SLOPE (%)	K	VEL (fps)	T(c) (hr.)	T(p) (hr.)	LAND TREATMENT (%) A	B	C	D	10 YEAR PEAK FLOW (24hr.) (cfs)	10 YEAR RUNOFF VOLUME (24hr.) (ac.ft.)	100 YEAR PEAK FLOW (24hr.) (cfs)	100 YEAR RUNOFF VOLUME (24hr.) (ac.ft.)
I40-3	0.0019 TOTAL =	415	7.0	1.69	3.0	3.90	0.03 0.03	0.02 0.02	65			35	1	0.1	3	0.1
I40-4	0.0012 TOTAL =	285	4.0	1.40	3.0	3.55	0.02 0.02	0.01 0.01	65			35	1	0.04	2	0.1
120.5 *	0.0162	-	-	-	-	-	-	0.13	100				3	0.1	13	0.4
120.6 *	0.0328	-	-	-	-	-	-	0.13	98			2	6	0.2	28	0.8
130.7 *	0.0170	-	-	-	-	-	-	0.13	86	12	2		3	0.1	15	0.4
130.8 *	0.0327	-	-	-	-	-	-	0.14	100				5	0.1	25	0.8
130.9 *	0.0095	-	-	-	-	-	-	0.13	86	12	2		2	0.1	8	0.2
140.1 *	0.0196	-	-	-	-	-	-	0.13	74	22	4		4	0.1	19	0.5
140.2 *	0.0892	-	-	-	-	-	-	0.13	92	4	4		16	0.5	79	2.2
150.1 *	0.1525	-	-	-	-	-	-	0.23	96		4		17	0.8	90	3.9
170.1	0.0642 TOTAL =	400 1600	9.0 27.0	2.25 1.69	0.7 2.0	1.05 2.60	0.11 0.17 0.28	0.07 0.11 0.18	98			2	9	0.4	43	1.6
P-1	0.0029	N/A	N/A	N/A	N/A	N/A	0.20	0.13	98			2	1	0.02	3	0.1
P-2	0.0036	N/A	N/A	N/A	N/A	N/A	0.20	0.13	98			2	1	0.02	3	0.1
P-3	0.0027	N/A	N/A	N/A	N/A	N/A	0.20	0.13	98			2	1	0.02	2	0.1
P-4	0.0036	N/A	N/A	N/A	N/A	N/A	0.20	0.13	98			2	1	0.02	3	0.1
P-5	0.0029	N/A	N/A	N/A	N/A	N/A	0.20	0.13	98			2	1	0.02	3	0.1
P-6	0.0043	N/A	N/A	N/A	N/A	N/A	0.20	0.13	98			2	1	0.03	4	0.1

* Hydrology paramaters taken from "Master Drainage Plan for the Atrisco Business Park", Revised Octover 1993, Easterling & Associates, Inc.

MASTER DRAINAGE PLAN
WEST SIDE TRANSIT FACILITY

AHYMO BASIN PARAMETER WORKSHEET
PEAK BASIN FLOWS AND VOLUMES

EXISTING CONDITIONS WITH THE WEST SIDE TRANSIT FACILITY FULLY DEVELOPED

BASIN	AREA (sq.mi.)	LENGTH (ft.)	ELEV. DIFF. (ft.)	SLOPE (%)	K	VEL (fps)	T(c) (hr.)	T(p) (hr.)	LAND TREATMENT (%) A	B	C	D	10 YEAR PEAK FLOW (24hr.) (cfs)	10 YEAR RUNOFF VOLUME (24hr.) (ac.ft.)	100 YEAR PEAK FLOW (24hr.) (cfs)	100 YEAR RUNOFF VOLUME (24hr.) (ac.ft.)
O-1E	0.0242	400	13.0	3.25	0.7	1.26	0.09	0.06								
		1350	40.0	2.96	2.0	3.44	0.11	0.07								
		400	9.0	2.25	3.0	4.50	0.02	0.02								
	TOTAL =						0.22	0.15	100				3	0.1	18	0.6
O-2E	0.0422	400	12.0	3.00	0.7	1.21	0.09	0.06								
		1450	50.0	3.45	2.0	3.71	0.11	0.07								
		1600	40.0	2.50	3.0	4.74	0.09	0.06								
	TOTAL =						0.29	0.20	100				4	0.2	24	1.0
O-3E	0.0164	400	14.0	3.50	0.7	1.31	0.08	0.06								
		780	20.0	2.56	2.0	3.20	0.07	0.05								
		900	20.0	2.22	3.0	4.47	0.06	0.04								
	TOTAL =						0.21	0.14	99			1	3	0.1	13	0.4
O-4E	0.0094	400	10.0	2.50	0.7	1.11	0.10	0.07								
		900	22.0	2.44	2.0	3.13	0.08	0.05								
		150	2.0	1.33	3.0	3.46	0.01	0.01								
	TOTAL =						0.19	0.13	98			2	2	0.1	8	0.2
O-5E	0.1097	400	16.0	4.00	0.7	1.40	0.08	0.05								
		1600	51.0	3.19	2.0	3.57	0.12	0.08								
		2100	52.0	2.48	3.0	4.72	0.12	0.08								
	TOTAL =						0.33	0.22	98			2	14	0.7	63	2.8
O-6E	0.0732	400	12.0	3.00	0.7	1.21	0.09	0.06								
		1600	57.0	3.56	2.0	3.77	0.12	0.08								
		650	21.0	3.23	3.0	5.39	0.03	0.02								
		1170	9.0	0.77	3.0	2.63	0.12	0.08								
	TOTAL =						0.37	0.24	100				6	0.3	36	1.7
O-7E	0.0523	400	19.0	4.75	0.7	1.53	0.07	0.05								
		150	12.0	8.00	2.0	5.66	0.01	0.00								
		2950	102.0	3.46	3.0	5.58	0.15	0.10								
	TOTAL =						0.23	0.15	100				7	0.2	39	1.2
O-8E	0.0099	1320	4.0	0.30	3.0	1.65	0.22	0.15								
							0.22	0.15	15			85	15	0.7	23	1.1
O-9E	0.0348	400	18.0	4.50	0.7	1.48	0.07	0.05								
		200	17.0	8.50	2.0	5.83	0.01	0.01								
		1000	43.0	4.30	3.0	6.22	0.04	0.03								
	TOTAL =						0.13	0.09	95			5	8	0.3	32	1.0
I40-1	0.0075	1765	33.0	1.87	3.0	4.10	0.12	0.08								
	TOTAL =						0.12	0.08	65			35	6	0.2	11	0.5
I40-2	0.0031	760	11.0	1.45	3.0	3.61	0.06	0.04								
	TOTAL =						0.06	0.04	65			35	2	0.1	5	0.2

MASTER DRAINAGE PLAN
WEST SIDE TRANSIT FACILITY

AHYMO BASIN PARAMETER WORKSHEET
PEAK BASIN FLOWS AND VOLUMES

EXISTING CONDITIONS WITH THE WEST SIDE TRANSIT FACILITY FULLY DEVELOPED

BASIN	AREA (sq.mi.)	LENGTH (ft.)	ELEV. DIFF. (ft.)	SLOPE (%)	K	VEL (fps)	T(c) (hr.)	T(p) (hr.)	LAND TREATMENT (%) A	B	C	D	10 YEAR PEAK FLOW (24hr.) (cfs)	10 YEAR RUNOFF VOLUME (24hr.) (ac.ft.)	100 YEAR PEAK FLOW (24hr.) (cfs)	100 YEAR RUNOFF VOLUME (24hr.) (ac.ft.)
I40-3	0.0019 TOTAL =	415	7.0	1.69	3.0	3.90	0.03 0.03	0.02 0.02	65			35	1	0.1	3	0.1
I40-4	0.0012 TOTAL =	285	4.0	1.40	3.0	3.55	0.02 0.02	0.01 0.01	65			35	1	0.04	2	0.1
120.5 *	0.0052	-	-	-	-	-	-	0.13	100				1	0.02	4	0.1
120.5A *	0.0020	-	-	-	-	-	-	0.13	100				0.30	0.01	2	0.05
130.7 *	0.0170	-	-	-	-	-	-	0.13	86	12	2		3	0.1	15	0.4
130.8 *	0.0301	-	-	-	-	-	-	0.14	100				5	0.1	23	0.7
130.9 *	0.0095	-	-	-	-	-	-	0.13	86	12	2		2	0.1	8	0.2
140.1 *	0.0196	-	-	-	-	-	-	0.13	74	22	4		4	0.1	19	0.5
140.2 *	0.0892	-	-	-	-	-	-	0.13	92	4	4		16	0.5	79	2.2
150.1 *	0.1525	-	-	-	-	-	-	0.23	96		4		17	0.8	90	3.9
A-1D	0.0240 TOTAL =	300 820 735	6.0 4.0 18.0	2.00 0.49 2.45	1.0 3.0 3.0	1.41 2.10 4.69	0.06 0.11 0.04 0.21	0.04 0.07 0.03 0.14		10		90	40	1.8	61	2.9
A-2D	0.0125 TOTAL =	370 760	12.0 4.0	3.24 0.53	1.0 3.0	1.80 2.18	0.06 0.10 0.15	0.04 0.06 0.10	15	9		76	18	0.8	29	1.3
G-1D	0.0229 TOTAL =	400 800	10.0 16.0	2.50 2.00	0.7 2.0	1.11 2.83	0.10 0.08 0.18	0.07 0.05 0.12	100				4	0.1	21	0.5
G-2D	0.0031 TOTAL =	1395	22.0	1.58	3.0	3.77	0.10 0.10	0.07 0.07		18		82	5	0.2	8	0.3

MASTER DRAINAGE PLAN
WEST SIDE TRANSIT FACILITY

AHYMO BASIN PARAMETER WORKSHEET
PEAK BASIN FLOWS AND VOLUMES

EXISTING CONDITIONS WITH THE WEST SIDE TRANSIT FACILITY FULLY DEVELOPED

BASIN	AREA (sq.mi.)	LENGTH (ft.)	ELEV. DIFF. (ft.)	SLOPE (%)	K	VEL (fps)	T(c) (hr.)	T(p) (hr.)	LAND TREATMENT (%)				10 YEAR		100 YEAR	
									A	B	C	D	PEAK FLOW (24hr.) (cfs)	RUNOFF VOLUME (24hr.) (ac.ft.)	PEAK FLOW (24hr.) (cfs)	RUNOFF VOLUME (24hr.) (ac.ft.)
G-3D	0.0351	400	9.0	2.25	0.7	1.05	0.11	0.07								
		1600	27.0	1.69	2.0	2.60	0.17	0.11								
	TOTAL =						0.28	0.18	100				4	0.2	22	0.8
G-4D	0.0033	1430	18.0	1.26	3.0	3.37	0.12	0.08								
	TOTAL =						0.12	0.08		18		82	5	0.2	8	0.4
P-1	0.0029	N/A	N/A	N/A	N/A	N/A	0.20	0.13	98			2	1	0.02	3	0.1
P-2	0.0036	N/A	N/A	N/A	N/A	N/A	0.20	0.13	98			2	1	0.02	3	0.1
P-3	0.0027	N/A	N/A	N/A	N/A	N/A	0.20	0.13	98			2	1	0.02	2	0.1
P-4	0.0036	N/A	N/A	N/A	N/A	N/A	0.20	0.13	98			2	1	0.02	3	0.1
P-5	0.0029	N/A	N/A	N/A	N/A	N/A	0.20	0.13	98			2	1	0.02	3	0.1
P-6	0.0043	N/A	N/A	N/A	N/A	N/A	0.20	0.13	98			2	1	0.03	4	0.1

* Hydrology paramaters taken from "Master Drainage Plan for the Atrisco Business Park", Revised Octover 1993, Easterling & Associates, Inc.

MASTER DRAINAGE PLAN
WEST SIDE TRANSIT FACILITY

AHYMO BASIN PARAMETER WORKSHEET
PEAK BASIN FLOWS AND VOLUMES
DEVELOPED INTERIM CONDITIONS

BASIN	AREA (sq.mi.)	LENGTH (ft.)	ELEV. DIFF. (ft.)	SLOPE (%)	K	VEL (fps)	T(c) (hr.)	T(p) (hr.)	LAND TREATMENT (%) A	B	C	D	10 YEAR PEAK FLOW (24hr.) (cfs)	10 YEAR RUNOFF VOLUME (24hr.) (ac.ft.)	100 YEAR PEAK FLOW (24hr.) (cfs)	100 YEAR RUNOFF VOLUME (24hr.) (ac.ft.)
O-1E	0.0242	400	13.0	3.25	0.7	1.26	0.09	0.06								
		1350	40.0	2.96	2.0	3.44	0.11	0.07								
		400	9.0	2.25	3.0	4.50	0.02	0.02								
	TOTAL =						0.22	0.15	100				3	0.1	18	0.6
O-2E	0.0422	400	12.0	3.00	0.7	1.21	0.09	0.06								
		1450	50.0	3.45	2.0	3.71	0.11	0.07								
		1600	40.0	2.50	3.0	4.74	0.09	0.06								
	TOTAL =						0.29	0.20	100				4	0.2	24	1.0
O-3E	0.0164	400	14.0	3.50	0.7	1.31	0.08	0.06								
		780	20.0	2.56	2.0	3.20	0.07	0.05								
		900	20.0	2.22	3.0	4.47	0.06	0.04								
	TOTAL =						0.21	0.14	99			1	3	0.1	13	0.4
O-4E	0.0094	400	10.0	2.50	0.7	1.11	0.10	0.07								
		900	22.0	2.44	2.0	3.13	0.08	0.05								
		150	2.0	1.33	3.0	3.46	0.01	0.01								
	TOTAL =						0.19	0.13	98			2	2	0.1	8	0.2
O-5E	0.1097	400	16.0	4.00	0.7	1.40	0.08	0.05								
		1600	51.0	3.19	2.0	3.57	0.12	0.08								
		2100	52.0	2.48	3.0	4.72	0.12	0.08								
	TOTAL =						0.33	0.22	98			2	14	0.7	63	2.8
O-6E	0.0732	400	12.0	3.00	0.7	1.21	0.09	0.06								
		1600	57.0	3.56	2.0	3.77	0.12	0.08								
		650	21.0	3.23	3.0	5.39	0.03	0.02								
		1170	9.0	0.77	3.0	2.63	0.12	0.08								
	TOTAL =						0.37	0.24	100				6	0.3	36	1.7
O-7E	0.0523	400	19.0	4.75	0.7	1.53	0.07	0.05								
		150	12.0	8.00	2.0	5.66	0.01	0.00								
		2950	102.0	3.46	3.0	5.58	0.15	0.10								
	TOTAL =						0.23	0.15	100				7	0.2	39	1.2
O-8E	0.0099	1320	4.0	0.30	3.0	1.65	0.22	0.15								
							0.22	0.15	15			85	15	0.7	23	1.1
O-9E	0.0348	400	18.0	4.50	0.7	1.48	0.07	0.05								
		200	17.0	8.50	2.0	5.83	0.01	0.01								
		1000	43.0	4.30	3.0	6.22	0.04	0.03								
	TOTAL =						0.13	0.09	95			5	8	0.3	32	1.0
I40-1	0.0075	1765	33.0	1.87	3.0	4.10	0.12	0.08								
	TOTAL =						0.12	0.08	65			35	6	0.2	11	0.5
I40-2	0.0031	760	11.0	1.45	3.0	3.61	0.06	0.04								
	TOTAL =						0.06	0.04	65			35	2	0.1	5	0.2

MASTER DRAINAGE PLAN
WEST SIDE TRANSIT FACILITY

AHYMO BASIN PARAMETER WORKSHEET
PEAK BASIN FLOWS AND VOLUMES
DEVELOPED INTERIM CONDITIONS

BASIN	AREA (sq.mi.)	LENGTH (ft.)	ELEV. DIFF. (ft.)	SLOPE (%)	K	VEL (fps)	T(c) (hr.)	T(p) (hr.)	LAND TREATMENT (%) A	B	C	D	10 YEAR PEAK FLOW (24hr.) (cfs)	10 YEAR RUNOFF VOLUME (24hr.) (ac.ft.)	100 YEAR PEAK FLOW (24hr.) (cfs)	100 YEAR RUNOFF VOLUME (24hr.) (ac.ft.)
I40-3	0.0019	415	7.0	1.69	3.0	3.90	0.03	0.02								
	TOTAL =						0.03	0.02	65			35	1	0.1	3	0.1
I40-4	0.0012	285	4.0	1.40	3.0	3.55	0.02	0.01								
	TOTAL =						0.02	0.01	65			35	1	0.04	2	0.1
A-1D	0.0257	175	5.0	2.86	1.0	1.69	0.03	0.02								
		340	10.0	2.94	2.0	3.43	0.03	0.02								
		775	24.0	3.10	3.0	5.28	0.04	0.03								
	TOTAL =						0.10	0.06		31		69	36	1.6	59	2.6
A-2D	0.0284	400	8.0	2.00	1.0	1.41	0.08	0.05								
		120	2.0	1.67	2.0	2.58	0.01	0.01								
		730	12.0	1.64	3.0	3.85	0.05	0.04								
	TOTAL =						0.14	0.10		16		84	46	2.0	71	3.3
A-3D	0.0240	300	6.0	2.00	1.0	1.41	0.06	0.04								
		820	4.0	0.49	3.0	2.10	0.11	0.07								
		735	18.0	2.45	3.0	4.69	0.04	0.03								
	TOTAL =						0.21	0.14		10		90	40	1.8	61	2.9
A-4D	0.0125	370	12.0	3.24	1.0	1.80	0.06	0.04								
		760	4.0	0.53	3.0	2.18	0.10	0.06								
	TOTAL =						0.15	0.10	15	9		76	18	0.8	29	1.3
C-1D	0.0115	400	8.0	2.00	1.0	1.41	0.08	0.05								
		250	19.0	7.60	2.0	5.51	0.01	0.01								
		475	31.0	6.53	3.0	7.66	0.02	0.01								
	TOTAL =						0.11	0.07		43		57	14	0.6	24	1.0
C-2D	0.0574	400	4.0	1.00	1.0	1.00	0.11	0.07								
		350	12.0	3.43	2.0	3.70	0.03	0.02								
		1520	52.0	3.42	3.0	5.55	0.08	0.05								
	TOTAL =						0.21	0.14		43		57	69	3.0	119	5.1
D-1D	0.0468	200	10.0	5.00	1.0	2.24	0.02	0.02								
		1050	32.0	3.05	3.0	5.24	0.06	0.04								
		800	8.0	1.00	3.0	3.00	0.07	0.05								
	TOTAL =						0.15	0.10		43		57	58	2.4	99	4.2
E-1D	0.0282	400	2.0	0.50	1.0	0.71	0.16	0.10								
		200	10.0	5.00	2.0	4.47	0.01	0.01								
		600	12.0	2.00	3.0	4.24	0.04	0.03								
	TOTAL =						0.21	0.14		29		71	39	1.7	64	2.9
E-2D	0.0172	400	10.0	2.50	1.0	1.58	0.07	0.05								
		1050	22.0	2.10	3.0	4.34	0.07	0.04								
	TOTAL =						0.14	0.09		10		90	29	1.3	45	2.1

MASTER DRAINAGE PLAN
WEST SIDE TRANSIT FACILITY

AHYMO BASIN PARAMETER WORKSHEET
PEAK BASIN FLOWS AND VOLUMES

DEVELOPED INTERIM CONDITIONS

BASIN	AREA (sq.mi.)	LENGTH (ft.)	ELEV. DIFF. (ft.)	SLOPE (%)	K	VEL (fps)	T(c) (hr.)	T(p) (hr.)	LAND TREATMENT (%) A	B	C	D	10 YEAR PEAK FLOW (24hr.) (cfs)	10 YEAR RUNOFF VOLUME (24hr.) (ac.ft.)	100 YEAR PEAK FLOW (24hr.) (cfs)	100 YEAR RUNOFF VOLUME (24hr.) (ac.ft.)
E-3D	0.0425	200	4.0	2.00	1.0	1.41	0.04	0.03								
		900	20.0	2.22	3.0	4.47	0.06	0.04								
		930	22.0	2.37	3.0	4.61	0.06	0.04								
	TOTAL =						0.15	0.10		43		57	53	2.2	90	3.8
E-4D	0.0030	1200	26.0	2.17	3.0	4.42	0.08	0.05								
	TOTAL =						0.08	0.05		10		90	5	0.2	8	0.4
F-1D	0.0396	250	5.0	2.00	1.0	1.41	0.05	0.03								
		850	20.0	2.35	3.0	4.60	0.05	0.03								
		700	4.0	0.57	3.0	2.27	0.09	0.06								
		200	2.0	1.00	3.0	3.00	0.02	0.01								
	TOTAL =						0.20	0.14		43		57	48	2.1	82	3.5
G-1D	0.0229	400	10.0	2.50	1.0	1.58	0.07	0.05								
		800	16.0	2.00	2.0	2.83	0.08	0.05								
	TOTAL =						0.15	0.10		20		80	36	1.6	56	2.5
G-2D	0.0031	1395	22.0	1.58	3.0	3.77	0.10	0.07								
	TOTAL =						0.10	0.07		18		82	5	0.2	8	0.3
G-3D	0.0351	400	7.0	1.75	1.0	1.32	0.08	0.06								
		1700	27.0	1.59	2.5	3.15	0.15	0.10								
	TOTAL =						0.23	0.16		20		80	49	2.4	78	3.9
G-4D	0.0033	1430	18.0	1.26	3.0	3.37	0.12	0.08								
	TOTAL =						0.12	0.08		18		82	5	0.2	8	0.4
P-1	0.0029	N/A	N/A	N/A	N/A	N/A	0.20	0.13	98			2	1	0.02	3	0.1
P-2	0.0036	N/A	N/A	N/A	N/A	N/A	0.20	0.13	98			2	1	0.02	3	0.1
P-3	0.0027	N/A	N/A	N/A	N/A	N/A	0.20	0.13	98			2	1	0.02	2	0.1
P-4	0.0036	N/A	N/A	N/A	N/A	N/A	0.20	0.13	98			2	1	0.02	3	0.1
P-5	0.0029	N/A	N/A	N/A	N/A	N/A	0.20	0.13	98			2	1	0.02	3	0.1
P-6	0.0043	N/A	N/A	N/A	N/A	N/A	0.20	0.13	98			2	1	0.03	4	0.1

MASTER DRAINAGE PLAN
WEST SIDE TRANSIT FACILITY

AHYMO BASIN PARAMETER WORKSHEET
PEAK BASIN FLOWS AND VOLUMES

DEVELOPED CONDITIONS

BASIN	AREA (sq.mi.)	LENGTH (ft.)	ELEV. DIFF. (ft.)	SLOPE (%)	K	VEL (fps)	T(c) (hr.)	T(p) (hr.)	LAND TREATMENT (%) A	B	C	D	10 YEAR PEAK FLOW (24hr.) (cfs)	10 YEAR RUNOFF VOLUME (24hr.) (ac.ft.)	100 YEAR PEAK FLOW (24hr.) (cfs)	100 YEAR RUNOFF VOLUME (24hr.) (ac.ft.)
I40-1	0.0075	1765	33.0	1.87	3.0	4.10	0.12	0.08								
	TOTAL =						0.12	0.08	65			35	6	0.2	11	0.5
I40-2	0.0031	760	11.0	1.45	3.0	3.61	0.06	0.04								
	TOTAL =						0.06	0.04	65			35	2	0.1	5	0.2
I40-3	0.0019	415	7.0	1.69	3.0	3.90	0.03	0.02								
	TOTAL =						0.03	0.02	65			35	1	0.1	3	0.1
I40-4	0.0012	285	4.0	1.40	3.0	3.55	0.02	0.01								
	TOTAL =						0.02	0.01	65			35	1	0.04	2	0.1
A-1D	0.0257	175	5.0	2.86	1.0	1.69	0.03	0.02								
		340	10.0	2.94	2.0	3.43	0.03	0.02								
		775	24.0	3.10	3.0	5.28	0.04	0.03								
	TOTAL =						0.10	0.06		31		69	36	1.6	59	2.6
A-2D	0.0284	400	8.0	2.00	1.0	1.41	0.08	0.05								
		120	2.0	1.67	2.0	2.58	0.01	0.01								
		730	12.0	1.64	3.0	3.85	0.05	0.04								
	TOTAL =						0.14	0.10		16		84	46	2.0	71	3.3
A-3D	0.0240	300	6.0	2.00	1.0	1.41	0.06	0.04								
		820	4.0	0.49	3.0	2.10	0.11	0.07								
		735	18.0	2.45	3.0	4.69	0.04	0.03								
	TOTAL =						0.21	0.14		10		90	40	1.8	61	2.9
A-4D	0.0125	370	12.0	3.24	1.0	1.80	0.06	0.04								
		760	4.0	0.53	3.0	2.18	0.10	0.06								
	TOTAL =						0.15	0.10	15	9		76	18	0.8	29	1.3
C-1D	0.0115	400	8.0	2.00	1.0	1.41	0.08	0.05								
		250	19.0	7.60	2.0	5.51	0.01	0.01								
		475	31.0	6.53	3.0	7.66	0.02	0.01								
	TOTAL =						0.11	0.07		43		57	14	0.6	24	1.0
C-2D	0.0574	400	4.0	1.00	1.0	1.00	0.11	0.07								
		350	12.0	3.43	2.0	3.70	0.03	0.02								
		1520	52.0	3.42	3.0	5.55	0.08	0.05								
	TOTAL =						0.21	0.14		43		57	69	3.0	119	5.1
D-1D	0.0468	200	10.0	5.00	1.0	2.24	0.02	0.02								
		1050	32.0	3.05	3.0	5.24	0.06	0.04								
		800	8.0	1.00	3.0	3.00	0.07	0.05								
	TOTAL =						0.15	0.10		43		57	58	2.4	99	4.2
E-1D	0.0282	400	2.0	0.50	1.0	0.71	0.16	0.10								
		200	10.0	5.00	2.0	4.47	0.01	0.01								
		600	12.0	2.00	3.0	4.24	0.04	0.03								
	TOTAL =						0.21	0.14		29		71	39	1.7	64	2.9

MASTER DRAINAGE PLAN
WEST SIDE TRANSIT FACILITY

AHYMO BASIN PARAMETER WORKSHEET
PEAK BASIN FLOWS AND VOLUMES

DEVELOPED CONDITIONS

BASIN	AREA (sq.mi.)	LENGTH (ft.)	ELEV. DIFF. (ft.)	SLOPE (%)	K	VEL (fps)	T(c) (hr.)	T(p) (hr.)	LAND TREATMENT (%) A	B	C	D	10 YEAR PEAK FLOW (24hr.) (cfs)	10 YEAR RUNOFF VOLUME (24hr.) (ac.ft.)	100 YEAR PEAK FLOW (24hr.) (cfs)	100 YEAR RUNOFF VOLUME (24hr.) (ac.ft.)
E-2D	0.0172	400	10.0	2.50	1.0	1.58	0.07	0.05								
		1050	22.0	2.10	3.0	4.34	0.07	0.04								
	TOTAL =						0.14	0.09		10		90	29	1.3	45	2.1
E-3D	0.0425	200	4.0	2.00	1.0	1.41	0.04	0.03								
		900	20.0	2.22	3.0	4.47	0.06	0.04								
		930	22.0	2.37	3.0	4.61	0.06	0.04								
	TOTAL =						0.15	0.10		43		57	53	2.2	90	3.8
E-4D	0.0030	1200	26.0	2.17	3.0	4.42	0.08	0.05								
	TOTAL =						0.08	0.05		10		90	5	0.2	8	0.4
F-1D	0.0396	250	5.0	2.00	1.0	1.41	0.05	0.03								
		850	20.0	2.35	3.0	4.60	0.05	0.03								
		700	4.0	0.57	3.0	2.27	0.09	0.06								
		200	2.0	1.00	3.0	3.00	0.02	0.01								
	TOTAL =						0.20	0.14		43		57	48	2.1	82	3.5
G-1D	0.0229	400	10.0	2.50	1.0	1.58	0.07	0.05								
		800	16.0	2.00	2.0	2.83	0.08	0.05								
	TOTAL =						0.15	0.10		20		80	36	1.6	56	2.5
G-2D	0.0031	1395	22.0	1.58	3.0	3.77	0.10	0.07								
	TOTAL =						0.10	0.07		18		82	5	0.2	8	0.3
G-3D	0.0351	400	7.0	1.75	1.0	1.32	0.08	0.06								
		1700	27.0	1.59	2.5	3.15	0.15	0.10								
	TOTAL =						0.23	0.16		20		80	49	2.4	78	3.9
G-4D	0.0033	1430	18.0	1.26	3.0	3.37	0.12	0.08								
	TOTAL =						0.12	0.08		18		82	5	0.2	8	0.4
P-1	0.0029	N/A	N/A	N/A	N/A	N/A	0.20	0.13	98			2	1	0.02	3	0.1
P-2	0.0036	N/A	N/A	N/A	N/A	N/A	0.20	0.13	98			2	2	0.05	5	0.1
P-3	0.0027	N/A	N/A	N/A	N/A	N/A	0.20	0.13	98			2	1	0.02	2	0.1
P-4	0.0036	N/A	N/A	N/A	N/A	N/A	0.20	0.13	98			2	1	0.02	3	0.1
P-6	0.0043	N/A	N/A	N/A	N/A	N/A	0.20	0.13	98			2	1	0.03	4	0.1

APPENDIX C

AHYMO SUMMARY TABLES

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994 RUN DATE (MON/DAY/YR) =01/23/2001
INPUT FILE = EXT10EE.IN USER NO.= C_ANDRSN.I01

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION
*S EXISTING CONDITIONS, WITHOUT I-40 INTERCEPTOR IN PLACE										
*S 10 YEAR, 24 HOUR STORM EVENT										
*S										
*S FILE NAME: EXT10EE.IN										
*S										
*S THIS MODEL ANALYZES THE UNSER DIVERSION PONDS WITH THE EXISTING POND										
*S OUTLET WORKS AND THE EXISTING POND GRADING.										
*S										
*S MASTER DRAINAGE PLAN FOR THE WEST SIDE TRANSIT FACILITY										
*S										
*S THIS AHYMO FILE ANALYZES THE UNSER DIVERSION (DETENTION PONDS) ASSUMING										
*S EXISTING CONDITIONS WITHOUT THE I-40 INTERCEPTOR IN PLACE.										
*S NOTES: UNSER DIVERSION POND NO. 5 IS USED IN EXISTING CONDITIONS.										
*S THE OUTLET RATING CURVES WERE ALSO BASED UPON TAILWATER										
*S ELEVATIONS SHOWN ON THE UNSER DIVERSION (AS-BUILT)										
*S CONSTRUCTION PLAN SET (PLANS BY CHUCK EASTERLING)										
*S DRAINAGE BASINS SOUTH OF INTERSTATE 40 WERE TAKEN FROM										
*S THE "MASTER DRAINAGE PLAN FOR THE ATRISCO BUSINESS PARK"										
*S REVISED OCTOBER 1993, PREPARED BY EASTERLING & ASSOCIATES, INC.										
*S DATE: 11/20/00										TIME= .00
START										RAIN24= 1.770
RAINFALL TYPE= 2										
*S BEGIN OFFSITE WATERSHED NORTH OF INTERSTATE 40 - EXISTING CONDITION										
COMPUTE NM HYD	O-1E	-	3	.02420	3.43	.108	.08362	1.550	.221	PER IMP= .00
COMPUTE NM HYD	O-2E	-	4	.04220	4.48	.188	.08362	1.600	.166	PER IMP= .00
ADD HYD	AP-1	3& 4	10	.06640	7.65	.296	.08361	1.550	.180	
ROUTE	AP-1R	10	2	.06640	5.48	.296	.08362	1.700	.128	
COMPUTE NM HYD	O-3E	-	3	.01640	2.72	.086	.09818	1.550	.259	PER IMP= 1.00
ADD HYD	PARTAP-2	2& 3	10	.08280	7.02	.382	.08650	1.650	.132	
COMPUTE NM HYD	AP-7&O-5E	-	3	.10970	13.54	.660	.11274	1.600	.193	PER IMP= 2.00
DIVIDE HYD	PART_AP-7	3	98	.10970	13.54	.660	.11274	1.600	.193	
	PART_AP-7	AND	99	.00000	.00	.000	.00000	-.050	.000	
ROUTE	AP-7R	99	2	.00000	.00	.000	.00000	-.050	.000	
ADD HYD	AP-2	10& 2	10	.08280	7.02	.382	.08650	1.650	.132	
ROUTE	AP-2R	10	2	.08280	6.98	.382	.08651	1.700	.132	
COMPUTE NM HYD	I40-3	-	3	.00190	1.40	.060	.59326	1.500	1.154	PER IMP= 35.00
ADD HYD	AP-3	2& 3	20	.08470	7.72	.442	.09786	1.650	.142	
COMPUTE NM HYD	AP-4&O-4E	-	3	.00940	1.75	.057	.11274	1.550	.292	PER IMP= 2.00
ROUTE	AP-4R	3	2	.00940	1.84	.057	.11280	1.550	.306	
COMPUTE NM HYD	I40-4	-	3	.00120	.89	.038	.59326	1.500	1.156	PER IMP= 35.00
ADD HYD	AP-5	2& 3	10	.01060	2.64	.094	.16710	1.550	.389	
ADD HYD	AP-3+AP-5	20&10	10	.09530	9.60	.537	.10556	1.600	.157	
ROUTE	AP-6	10	12	.09530	9.61	.537	.10557	1.600	.158	
ROUTE	AP-7R-CUL	98	2	.10970	13.58	.660	.11274	1.600	.193	
COMPUTE NM HYD	I40-2	-	3	.00310	2.28	.098	.59326	1.500	1.152	PER IMP= 35.00
ADD HYD	AP-8	2& 3	20	.11280	15.19	.758	.12594	1.600	.210	
COMPUTE NM HYD	O-6E	-	3	.07320	6.49	.326	.08362	1.650	.138	PER IMP= .00
COMPUTE NM HYD	O-7E	-	4	.05230	7.41	.233	.08362	1.550	.221	PER IMP= .00
ADD HYD	AP-9	3& 4	10	.12550	13.19	.560	.08362	1.600	.164	

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION
ROUTE	AP-9R	10	2	.12550	13.27	.560	.08362	1.600	.165	
COMPUTE NM HYD	O-8E	-	3	.00990	14.68	.698	1.32131	1.500	2.318	PER IMP= 85.00
ADD HYD	AP-10	2& 3	10	.13540	26.66	1.257	.17411	1.550	.308	
ROUTE	AP-10R	10	2	.13540	21.80	1.257	.17411	1.650	.252	
COMPUTE NM HYD	O-9E	-	3	.03480	8.15	.290	.15643	1.500	.366	PER IMP= 5.00
ADD HYD	AP-11	2& 3	10	.17020	27.81	1.548	.17049	1.600	.255	
ROUTE	AP-11R	10	2	.17020	27.65	1.548	.17050	1.600	.254	
COMPUTE NM HYD	I40-1	-	3	.00750	5.52	.237	.59326	1.500	1.150	PER IMP= 35.00
ADD HYD	AP-12	2& 3	30	.17770	31.52	1.785	.18833	1.600	.277	
ROUTE	AP-8R	20	22	.11280	15.03	.758	.12595	1.650	.208	
COMPUTE NM HYD	130.70	-	1	.01696	3.27	.092	.10184	1.550	.301	PER IMP= .00
COMPUTE NM HYD	130.90	-	2	.00954	1.84	.052	.10184	1.550	.301	PER IMP= .00
ADD HYD	130.91	1& 2	1	.02650	5.11	.144	.10183	1.550	.301	
ROUTE	AP-12R	30	3	.17770	30.80	1.785	.18834	1.650	.271	
ADD HYD	130.72	1& 3	4	.20420	34.10	1.929	.17711	1.650	.261	
ADD HYD	AP-13	4&22	2	.31700	49.13	2.686	.15890	1.650	.242	
ROUTE	AP-13R	2	3	.31700	44.96	2.687	.15890	1.700	.222	
COMPUTE NM HYD	130.80	-	1	.03270	4.91	.146	.08362	1.550	.235	PER IMP= .00
ADD HYD	130.81	3& 1	10	.34970	47.65	2.832	.15186	1.700	.213	
COMPUTE NM HYD	P-5	-	3	.00290	.55	.017	.11274	1.550	.294	PER IMP= 2.00
ADD HYD	AP-15UB	10& 3	11	.35260	47.93	2.849	.15149	1.700	.212	
ROUTE	AP-6R	12	1	.09530	8.36	.537	.10557	1.750	.137	
COMPUTE NM HYD	120.50	-	2	.01620	2.55	.072	.08362	1.550	.246	PER IMP= .00
ADD HYD	120.51	2& 1	3	.11150	9.35	.609	.10238	1.700	.131	
DIVIDE HYD	AP-6A	3	15	.11150	9.81	.639	.10749	1.700	.138	
	120.53	AND	16	.11150	.47	.030	.00512	1.700	.007	
COMPUTE NM HYD	120.60	-	6	.03280	6.10	.197	.11274	1.550	.291	PER IMP= 2.00
DIVIDE HYD	AP-6B	6	17	.03280	6.16	.199	.11386	1.550	.294	

	120.62	AND	16	.03280	.06	.002	.00113	1.550	.003	
ADD HYD	120.63	15&17	10	.14430	13.69	.838	.10894	1.600	.148	
COMPUTE NM HYD	P-6	-	3	.00430	.81	.026	.11274	1.550	.293	PER IMP= 2.00
ADD HYD	AP-14	10& 3	18	.14860	14.37	.863	.10895	1.600	.151	
ROUTE RESERVOIR	AP-14R	18	2	.14860	4.83	.863	.10895	2.200	.051	AC-FT= .399
ROUTE	POND6ROUTE36	2	12	.14860	4.83	.863	.10895	2.200	.051	
ROUTE	POND6ROUTE48	12	22	.14860	4.83	.863	.10895	2.200	.051	
DIVIDE HYD	AP-15	11	17	.35260	51.28	3.048	.16209	1.700	.227	
	130.83	AND	16	.35260	3.36	.199	.01060	1.700	.015	
ROUTE RESERVOIR	AP-15R	17	5	.35260	23.55	3.048	.16209	2.050	.104	AC-FT= 1.291
ROUTE	P-5R	5	11	.35260	23.72	3.048	.16209	2.050	.105	
ADD HYD	POND-5R&6R	11&22	10	.50120	28.50	3.912	.14633	2.050	.089	
COMPUTE NM HYD	P-4	-	3	.00360	.68	.022	.11274	1.550	.293	PER IMP= 2.00
ADD HYD	AP-15A	10& 3	10	.50480	28.58	3.932	.14606	2.050	.088	
ROUTE RESERVOIR	AP-15AR	10	5	.50480	6.96	3.851	.14302	4.300	.022	AC-FT= 1.991
ROUTE	P-4R	5	12	.50480	6.96	3.851	.14302	4.300	.022	
COMPUTE NM HYD	AP-16	-	1	.01960	4.43	.124	.11841	1.550	.353	PER IMP= .00
ROUTE	140.11	1	22	.01960	3.49	.124	.11845	1.600	.278	
ROUTE	140.12	22	15	.01960	3.47	.124	.11845	1.650	.276	
COMPUTE NM HYD	140.20	-	1	.08920	16.45	.462	.09721	1.550	.288	PER IMP= .00
ADD HYD	AP-17UB	15& 1	2	.10880	18.28	.586	.10103	1.550	.262	
DIVIDE HYD	AP-17	2	17	.10880	19.74	.633	.10911	1.550	.283	
	140.23	AND	16	.10880	1.46	.047	.00808	1.550	.021	
ADD HYD	140.24	17&12	2	.61360	19.75	4.484	.13701	1.550	.050	

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION
COMPUTE NM HYD	P-3	-	3	.00270	.51	.016	.11274	1.550	.294	PER IMP= 2.00
ADD HYD	AP-18	2& 3	10	.61630	20.25	4.499	.13687	1.550	.051	
ROUTE RESERVOIR	AP-18R	10	4	.61630	5.73	4.412	.13422	6.350	.015	AC-FT= 1.241
ROUTE	P-3R	4	2	.61630	5.73	4.412	.13421	6.350	.015	
COMPUTE NM HYD	P-2	-	3	.00360	.68	.022	.11274	1.550	.293	PER IMP= 2.00
ADD HYD	AP-18A	2& 3	10	.61990	5.73	4.432	.13406	6.350	.014	
ROUTE RESERVOIR	AP-18AR	10	5	.61990	4.10	3.664	.11082	9.500	.010	AC-FT= 1.828
ROUTE	P-2R	5	10	.61990	4.10	3.664	.11081	9.500	.010	
COMPUTE NM HYD	AP-19UB	-	1	.15250	16.02	.753	.09263	1.650	.164	PER IMP= .00
DIVIDE HYD	AP-19	1	17	.15250	16.82	.791	.09726	1.650	.172	
	150.12	AND	16	.15250	.80	.038	.00463	1.650	.008	
ADD HYD	150.13	10&17	10	.77240	16.83	4.455	.10814	1.650	.034	
COMPUTE NM HYD	P-1	-	3	.00290	.55	.017	.11274	1.550	.294	PER IMP= 2.00
ADD HYD	AP-20	10& 3	10	.77530	17.29	4.471	.10813	1.600	.035	
ROUTE RESERVOIR	150.14	10	5	.77530	4.58	4.457	.10779	2.100	.009	AC-FT= .437
COMPUTE NM HYD	AP-21&170.1	-	3	.06420	9.31	.386	.11274	1.550	.227	PER IMP= 2.00
FINISH										

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994 RUN DATE (MON/DAY/YR) =01/25/2001
INPUT FILE = EXT100EE.IN USER NO.= C_ANDRSN.I01

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION
*S EXISTING CONDITIONS, WITHOUT I-40 INTERCEPTOR IN PLACE										
*S 100 YEAR, 24 HOUR STORM EVENT										
*S										
*S FILE NAME: EXT100EE.IN										
*S										
*S THIS MODEL ANALYZES THE UNSER DIVERSION PONDS WITH THE EXISTING POND										
*S OUTLET WORKS AND THE EXISTING POND GRADING.										
*S										
*S MASTER DRAINAGE PLAN FOR THE WEST SIDE TRANSIT FACILITY										
*S										
*S THIS AHYMO FILE ANALYZES THE UNSER DIVERSION (DETENTION PONDS) ASSUMING										
*S EXISTING CONDITIONS WITHOUT THE I-40 INTERCEPTOR IN PLACE.										
*S NOTES: UNSER DIVERSION POND NO. 5 IS USED IN EXISTING CONDITIONS.										
*S THE OUTLET RATING CURVES WERE ALSO BASED UPON TAILWATER										
*S ELEVATIONS SHOWN ON THE UNSER DIVERSION (AS-BUILT)										
*S CONSTRUCTION PLAN SET (PLANS BY CHUCK EASTERLING)										
*S DRAINAGE BASINS SOUTH OF INTERSTATE 40 WERE TAKEN FROM										
*S THE "MASTER DRAINAGE PLAN FOR THE ATRISCO BUSINESS PARK"										
*S REVISED OCTOBER 1993, PREPARED BY EASTERLING & ASSOCIATES, INC.										
*S DATE: 11/20/00										TIME= .00
START										RAIN24= 2.660
RAINFALL TYPE= 2										
*S BEGIN OFFSITE WATERSHED NORTH OF INTERSTATE 40 - EXISTING CONDITION										
COMPUTE NM HYD	O-1E	-	3	.02420	18.06	.567	.43936	1.550	1.166	PER IMP= .00
COMPUTE NM HYD	O-2E	-	4	.04220	24.34	.989	.43936	1.600	.901	PER IMP= .00
ADD HYD	AP-1	3& 4	10	.06640	41.88	1.556	.43936	1.550	.986	
ROUTE	AP-1R	10	2	.06640	36.11	1.556	.43938	1.650	.850	
COMPUTE NM HYD	O-3E	-	3	.01640	13.10	.402	.45924	1.500	1.248	PER IMP= 1.00
ADD HYD	PARTAP-2	2& 3	10	.08280	44.73	1.958	.44330	1.650	.844	
COMPUTE NM HYD	AP-7&O-5E	-	3	.10970	63.37	2.803	.47912	1.600	.903	PER IMP= 2.00
DIVIDE HYD	PART_AP-7	3	98	.08594	32.00	2.196	.47911	1.500	.582	
	PART_AP-7	AND	99	.02376	31.37	.607	.47911	1.600	2.062	
ROUTE	AP-7R	99	2	.02376	31.64	.608	.47944	1.600	2.081	
ADD HYD	AP-2	10& 2	10	.10656	76.00	2.565	.45135	1.600	1.114	
ROUTE	AP-2R	10	2	.10656	79.83	2.565	.45136	1.600	1.171	
COMPUTE NM HYD	I40-3	-	3	.00190	2.86	.115	1.13512	1.500	2.350	PER IMP= 35.00
ADD HYD	AP-3	2& 3	20	.10846	81.89	2.680	.46333	1.600	1.180	
COMPUTE NM HYD	AP-4&O-4E	-	3	.00940	8.11	.240	.47912	1.500	1.348	PER IMP= 2.00
ROUTE	AP-4R	3	2	.00940	8.17	.240	.47921	1.500	1.358	
COMPUTE NM HYD	I40-4	-	3	.00120	1.81	.073	1.13512	1.500	2.357	PER IMP= 35.00
ADD HYD	AP-5	2& 3	10	.01060	9.98	.313	.55337	1.500	1.471	
ADD HYD	AP-3+AP-5	20&10	10	.11906	89.90	2.993	.47135	1.600	1.180	
ROUTE	AP-6	10	12	.11906	87.97	2.993	.47135	1.650	1.154	
ROUTE	AP-7R-CUL	98	2	.08594	33.23	2.196	.47912	1.500	.604	
COMPUTE NM HYD	I40-2	-	3	.00310	4.65	.188	1.13512	1.500	2.346	PER IMP= 35.00
ADD HYD	AP-8	2& 3	20	.08904	37.88	2.384	.50195	1.500	.665	
COMPUTE NM HYD	O-6E	-	3	.07320	35.69	1.715	.43936	1.600	.762	PER IMP= .00
COMPUTE NM HYD	O-7E	-	4	.05230	39.03	1.226	.43936	1.550	1.166	PER IMP= .00
ADD HYD	AP-9	3& 4	10	.12550	71.47	2.941	.43936	1.550	.890	

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION
ROUTE	AP-9R	10	2	.12550	70.89	2.941	.43937	1.600	.883	
COMPUTE NM HYD	O-8E	-	3	.00990	23.00	1.124	2.12906	1.500	3.630	PER IMP= 85.00
ADD HYD	AP-10	2& 3	10	.13540	92.59	4.065	.56290	1.550	1.068	
ROUTE	AP-10R	10	2	.13540	89.10	4.065	.56291	1.600	1.028	
COMPUTE NM HYD	O-9E	-	3	.03480	31.99	1.000	.53875	1.500	1.436	PER IMP= 5.00
ADD HYD	AP-11	2& 3	10	.17020	114.14	5.065	.55797	1.600	1.048	
ROUTE	AP-11R	10	2	.17020	114.79	5.065	.55797	1.600	1.054	
COMPUTE NM HYD	I40-1	-	3	.00750	11.24	.454	1.13512	1.500	2.342	PER IMP= 35.00
ADD HYD	AP-12	2& 3	30	.17770	122.86	5.519	.58232	1.600	1.080	
ROUTE	AP-8R	20	22	.08904	38.28	2.384	.50196	1.550	.672	
COMPUTE NM HYD	130.70	-	1	.01696	15.02	.425	.46988	1.500	1.384	PER IMP= .00
COMPUTE NM HYD	130.90	-	2	.00954	8.45	.239	.46988	1.500	1.385	PER IMP= .00
ADD HYD	130.91	1& 2	1	.02650	23.48	.664	.46988	1.500	1.384	
ROUTE	AP-12R	30	3	.17770	121.65	5.519	.58233	1.600	1.070	
ADD HYD	130.72	1& 3	4	.20420	140.42	6.183	.56773	1.600	1.074	
ADD HYD	AP-13	4&22	2	.29324	176.26	8.567	.54776	1.600	.939	
ROUTE	AP-13R	2	3	.29324	172.47	8.567	.54776	1.650	.919	
COMPUTE NM HYD	130.80	-	1	.03270	25.40	.766	.43936	1.550	1.214	PER IMP= .00
ADD HYD	130.81	3& 1	10	.32594	189.38	9.333	.53688	1.650	.908	
COMPUTE NM HYD	P-5	-	3	.00290	2.51	.074	.47912	1.500	1.353	PER IMP= 2.00
ADD HYD	AP-15UB	10& 3	11	.32884	190.90	9.406	.53633	1.650	.907	
ROUTE	AP-6R	12	1	.11906	84.75	2.993	.47135	1.650	1.112	
COMPUTE NM HYD	120.50	-	2	.01620	13.33	.380	.43936	1.500	1.286	PER IMP= .00
ADD HYD	120.51	2& 1	3	.13526	92.90	3.373	.46752	1.650	1.073	
DIVIDE HYD	AP-6A	3	15	.13526	97.54	3.541	.49089	1.650	1.127	
	120.53	AND	16	.13526	4.64	.169	.02338	1.650	.054	
COMPUTE NM HYD	120.60	-	6	.03280	28.25	.838	.47912	1.500	1.346	PER IMP= 2.00
DIVIDE HYD	AP-6B	6	17	.03280	28.53	.846	.48390	1.500	1.359	

	120.62	AND	16	.03280	.28	.008	.00479	1.500	.013	
ADD HYD	120.63	15&17	10	.16806	114.84	4.388	.48953	1.650	1.068	
COMPUTE NM HYD	P-6	-	3	.00430	3.72	.110	.47912	1.500	1.350	PER IMP= 2.00
ADD HYD	AP-14	10& 3	18	.17236	117.09	4.498	.48926	1.650	1.061	
ROUTE RESERVOIR	AP-14R	18	2	.17236	9.77	4.498	.48926	2.400	.089	AC-FT= 3.305
ROUTE	POND6ROUTE36	2	12	.17236	9.77	4.498	.48926	2.450	.089	
ROUTE	POND6ROUTE48	12	22	.17236	9.77	4.498	.48926	2.450	.089	
DIVIDE HYD	AP-15	11	17	.32884	204.27	10.065	.57387	1.650	.971	
	130.83	AND	16	.32884	13.36	.658	.03754	1.650	.063	
ROUTE RESERVOIR	AP-15R	17	5	.32884	130.75	10.065	.57387	1.850	.621	AC-FT= 2.666
ROUTE	P-5R	5	11	.32884	131.18	10.065	.57388	1.800	.623	
ADD HYD	POND-5R&6R	11&22	10	.50120	139.93	14.562	.54477	1.800	.436	
COMPUTE NM HYD	P-4	-	3	.00360	3.11	.092	.47912	1.500	1.352	PER IMP= 2.00
ADD HYD	AP-15A	10& 3	10	.50480	140.72	14.654	.54429	1.800	.436	
ROUTE RESERVOIR	AP-15AR	10	5	.50480	93.63	14.571	.54122	2.200	.290	AC-FT= 4.641
ROUTE	P-4R	5	12	.50480	94.22	14.571	.54123	2.200	.292	
COMPUTE NM HYD	AP-16	-	1	.01960	18.50	.522	.49901	1.500	1.475	PER IMP= .00
ROUTE	140.11	1	22	.01960	16.56	.522	.49908	1.550	1.320	
ROUTE	140.12	22	15	.01960	17.13	.522	.49909	1.600	1.366	
COMPUTE NM HYD	140.20	-	1	.08920	78.50	2.196	.46166	1.500	1.375	PER IMP= .00
ADD HYD	AP-17UB	15& 1	2	.10880	91.60	2.718	.46840	1.550	1.316	
DIVIDE HYD	AP-17	2	17	.10880	98.93	2.935	.50587	1.550	1.421	
	140.23	AND	16	.10880	7.33	.217	.03747	1.550	.105	
ADD HYD	140.24	17&12	2	.61360	100.79	17.507	.53496	2.200	.257	

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION
COMPUTE NM HYD	P-3	-	3	.00270	2.34	.069	.47912	1.500	1.354	PER IMP= 2.00
ADD HYD	AP-18	2& 3	10	.61630	101.53	17.575	.53469	1.550	.257	
ROUTE RESERVOIR	AP-18R	10	4	.61630	88.48	17.467	.53141	2.350	.224	AC-FT= 2.796
ROUTE	P-3R	4	2	.61630	88.54	17.467	.53141	2.300	.224	
COMPUTE NM HYD	P-2	-	3	.00360	3.11	.092	.47912	1.500	1.352	PER IMP= 2.00
ADD HYD	AP-18A	2& 3	10	.61990	88.70	17.559	.53109	2.300	.224	
ROUTE RESERVOIR	AP-18AR	10	5	.61990	16.86	16.695	.50497	4.550	.042	AC-FT= 6.420
ROUTE	P-2R	5	10	.61990	16.86	16.694	.50495	4.550	.042	
COMPUTE NM HYD	AP-19UB	-	1	.15250	85.35	3.692	.45390	1.600	.874	PER IMP= .00
DIVIDE HYD	AP-19	1	17	.15250	89.62	3.876	.47660	1.600	.918	
	150.12	AND	16	.15250	4.27	.185	.02270	1.600	.044	
ADD HYD	150.13	10&17	10	.77240	89.63	20.571	.49935	1.600	.181	
COMPUTE NM HYD	P-1	-	3	.00290	2.51	.074	.47912	1.500	1.353	PER IMP= 2.00
ADD HYD	AP-20	10& 3	10	.77530	91.63	20.644	.49926	1.600	.185	
ROUTE RESERVOIR	150.14	10	5	.77530	12.02	20.590	.49796	6.800	.024	AC-FT= 4.419
COMPUTE NM HYD	AP-21&170.1	-	3	.06420	43.17	1.640	.47912	1.550	1.051	PER IMP= 2.00
FINISH										

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994 RUN DATE (MON/DAY/YR) =01/23/2001
INPUT FILE = EXT10PE.IN USER NO.= C_ANDRSN.101

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION	PAGE = 1
*S EXISTING CONDITIONS, WITHOUT I-40 INTERCEPTOR IN PLACE											
*S 10 YEAR, 24 HOUR STORM EVENT											
*S											
*S FILE NAME: EXT10PE.IN											
*S											
*S THIS MODEL ANALYZES THE UNSER DIVERSION PONDS WITH THE PROPOSED POND											
*S OUTLET WORKS AND THE EXISTING POND GRADING.											
*S											
*S MASTER DRAINAGE PLAN FOR THE WEST SIDE TRANSIT FACILITY											
*S											
*S THIS AHYMO FILE ANALYZES THE UNSER DIVERSION (DETENTION PONDS) ASSUMING											
*S EXISTING CONDITIONS WITHOUT THE I-40 INTERCEPTOR IN PLACE.											
*S NOTES: UNSER DIVERSION POND NO. 5 IS USED IN EXISTING CONDITIONS.											
*S THE OUTLET RATING CURVES WERE ALSO BASED UPON TAILWATER											
*S ELEVATIONS SHOWN ON THE UNSER DIVERSION (AS-BUILT)											
*S CONSTRUCTION PLAN SET (PLANS BY CHUCK EASTERLING)											
*S DRAINAGE BASINS SOUTH OF INTERSTATE 40 WERE TAKEN FROM											
*S THE "MASTER DRAINAGE PLAN FOR THE ATRISCO BUSINESS PARK"											
*S REVISED OCTOBER 1993, PREPARED BY EASTERLING & ASSOCIATES, INC.											
*S DATE: 11/20/00											TIME= .00
START											RAIN24= 1.770
RAINFALL TYPE= 2											
*S BEGIN OFFSITE WATERSHED NORTH OF INTERSTATE 40 - EXISTING CONDITION											
COMPUTE NM HYD	O-1E	-	3	.02420	3.43	.108	.08362	1.550	.221	PER IMP=	.00
COMPUTE NM HYD	O-2E	-	4	.04220	4.48	.188	.08362	1.600	.166	PER IMP=	.00
ADD HYD	AP-1	3& 4	10	.06640	7.65	.296	.08361	1.550	.180		
ROUTE	AP-1R	10	2	.06640	5.45	.296	.08362	1.700	.128		
COMPUTE NM HYD	O-3E	-	3	.01640	2.72	.086	.09818	1.550	.259	PER IMP=	1.00
ADD HYD	PARTAP-2	2& 3	10	.08280	7.02	.382	.08650	1.650	.132		
COMPUTE NM HYD	AP-7&O-5E	-	3	.10970	13.54	.660	.11274	1.600	.193	PER IMP=	2.00
DIVIDE HYD	PART_AP-7	3	98	.10970	13.54	.660	.11274	1.600	.193		
	PART_AP-7	AND	99	.00000	.00	.000	.00000	-.050	.000		
ROUTE	AP-7R	99	2	.00000	.00	.000	.00000	-.050	.000		
ADD HYD	AP-2	10& 2	10	.08280	7.02	.382	.08650	1.650	.132		
ROUTE	AP-2R	10	2	.08280	6.98	.382	.08651	1.700	.132		
COMPUTE NM HYD	I40-3	-	3	.00190	1.40	.060	.59326	1.500	1.154	PER IMP=	35.00
ADD HYD	AP-3	2& 3	20	.08470	7.72	.442	.09786	1.650	.142		
COMPUTE NM HYD	AP-4&O-4E	-	3	.00940	1.75	.057	.11274	1.550	.292	PER IMP=	2.00
ROUTE	AP-4R	3	2	.00940	1.84	.057	.11280	1.550	.306		
COMPUTE NM HYD	I40-4	-	3	.00120	.89	.038	.59326	1.500	1.156	PER IMP=	35.00
ADD HYD	AP-5	2& 3	10	.01060	2.64	.094	.16710	1.550	.389		
ADD HYD	AP-3+AP-5	20&10	10	.09530	9.60	.537	.10556	1.600	.157		
ROUTE	AP-6	10	12	.09530	9.61	.537	.10557	1.600	.158		
ROUTE	AP-7R-CUL	98	2	.10970	13.58	.660	.11274	1.600	.193		
COMPUTE NM HYD	I40-2	-	3	.00310	2.28	.098	.59326	1.500	1.152	PER IMP=	35.00
ADD HYD	AP-8	2& 3	20	.11280	15.19	.758	.12594	1.600	.210		
COMPUTE NM HYD	O-6E	-	3	.07320	6.49	.326	.08362	1.650	.138	PER IMP=	.00
COMPUTE NM HYD	O-7E	-	4	.05230	7.41	.233	.08362	1.550	.221	PER IMP=	.00
ADD HYD	AP-9	3& 4	10	.12550	13.19	.560	.08362	1.600	.164		

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION	PAGE = 2
ROUTE	AP-9R	10	2	.12550	13.27	.560	.08362	1.600	.165		
COMPUTE NM HYD	O-8E	-	3	.00990	14.68	.698	1.32131	1.500	2.318	PER IMP=	85.00
ADD HYD	AP-10	2& 3	10	.13540	26.66	1.257	.17411	1.550	.308		
ROUTE	AP-10R	10	2	.13540	21.80	1.257	.17411	1.650	.252		
COMPUTE NM HYD	O-9E	-	3	.03480	8.15	.290	.15643	1.500	.366	PER IMP=	5.00
ADD HYD	AP-11	2& 3	10	.17020	27.81	1.548	.17049	1.600	.255		
ROUTE	AP-11R	10	2	.17020	27.65	1.548	.17050	1.600	.254		
COMPUTE NM HYD	I40-1	-	3	.00750	5.52	.237	.59326	1.500	1.150	PER IMP=	35.00
ADD HYD	AP-12	2& 3	30	.17770	31.52	1.785	.18833	1.600	.277		
ROUTE	AP-8R	20	22	.11280	15.03	.758	.12595	1.650	.208		
COMPUTE NM HYD	130.70	-	1	.01696	3.27	.092	.10184	1.550	.301	PER IMP=	.00
COMPUTE NM HYD	130.90	-	2	.00954	1.84	.052	.10184	1.550	.301	PER IMP=	.00
ADD HYD	130.91	1& 2	1	.02650	5.11	.144	.10183	1.550	.301		
ROUTE	AP-12R	30	3	.17770	30.80	1.785	.18834	1.650	.271		
ADD HYD	130.72	1& 3	4	.20420	34.10	1.929	.17711	1.650	.261		
ADD HYD	AP-13	4&22	2	.31700	49.13	2.686	.15890	1.650	.242		
ROUTE	AP-13R	2	3	.31700	44.96	2.687	.15890	1.700	.222		
COMPUTE NM HYD	130.80	-	1	.03270	4.91	.146	.08362	1.550	.235	PER IMP=	.00
ADD HYD	130.81	3& 1	10	.34970	47.65	2.832	.15186	1.700	.213		
COMPUTE NM HYD	P-5	-	3	.00290	.55	.017	.11274	1.550	.294	PER IMP=	2.00
ADD HYD	AP-15UB	10& 3	11	.35260	47.93	2.849	.15149	1.700	.212		
ROUTE	AP-6R	12	1	.09530	8.36	.537	.10557	1.750	.137		
COMPUTE NM HYD	120.50	-	2	.01620	2.55	.072	.08362	1.550	.246	PER IMP=	.00
ADD HYD	120.51	2& 1	3	.11150	9.35	.609	.10238	1.700	.131		
DIVIDE HYD	AP-6A	3	15	.11150	9.81	.639	.10749	1.700	.138		
	120.53	AND	16	.11150	.47	.030	.00512	1.700	.007		
COMPUTE NM HYD	120.60	-	6	.03280	6.10	.197	.11274	1.550	.291	PER IMP=	2.00
DIVIDE HYD	AP-6B	6	17	.03280	6.16	.199	.11386	1.550	.294		

	120.62	AND	16	.03280	.06	.002	.00113	1.550	.003		
ADD HYD	120.63	15&17	10	.14430	13.69	.838	.10894	1.600	.148		
COMPUTE NM HYD	P-6	-	3	.00430	.81	.026	.11274	1.550	.293	PER IMP=	2.00
ADD HYD	AP-14	10& 3	18	.14860	14.37	.863	.10895	1.600	.151		
ROUTE RESERVOIR	AP-14R	18	2	.14860	7.30	.863	.10895	2.050	.077	AC-FT=	.313
ROUTE	POND6ROUTE36	2	12	.14860	7.30	.863	.10895	2.050	.077		
ROUTE	POND6ROUTE48	12	22	.14860	7.30	.863	.10895	2.050	.077		
DIVIDE HYD	AP-15	11	17	.35260	51.28	3.048	.16209	1.700	.227		
	130.83	AND	16	.35260	3.36	.199	.01060	1.700	.015		
ROUTE RESERVOIR	AP-15R	17	5	.35260	23.55	3.048	.16209	2.050	.104	AC-FT=	1.291
ROUTE	P-5R	5	11	.35260	23.72	3.048	.16209	2.050	.105		
ADD HYD	POND-5R&6R	11&22	10	.50120	31.03	3.912	.14633	2.050	.097		
COMPUTE NM HYD	P-4	-	3	.00360	.68	.022	.11274	1.550	.293	PER IMP=	2.00
ADD HYD	AP-15A	10& 3	10	.50480	31.10	3.932	.14606	2.050	.096		
ROUTE RESERVOIR	AP-15AR	10	5	.50480	21.36	3.854	.14313	2.400	.066	AC-FT=	.661
ROUTE	P-4R	5	12	.50480	21.36	3.854	.14313	2.400	.066		
COMPUTE NM HYD	AP-16	-	1	.01960	4.43	.124	.11841	1.550	.353	PER IMP=	.00
ROUTE	140.11	1	22	.01960	3.49	.124	.11845	1.600	.278		
ROUTE	140.12	22	15	.01960	3.47	.124	.11845	1.650	.276		
COMPUTE NM HYD	140.20	-	1	.08920	16.45	.462	.09721	1.550	.288	PER IMP=	.00
ADD HYD	AP-17UB	15& 1	2	.10880	18.28	.586	.10103	1.550	.262		
DIVIDE HYD	AP-17	2	17	.10880	19.74	.633	.10911	1.550	.283		
	140.23	AND	16	.10880	1.46	.047	.00808	1.550	.021		
ADD HYD	140.24	17&12	2	.61360	22.64	4.487	.13710	2.350	.058		

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION	
COMPUTE NM HYD	P-3	-	3	.00270	.51	.016	.11274	1.550	.294	PER IMP=	2.00
ADD HYD	AP-18	2& 3	10	.61630	22.66	4.502	.13697	2.350	.057		
ROUTE RESERVOIR	AP-18R	10	4	.61630	19.25	4.433	.13487	2.750	.049	AC-FT=	.870
ROUTE	P-3R	4	2	.61630	19.25	4.433	.13487	2.750	.049		
COMPUTE NM HYD	P-2	-	3	.00360	.68	.022	.11274	1.550	.293	PER IMP=	2.00
ADD HYD	AP-18A	2& 3	10	.61990	19.27	4.454	.13472	2.750	.049		
ROUTE RESERVOIR	AP-18AR	10	5	.61990	14.08	3.724	.11264	3.550	.035	AC-FT=	1.437
ROUTE	P-2R	5	10	.61990	14.08	3.724	.11264	3.550	.035		
COMPUTE NM HYD	AP-19UB	-	1	.15250	16.02	.753	.09263	1.650	.164	PER IMP=	.00
DIVIDE HYD	AP-19	1	17	.15250	16.82	.791	.09726	1.650	.172		
	150.12	AND	16	.15250	.80	.038	.00463	1.650	.008		
ADD HYD	150.13	10&17	10	.77240	16.83	4.515	.10960	1.650	.034		
COMPUTE NM HYD	P-1	-	3	.00290	.55	.017	.11274	1.550	.294	PER IMP=	2.00
ADD HYD	AP-20	10& 3	10	.77530	17.29	4.531	.10959	1.600	.035		
ROUTE RESERVOIR	150.14	10	5	.77530	13.48	4.531	.10957	4.000	.027	AC-FT=	.402
COMPUTE NM HYD	AP-21&170.1	-	3	.06420	9.31	.386	.11274	1.550	.227	PER IMP=	2.00

FINISH

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994 RUN DATE (MON/DAY/YR) =01/25/2001
INPUT FILE = EXT100PE.IN USER NO.= C_ANDRSN.I01

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION
*S EXISTING CONDITIONS, WITHOUT I-40 INTERCEPTOR IN PLACE										
*S 100 YEAR, 24 HOUR STORM EVENT										
*S										
*S FILE NAME: EXT100PE.IN										
*S										
*S THIS MODEL ANALYZES THE UNSER DIVERSION PONDS WITH THE PROPOSED POND										
*S OUTLET WORKS AND THE EXISTING POND GRADING.										
*S										
*S MASTER DRAINAGE PLAN FOR THE WEST SIDE TRANSIT FACILITY										
*S										
*S THIS AHYMO FILE ANALYZES THE UNSER DIVERSION (DETENTION PONDS) ASSUMING										
*S EXISTING CONDITIONS WITHOUT THE I-40 INTERCEPTOR IN PLACE.										
*S NOTES: UNSER DIVERSION POND NO. 5 IS USED IN EXISTING CONDITIONS.										
*S THE OUTLET RATING CURVES WERE ALSO BASED UPON TAILWATER										
*S ELEVATIONS SHOWN ON THE UNSER DIVERSION (AS-BUILT)										
*S CONSTRUCTION PLAN SET (PLANS BY CHUCK EASTERLING)										
*S DRAINAGE BASINS SOUTH OF INTERSTATE 40 WERE TAKEN FROM										
*S THE "MASTER DRAINAGE PLAN FOR THE ATRISCO BUSINESS PARK"										
*S REVISED OCTOBER 1993, PREPARED BY EASTERLING & ASSOCIATES, INC.										
*S DATE: 11/20/00										TIME= .00
START										RAIN24= 2.660
RAINFALL TYPE= 2										
*S BEGIN OFFSITE WATERSHED NORTH OF INTERSTATE 40 - EXISTING CONDITION										
COMPUTE NM HYD	O-1E	-	3	.02420	18.06	.567	.43936	1.550	1.166	PER IMP= .00
COMPUTE NM HYD	O-2E	-	4	.04220	24.34	.989	.43936	1.600	.901	PER IMP= .00
ADD HYD	AP-1	3& 4	10	.06640	41.88	1.556	.43936	1.550	.986	
ROUTE	AP-1R	10	2	.06640	36.11	1.556	.43938	1.650	.850	
COMPUTE NM HYD	O-3E	-	3	.01640	13.10	.402	.45924	1.500	1.248	PER IMP= 1.00
ADD HYD	PARTAP-2	2& 3	10	.08280	44.73	1.958	.44330	1.650	.844	
COMPUTE NM HYD	AP-7&O-5E	-	3	.10970	63.37	2.803	.47912	1.600	.903	PER IMP= 2.00
DIVIDE HYD	PART AP-7	3	98	.08594	32.00	2.196	.47911	1.500	.582	
	PART AP-7	AND	99	.02376	31.37	.607	.47911	1.600	2.062	
ROUTE	AP-7R	99	2	.02376	31.64	.608	.47944	1.600	2.081	
ADD HYD	AP-2	10& 2	10	.10656	76.00	2.565	.45135	1.600	1.114	
ROUTE	AP-2R	10	2	.10656	79.83	2.565	.45136	1.600	1.171	
COMPUTE NM HYD	I40-3	-	3	.00190	2.86	.115	1.13512	1.500	2.350	PER IMP= 35.00
ADD HYD	AP-3	2& 3	20	.10846	81.89	2.680	.46333	1.600	1.180	
COMPUTE NM HYD	AP-4&O-4E	-	3	.00940	8.11	.240	.47912	1.500	1.348	PER IMP= 2.00
ROUTE	AP-4R	3	2	.00940	8.17	.240	.47921	1.500	1.358	
COMPUTE NM HYD	I40-4	-	3	.00120	1.81	.073	1.13512	1.500	2.357	PER IMP= 35.00
ADD HYD	AP-5	2& 3	10	.01060	9.98	.313	.55337	1.500	1.471	
ADD HYD	AP-3+AP-5	20&10	10	.11906	89.90	2.993	.47135	1.600	1.180	
ROUTE	AP-6	10	12	.11906	87.97	2.993	.47135	1.650	1.154	
ROUTE	AP-7R-CUL	98	2	.08594	33.23	2.196	.47912	1.500	.604	
COMPUTE NM HYD	I40-2	-	3	.00310	4.65	.188	1.13512	1.500	2.346	PER IMP= 35.00
ADD HYD	AP-8	2& 3	20	.08904	37.88	2.384	.50195	1.500	.665	
COMPUTE NM HYD	O-6E	-	3	.07320	35.69	1.715	.43936	1.600	.762	PER IMP= .00
COMPUTE NM HYD	O-7E	-	4	.05230	39.03	1.226	.43936	1.550	1.166	PER IMP= .00
ADD HYD	AP-9	3& 4	10	.12550	71.47	2.941	.43936	1.550	.890	

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION
ROUTE	AP-9R	10	2	.12550	70.89	2.941	.43937	1.600	.883	
COMPUTE NM HYD	O-8E	-	3	.00990	23.00	1.124	2.12906	1.500	3.630	PER IMP= 85.00
ADD HYD	AP-10	2& 3	10	.13540	92.59	4.065	.56290	1.550	1.068	
ROUTE	AP-10R	10	2	.13540	89.10	4.065	.56291	1.600	1.028	
COMPUTE NM HYD	O-9E	-	3	.03480	31.99	1.000	.53875	1.500	1.436	PER IMP= 5.00
ADD HYD	AP-11	2& 3	10	.17020	114.14	5.065	.55797	1.600	1.048	
ROUTE	AP-11R	10	2	.17020	114.79	5.065	.55797	1.600	1.054	
COMPUTE NM HYD	I40-1	-	3	.00750	11.24	.454	1.13512	1.500	2.342	PER IMP= 35.00
ADD HYD	AP-12	2& 3	30	.17770	122.86	5.519	.58232	1.600	1.080	
ROUTE	AP-8R	20	22	.08904	38.28	2.384	.50196	1.550	.672	
COMPUTE NM HYD	130.70	-	1	.01696	15.02	.425	.46988	1.500	1.384	PER IMP= .00
COMPUTE NM HYD	130.90	-	2	.00954	8.45	.239	.46988	1.500	1.385	PER IMP= .00
ADD HYD	130.91	1& 2	1	.02650	23.48	.664	.46988	1.500	1.384	
ROUTE	AP-12R	30	3	.17770	121.65	5.519	.58233	1.600	1.070	
ADD HYD	130.72	1& 3	4	.20420	140.42	6.183	.56773	1.600	1.074	
ADD HYD	AP-13	4&22	2	.29324	176.26	8.567	.54776	1.600	.939	
ROUTE	AP-13R	2	3	.29324	172.47	8.567	.54776	1.650	.919	
COMPUTE NM HYD	130.80	-	1	.03270	25.40	.766	.43936	1.550	1.214	PER IMP= .00
ADD HYD	130.81	3& 1	10	.32594	189.38	9.333	.53688	1.650	.908	
COMPUTE NM HYD	P-5	-	3	.00290	2.51	.074	.47912	1.500	1.353	PER IMP= 2.00
ADD HYD	AP-15UB	10& 3	11	.32884	190.90	9.406	.53633	1.650	.907	
ROUTE	AP-6R	12	1	.11906	84.75	2.993	.47135	1.650	1.112	
COMPUTE NM HYD	120.50	-	2	.01620	13.33	.380	.43936	1.500	1.286	PER IMP= .00
ADD HYD	120.51	2& 1	3	.13526	92.90	3.373	.46752	1.650	1.073	
DIVIDE HYD	AP-6A	3	15	.13526	97.54	3.541	.49089	1.650	1.127	
	120.53	AND	16	.13526	4.64	.169	.02338	1.650	.054	
COMPUTE NM HYD	120.60	-	6	.03280	28.25	.838	.47912	1.500	1.346	PER IMP= 2.00
DIVIDE HYD	AP-6B	6	17	.03280	28.53	.846	.48390	1.500	1.359	

	120.62	AND	16	.03280	.28	.008	.00479	1.500	.013		
ADD HYD	120.63	15&17	10	.16806	114.84	4.388	.48953	1.650	1.068		
COMPUTE NM HYD	P-6	-	3	.00430	3.72	.110	.47912	1.500	1.350	PER IMP=	2.00
ADD HYD	AP-14	10& 3	18	.17236	117.09	4.498	.48926	1.650	1.061		
ROUTE RESERVOIR	AP-14R	18	2	.17236	38.13	4.498	.48926	1.900	.346	AC-FT=	2.250
ROUTE	POND6ROUTE36	2	12	.17236	38.12	4.498	.48926	1.900	.346		
ROUTE	POND6ROUTE48	12	22	.17236	38.12	4.498	.48926	1.900	.346		
DIVIDE HYD	AP-15	11	17	.32884	204.27	10.065	.57387	1.650	.971		
	130.83	AND	16	.32884	13.36	.658	.03754	1.650	.063		
ROUTE RESERVOIR	AP-15R	17	5	.32884	130.75	10.065	.57387	1.850	.621	AC-FT=	2.666
ROUTE	P-5R	5	11	.32884	131.18	10.065	.57388	1.800	.623		
ADD HYD	POND-5R&6R	11&22	10	.50120	168.51	14.562	.54477	1.900	.525		
COMPUTE NM HYD	P-4	-	3	.00360	3.11	.092	.47912	1.500	1.352	PER IMP=	2.00
ADD HYD	AP-15A	10& 3	10	.50480	168.96	14.654	.54429	1.900	.523		
ROUTE RESERVOIR	AP-15AR	10	5	.50480	95.22	14.575	.54136	2.250	.295	AC-FT=	4.809
ROUTE	P-4R	5	12	.50480	95.31	14.575	.54136	2.250	.295		
COMPUTE NM HYD	AP-16	-	1	.01960	18.50	.522	.49901	1.500	1.475	PER IMP=	.00
ROUTE	140.11	1	22	.01960	16.56	.522	.49908	1.550	1.320		
ROUTE	140.12	22	15	.01960	17.13	.522	.49909	1.600	1.366		
COMPUTE NM HYD	140.20	-	1	.08920	78.50	2.196	.46166	1.500	1.375	PER IMP=	.00
ADD HYD	AP-17UB	15& 1	2	.10880	91.60	2.718	.46840	1.550	1.316		
DIVIDE HYD	AP-17	2	17	.10880	98.93	2.935	.50587	1.550	1.421		
	140.23	AND	16	.10880	7.33	.217	.03747	1.550	.105		
ADD HYD	140.24	17&12	2	.61360	101.44	17.510	.53507	1.550	.258		

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION	
COMPUTE NM HYD	P-3	-	3	.00270	2.34	.069	.47912	1.500	1.354	PER IMP=	2.00
ADD HYD	AP-18	2& 3	10	.61630	103.70	17.578	.53480	1.550	.263		
ROUTE RESERVOIR	AP-18R	10	4	.61630	90.35	17.510	.53270	2.600	.229	AC-FT=	2.898
ROUTE	P-3R	4	2	.61630	90.34	17.510	.53270	2.600	.229		
COMPUTE NM HYD	P-2	-	3	.00360	3.11	.092	.47912	1.500	1.352	PER IMP=	2.00
ADD HYD	AP-18A	2& 3	10	.61990	90.42	17.601	.53238	2.600	.228		
ROUTE RESERVOIR	AP-18AR	10	5	.61990	50.71	16.871	.51028	3.400	.128	AC-FT=	6.005
ROUTE	P-2R	5	10	.61990	50.70	16.871	.51028	3.400	.128		
COMPUTE NM HYD	AP-19UB	-	1	.15250	85.35	3.692	.45390	1.600	.874	PER IMP=	.00
DIVIDE HYD	AP-19	1	17	.15250	89.62	3.876	.47660	1.600	.918		
	150.12	AND	16	.15250	4.27	.185	.02270	1.600	.044		
ADD HYD	150.13	10&17	10	.77240	89.64	20.747	.50363	1.600	.181		
COMPUTE NM HYD	P-1	-	3	.00290	2.51	.074	.47912	1.500	1.353	PER IMP=	2.00
ADD HYD	AP-20	10& 3	10	.77530	91.63	20.820	.50352	1.600	.185		
ROUTE RESERVOIR	150.14	10	5	.77530	50.75	20.820	.50350	3.700	.102	AC-FT=	1.729
COMPUTE NM HYD	AP-21&170.1	-	3	.06420	43.17	1.640	.47912	1.550	1.051	PER IMP=	2.00
FINISH											

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994 RUN DATE (MON/DAY/YR) =01/23/2001
INPUT FILE = EXT10EP.IN USER NO.= C_ANDRSN.I01

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION
*$ EXISTING CONDITIONS, WITHOUT I-40 INTERCEPTOR IN PLACE										
*$ 10 YEAR, 24 HOUR STORM EVENT										
*$										
*$ FILE NAME: EXT10EP.IN										
*$										
*$ THIS MODEL ANALYZES THE UNSER DIVERSION PONDS WITH THE EXISTING POND										
*$ OUTLET WORKS AND THE PROPOSED POND GRADING.										
*$										
*$ MASTER DRAINAGE PLAN FOR THE WEST SIDE TRANSIT FACILITY										
*$										
*$ THIS AHYMO FILE ANALYZES THE UNSER DIVERSION (DETENTION PONDS) ASSUMING										
*$ EXISTING CONDITIONS WITHOUT THE I-40 INTERCEPTOR IN PLACE.										
*$ NOTES: UNSER DIVERSION POND NO. 5 IS USED IN EXISTING CONDITIONS.										
*$ THE OUTLET RATING CURVES WERE ALSO BASED UPON TAILWATER										
*$ ELEVATIONS SHOWN ON THE UNSER DIVERSION (AS-BUILT)										
*$ CONSTRUCTION PLAN SET (PLANS BY CHUCK EASTERLING)										
*$ DRAINAGE BASINS SOUTH OF INTERSTATE 40 WERE TAKEN FROM										
*$ THE "MASTER DRAINAGE PLAN FOR THE ATRISCO BUSINESS PARK"										
*$ REVISED OCTOBER 1993, PREPARED BY EASTERLING & ASSOCIATES, INC.										
*$ DATE: 11/20/00										
START										TIME= .00
RAINFALL TYPE= 2										RAIN24= 1.770
*$ BEGIN OFFSITE WATERSHED NORTH OF INTERSTATE 40 - EXISTING CONDITION										
COMPUTE NM HYD	O-1E	-	3	.02420	3.43	.108	.08362	1.550	.221	PER IMP= .00
COMPUTE NM HYD	O-2E	-	4	.04220	4.48	.188	.08362	1.600	.166	PER IMP= .00
ADD HYD	AP-1	3& 4	10	.06640	7.65	.296	.08361	1.550	.180	
ROUTE	AP-1R	10	2	.06640	5.45	.296	.08362	1.700	.128	
COMPUTE NM HYD	O-3E	-	3	.01640	2.72	.086	.09818	1.550	.259	PER IMP= 1.00
ADD HYD	PARTAP-2	2& 3	10	.08280	7.02	.382	.08650	1.650	.132	
COMPUTE NM HYD	AP-7&O-5E	-	3	.10970	13.54	.660	.11274	1.600	.193	PER IMP= 2.00
DIVIDE HYD	PART_AP-7	3	98	.10970	13.54	.660	.11274	1.600	.193	
	PART_AP-7	AND	99	.00000	.00	.000	.00000	-.050	.000	
ROUTE	AP-7R	99	2	.00000	.00	.000	.00000	-.050	.000	
ADD HYD	AP-2	10& 2	10	.08280	7.02	.382	.08650	1.650	.132	
ROUTE	AP-2R	10	2	.08280	6.98	.382	.08651	1.700	.132	
COMPUTE NM HYD	I40-3	-	3	.00190	1.40	.060	.59326	1.500	1.154	PER IMP= 35.00
ADD HYD	AP-3	2& 3	20	.08470	7.72	.442	.09786	1.650	.142	
COMPUTE NM HYD	AP-4&O-4E	-	3	.00940	1.75	.057	.11274	1.550	.292	PER IMP= 2.00
ROUTE	AP-4R	3	2	.00940	1.84	.057	.11280	1.550	.306	
COMPUTE NM HYD	I40-4	-	3	.00120	.89	.038	.59326	1.500	1.156	PER IMP= 35.00
ADD HYD	AP-5	2& 3	10	.01060	2.64	.094	.16710	1.550	.389	
ADD HYD	AP-3+AP-5	20&10	10	.09530	9.60	.537	.10556	1.600	.157	
ROUTE	AP-6	10	12	.09530	9.61	.537	.10557	1.600	.158	
ROUTE	AP-7R-CUL	98	2	.10970	13.58	.660	.11274	1.600	.193	
COMPUTE NM HYD	I40-2	-	3	.00310	2.28	.098	.59326	1.500	1.152	PER IMP= 35.00
ADD HYD	AP-8	2& 3	20	.11280	15.19	.758	.12594	1.600	.210	
COMPUTE NM HYD	O-6E	-	3	.07320	6.49	.326	.08362	1.650	.138	PER IMP= .00
COMPUTE NM HYD	O-7E	-	4	.05230	7.41	.233	.08362	1.550	.221	PER IMP= .00
ADD HYD	AP-9	3& 4	10	.12550	13.19	.560	.08362	1.600	.164	

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION
ROUTE	AP-9R	10	2	.12550	13.27	.560	.08362	1.600	.165	
COMPUTE NM HYD	O-8E	-	3	.00990	14.68	.698	1.32131	1.500	2.318	PER IMP= 85.00
ADD HYD	AP-10	2& 3	10	.13540	26.66	1.257	.17411	1.550	.308	
ROUTE	AP-10R	10	2	.13540	21.80	1.257	.17411	1.650	.252	
COMPUTE NM HYD	O-9E	-	3	.03480	8.15	.290	.15643	1.500	.366	PER IMP= 5.00
ADD HYD	AP-11	2& 3	10	.17020	27.81	1.548	.17049	1.600	.255	
ROUTE	AP-11R	10	2	.17020	27.65	1.548	.17050	1.600	.254	
COMPUTE NM HYD	I40-1	-	3	.00750	5.52	.237	.59326	1.500	1.150	PER IMP= 35.00
ADD HYD	AP-12	2& 3	30	.17770	31.52	1.785	.18833	1.600	.277	
ROUTE	AP-8R	20	22	.11280	15.03	.758	.12595	1.650	.208	
COMPUTE NM HYD	130.70	-	1	.01696	3.27	.092	.10184	1.550	.301	PER IMP= .00
COMPUTE NM HYD	130.90	-	2	.00954	1.84	.052	.10184	1.550	.301	PER IMP= .00
ADD HYD	130.91	1& 2	1	.02650	5.11	.144	.10183	1.550	.301	
ROUTE	AP-12R	30	3	.17770	30.80	1.785	.18834	1.650	.271	
ADD HYD	130.72	1& 3	4	.20420	34.10	1.929	.17711	1.650	.261	
ADD HYD	AP-13	4&22	2	.31700	49.13	2.686	.15890	1.650	.242	
ROUTE	AP-13R	2	3	.31700	44.96	2.687	.15890	1.700	.222	
COMPUTE NM HYD	130.80	-	1	.03270	4.91	.146	.08362	1.550	.235	PER IMP= .00
ADD HYD	130.81	3& 1	10	.34970	47.65	2.832	.15186	1.700	.213	
COMPUTE NM HYD	P-5	-	3	.00290	.55	.017	.11274	1.550	.294	PER IMP= 2.00
ADD HYD	AP-15UB	10& 3	11	.35260	47.93	2.849	.15149	1.700	.212	
ROUTE	AP-6R	12	1	.09530	8.36	.537	.10557	1.750	.137	
COMPUTE NM HYD	120.50	-	2	.01620	2.55	.072	.08362	1.550	.246	PER IMP= .00
ADD HYD	120.51	2& 1	3	.11150	9.35	.609	.10238	1.700	.131	
DIVIDE HYD	AP-6A	3	15	.11150	9.81	.639	.10749	1.700	.138	
	120.53	AND	16	.11150	.47	.030	.00512	1.700	.007	
COMPUTE NM HYD	120.60	-	6	.03280	6.10	.197	.11274	1.550	.291	PER IMP= 2.00
DIVIDE HYD	AP-6B	6	17	.03280	6.16	.199	.11386	1.550	.294	

	120.62	AND	16	.03280	.06	.002	.00113	1.550	.003	
ADD HYD	120.63	15&17	10	.14430	13.69	.838	.10894	1.600	.148	
COMPUTE NM HYD	P-6	-	3	.00430	.81	.026	.11274	1.550	.293	PER IMP= 2.00
ADD HYD	AP-14	10& 3	18	.14860	14.37	.863	.10895	1.600	.151	
ROUTE RESERVOIR	AP-14R	18	2	.14860	4.37	.863	.10895	2.250	.046	AC-FT= .445
ROUTE	POND6ROUTE36	2	12	.14860	4.37	.863	.10895	2.250	.046	
ROUTE	POND6ROUTE48	12	22	.14860	4.37	.863	.10895	2.250	.046	
DIVIDE HYD	AP-15	11	17	.35260	51.28	3.048	.16209	1.700	.227	
	130.83	AND	16	.35260	3.36	.199	.01060	1.700	.015	
ROUTE RESERVOIR	AP-15R	17	5	.35260	23.55	3.048	.16209	2.050	.104	AC-FT= 1.291
ROUTE	P-5R	5	11	.35260	23.72	3.048	.16209	2.050	.105	
ADD HYD	POND-5R&6R	11&22	10	.50120	28.04	3.912	.14633	2.050	.087	
COMPUTE NM HYD	P-4	-	3	.00360	.68	.022	.11274	1.550	.293	PER IMP= 2.00
ADD HYD	AP-15A	10& 3	10	.50480	28.12	3.932	.14606	2.050	.087	
ROUTE RESERVOIR	AP-15AR	10	5	.50480	6.89	3.850	.14302	4.450	.021	AC-FT= 1.953
ROUTE	P-4R	5	12	.50480	6.89	3.850	.14302	4.450	.021	
COMPUTE NM HYD	AP-16	-	1	.01960	4.43	.124	.11841	1.550	.353	PER IMP= .00
ROUTE	140.11	1	22	.01960	3.49	.124	.11845	1.600	.278	
ROUTE	140.12	22	15	.01960	3.47	.124	.11845	1.650	.276	
COMPUTE NM HYD	140.20	-	1	.08920	16.45	.462	.09721	1.550	.288	PER IMP= .00
ADD HYD	AP-17UB	15& 1	2	.10880	18.28	.586	.10103	1.550	.262	
DIVIDE HYD	AP-17	2	17	.10880	19.74	.633	.10911	1.550	.283	
	140.23	AND	16	.10880	1.46	.047	.00808	1.550	.021	
ADD HYD	140.24	17&12	2	.61360	19.74	4.484	.13700	1.550	.050	

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 3 NOTATION
COMPUTE NM HYD	P-3	-	3	.00270	.51	.016	.11274	1.550	.294	PER IMP= 2.00
ADD HYD	AP-18	2& 3	10	.61630	20.25	4.499	.13687	1.550	.051	
ROUTE RESERVOIR	AP-18R	10	4	.61630	5.70	4.411	.13421	6.400	.014	AC-FT= 1.233
ROUTE	P-3R	4	2	.61630	5.70	4.411	.13420	6.400	.014	
COMPUTE NM HYD	P-2	-	3	.00360	.68	.022	.11274	1.550	.293	PER IMP= 2.00
ADD HYD	AP-18A	2& 3	10	.61990	5.70	4.432	.13405	6.400	.014	
ROUTE RESERVOIR	AP-18AR	10	5	.61990	3.51	3.312	.10017	10.100	.009	AC-FT= 2.245
ROUTE	P-2R	5	10	.61990	3.51	3.311	.10016	10.100	.009	
COMPUTE NM HYD	AP-19UB	-	1	.15250	16.02	.753	.09263	1.650	.164	PER IMP= .00
DIVIDE HYD	AP-19	1	17	.15250	16.82	.791	.09726	1.650	.172	
	150.12	AND	16	.15250	.80	.038	.00463	1.650	.008	
ADD HYD	150.13	10&17	10	.77240	16.83	4.102	.09959	1.650	.034	
COMPUTE NM HYD	P-1	-	3	.00290	.55	.017	.11274	1.550	.294	PER IMP= 2.00
ADD HYD	AP-20	10& 3	10	.77530	17.28	4.119	.09961	1.600	.035	
ROUTE RESERVOIR	150.14	10	5	.77530	3.68	4.032	.09751	2.150	.007	AC-FT= .501
COMPUTE NM HYD	AP-21&170.1	-	3	.06420	9.31	.386	.11274	1.550	.227	PER IMP= 2.00
FINISH										

AHYMO SUMMARY TABLE (AHYM0194) - AMAFCA Hydrologic Model - January, 1994 RUN DATE (MON/DAY/YR) =01/25/2001
INPUT FILE = EXT100EP.IN USER NO.= C_ANDRSN.I01

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION	PAGE = 1
*S EXISTING CONDITIONS, WITHOUT I-40 INTERCEPTOR IN PLACE											
*S 100 YEAR, 24 HOUR STORM EVENT											
*S											
*S FILE NAME: EXT100EP.IN											
*S											
*S THIS MODEL ANALYZES THE UNSER DIVERSION PONDS WITH THE EXISTING POND											
*S OUTLET WORKS AND THE PROPOSED POND GRADING.											
*S											
*S MASTER DRAINAGE PLAN FOR THE WEST SIDE TRANSIT FACILITY											
*S											
*S THIS AHYMO FILE ANALYZES THE UNSER DIVERSION (DETENTION PONDS) ASSUMING											
*S EXISTING CONDITIONS WITHOUT THE I-40 INTERCEPTOR IN PLACE.											
*S NOTES: UNSER DIVERSION POND NO. 5 IS USED IN EXISTING CONDITIONS.											
*S THE OUTLET RATING CURVES WERE ALSO BASED UPON TAILWATER											
*S ELEVATIONS SHOWN ON THE UNSER DIVERSION (AS-BUILT)											
*S CONSTRUCTION PLAN SET (PLANS BY CHUCK EASTERLING)											
*S DRAINAGE BASINS SOUTH OF INTERSTATE 40 WERE TAKEN FROM											
*S THE "MASTER DRAINAGE PLAN FOR THE ATRISCO BUSINESS PARK"											
*S REVISED OCTOBER 1993, PREPARED BY EASTERLING & ASSOCIATES, INC.											
*S DATE: 11/20/00											
START											TIME= .00
RAINFALL TYPE= 2											RAIN24= 2.660
*S BEGIN OFFSITE WATERSHED NORTH OF INTERSTATE 40 - EXISTING CONDITION											
COMPUTE NM HYD	0-1E	-	3	.02420	18.06	.567	.43936	1.550	1.166	PER IMP=	.00
COMPUTE NM HYD	0-2E	-	4	.04220	24.34	.989	.43936	1.600	.901	PER IMP=	.00
ADD HYD	AP-1	3& 4	10	.06640	41.88	1.556	.43936	1.550	.986		
ROUTE	AP-1R	10	2	.06640	36.11	1.556	.43938	1.650	.850		
COMPUTE NM HYD	0-3E	-	3	.01640	13.10	.402	.45924	1.500	1.248	PER IMP=	1.00
ADD HYD	PARTAP-2	2& 3	10	.08280	44.73	1.958	.44330	1.650	.844		
COMPUTE NM HYD	AP-7&0-5E	-	3	.10970	63.37	2.803	.47912	1.600	.903	PER IMP=	2.00
DIVIDE HYD	PART_AP-7	3	98	.08594	32.00	2.196	.47911	1.500	.582		
	PART_AP-7	AND	99	.02376	31.37	.607	.47911	1.600	2.062		
ROUTE	AP-7R	99	2	.02376	31.64	.608	.47944	1.600	2.081		
ADD HYD	AP-2	10& 2	10	.10656	76.00	2.565	.45135	1.600	1.114		
ROUTE	AP-2R	10	2	.10656	79.83	2.565	.45136	1.600	1.171		
COMPUTE NM HYD	I40-3	-	3	.00190	2.86	.115	1.13512	1.500	2.350	PER IMP=	35.00
ADD HYD	AP-3	2& 3	20	.10846	81.89	2.680	.46333	1.600	1.180		
COMPUTE NM HYD	AP-4&0-4E	-	3	.00940	8.11	.240	.47912	1.500	1.348	PER IMP=	2.00
ROUTE	AP-4R	3	2	.00940	8.17	.240	.47921	1.500	1.358		
COMPUTE NM HYD	I40-4	-	3	.00120	1.81	.073	1.13512	1.500	2.357	PER IMP=	35.00
ADD HYD	AP-5	2& 3	10	.01060	9.98	.313	.55337	1.500	1.471		
ADD HYD	AP-3+AP-5	20&10	10	.11906	89.90	2.993	.47135	1.600	1.180		
ROUTE	AP-6	10	12	.11906	87.97	2.993	.47135	1.650	1.154		
ROUTE	AP-7R-CUL	98	2	.08594	33.23	2.196	.47912	1.500	.604		
COMPUTE NM HYD	I40-2	-	3	.00310	4.65	.188	1.13512	1.500	2.346	PER IMP=	35.00
ADD HYD	AP-8	2& 3	20	.08904	37.88	2.384	.50195	1.500	.665		
COMPUTE NM HYD	0-6E	-	3	.07320	35.69	1.715	.43936	1.600	.762	PER IMP=	.00
COMPUTE NM HYD	0-7E	-	4	.05230	39.03	1.226	.43936	1.550	1.166	PER IMP=	.00
ADD HYD	AP-9	3& 4	10	.12550	71.47	2.941	.43936	1.550	.890		

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION	PAGE = 2
ROUTE	AP-9R	10	2	.12550	70.89	2.941	.43937	1.600	.883		
COMPUTE NM HYD	0-8E	-	3	.00990	23.00	1.124	2.12906	1.500	3.630	PER IMP=	85.00
ADD HYD	AP-10	2& 3	10	.13540	92.59	4.065	.56290	1.550	1.068		
ROUTE	AP-10R	10	2	.13540	89.10	4.065	.56291	1.600	1.028		
COMPUTE NM HYD	0-9E	-	3	.03480	31.99	1.000	.53875	1.500	1.436	PER IMP=	5.00
ADD HYD	AP-11	2& 3	10	.17020	114.14	5.065	.55797	1.600	1.048		
ROUTE	AP-11R	10	2	.17020	114.79	5.065	.55797	1.600	1.054		
COMPUTE NM HYD	I40-1	-	3	.00750	11.24	.454	1.13512	1.500	2.342	PER IMP=	35.00
ADD HYD	AP-12	2& 3	30	.17770	122.86	5.519	.58232	1.600	1.080		
ROUTE	AP-8R	20	22	.08904	38.28	2.384	.50196	1.550	.672		
COMPUTE NM HYD	130.70	-	1	.01696	15.02	.425	.46988	1.500	1.384	PER IMP=	.00
COMPUTE NM HYD	130.90	-	2	.00954	8.45	.239	.46988	1.500	1.385	PER IMP=	.00
ADD HYD	130.91	1& 2	1	.02650	23.48	.664	.46988	1.500	1.384		
ROUTE	AP-12R	30	3	.17770	121.65	5.519	.58233	1.600	1.070		
ADD HYD	130.72	1& 3	4	.20420	140.42	6.183	.56773	1.600	1.074		
ADD HYD	AP-13	4&22	2	.29324	176.26	8.567	.54776	1.600	.939		
ROUTE	AP-13R	2	3	.29324	172.47	8.567	.54776	1.650	.919		
COMPUTE NM HYD	130.80	-	1	.03270	25.40	.766	.43936	1.550	1.214	PER IMP=	.00
ADD HYD	130.81	3& 1	10	.32594	189.38	9.333	.53688	1.650	.908		
COMPUTE NM HYD	P-5	-	3	.00290	2.51	.074	.47912	1.500	1.353	PER IMP=	2.00
ADD HYD	AP-15UB	10& 3	11	.32884	190.90	9.406	.53633	1.650	.907		
ROUTE	AP-6R	12	1	.11906	84.75	2.993	.47135	1.650	1.112		
COMPUTE NM HYD	120.50	-	2	.01620	13.33	.380	.43936	1.500	1.286	PER IMP=	.00
ADD HYD	120.51	2& 1	3	.13526	92.90	3.373	.46752	1.650	1.073		
DIVIDE HYD	AP-6A	3	15	.13526	97.54	3.541	.49089	1.650	1.127		
	120.53	AND	16	.13526	4.64	.169	.02338	1.650	.054		
COMPUTE NM HYD	120.60	-	6	.03280	28.25	.838	.47912	1.500	1.346	PER IMP=	2.00
DIVIDE HYD	AP-6B	6	17	.03280	28.53	.846	.48390	1.500	1.359		

	120.62	AND	16	.03280	.28	.008	.00479	1.500	.013	
ADD HYD	120.63	15&17	10	.16806	114.84	4.388	.48953	1.650	1.068	
COMPUTE NM HYD	P-6	-	3	.00430	3.72	.110	.47912	1.500	1.350	PER IMP= 2.00
ADD HYD	AP-14	10& 3	18	.17236	117.09	4.498	.48926	1.650	1.061	
ROUTE RESERVOIR	AP-14R	18	2	.17236	8.98	4.498	.48926	2.450	.081	AC-FT= 3.376
ROUTE	POND6ROUTE36	2	12	.17236	8.98	4.498	.48926	2.450	.081	
ROUTE	POND6ROUTE48	12	22	.17236	8.98	4.498	.48926	2.450	.081	
DIVIDE HYD	AP-15	11	17	.32884	204.27	10.065	.57387	1.650	.971	
	130.83	AND	16	.32884	13.36	.658	.03754	1.650	.063	
ROUTE RESERVOIR	AP-15R	17	5	.32884	130.75	10.065	.57387	1.850	.621	AC-FT= 2.666
ROUTE	P-5R	5	11	.32884	131.18	10.065	.57388	1.800	.623	
ADD HYD	POND-5R&6R	11&22	10	.50120	139.22	14.562	.54477	1.800	.434	
COMPUTE NM HYD	P-4	-	3	.00360	3.11	.092	.47912	1.500	1.352	PER IMP= 2.00
ADD HYD	AP-15A	10& 3	10	.50480	140.01	14.654	.54429	1.800	.433	
ROUTE RESERVOIR	AP-15AR	10	5	.50480	93.42	14.571	.54122	2.200	.289	AC-FT= 4.619
ROUTE	P-4R	5	12	.50480	93.98	14.571	.54122	2.200	.291	
COMPUTE NM HYD	AP-16	-	1	.01960	18.50	.522	.49901	1.500	1.475	PER IMP= .00
ROUTE	140.11	1	22	.01960	16.56	.522	.49908	1.550	1.320	
ROUTE	140.12	22	15	.01960	17.13	.522	.49909	1.600	1.366	
COMPUTE NM HYD	140.20	-	1	.08920	78.50	2.196	.46166	1.500	1.375	PER IMP= .00
ADD HYD	AP-17UB	15& 1	2	.10880	91.60	2.718	.46840	1.550	1.316	
DIVIDE HYD	AP-17	2	17	.10880	98.93	2.935	.50587	1.550	1.421	
	140.23	AND	16	.10880	7.33	.217	.03747	1.550	.105	
ADD HYD	140.24	17&12	2	.61360	100.55	17.506	.53495	2.200	.256	

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION
COMPUTE NM HYD	P-3	-	3	.00270	2.34	.069	.47912	1.500	1.354	PER IMP= 2.00
ADD HYD	AP-18	2& 3	10	.61630	101.44	17.575	.53468	1.550	.257	
ROUTE RESERVOIR	AP-18R	10	4	.61630	88.19	17.465	.53134	2.300	.224	AC-FT= 2.780
ROUTE	P-3R	4	2	.61630	88.27	17.465	.53134	2.300	.224	
COMPUTE NM HYD	P-2	-	3	.00360	3.11	.092	.47912	1.500	1.352	PER IMP= 2.00
ADD HYD	AP-18A	2& 3	10	.61990	88.43	17.556	.53102	2.300	.223	
ROUTE RESERVOIR	AP-18AR	10	5	.61990	10.20	16.094	.48681	6.300	.026	AC-FT= 7.608
ROUTE	P-2R	5	10	.61990	10.20	16.093	.48677	6.350	.026	
COMPUTE NM HYD	AP-19UB	-	1	.15250	85.35	3.692	.45390	1.600	.874	PER IMP= .00
DIVIDE HYD	AP-19	1	17	.15250	89.62	3.876	.47660	1.600	.918	
	150.12	AND	16	.15250	4.27	.185	.02270	1.600	.044	
ADD HYD	150.13	10&17	10	.77240	89.63	19.970	.48476	1.600	.181	
COMPUTE NM HYD	P-1	-	3	.00290	2.51	.074	.47912	1.500	1.353	PER IMP= 2.00
ADD HYD	AP-20	10& 3	10	.77530	91.62	20.043	.48472	1.600	.185	
ROUTE RESERVOIR	150.14	10	5	.77530	9.61	19.636	.47487	10.250	.019	AC-FT= 3.475
COMPUTE NM HYD	AP-21&170.1	-	3	.06420	43.17	1.640	.47912	1.550	1.051	PER IMP= 2.00
FINISH										

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994 RUN DATE (MON/DAY/YR) =01/25/2001
INPUT FILE = EXT10PP.IN USER NO.= C_ANDRSN.I01

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION
										PAGE = 1
*S EXISTING CONDITIONS, WITHOUT I-40 INTERCEPTOR IN PLACE										
*S 10 YEAR, 24 HOUR STORM EVENT										
*S										
*S FILE NAME: EXT10PP.IN										
*S										
*S THIS MODEL ANALYZES THE UNSER DIVERSION PONDS WITH THE PROPOSED POND										
*S OUTLET WORKS AND THE PROPOSED POND GRADING.										
*S										
*S MASTER DRAINAGE PLAN FOR THE WEST SIDE TRANSIT FACILITY										
*S										
*S THIS AHYMO FILE ANALYZES THE UNSER DIVERSION (DETENTION PONDS) ASSUMING										
*S EXISTING CONDITIONS WITHOUT THE I-40 INTERCEPTOR IN PLACE.										
*S NOTES: UNSER DIVERSION POND NO. 5 IS USED IN EXISTING CONDITIONS.										
*S THE OUTLET RATING CURVES WERE ALSO BASED UPON TAILWATER										
*S ELEVATIONS SHOWN ON THE UNSER DIVERSION (AS-BUILT)										
*S CONSTRUCTION PLAN SET (PLANS BY CHUCK EASTERLING)										
*S DRAINAGE BASINS SOUTH OF INTERSTATE 40 WERE TAKEN FROM										
*S THE "MASTER DRAINAGE PLAN FOR THE ATRISCO BUSINESS PARK"										
*S REVISED OCTOBER 1993, PREPARED BY EASTERLING & ASSOCIATES, INC.										
*S DATE: 11/20/00										
START										TIME= .00
RAINFALL TYPE= 2										RAIN24= 1.770
*S BEGIN OFFSITE WATERSHED NORTH OF INTERSTATE 40 - EXISTING CONDITION										
COMPUTE NM HYD	O-1E	-	3	.02420	3.43	.108	.08362	1.550	.221	PER IMP= .00
COMPUTE NM HYD	O-2E	-	4	.04220	4.48	.188	.08362	1.600	.166	PER IMP= .00
ADD HYD	AP-1	3& 4	10	.06640	7.65	.296	.08361	1.550	.180	
ROUTE	AP-1R	10	2	.06640	5.45	.296	.08362	1.700	.128	
COMPUTE NM HYD	O-3E	-	3	.01640	2.72	.086	.09818	1.550	.259	PER IMP= 1.00
ADD HYD	PARTAP-2	2& 3	10	.08280	7.02	.382	.08650	1.650	.132	
COMPUTE NM HYD	AP-7&O-5E	-	3	.10970	13.54	.660	.11274	1.600	.193	PER IMP= 2.00
DIVIDE HYD	PART_AP-7	3	98	.10970	13.54	.660	.11274	1.600	.193	
	PART_AP-7	AND	99	.00000	.00	.000	.00000	-.050	.000	
ROUTE	AP-7R	99	2	.00000	.00	.000	.00000	-.050	.000	
ADD HYD	AP-2	10& 2	10	.08280	7.02	.382	.08650	1.650	.132	
ROUTE	AP-2R	10	2	.08280	6.98	.382	.08651	1.700	.132	
COMPUTE NM HYD	I40-3	-	3	.00190	1.40	.060	.59326	1.500	1.154	PER IMP= 35.00
ADD HYD	AP-3	2& 3	20	.08470	7.72	.442	.09786	1.650	.142	
COMPUTE NM HYD	AP-4&O-4E	-	3	.00940	1.75	.057	.11274	1.550	.292	PER IMP= 2.00
ROUTE	AP-4R	3	2	.00940	1.84	.057	.11280	1.550	.306	
COMPUTE NM HYD	I40-4	-	3	.00120	.89	.038	.59326	1.500	1.156	PER IMP= 35.00
ADD HYD	AP-5	2& 3	10	.01060	2.64	.094	.16710	1.550	.389	
ADD HYD	AP-3+AP-5	20&10	10	.09530	9.60	.537	.10556	1.600	.157	
ROUTE	AP-6	10	12	.09530	9.61	.537	.10557	1.600	.158	
ROUTE	AP-7R-CUL	98	2	.10970	13.58	.660	.11274	1.600	.193	
COMPUTE NM HYD	I40-2	-	3	.00310	2.28	.098	.59326	1.500	1.152	PER IMP= 35.00
ADD HYD	AP-8	2& 3	20	.11280	15.19	.758	.12594	1.600	.210	
COMPUTE NM HYD	O-6E	-	3	.07320	6.49	.326	.08362	1.650	.138	PER IMP= .00
COMPUTE NM HYD	O-7E	-	4	.05230	7.41	.233	.08362	1.550	.221	PER IMP= .00
ADD HYD	AP-9	3& 4	10	.12550	13.19	.560	.08362	1.600	.164	

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION
										PAGE = 2
ROUTE	AP-9R	10	2	.12550	13.27	.560	.08362	1.600	.165	
COMPUTE NM HYD	O-8E	-	3	.00990	14.68	.698	1.32131	1.500	2.318	PER IMP= 85.00
ADD HYD	AP-10	2& 3	10	.13540	26.66	1.257	.17411	1.550	.308	
ROUTE	AP-10R	10	2	.13540	21.80	1.257	.17411	1.650	.252	
COMPUTE NM HYD	O-9E	-	3	.03480	8.15	.290	.15643	1.500	.366	PER IMP= 5.00
ADD HYD	AP-11	2& 3	10	.17020	27.81	1.548	.17049	1.600	.255	
ROUTE	AP-11R	10	2	.17020	27.65	1.548	.17050	1.600	.254	
COMPUTE NM HYD	I40-1	-	3	.00750	5.52	.237	.59326	1.500	1.150	PER IMP= 35.00
ADD HYD	AP-12	2& 3	30	.17770	31.52	1.785	.18833	1.600	.277	
ROUTE	AP-8R	20	22	.11280	15.03	.758	.12595	1.650	.208	
COMPUTE NM HYD	130.70	-	1	.01696	3.27	.092	.10184	1.550	.301	PER IMP= .00
COMPUTE NM HYD	130.90	-	2	.00954	1.84	.052	.10184	1.550	.301	PER IMP= .00
ADD HYD	130.91	1& 2	1	.02650	5.11	.144	.10183	1.550	.301	
ROUTE	AP-12R	30	3	.17770	30.80	1.785	.18834	1.650	.271	
ADD HYD	130.72	1& 3	4	.20420	34.10	1.929	.17711	1.650	.261	
ADD HYD	AP-13	4&22	2	.31700	49.13	2.686	.15890	1.650	.242	
ROUTE	AP-13R	2	3	.31700	44.96	2.687	.15890	1.700	.222	
COMPUTE NM HYD	130.80	-	1	.03270	4.91	.146	.08362	1.550	.235	PER IMP= .00
ADD HYD	130.81	3& 1	10	.34970	47.65	2.832	.15186	1.700	.213	
COMPUTE NM HYD	P-5	-	3	.00290	.55	.017	.11274	1.550	.294	PER IMP= 2.00
ADD HYD	AP-15UB	10& 3	11	.35260	47.93	2.849	.15149	1.700	.212	
ROUTE	AP-6R	12	1	.09530	8.36	.537	.10557	1.750	.137	
COMPUTE NM HYD	120.50	-	2	.01620	2.55	.072	.08362	1.550	.246	PER IMP= .00
ADD HYD	120.51	2& 1	3	.11150	9.35	.609	.10238	1.700	.131	
DIVIDE HYD	AP-6A	3	15	.11150	9.81	.639	.10749	1.700	.138	
	120.53	AND	16	.11150	.47	.030	.00512	1.700	.007	
COMPUTE NM HYD	120.60	-	6	.03280	6.10	.197	.11274	1.550	.291	PER IMP= 2.00
DIVIDE HYD	AP-6B	6	17	.03280	6.16	.199	.11386	1.550	.294	

	120.62	AND	16	.03280	.06	.002	.00113	1.550	.003	
ADD HYD	120.63	15&17	10	.14430	13.69	.838	.10894	1.600	.148	
COMPUTE NM HYD	P-6	-	3	.00430	.81	.026	.11274	1.550	.293	PER IMP= 2.00
ADD HYD	AP-14	10& 3	18	.14860	14.37	.863	.10895	1.600	.151	
ROUTE RESERVOIR	AP-14R	18	2	.14860	5.77	.863	.10895	2.150	.061	AC-FT= .402
ROUTE	POND6ROUTE36	2	12	.14860	5.78	.863	.10895	2.150	.061	
ROUTE	POND6ROUTE48	12	22	.14860	5.78	.863	.10895	2.150	.061	
DIVIDE HYD	AP-15	11	17	.35260	51.28	3.048	.16209	1.700	.227	
	130.83	AND	16	.35260	3.36	.199	.01060	1.700	.015	
ROUTE RESERVOIR	AP-15R	17	5	.35260	23.55	3.048	.16209	2.050	.104	AC-FT= 1.291
ROUTE	P-5R	5	11	.35260	23.72	3.048	.16209	2.050	.105	
ADD HYD	POND-5R&6R	11&22	10	.50120	29.41	3.912	.14633	2.050	.092	
COMPUTE NM HYD	P-4	-	3	.00360	.68	.022	.11274	1.550	.293	PER IMP= 2.00
ADD HYD	AP-15A	10& 3	10	.50480	29.49	3.932	.14606	2.050	.091	
ROUTE RESERVOIR	AP-15AR	10	5	.50480	20.35	3.853	.14312	2.400	.063	AC-FT= .619
ROUTE	P-4R	5	12	.50480	20.36	3.853	.14312	2.400	.063	
COMPUTE NM HYD	AP-16	-	1	.01960	4.43	.124	.11841	1.550	.353	PER IMP= .00
ROUTE	140.11	1	22	.01960	3.49	.124	.11845	1.600	.278	
ROUTE	140.12	22	15	.01960	3.47	.124	.11845	1.650	.276	
COMPUTE NM HYD	140.20	-	1	.08920	16.45	.462	.09721	1.550	.288	PER IMP= .00
ADD HYD	AP-17UB	15& 1	2	.10880	18.28	.586	.10103	1.550	.262	
DIVIDE HYD	AP-17	2	17	.10880	19.74	.633	.10911	1.550	.283	
	140.23	AND	16	.10880	1.46	.047	.00808	1.550	.021	
ADD HYD	140.24	17&12	2	.61360	21.61	4.486	.13709	2.350	.055	

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 3 NOTATION
COMPUTE NM HYD	P-3	-	3	.00270	.51	.016	.11274	1.550	.294	PER IMP= 2.00
ADD HYD	AP-18	2& 3	10	.61630	21.64	4.502	.13696	2.350	.055	
ROUTE RESERVOIR	AP-18R	10	4	.61630	18.29	4.433	.13487	2.750	.046	AC-FT= .839
ROUTE	P-3R	4	2	.61630	18.29	4.433	.13486	2.750	.046	
COMPUTE NM HYD	P-2	-	3	.00360	.68	.022	.11274	1.550	.293	PER IMP= 2.00
ADD HYD	AP-18A	2& 3	10	.61990	18.31	4.454	.13471	2.750	.046	
ROUTE RESERVOIR	AP-18AR	10	5	.61990	11.34	3.407	.10307	4.050	.029	AC-FT= 1.823
ROUTE	P-2R	5	10	.61990	11.34	3.407	.10306	4.050	.029	
COMPUTE NM HYD	AP-19UB	-	1	.15250	16.02	.753	.09263	1.650	.164	PER IMP= .00
DIVIDE HYD	AP-19	1	17	.15250	16.82	.791	.09726	1.650	.172	
	150.12	AND	16	.15250	.80	.038	.00463	1.650	.008	
ADD HYD	150.13	10&17	10	.77240	16.83	4.199	.10192	1.650	.034	
COMPUTE NM HYD	P-1	-	3	.00290	.55	.017	.11274	1.550	.294	PER IMP= 2.00
ADD HYD	AP-20	10& 3	10	.77530	17.29	4.215	.10193	1.600	.035	
ROUTE RESERVOIR	150.14	10	5	.77530	10.61	4.164	.10070	4.650	.021	AC-FT= .550
COMPUTE NM HYD	AP-21&170.1	-	3	.06420	9.31	.386	.11274	1.550	.227	PER IMP= 2.00
FINISH										

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994 RUN DATE (MON/DAY/YR) =01/25/2001
INPUT FILE = EXT100PP.IN USER NO.= C_ANDRSN.I01

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION
*S EXISTING CONDITIONS, WITHOUT I-40 INTERCEPTOR IN PLACE										
*S 100 YEAR, 24 HOUR STORM EVENT										
*S										
*S FILE NAME: EXT100PP.IN										
*S										
*S THIS MODEL ANALYZES THE UNSER DIVERSION PONDS WITH THE PROPOSED POND										
*S OUTLET WORKS AND THE PROPOSED POND GRADING.										
*S										
*S MASTER DRAINAGE PLAN FOR THE WEST SIDE TRANSIT FACILITY										
*S										
*S THIS AHYMO FILE ANALYZES THE UNSER DIVERSION (DETENTION PONDS) ASSUMING										
*S EXISTING CONDITIONS WITHOUT THE I-40 INTERCEPTOR IN PLACE.										
*S NOTES: UNSER DIVERSION POND NO. 5 IS USED IN EXISTING CONDITIONS.										
*S THE OUTLET RATING CURVES WERE ALSO BASED UPON TAILWATER										
*S ELEVATIONS SHOWN ON THE UNSER DIVERSION (AS-BUILT)										
*S CONSTRUCTION PLAN SET (PLANS BY CHUCK EASTERLING)										
*S DRAINAGE BASINS SOUTH OF INTERSTATE 40 WERE TAKEN FROM										
*S THE "MASTER DRAINAGE PLAN FOR THE ATRISCO BUSINESS PARK"										
*S REVISED OCTOBER 1993, PREPARED BY EASTERLING & ASSOCIATES, INC.										
*S DATE: 11/20/00										
START										TIME= .00
RAINFALL TYPE= 2										RAIN24= 2.660
*S BEGIN OFFSITE WATERSHED NORTH OF INTERSTATE 40 - EXISTING CONDITION										
COMPUTE NM HYD	O-1E	-	3	.02420	18.06	.567	.43936	1.550	1.166	PER IMP= .00
COMPUTE NM HYD	O-2E	-	4	.04220	24.34	.989	.43936	1.600	.901	PER IMP= .00
ADD HYD	AP-1	3& 4	10	.06640	41.88	1.556	.43936	1.550	.986	
ROUTE	AP-1R	10	2	.06640	36.11	1.556	.43938	1.650	.850	
COMPUTE NM HYD	O-3E	-	3	.01640	13.10	.402	.45924	1.500	1.248	PER IMP= 1.00
ADD HYD	PARTAP-2	2& 3	10	.08280	44.73	1.958	.44330	1.650	.844	
COMPUTE NM HYD	AP-7&O-5E	-	3	.10970	63.37	2.803	.47912	1.600	.903	PER IMP= 2.00
DIVIDE HYD	PART_AP-7	3	98	.08594	32.00	2.196	.47911	1.500	.582	
	PART_AP-7	AND	99	.02376	31.37	.607	.47911	1.600	2.062	
ROUTE	AP-7R	99	2	.02376	31.64	.608	.47944	1.600	2.081	
ADD HYD	AP-2	10& 2	10	.10656	76.00	2.565	.45135	1.600	1.114	
ROUTE	AP-2R	10	2	.10656	79.83	2.565	.45136	1.600	1.171	
COMPUTE NM HYD	I40-3	-	3	.00190	2.86	.115	1.13512	1.500	2.350	PER IMP= 35.00
ADD HYD	AP-3	2& 3	20	.10846	81.89	2.680	.46333	1.600	1.180	
COMPUTE NM HYD	AP-4&O-4E	-	3	.00940	8.11	.240	.47912	1.500	1.348	PER IMP= 2.00
ROUTE	AP-4R	3	2	.00940	8.17	.240	.47921	1.500	1.358	
COMPUTE NM HYD	I40-4	-	3	.00120	1.81	.073	1.13512	1.500	2.357	PER IMP= 35.00
ADD HYD	AP-5	2& 3	10	.01060	9.98	.313	.55337	1.500	1.471	
ADD HYD	AP-3+AP-5	20&10	10	.11906	89.90	2.993	.47135	1.600	1.180	
ROUTE	AP-6	10	12	.11906	87.97	2.993	.47135	1.650	1.154	
ROUTE	AP-7R-CUL	98	2	.08594	33.23	2.196	.47912	1.500	.604	
COMPUTE NM HYD	I40-2	-	3	.00310	4.65	.188	1.13512	1.500	2.346	PER IMP= 35.00
ADD HYD	AP-8	2& 3	20	.08904	37.88	2.384	.50195	1.500	.665	
COMPUTE NM HYD	O-6E	-	3	.07320	35.69	1.715	.43936	1.600	.762	PER IMP= .00
COMPUTE NM HYD	O-7E	-	4	.05230	39.03	1.226	.43936	1.550	1.166	PER IMP= .00
ADD HYD	AP-9	3& 4	10	.12550	71.47	2.941	.43936	1.550	.890	

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION
ROUTE	AP-9R	10	2	.12550	70.89	2.941	.43937	1.600	.883	
COMPUTE NM HYD	O-8E	-	3	.00990	23.00	1.124	2.12906	1.500	3.630	PER IMP= 85.00
ADD HYD	AP-10	2& 3	10	.13540	92.59	4.065	.56290	1.550	1.068	
ROUTE	AP-10R	10	2	.13540	89.10	4.065	.56291	1.600	1.028	
COMPUTE NM HYD	O-9E	-	3	.03480	31.99	1.000	.53875	1.500	1.436	PER IMP= 5.00
ADD HYD	AP-11	2& 3	10	.17020	114.14	5.065	.55797	1.600	1.048	
ROUTE	AP-11R	10	2	.17020	114.79	5.065	.55797	1.600	1.054	
COMPUTE NM HYD	I40-1	-	3	.00750	11.24	.454	1.13512	1.500	2.342	PER IMP= 35.00
ADD HYD	AP-12	2& 3	30	.17770	122.86	5.519	.58232	1.600	1.080	
ROUTE	AP-8R	20	22	.08904	38.28	2.384	.50196	1.550	.672	
COMPUTE NM HYD	130.70	-	1	.01696	15.02	.425	.46988	1.500	1.384	PER IMP= .00
COMPUTE NM HYD	130.90	-	2	.00954	8.45	.239	.46988	1.500	1.385	PER IMP= .00
ADD HYD	130.91	1& 2	1	.02650	23.48	.664	.46988	1.500	1.384	
ROUTE	AP-12R	30	3	.17770	121.65	5.519	.58233	1.600	1.070	
ADD HYD	130.72	1& 3	4	.20420	140.42	6.183	.56773	1.600	1.074	
ADD HYD	AP-13	4&22	2	.29324	176.26	8.567	.54776	1.600	.939	
ROUTE	AP-13R	2	3	.29324	172.47	8.567	.54776	1.650	.919	
COMPUTE NM HYD	130.80	-	1	.03270	25.40	.766	.43936	1.550	1.214	PER IMP= .00
ADD HYD	130.81	3& 1	10	.32594	189.38	9.333	.53688	1.650	.908	
COMPUTE NM HYD	P-5	-	3	.00290	2.51	.074	.47912	1.500	1.353	PER IMP= 2.00
ADD HYD	AP-15UB	10& 3	11	.32884	190.90	9.406	.53633	1.650	.907	
ROUTE	AP-6R	12	1	.11906	84.75	2.993	.47135	1.650	1.112	
COMPUTE NM HYD	120.50	-	2	.01620	13.33	.380	.43936	1.500	1.286	PER IMP= .00
ADD HYD	120.51	2& 1	3	.13526	92.90	3.373	.46752	1.650	1.073	
DIVIDE HYD	AP-6A	3	15	.13526	97.54	3.541	.49089	1.650	1.127	
	120.53	AND	16	.13526	4.64	.169	.02338	1.650	.054	
COMPUTE NM HYD	120.60	-	6	.03280	28.25	.838	.47912	1.500	1.346	PER IMP= 2.00
DIVIDE HYD	AP-6B	6	17	.03280	28.53	.846	.48390	1.500	1.359	

	120.62	AND	16	.03280	.28	.008	.00479	1.500	.013	
ADD HYD	120.63	15&17	10	.16806	114.84	4.388	.48953	1.650	1.068	
COMPUTE NM HYD	P-6	-	3	.00430	3.72	.110	.47912	1.500	1.350	PER IMP= 2.00
ADD HYD	AP-14	10& 3	18	.17236	117.09	4.498	.48926	1.650	1.061	
ROUTE RESERVOIR	AP-14R	18	2	.17236	33.14	4.498	.48926	1.950	.300	AC-FT= 2.440
ROUTE	POND6ROUTE36	2	12	.17236	33.14	4.498	.48926	1.950	.300	
ROUTE	POND6ROUTE48	12	22	.17236	33.15	4.498	.48926	1.950	.301	
DIVIDE HYD	AP-15	11	17	.32884	204.27	10.065	.57387	1.650	.971	
	130.83	AND	16	.32884	13.36	.658	.03754	1.650	.063	
ROUTE RESERVOIR	AP-15R	17	5	.32884	130.75	10.065	.57387	1.850	.621	AC-FT= 2.666
ROUTE	P-5R	5	11	.32884	131.18	10.065	.57388	1.800	.623	
ADD HYD	POND-5R&6R	11&22	10	.50120	163.37	14.562	.54477	1.900	.509	
COMPUTE NM HYD	P-4	-	3	.00360	3.11	.092	.47912	1.500	1.352	PER IMP= 2.00
ADD HYD	AP-15A	10& 3	10	.50480	163.82	14.654	.54429	1.900	.507	
ROUTE RESERVOIR	AP-15AR	10	5	.50480	93.54	14.574	.54135	2.250	.290	AC-FT= 4.632
ROUTE	P-4R	5	12	.50480	93.57	14.574	.54135	2.250	.290	
COMPUTE NM HYD	AP-16	-	1	.01960	18.50	.522	.49901	1.500	1.475	PER IMP= .00
ROUTE	140.11	1	22	.01960	16.56	.522	.49908	1.550	1.320	
ROUTE	140.12	22	15	.01960	17.13	.522	.49909	1.600	1.366	
COMPUTE NM HYD	140.20	-	1	.08920	78.50	2.196	.46166	1.500	1.375	PER IMP= .00
ADD HYD	AP-17UB	15& 1	2	.10880	91.60	2.718	.46840	1.550	1.316	
DIVIDE HYD	AP-17	2	17	.10880	98.93	2.935	.50587	1.550	1.421	
	140.23	AND	16	.10880	7.33	.217	.03747	1.550	.105	
ADD HYD	140.24	17&12	2	.61360	100.56	17.510	.53505	1.550	.256	

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION
COMPUTE NM HYD	P-3	-	3	.00270	2.34	.069	.47912	1.500	1.354	PER IMP= 2.00
ADD HYD	AP-18	2& 3	10	.61630	102.82	17.578	.53478	1.550	.261	
ROUTE RESERVOIR	AP-18R	10	4	.61630	89.08	17.509	.53269	2.550	.226	AC-FT= 2.829
ROUTE	P-3R	4	2	.61630	89.06	17.509	.53269	2.550	.226	
COMPUTE NM HYD	P-2	-	3	.00360	3.11	.092	.47912	1.500	1.352	PER IMP= 2.00
ADD HYD	AP-18A	2& 3	10	.61990	89.15	17.601	.53237	2.550	.225	
ROUTE RESERVOIR	AP-18AR	10	5	.61990	44.89	16.554	.50070	3.600	.113	AC-FT= 6.543
ROUTE	P-2R	5	10	.61990	44.89	16.554	.50070	3.600	.113	
COMPUTE NM HYD	AP-19UB	-	1	.15250	85.35	3.692	.45390	1.600	.874	PER IMP= .00
DIVIDE HYD	AP-19	1	17	.15250	89.62	3.876	.47660	1.600	.918	
	150.12	AND	16	.15250	4.27	.185	.02270	1.600	.044	
ADD HYD	150.13	10&17	10	.77240	89.64	20.430	.49594	1.600	.181	
COMPUTE NM HYD	P-1	-	3	.00290	2.51	.074	.47912	1.500	1.353	PER IMP= 2.00
ADD HYD	AP-20	10& 3	10	.77530	91.63	20.503	.49586	1.600	.185	
ROUTE RESERVOIR	150.14	10	5	.77530	45.54	20.452	.49463	3.900	.092	AC-FT= 2.153
COMPUTE NM HYD	AP-21&170.1	-	3	.06420	43.17	1.640	.47912	1.550	1.051	PER IMP= 2.00
FINISH										

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994 RUN DATE (MON/DAY/YR) =01/30/2001
INPUT FILE = WTF10EE.IN USER NO.= C_ANDRSN.I01

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION
										PAGE = 1
*S EXISTING CONDITIONS, WITHOUT I-40 INTERCEPTOR IN PLACE AND										
*S WITH THE WEST SIDE TRANSIT FACILITY FULLY DEVELOPED										
*S 10 YEAR, 24 HOUR STORM EVENT										
*S										
*S FILE NAME: WTF10EE.IN										
*S										
*S THIS MODEL ANALYZES THE UNSER DIVERSION PONDS WITH THE EXISTING POND										
*S OUTLET WORKS AND THE EXISTING POND GRADING.										
*S										
*S MASTER DRAINAGE PLAN FOR THE WEST SIDE TRANSIT FACILITY										
*S										
*S THIS AHYMO FILE ANALYZES THE UNSER DIVERSION (DETENTION PONDS) ASSUMING										
*S EXISTING CONDITIONS WITHOUT THE I-40 INTERCEPTOR IN PLACE										
*S AND THE WEST SIDE TRANSIT FACILITY FULLY DEVELOPED.										
*S										
*S NOTES: UNSER DIVERSION POND NO. 5 IS USED IN EXISTING CONDITIONS.										
*S THE OUTLET RATING CURVES WERE ALSO BASED UPON TAILWATER										
*S ELEVATIONS SHOWN ON THE UNSER DIVERSION (AS-BUILT)										
*S CONSTRUCTION PLAN SET (PLANS BY CHUCK EASTERLING).										
*S DRAINAGE BASINS SOUTH OF INTERSTATE 40 (WITH THE EXCEPTION										
*S OF THE WEST SIDE TRANSIT FACILITY AREA) WERE TAKEN FROM										
*S THE "MASTER DRAINAGE PLAN FOR THE ATRISCO BUSINESS PARK"										
*S REVISED OCTOBER 1993, PREPARED BY EASTERLING & ASSOCIATES, INC.										
*S DATE: 11/21/00										
START										TIME= .00
RAINFALL TYPE= 2										RAIN24= 1.770
*S BEGIN OFFSITE WATERSHED NORTH OF INTERSTATE 40 - EXISTING CONDITION										
COMPUTE NM HYD	O-1E	-	3	.02420	3.43	.108	.08362	1.550	.221	PER IMP= .00
COMPUTE NM HYD	O-2E	-	4	.04220	4.48	.188	.08362	1.600	.166	PER IMP= .00
ADD HYD	AP-1	3& 4	10	.06640	7.65	.296	.08361	1.550	.180	
ROUTE	AP-1R	10	2	.06640	5.45	.296	.08362	1.700	.128	
COMPUTE NM HYD	O-3E	-	3	.01640	2.72	.086	.09818	1.550	.259	PER IMP= 1.00
ADD HYD	PARTAP-2	2& 3	10	.08280	7.02	.382	.08650	1.650	.132	
COMPUTE NM HYD	AP-7&O-5E	-	3	.10970	13.54	.660	.11274	1.600	.193	PER IMP= 2.00
DIVIDE HYD	PART_AP-7	3	98	.10970	13.54	.660	.11274	1.600	.193	
	PART_AP-7	AND	99	.00000	.00	.000	.00000	-.050	.000	
ROUTE	AP-7R	99	2	.00000	.00	.000	.00000	-.050	.000	
ADD HYD	AP-2	10& 2	10	.08280	7.02	.382	.08650	1.650	.132	
ROUTE	AP-2R	10	2	.08280	6.98	.382	.08651	1.700	.132	
COMPUTE NM HYD	I40-3	-	3	.00190	1.40	.060	.59326	1.500	1.154	PER IMP= 35.00
ADD HYD	AP-3	2& 3	20	.08470	7.72	.442	.09786	1.650	.142	
COMPUTE NM HYD	AP-4&O-4E	-	3	.00940	1.75	.057	.11274	1.550	.292	PER IMP= 2.00
ROUTE	AP-4R	3	2	.00940	1.84	.057	.11280	1.550	.306	
COMPUTE NM HYD	I40-4	-	3	.00120	.89	.038	.59326	1.500	1.156	PER IMP= 35.00
ADD HYD	AP-5	2& 3	10	.01060	2.64	.094	.16710	1.550	.389	
ADD HYD	AP-3+AP-5	20&10	10	.09530	9.60	.537	.10556	1.600	.157	
ROUTE	PARTAP-6	10	12	.09530	9.61	.537	.10557	1.600	.158	
ROUTE	AP-7R-CUL	98	2	.10970	13.58	.660	.11274	1.600	.193	
COMPUTE NM HYD	I40-2	-	3	.00310	2.28	.098	.59326	1.500	1.152	PER IMP= 35.00

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION
										PAGE = 2
ADD HYD	AP-8	2& 3	20	.11280	15.19	.758	.12594	1.600	.210	
COMPUTE NM HYD	O-6E	-	3	.07320	6.49	.326	.08362	1.650	.138	PER IMP= .00
COMPUTE NM HYD	O-7E	-	4	.05230	7.41	.233	.08362	1.550	.221	PER IMP= .00
ADD HYD	AP-9	3& 4	10	.12550	13.19	.560	.08362	1.600	.164	
ROUTE	AP-9R	10	2	.12550	13.27	.560	.08362	1.600	.165	
COMPUTE NM HYD	O-8E	-	3	.00990	14.68	.698	1.32131	1.500	2.318	PER IMP= 85.00
ADD HYD	AP-10	2& 3	10	.13540	26.66	1.257	.17411	1.550	.308	
ROUTE	AP-10R	10	2	.13540	21.80	1.257	.17411	1.650	.252	
COMPUTE NM HYD	O-9E	-	3	.03480	8.15	.290	.15643	1.500	.366	PER IMP= 5.00
ADD HYD	AP-11	2& 3	10	.17020	27.81	1.548	.17049	1.600	.255	
ROUTE	AP-11R	10	2	.17020	27.65	1.548	.17050	1.600	.254	
COMPUTE NM HYD	I40-1	-	3	.00750	5.52	.237	.59326	1.500	1.150	PER IMP= 35.00
ADD HYD	AP-12	2& 3	30	.17770	31.52	1.785	.18833	1.600	.277	
ROUTE	AP-8R	20	22	.11280	15.03	.758	.12595	1.650	.208	
COMPUTE NM HYD	130.70	-	1	.01696	3.27	.092	.10184	1.550	.301	PER IMP= .00
COMPUTE NM HYD	130.90	-	2	.00954	1.84	.052	.10184	1.550	.301	PER IMP= .00
ADD HYD	130.91	1& 2	1	.02650	5.11	.144	.10183	1.550	.301	
ROUTE	AP-12R	30	3	.17770	30.80	1.785	.18834	1.650	.271	
ADD HYD	130.72	1& 3	4	.20420	34.10	1.929	.17711	1.650	.261	
ADD HYD	AP-13	4&22	2	.31700	49.13	2.686	.15890	1.650	.242	
ROUTE	AP-13R	2	3	.31700	44.96	2.687	.15890	1.700	.222	
COMPUTE NM HYD	130.80	-	1	.03010	4.52	.134	.08362	1.550	.235	PER IMP= .00
ADD HYD	130.81	3& 1	10	.34710	47.44	2.821	.15237	1.700	.214	
COMPUTE NM HYD	P-5	-	3	.00290	.55	.017	.11274	1.550	.294	PER IMP= 2.00
ADD HYD	AP-15UB	10& 3	11	.35000	47.71	2.837	.15199	1.700	.213	
COMPUTE NM HYD	120.50	-	3	.00520	.82	.023	.08362	1.550	.246	PER IMP= .00
ADD HYD	AP-6	3&12	10	.10050	10.32	.560	.10443	1.600	.160	
ROUTE	AP-6R	10	2	.10050	10.45	.560	.10444	1.650	.162	

COMPUTE NM HYD	A-1D	-	3	.02400	39.69	1.802	1.40796	1.500	2.584	PER IMP=	90.00
ADD HYD	AP-6A	2& 3	10	.12450	43.13	2.362	.35571	1.500	.541		
COMPUTE NM HYD	AP-6B&A-2D	-	3	.01250	18.04	.801	1.20080	1.500	2.256	PER IMP=	76.00
COMPUTE NM HYD	120.5A	-	4	.00200	.32	.009	.08362	1.550	.247	PER IMP=	.00
ADD HYD	AP-6B+120.5A	3& 4	20	.01450	18.35	.809	1.04667	1.500	1.977		
ADD HYD	AP-6A+	10&20	10	.13900	61.48	3.171	.42779	1.500	.691		
COMPUTE NM HYD	P-6	-	3	.00430	.81	.026	.11274	1.550	.293	PER IMP=	2.00
ADD HYD	AP-14	10& 3	18	.14330	62.27	3.196	.41823	1.500	.679		
ROUTE RESERVOIR	AP-14R	18	2	.14330	8.05	3.196	.41823	2.200	.088	AC-FT=	1.888
ROUTE	POND6ROUTE36	2	12	.14330	8.05	3.196	.41823	2.200	.088		
ROUTE	POND6ROUTE48	12	22	.14330	8.05	3.196	.41823	2.200	.088		
DIVIDE HYD	AP-15	11	17	.35000	51.05	3.036	.16263	1.700	.228		
	130.83	AND	16	.35000	3.34	.199	.01064	1.700	.015		
ROUTE RESERVOIR	AP-15R	17	5	.35000	23.22	3.036	.16263	2.050	.104	AC-FT=	1.289
ROUTE	P-5R	5	11	.35000	23.50	3.036	.16263	2.050	.105		
ADD HYD	POND-5R&6R	11&22	10	.49330	31.46	6.232	.23688	2.050	.100		
COMPUTE NM HYD	P-4	-	3	.00360	.68	.022	.11274	1.550	.293	PER IMP=	2.00
ADD HYD	AP-15A	10& 3	10	.49690	31.54	6.253	.23594	2.050	.099		
ROUTE RESERVOIR	AP-15AR	10	5	.49690	8.29	6.169	.23279	5.150	.026	AC-FT=	2.925
ROUTE	P-4R	5	12	.49690	8.29	6.169	.23279	5.150	.026		
COMPUTE NM HYD	AP-16	-	1	.01960	4.43	.124	.11841	1.550	.353	PER IMP=	.00
ROUTE	140.11	1	22	.01960	3.49	.124	.11845	1.600	.278		
ROUTE	140.12	22	15	.01960	3.47	.124	.11845	1.650	.276		
COMPUTE NM HYD	140.20	-	1	.08920	16.45	.462	.09721	1.550	.288	PER IMP=	.00

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION	PAGE = 3
ADD HYD	AP-17UB	15& 1	2	.10880	18.28	.586	.10103	1.550	.262		
DIVIDE HYD	AP-17	2	17	.10880	19.74	.633	.10911	1.550	.283		
	140.23	AND	16	.10880	1.46	.047	.00808	1.550	.021		
ADD HYD	140.24	17&12	2	.60570	19.89	6.802	.21057	1.550	.051		
COMPUTE NM HYD	P-3	-	3	.00270	.51	.016	.11274	1.550	.294	PER IMP=	2.00
ADD HYD	AP-18	2& 3	10	.60840	20.40	6.818	.21011	1.550	.052		
ROUTE RESERVOIR	AP-18R	10	4	.60840	6.86	6.699	.20644	7.700	.018	AC-FT=	1.597
ROUTE	P-3R	4	2	.60840	6.86	6.698	.20643	7.700	.018		
COMPUTE NM HYD	P-2	-	3	.00360	.68	.022	.11274	1.550	.293	PER IMP=	2.00
ADD HYD	AP-18A	2& 3	10	.61200	6.86	6.719	.20585	7.700	.018		
ROUTE RESERVOIR	AP-18AR	10	5	.61200	5.41	5.891	.18050	11.700	.014	AC-FT=	2.315
ROUTE	P-2R	5	10	.61200	5.41	5.891	.18048	11.700	.014		
COMPUTE NM HYD	AP-19UB	-	1	.15250	16.02	.753	.09263	1.650	.164	PER IMP=	.00
DIVIDE HYD	AP-19	1	17	.15250	16.82	.791	.09726	1.650	.172		
	150.12	AND	16	.15250	.80	.038	.00463	1.650	.008		
ADD HYD	150.13	10&17	10	.76450	16.83	6.682	.16388	1.650	.034		
COMPUTE NM HYD	P-1	-	3	.00290	.55	.017	.11274	1.550	.294	PER IMP=	2.00
ADD HYD	AP-20	10& 3	10	.76740	17.29	6.698	.16366	1.600	.035		
ROUTE RESERVOIR	150.14	10	5	.76740	5.04	6.664	.16282	13.650	.010	AC-FT=	.550
COMPUTE NM HYD	AP-21&G-1D	-	3	.02290	3.61	.102	.08362	1.550	.246	PER IMP=	.00
COMPUTE NM HYD	AP-22&G-2D	-	4	.00310	4.90	.215	1.30254	1.500	2.469	PER IMP=	82.00
ROUTE	AP-22R	4	12	.00310	4.76	.215	1.30270	1.500	2.398		
COMPUTE NM HYD	AP-23&G-3D	-	5	.03510	4.10	.157	.08362	1.600	.183	PER IMP=	.00
COMPUTE NM HYD	G-4D	-	6	.00330	5.08	.220	1.24983	1.500	2.407	PER IMP=	78.00
ADD HYD	AP-24	12& 6	10	.00640	9.84	.435	1.27528	1.500	2.403		
FINISH											

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994 RUN DATE (MON/DAY/YR) =01/26/2001
INPUT FILE = WTF100EE.IN USER NO.= C_ANDRSN.I01

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION
*S EXISTING CONDITIONS, WITHOUT I-40 INTERCEPTOR IN PLACE AND										
*S WITH THE WEST SIDE TRANSIT FACILITY FULLY DEVELOPED										
*S 100 YEAR, 24 HOUR STORM EVENT										
*S										
*S FILE NAME: WTF100EE.IN										
*S										
*S THIS MODEL ANALYZES THE UNSER DIVERSION PONDS WITH THE EXISTING POND										
*S OUTLET WORKS AND THE EXISTING POND GRADING.										
*S										
*S MASTER DRAINAGE PLAN FOR THE WEST SIDE TRANSIT FACILITY										
*S										
*S THIS AHYMO FILE ANALYZES THE UNSER DIVERSION (DETENTION PONDS) ASSUMING										
*S EXISTING CONDITIONS WITHOUT THE I-40 INTERCEPTOR IN PLACE										
*S AND THE WEST SIDE TRANSIT FACILITY FULLY DEVELOPED.										
*S										
*S NOTES: UNSER DIVERSION POND NO. 5 IS USED IN EXISTING CONDITIONS.										
*S THE OUTLET RATING CURVES WERE ALSO BASED UPON TAILWATER										
*S ELEVATIONS SHOWN ON THE UNSER DIVERSION (AS-BUILT)										
*S CONSTRUCTION PLAN SET (PLANS BY CHUCK EASTERLING).										
*S DRAINAGE BASINS SOUTH OF INTERSTATE 40 (WITH THE EXCEPTION										
*S OF THE WEST SIDE TRANSIT FACILITY AREA) WERE TAKEN FROM										
*S THE "MASTER DRAINAGE PLAN FOR THE ATRISCO BUSINESS PARK"										
*S REVISED OCTOBER 1993, PREPARED BY EASTERLING & ASSOCIATES, INC.										
*S DATE: 11/21/00										
START										TIME= .00
RAINFALL TYPE= 2										RAIN24= 2.660
*S BEGIN OFFSITE WATERSHED NORTH OF INTERSTATE 40 - EXISTING CONDITION										
COMPUTE NM HYD	O-1E	-	3	.02420	18.06	.567	.43936	1.550	1.166	PER IMP= .00
COMPUTE NM HYD	O-2E	-	4	.04220	24.34	.989	.43936	1.600	.901	PER IMP= .00
ADD HYD	AP-1	3& 4	10	.06640	41.88	1.556	.43936	1.550	.986	
ROUTE	AP-1R	10	2	.06640	36.11	1.556	.43938	1.650	.850	
COMPUTE NM HYD	O-3E	-	3	.01640	13.10	.402	.45924	1.500	1.248	PER IMP= 1.00
ADD HYD	PARTAP-2	2& 3	10	.08280	44.73	1.958	.44330	1.650	.844	
COMPUTE NM HYD	AP-7&O-5E	-	3	.10970	63.37	2.803	.47912	1.600	.903	PER IMP= 2.00
DIVIDE HYD	PART_AP-7	3	98	.08594	32.00	2.196	.47911	1.500	.582	
	PART_AP-7	AND	99	.02376	31.37	.607	.47911	1.600	2.062	
ROUTE	AP-7R	99	2	.02376	31.64	.608	.47944	1.600	2.081	
ADD HYD	AP-2	10& 2	10	.10656	76.00	2.565	.45135	1.600	1.114	
ROUTE	AP-2R	10	2	.10656	79.83	2.565	.45136	1.600	1.171	
COMPUTE NM HYD	I40-3	-	3	.00190	2.86	.115	1.13512	1.500	2.350	PER IMP= 35.00
ADD HYD	AP-3	2& 3	20	.10846	81.89	2.680	.46333	1.600	1.180	
COMPUTE NM HYD	AP-4&O-4E	-	3	.00940	8.11	.240	.47912	1.500	1.348	PER IMP= 2.00
ROUTE	AP-4R	3	2	.00940	8.17	.240	.47921	1.500	1.358	
COMPUTE NM HYD	I40-4	-	3	.00120	1.81	.073	1.13512	1.500	2.357	PER IMP= 35.00
ADD HYD	AP-5	2& 3	10	.01060	9.98	.313	.55337	1.500	1.471	
ADD HYD	AP-3+AP-5	20&10	10	.11906	89.90	2.993	.47135	1.600	1.180	
ROUTE	PARTAP-6	10	12	.11906	87.97	2.993	.47135	1.650	1.154	
ROUTE	AP-7R-CUL	98	2	.08594	33.23	2.196	.47912	1.500	.604	
COMPUTE NM HYD	I40-2	-	3	.00310	4.65	.188	1.13512	1.500	2.346	PER IMP= 35.00

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION
ADD HYD	AP-8	2& 3	20	.08904	37.88	2.384	.50195	1.500	.665	
COMPUTE NM HYD	O-6E	-	3	.07320	35.69	1.715	.43936	1.600	.762	PER IMP= .00
COMPUTE NM HYD	O-7E	-	4	.05230	39.03	1.226	.43936	1.550	1.166	PER IMP= .00
ADD HYD	AP-9	3& 4	10	.12550	71.47	2.941	.43936	1.550	.890	
ROUTE	AP-9R	10	2	.12550	70.89	2.941	.43937	1.600	.883	
COMPUTE NM HYD	O-8E	-	3	.00990	23.00	1.124	2.12906	1.500	3.630	PER IMP= 85.00
ADD HYD	AP-10	2& 3	10	.13540	92.59	4.065	.56290	1.550	1.068	
ROUTE	AP-10R	10	2	.13540	89.10	4.065	.56291	1.600	1.028	
COMPUTE NM HYD	O-9E	-	3	.03480	31.99	1.000	.53875	1.500	1.436	PER IMP= 5.00
ADD HYD	AP-11	2& 3	10	.17020	114.14	5.065	.55797	1.600	1.048	
ROUTE	AP-11R	10	2	.17020	114.79	5.065	.55797	1.600	1.054	
COMPUTE NM HYD	I40-1	-	3	.00750	11.24	.454	1.13512	1.500	2.342	PER IMP= 35.00
ADD HYD	AP-12	2& 3	30	.17770	122.86	5.519	.58232	1.600	1.080	
ROUTE	AP-8R	20	22	.08904	38.28	2.384	.50196	1.550	.672	
COMPUTE NM HYD	130.70	-	1	.01696	15.02	.425	.46988	1.500	1.384	PER IMP= .00
COMPUTE NM HYD	130.90	-	2	.00954	8.45	.239	.46988	1.500	1.385	PER IMP= .00
ADD HYD	130.91	1& 2	1	.02650	23.48	.664	.46988	1.500	1.384	
ROUTE	AP-12R	30	3	.17770	121.65	5.519	.58233	1.600	1.070	
ADD HYD	130.72	1& 3	4	.20420	140.42	6.183	.56773	1.600	1.074	
ADD HYD	AP-13	4&22	2	.29324	176.26	8.567	.54776	1.600	.939	
ROUTE	AP-13R	2	3	.29324	172.47	8.567	.54776	1.650	.919	
COMPUTE NM HYD	130.80	-	1	.03010	23.38	.705	.43936	1.550	1.214	PER IMP= .00
ADD HYD	130.81	3& 1	10	.32334	188.04	9.272	.53767	1.650	.909	
COMPUTE NM HYD	P-5	-	3	.00290	2.51	.074	.47912	1.500	1.353	PER IMP= 2.00
ADD HYD	AP-15UB	10& 3	11	.32624	189.56	9.345	.53710	1.650	.908	
COMPUTE NM HYD	120.50	-	3	.00520	4.28	.122	.43936	1.500	1.287	PER IMP= .00
ADD HYD	AP-6	3&12	10	.12426	90.59	3.115	.47001	1.650	1.139	
ROUTE	AP-6R	10	2	.12426	91.47	3.115	.47001	1.650	1.150	

COMPUTE NM HYD	A-1D	-	3	.02400	61.30	2.881	2.25115	1.500	3.991	PER IMP=	90.00
ADD HYD	AP-6A	2& 3	10	.14826	126.03	5.996	.75833	1.650	1.328		
COMPUTE NM HYD	AP-6B&A-2D	-	3	.01250	29.07	1.312	1.96827	1.500	3.634	PER IMP=	76.00
COMPUTE NM HYD	120.5A	-	4	.00200	1.65	.047	.43936	1.500	1.290	PER IMP=	.00
ADD HYD	AP-6B+120.5A	3& 4	20	.01450	30.72	1.359	1.75736	1.500	3.310		
ADD HYD	AP-6A+	10&20	10	.16276	146.46	7.355	.84732	1.600	1.406		
COMPUTE NM HYD	P-6	-	3	.00430	3.72	.110	.47912	1.500	1.350	PER IMP=	2.00
ADD HYD	AP-14	10& 3	18	.16705	149.41	7.465	.83784	1.600	1.397		
ROUTE RESERVOIR	AP-14R	18	2	.16706	16.89	7.465	.83784	2.250	.158	AC-FT=	5.134
ROUTE	POND6ROUTE36	2	12	.16706	16.85	7.465	.83784	2.250	.158		
ROUTE	POND6ROUTE48	12	22	.16706	16.98	7.465	.83784	2.250	.159		
DIVIDE HYD	AP-15	11	17	.32624	202.83	9.999	.57470	1.650	.971		
	130.83	AND	16	.32624	13.27	.654	.03760	1.650	.064		
ROUTE RESERVOIR	AP-15R	17	5	.32624	130.19	9.999	.57470	1.850	.624	AC-FT=	2.645
ROUTE	P-5R	5	11	.32624	131.24	9.999	.57470	1.800	.629		
ADD HYD	POND-5R&6R	11&22	10	.49330	141.70	17.465	.66382	1.800	.449		
COMPUTE NM HYD	P-4	-	3	.00360	3.11	.092	.47912	1.500	1.352	PER IMP=	2.00
ADD HYD	AP-15A	10& 3	10	.49690	142.49	17.556	.66246	1.800	.448		
ROUTE RESERVOIR	AP-15AR	10	5	.49690	94.20	17.471	.65925	2.200	.296	AC-FT=	4.701
ROUTE	P-4R	5	12	.49690	94.82	17.471	.65925	2.200	.298		
COMPUTE NM HYD	AP-16	-	1	.01960	18.50	.522	.49901	1.500	1.475	PER IMP=	.00
ROUTE	140.11	1	22	.01960	16.56	.522	.49908	1.550	1.320		
ROUTE	140.12	22	15	.01960	17.13	.522	.49909	1.600	1.366		
COMPUTE NM HYD	140.20	-	1	.08920	78.50	2.196	.46166	1.500	1.375	PER IMP=	.00

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION	
ADD HYD	AP-17UB	15& 1	2	.10880	91.60	2.718	.46840	1.550	1.316		
DIVIDE HYD	AP-17	2	17	.10880	98.93	2.935	.50587	1.550	1.421		
	140.23	AND	16	.10880	7.33	.217	.03747	1.550	.105		
ADD HYD	140.24	17&12	2	.60570	101.94	20.406	.63170	2.100	.263		
COMPUTE NM HYD	P-3	-	3	.00270	2.34	.069	.47912	1.500	1.354	PER IMP=	2.00
ADD HYD	AP-18	2& 3	10	.60840	102.13	20.474	.63099	2.100	.262		
ROUTE RESERVOIR	AP-18R	10	4	.60840	89.26	20.314	.62604	2.350	.229	AC-FT=	2.839
ROUTE	P-3R	4	2	.60840	89.28	20.313	.62602	2.400	.229		
COMPUTE NM HYD	P-2	-	3	.00360	3.11	.092	.47912	1.500	1.352	PER IMP=	2.00
ADD HYD	AP-18A	2& 3	10	.61200	89.41	20.405	.62514	2.400	.228		
ROUTE RESERVOIR	AP-18AR	10	5	.61200	20.58	19.387	.59397	3.900	.053	AC-FT=	6.532
ROUTE	P-2R	5	10	.61200	20.58	19.386	.59394	3.950	.053		
COMPUTE NM HYD	AP-19UB	-	1	.15250	85.35	3.692	.45390	1.600	.874	PER IMP=	.00
DIVIDE HYD	AP-19	1	17	.15250	89.62	3.876	.47660	1.600	.918		
	150.12	AND	16	.15250	4.27	.185	.02270	1.600	.044		
ADD HYD	150.13	10&17	10	.76450	89.63	23.262	.57053	1.600	.183		
COMPUTE NM HYD	P-1	-	3	.00290	2.51	.074	.47912	1.500	1.353	PER IMP=	2.00
ADD HYD	AP-20	10& 3	10	.76740	91.63	23.336	.57017	1.600	.187		
ROUTE RESERVOIR	150.14	10	5	.76740	15.14	23.184	.56646	6.400	.031	AC-FT=	4.897
COMPUTE NM HYD	AP-21&G-1D	-	3	.02290	18.84	.537	.43936	1.500	1.286	PER IMP=	.00
COMPUTE NM HYD	AP-22&G-2D	-	4	.00310	7.72	.349	2.11028	1.500	3.893	PER IMP=	82.00
ROUTE	AP-22R	4	12	.00310	7.56	.349	2.11044	1.500	3.808		
COMPUTE NM HYD	AP-23&G-3D	-	5	.03510	22.33	.822	.43936	1.550	.994	PER IMP=	.00
COMPUTE NM HYD	G-4D	-	6	.00330	8.10	.359	2.03985	1.500	3.837	PER IMP=	78.00
ADD HYD	AP-24	12& 6	10	.00640	15.66	.708	2.07389	1.500	3.823		
FINISH											

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994 RUN DATE (MON/DAY/YR) =01/30/2001
INPUT FILE = WTF10PE.IN USER NO.= C_ANDRSN.I01

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION
*S EXISTING CONDITIONS, WITHOUT I-40 INTERCEPTOR IN PLACE AND										
*S WITH THE WEST SIDE TRANSIT FACILITY FULLY DEVELOPED										
*S 10 YEAR, 24 HOUR STORM EVENT										
*S										
*S FILE NAME: WTF10PE.IN										
*S										
*S THIS MODEL ANALYZES THE UNSER DIVERSION PONDS WITH THE PROPOSED POND										
*S OUTLET WORKS AND THE EXISTING POND GRADING.										
*S										
*S MASTER DRAINAGE PLAN FOR THE WEST SIDE TRANSIT FACILITY										
*S										
*S THIS AHYMO FILE ANALYZES THE UNSER DIVERSION (DETENTION PONDS) ASSUMING										
*S EXISTING CONDITIONS WITHOUT THE I-40 INTERCEPTOR IN PLACE										
*S AND THE WEST SIDE TRANSIT FACILITY FULLY DEVELOPED.										
*S										
*S NOTES: UNSER DIVERSION POND NO. 5 IS USED IN EXISTING CONDITIONS.										
*S THE OUTLET RATING CURVES WERE ALSO BASED UPON TAILWATER										
*S ELEVATIONS SHOWN ON THE UNSER DIVERSION (AS-BUILT)										
*S CONSTRUCTION PLAN SET (PLANS BY CHUCK EASTERLING).										
*S DRAINAGE BASINS SOUTH OF INTERSTATE 40 (WITH THE EXCEPTION										
*S OF THE WEST SIDE TRANSIT FACILITY AREA) WERE TAKEN FROM										
*S THE "MASTER DRAINAGE PLAN FOR THE ATRISCO BUSINESS PARK"										
*S REVISED OCTOBER 1993, PREPARED BY EASTERLING & ASSOCIATES, INC.										
*S DATE: 11/21/00										
START										TIME= .00
RAINFALL TYPE= 2										RAIN24= 1.770
*S BEGIN OFFSITE WATERSHED NORTH OF INTERSTATE 40 - EXISTING CONDITION										
COMPUTE NM HYD	O-1E	-	3	.02420	3.43	.108	.08362	1.550	.221	PER IMP= .00
COMPUTE NM HYD	O-2E	-	4	.04220	4.48	.188	.08362	1.600	.166	PER IMP= .00
ADD HYD	AP-1	3& 4	10	.06640	7.65	.296	.08361	1.550	.180	
ROUTE	AP-1R	10	2	.06640	5.45	.296	.08362	1.700	.128	
COMPUTE NM HYD	O-3E	-	3	.01640	2.72	.086	.09818	1.550	.259	PER IMP= 1.00
ADD HYD	PARTAP-2	2& 3	10	.08280	7.02	.382	.08650	1.650	.132	
COMPUTE NM HYD	AP-7&O-5E	-	3	.10970	13.54	.660	.11274	1.600	.193	PER IMP= 2.00
DIVIDE HYD	PART_AP-7	3	98	.10970	13.54	.660	.11274	1.500	.193	
	PART_AP-7	AND	99	.00000	.00	.000	.00000	-.050	.000	
ROUTE	AP-7R	99	2	.00000	.00	.000	.00000	-.050	.000	
ADD HYD	AP-2	10& 2	10	.08280	7.02	.382	.08650	1.650	.132	
ROUTE	AP-2R	10	2	.08280	6.98	.382	.08651	1.700	.132	
COMPUTE NM HYD	I40-3	-	3	.00190	1.40	.060	.59326	1.500	1.154	PER IMP= 35.00
ADD HYD	AP-3	2& 3	20	.08470	7.72	.442	.09786	1.650	.142	
COMPUTE NM HYD	AP-4&O-4E	-	3	.00940	1.75	.057	.11274	1.550	.292	PER IMP= 2.00
ROUTE	AP-4R	3	2	.00940	1.84	.057	.11280	1.550	.306	
COMPUTE NM HYD	I40-4	-	3	.00120	.89	.038	.59326	1.500	1.156	PER IMP= 35.00
ADD HYD	AP-5	2& 3	10	.01060	2.64	.094	.16710	1.550	.389	
ADD HYD	AP-3+AP-5	20&10	10	.09530	9.60	.537	.10556	1.600	.157	
ROUTE	PARTAP-6	10	12	.09530	9.61	.537	.10557	1.600	.158	
ROUTE	AP-7R-CUL	98	2	.10970	13.58	.660	.11274	1.600	.193	
COMPUTE NM HYD	I40-2	-	3	.00310	2.28	.098	.59326	1.500	1.152	PER IMP= 35.00

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION
ADD HYD	AP-8	2& 3	20	.11280	15.19	.758	.12594	1.600	.210	
COMPUTE NM HYD	O-6E	-	3	.07320	6.49	.326	.08362	1.650	.138	PER IMP= .00
COMPUTE NM HYD	O-7E	-	4	.05230	7.41	.233	.08362	1.550	.221	PER IMP= .00
ADD HYD	AP-9	3& 4	10	.12550	13.19	.560	.08362	1.600	.164	
ROUTE	AP-9R	10	2	.12550	13.27	.560	.08362	1.600	.165	
COMPUTE NM HYD	O-8E	-	3	.00990	14.68	.698	1.32131	1.500	2.318	PER IMP= 85.00
ADD HYD	AP-10	2& 3	10	.13540	26.66	1.257	.17411	1.550	.308	
ROUTE	AP-10R	10	2	.13540	21.80	1.257	.17411	1.650	.252	
COMPUTE NM HYD	O-9E	-	3	.03480	8.15	.290	.15643	1.500	.366	PER IMP= 5.00
ADD HYD	AP-11	2& 3	10	.17020	27.81	1.548	.17049	1.600	.255	
ROUTE	AP-11R	10	2	.17020	27.65	1.548	.17050	1.600	.254	
COMPUTE NM HYD	I40-1	-	3	.00750	5.52	.237	.59326	1.500	1.150	PER IMP= 35.00
ADD HYD	AP-12	2& 3	30	.17770	31.52	1.785	.18833	1.600	.277	
ROUTE	AP-8R	20	22	.11280	15.03	.758	.12595	1.650	.208	
COMPUTE NM HYD	130.70	-	1	.01696	3.27	.092	.10184	1.550	.301	PER IMP= .00
COMPUTE NM HYD	130.90	-	2	.00954	1.84	.052	.10184	1.550	.301	PER IMP= .00
ADD HYD	130.91	1& 2	1	.02650	5.11	.144	.10183	1.550	.301	
ROUTE	AP-12R	30	3	.17770	30.80	1.785	.18834	1.650	.271	
ADD HYD	130.72	1& 3	4	.20420	34.10	1.929	.17711	1.650	.261	
ADD HYD	AP-13	4&22	2	.31700	49.13	2.686	.15890	1.650	.242	
ROUTE	AP-13R	2	3	.31700	44.96	2.687	.15890	1.700	.222	
COMPUTE NM HYD	130.80	-	1	.03010	4.52	.134	.08362	1.550	.235	PER IMP= .00
ADD HYD	130.81	3& 1	10	.34710	47.44	2.821	.15237	1.700	.214	
COMPUTE NM HYD	P-5	-	3	.00290	.55	.017	.11274	1.550	.294	PER IMP= 2.00
ADD HYD	AP-15UB	10& 3	11	.35000	47.71	2.837	.15199	1.700	.213	
COMPUTE NM HYD	120.50	-	3	.00520	.82	.023	.08362	1.550	.246	PER IMP= .00
ADD HYD	AP-6	3&12	10	.10050	10.32	.560	.10443	1.600	.160	
ROUTE	AP-6R	10	2	.10050	10.45	.560	.10444	1.650	.162	

COMPUTE NM HYD	A-1D	-	3	.02400	39.69	1.802	1.40796	1.500	2.584	PER IMP=	90.00
ADD HYD	AP-6A	2& 3	10	.12450	43.13	2.362	.35571	1.500	.541		
COMPUTE NM HYD	AP-6B&A-2D	-	3	.01250	18.04	.801	1.20080	1.500	2.256	PER IMP=	76.00
COMPUTE NM HYD	120.5A	-	4	.00200	.32	.009	.08362	1.550	.247	PER IMP=	.00
ADD HYD	AP-6B+120.5A	3& 4	20	.01450	18.35	.809	1.04667	1.500	1.977		
ADD HYD	AP-6A+	10&20	10	.13900	61.48	3.171	.42779	1.500	.691		
COMPUTE NM HYD	P-6	-	3	.00430	.81	.026	.11274	1.550	.293	PER IMP=	2.00
ADD HYD	AP-14	10& 3	18	.14330	62.27	3.196	.41823	1.500	.679		
ROUTE RESERVOIR	AP-14R	18	2	.14330	22.83	3.196	.41823	1.950	.249	AC-FT=	1.230
ROUTE	POND6ROUTE36	2	12	.14330	22.84	3.196	.41823	1.950	.249		
ROUTE	POND6ROUTE48	12	22	.14330	22.85	3.196	.41823	1.950	.249		
DIVIDE HYD	AP-15	11	17	.35000	51.05	3.036	.16263	1.700	.228		
	130.83	AND	16	.35000	3.34	.199	.01064	1.700	.015		
ROUTE RESERVOIR	AP-15R	17	5	.35000	23.22	3.036	.16263	2.050	.104	AC-FT=	1.289
ROUTE	P-5R	5	11	.35000	23.50	3.036	.16263	2.050	.105		
ADD HYD	POND-5R&6R	11&22	10	.49330	46.02	6.232	.23688	2.050	.146		
COMPUTE NM HYD	P-4	-	3	.00360	.68	.022	.11274	1.550	.293	PER IMP=	2.00
ADD HYD	AP-15A	10& 3	10	.49690	46.10	6.253	.23594	2.050	.145		
ROUTE RESERVOIR	AP-15AR	10	5	.49690	31.93	6.174	.23296	2.400	.100	AC-FT=	1.106
ROUTE	P-4R	5	12	.49690	31.93	6.174	.23296	2.400	.100		
COMPUTE NM HYD	AP-16	-	1	.01960	4.43	.124	.11841	1.550	.353	PER IMP=	.00
ROUTE	140.11	1	22	.01960	3.49	.124	.11845	1.600	.278		
ROUTE	140.12	22	15	.01960	3.47	.124	.11845	1.650	.276		
COMPUTE NM HYD	140.20	-	1	.08920	16.45	.462	.09721	1.550	.288	PER IMP=	.00

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 3 NOTATION	
ADD HYD	AP-17UB	15& 1	2	.10880	18.28	.586	.10103	1.550	.262		
DIVIDE HYD	AP-17	2	17	.10880	19.74	.633	.10911	1.550	.283		
	140.23	AND	16	.10880	1.46	.047	.00808	1.550	.021		
ADD HYD	140.24	17&12	2	.60570	33.13	6.807	.21071	2.350	.085		
COMPUTE NM HYD	P-3	-	3	.00270	.51	.016	.11274	1.550	.294	PER IMP=	2.00
ADD HYD	AP-18	2& 3	10	.60840	33.15	6.822	.21025	2.350	.085		
ROUTE RESERVOIR	AP-18R	10	4	.60840	27.49	6.753	.20813	2.850	.071	AC-FT=	1.219
ROUTE	P-3R	4	2	.60840	27.49	6.753	.20813	2.850	.071		
COMPUTE NM HYD	P-2	-	3	.00360	.68	.022	.11274	1.550	.293	PER IMP=	2.00
ADD HYD	AP-18A	2& 3	10	.61200	27.51	6.774	.20754	2.850	.070		
ROUTE RESERVOIR	AP-18AR	10	5	.61200	21.71	6.043	.18513	3.700	.055	AC-FT=	1.949
ROUTE	P-2R	5	10	.61200	21.71	6.043	.18513	3.700	.055		
COMPUTE NM HYD	AP-19UB	-	1	.15250	16.02	.753	.09263	1.650	.164	PER IMP=	.00
DIVIDE HYD	AP-19	1	17	.15250	16.82	.791	.09726	1.650	.172		
	150.12	AND	16	.15250	.80	.038	.00463	1.650	.008		
ADD HYD	150.13	10&17	10	.76450	22.10	6.834	.16760	3.650	.045		
COMPUTE NM HYD	P-1	-	3	.00290	.55	.017	.11274	1.550	.294	PER IMP=	2.00
ADD HYD	AP-20	10& 3	10	.76740	22.11	6.850	.16737	3.650	.045		
ROUTE RESERVOIR	150.14	10	5	.76740	20.85	6.849	.16735	4.100	.042	AC-FT=	.664
COMPUTE NM HYD	AP-21&G-1D	-	3	.02290	3.61	.102	.08362	1.550	.246	PER IMP=	.00
COMPUTE NM HYD	AP-22&G-2D	-	4	.00310	4.90	.215	1.30254	1.500	2.469	PER IMP=	82.00
ROUTE	AP-22R	4	12	.00310	4.76	.215	1.30270	1.500	2.398		
COMPUTE NM HYD	AP-23&G-3D	-	5	.03510	4.10	.157	.08362	1.600	.183	PER IMP=	.00
COMPUTE NM HYD	G-4D	-	6	.00330	5.08	.220	1.24983	1.500	2.407	PER IMP=	78.00
ADD HYD	AP-24	12& 6	10	.00640	9.84	.435	1.27528	1.500	2.403		
FINISH											

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994 RUN DATE (MON/DAY/YR) =01/29/2001
INPUT FILE = WTF100PE.IN USER NO.= C_ANDRSN.I01

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION
										PAGE = 1
*$ EXISTING CONDITIONS, WITHOUT I-40 INTERCEPTOR IN PLACE AND										
*$ WITH THE WEST SIDE TRANSIT FACILITY FULLY DEVELOPED										
*$ 100 YEAR, 24 HOUR STORM EVENT										
*$										
*$ FILE NAME: WTF100PE.IN										
*$										
*$ THIS MODEL ANALYZES THE UNSER DIVERSION PONDS WITH THE PROPOSED POND										
*$ OUTLET WORKS AND THE EXISTING POND GRADING.										
*$										
*$ MASTER DRAINAGE PLAN FOR THE WEST SIDE TRANSIT FACILITY										
*$										
*$ THIS AHYMO FILE ANALYZES THE UNSER DIVERSION (DETENTION PONDS) ASSUMING										
*$ EXISTING CONDITIONS WITHOUT THE I-40 INTERCEPTOR IN PLACE										
*$ AND THE WEST SIDE TRANSIT FACILITY FULLY DEVELOPED.										
*$										
*$ NOTES: UNSER DIVERSION POND NO. 5 IS USED IN EXISTING CONDITIONS.										
*$ THE OUTLET RATING CURVES WERE ALSO BASED UPON TAILWATER										
*$ ELEVATIONS SHOWN ON THE UNSER DIVERSION (AS-BUILT)										
*$ CONSTRUCTION PLAN SET (PLANS BY CHUCK EASTERLING).										
*$ DRAINAGE BASINS SOUTH OF INTERSTATE 40 (WITH THE EXCEPTION										
*$ OF THE WEST SIDE TRANSIT FACILITY AREA) WERE TAKEN FROM										
*$ THE "MASTER DRAINAGE PLAN FOR THE ATRISCO BUSINESS PARK"										
*$ REVISED OCTOBER 1993, PREPARED BY EASTERLING & ASSOCIATES, INC.										
*$ DATE: 11/21/00										
START										TIME= .00
RAINFALL TYPE= 2										RAIN24= 2.660
*$ BEGIN OFFSITE WATERSHED NORTH OF INTERSTATE 40 - EXISTING CONDITION										
COMPUTE NM HYD	O-1E	-	3	.02420	18.06	.567	.43936	1.550	1.166	PER IMP= .00
COMPUTE NM HYD	O-2E	-	4	.04220	24.34	.989	.43936	1.600	.901	PER IMP= .00
ADD HYD	AP-1	3& 4	10	.06640	41.88	1.556	.43936	1.550	.986	
ROUTE	AP-1R	10	2	.06640	36.11	1.556	.43938	1.650	.850	
COMPUTE NM HYD	O-3E	-	3	.01640	13.10	.402	.45924	1.500	1.248	PER IMP= 1.00
ADD HYD	PARTAP-2	2& 3	10	.08280	44.73	1.958	.44330	1.650	.844	
COMPUTE NM HYD	AP-7&O-5E	-	3	.10970	63.37	2.803	.47912	1.600	.903	PER IMP= 2.00
DIVIDE HYD	PART_AP-7	3	98	.08594	32.00	2.196	.47911	1.500	.582	
	PART_AP-7	AND	99	.02376	31.37	.607	.47911	1.600	2.062	
ROUTE	AP-7R	99	2	.02376	31.64	.608	.47944	1.600	2.081	
ADD HYD	AP-2	10& 2	10	.10656	76.00	2.565	.45135	1.600	1.114	
ROUTE	AP-2R	10	2	.10656	79.83	2.565	.45136	1.600	1.171	
COMPUTE NM HYD	I40-3	-	3	.00190	2.86	.115	1.13512	1.500	2.350	PER IMP= 35.00
ADD HYD	AP-3	2& 3	20	.10846	81.89	2.680	.46333	1.600	1.180	
COMPUTE NM HYD	AP-4&O-4E	-	3	.00940	8.11	.240	.47912	1.500	1.348	PER IMP= 2.00
ROUTE	AP-4R	3	2	.00940	8.17	.240	.47921	1.500	1.358	
COMPUTE NM HYD	I40-4	-	3	.00120	1.81	.073	1.13512	1.500	2.357	PER IMP= 35.00
ADD HYD	AP-5	2& 3	10	.01060	9.98	.313	.55337	1.500	1.471	
ADD HYD	AP-3+AP-5	20&10	10	.11906	89.90	2.993	.47135	1.600	1.180	
ROUTE	PARTAP-6	10	12	.11906	87.97	2.993	.47135	1.650	1.154	
ROUTE	AP-7R-CUL	98	2	.08594	33.23	2.196	.47912	1.500	.604	
COMPUTE NM HYD	I40-2	-	3	.00310	4.65	.188	1.13512	1.500	2.346	PER IMP= 35.00

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION
										PAGE = 2
ADD HYD	AP-8	2& 3	20	.08904	37.88	2.384	.50195	1.500	.665	
COMPUTE NM HYD	O-6E	-	3	.07320	35.69	1.715	.43936	1.600	.762	PER IMP= .00
COMPUTE NM HYD	O-7E	-	4	.05230	39.03	1.226	.43936	1.550	1.166	PER IMP= .00
ADD HYD	AP-9	3& 4	10	.12550	71.47	2.941	.43936	1.550	.890	
ROUTE	AP-9R	10	2	.12550	70.89	2.941	.43937	1.600	.883	
COMPUTE NM HYD	O-8E	-	3	.00990	23.00	1.124	2.12906	1.500	3.630	PER IMP= 85.00
ADD HYD	AP-10	2& 3	10	.13540	92.59	4.065	.56290	1.550	1.068	
ROUTE	AP-10R	10	2	.13540	89.10	4.065	.56291	1.600	1.028	
COMPUTE NM HYD	O-9E	-	3	.03480	31.99	1.000	.53875	1.500	1.436	PER IMP= 5.00
ADD HYD	AP-11	2& 3	10	.17020	114.14	5.065	.55797	1.600	1.048	
ROUTE	AP-11R	10	2	.17020	114.79	5.065	.55797	1.600	1.054	
COMPUTE NM HYD	I40-1	-	3	.00750	11.24	.454	1.13512	1.500	2.342	PER IMP= 35.00
ADD HYD	AP-12	2& 3	30	.17770	122.86	5.519	.58232	1.600	1.080	
ROUTE	AP-8R	20	22	.08904	38.28	2.384	.50196	1.550	.672	
COMPUTE NM HYD	130.70	-	1	.01696	15.02	.425	.46988	1.500	1.384	PER IMP= .00
COMPUTE NM HYD	130.90	-	2	.00954	8.45	.239	.46988	1.500	1.385	PER IMP= .00
ADD HYD	130.91	1& 2	1	.02650	23.48	.664	.46988	1.500	1.384	
ROUTE	AP-12R	30	3	.17770	121.65	5.519	.58233	1.600	1.070	
ADD HYD	130.72	1& 3	4	.20420	140.42	6.183	.56773	1.600	1.074	
ADD HYD	AP-13	4&22	2	.29324	176.26	8.567	.54776	1.600	.939	
ROUTE	AP-13R	2	3	.29324	172.47	8.567	.54776	1.650	.919	
COMPUTE NM HYD	130.80	-	1	.03010	23.38	.705	.43936	1.550	1.214	PER IMP= .00
ADD HYD	130.81	3& 1	10	.32334	188.04	9.272	.53767	1.650	.909	
COMPUTE NM HYD	P-5	-	3	.00290	2.51	.074	.47912	1.500	1.353	PER IMP= 2.00
ADD HYD	AP-15UB	10& 3	11	.32624	189.56	9.345	.53710	1.650	.908	
COMPUTE NM HYD	120.50	-	3	.00520	4.28	.122	.43936	1.500	1.287	PER IMP= .00
ADD HYD	AP-6	3&12	10	.12426	90.59	3.115	.47001	1.650	1.139	
ROUTE	AP-6R	10	2	.12426	91.47	3.115	.47001	1.650	1.150	

COMPUTE NM HYD	A-1D	-	3	.02400	61.30	2.881	2.25115	1.500	3.991	PER IMP=	90.00
ADD HYD	AP-6A	2& 3	10	.14826	126.03	5.996	.75833	1.650	1.328		
COMPUTE NM HYD	AP-6B&A-2D	-	3	.01250	29.07	1.312	1.96827	1.500	3.634	PER IMP=	76.00
COMPUTE NM HYD	120.5A	-	4	.00200	1.65	.047	.43936	1.500	1.290	PER IMP=	.00
ADD HYD	AP-6B+120.5A	3& 4	20	.01450	30.72	1.359	1.75736	1.500	3.310		
ADD HYD	AP-6A+	10&20	10	.16276	146.46	7.355	.84732	1.600	1.406		
COMPUTE NM HYD	P-6	-	3	.00430	3.72	.110	.47912	1.500	1.350	PER IMP=	2.00
ADD HYD	AP-14	10& 3	18	.16706	149.41	7.465	.83784	1.600	1.397		
ROUTE RESERVOIR	AP-14R	18	2	.16706	48.71	7.465	.83784	1.950	.456	AC-FT=	3.459
ROUTE	POND6ROUTE36	2	12	.16706	48.73	7.465	.83784	1.950	.456		
ROUTE	POND6ROUTE48	12	22	.16706	48.70	7.465	.83784	1.950	.455		
DIVIDE HYD	AP-15	11	17	.32624	202.83	9.999	.57470	1.650	.971		
	130.83	AND	16	.32624	13.27	.654	.03760	1.650	.064		
ROUTE RESERVOIR	AP-15R	17	5	.32624	130.19	9.999	.57470	1.850	.624	AC-FT=	2.645
ROUTE	P-5R	5	11	.32624	131.24	9.999	.57470	1.800	.629		
ADD HYD	POND-5R&6R	11&22	10	.49330	178.75	17.465	.66382	1.900	.566		
COMPUTE NM HYD	P-4	-	3	.00360	3.11	.092	.47912	1.500	1.352	PER IMP=	2.00
ADD HYD	AP-15A	10& 3	10	.49690	179.37	17.556	.66246	1.800	.564		
ROUTE RESERVOIR	AP-15AR	10	5	.49690	99.87	17.477	.65947	2.250	.314	AC-FT=	5.298
ROUTE	P-4R	5	12	.49690	99.92	17.477	.65947	2.300	.314		
COMPUTE NM HYD	AP-16	-	1	.01960	18.50	.522	.49901	1.500	1.475	PER IMP=	.00
ROUTE	140.11	1	22	.01960	16.56	.522	.49908	1.550	1.320		
ROUTE	140.12	22	15	.01960	17.13	.522	.49909	1.600	1.366		
COMPUTE NM HYD	140.20	-	1	.08920	78.50	2.196	.46166	1.500	1.375	PER IMP=	.00

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION	
ADD HYD	AP-17UB	15& 1	2	.10880	91.60	2.718	.46840	1.550	1.316		
DIVIDE HYD	AP-17	2	17	.10880	98.93	2.935	.50587	1.550	1.421		
	140.23	AND	16	.10880	7.33	.217	.03747	1.550	.105		
ADD HYD	140.24	17&12	2	.60570	107.19	20.412	.63188	1.550	.277		
COMPUTE NM HYD	P-3	-	3	.00270	2.34	.069	.47912	1.500	1.354	PER IMP=	2.00
ADD HYD	AP-18	2& 3	10	.60840	109.44	20.480	.63117	1.550	.281		
ROUTE RESERVOIR	AP-18R	10	4	.60840	94.06	20.412	.62905	2.700	.242	AC-FT=	3.118
ROUTE	P-3R	4	2	.60840	94.09	20.412	.62905	2.700	.242		
COMPUTE NM HYD	P-2	-	3	.00360	3.11	.092	.47912	1.500	1.352	PER IMP=	2.00
ADD HYD	AP-18A	2& 3	10	.61200	94.15	20.503	.62816	2.700	.240		
ROUTE RESERVOIR	AP-18AR	10	5	.61200	71.15	19.771	.60573	3.300	.182	AC-FT=	6.618
ROUTE	P-2R	5	10	.61200	71.17	19.771	.60573	3.300	.182		
COMPUTE NM HYD	AP-19UB	-	1	.15250	85.35	3.692	.45390	1.600	.874	PER IMP=	.00
DIVIDE HYD	AP-19	1	17	.15250	89.62	3.876	.47660	1.600	.918		
	150.12	AND	16	.15250	4.27	.185	.02270	1.600	.044		
ADD HYD	150.13	10&17	10	.76450	89.64	23.647	.57997	1.600	.183		
COMPUTE NM HYD	P-1	-	3	.00290	2.51	.074	.47912	1.500	1.353	PER IMP=	2.00
ADD HYD	AP-20	10& 3	10	.76740	91.63	23.721	.57957	1.600	.187		
ROUTE RESERVOIR	150.14	10	5	.76740	58.70	23.720	.57956	3.650	.120	AC-FT=	2.146
COMPUTE NM HYD	AP-21&G-1D	-	3	.02290	18.84	.537	.43936	1.500	1.286	PER IMP=	.00
COMPUTE NM HYD	AP-22&G-2D	-	4	.00310	7.72	.349	2.11028	1.500	3.893	PER IMP=	82.00
ROUTE	AP-22R	4	12	.00310	7.56	.349	2.11044	1.500	3.808		
COMPUTE NM HYD	AP-23&G-3D	-	5	.03510	22.33	.822	.43936	1.550	.994	PER IMP=	.00
COMPUTE NM HYD	G-4D	-	6	.00330	8.10	.359	2.03985	1.500	3.837	PER IMP=	78.00
ADD HYD	AP-24	12& 6	10	.00640	15.66	.708	2.07389	1.500	3.823		

FINISH

```
AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994          RUN DATE (MON/DAY/YR) =01/30/2001
INPUT FILE = WTF10EP.IN                                                                 USER NO.= C_ANDRSN.I01

                                FROM  TO                    PEAK          RUNOFF                TIME TO   CFS        PAGE =   1
                        HYDROGRAPH  ID   ID        AREA   DISCHARGE       VOLUME      RUNOFF      PEAK     PER
COMMAND       IDENTIFICATION  NO.  NO.      (SQ MI)    (CFS)        (AC-FT)     (INCHES)    (HOURS)  ACRE     NOTATION

*S EXISTING CONDITIONS, WITHOUT I-40 INTERCEPTOR IN PLACE AND
*S WITH THE WEST SIDE TRANSIT FACILITY FULLY DEVELOPED
*S 10 YEAR, 24 HOUR STORM EVENT
*S
*S FILE NAME: WTF10EP.IN
*S
*S THIS MODEL ANALYZES THE UNSER DIVERSION PONDS WITH THE EXISTING POND
*S OUTLET WORKS AND THE PROPOSED POND GRADING.
*S
*S MASTER DRAINAGE PLAN FOR THE WEST SIDE TRANSIT FACILITY
*S
*S THIS AHYMO FILE ANALYZES THE UNSER DIVERSION (DETENTION PONDS) ASSUMING
*S EXISTING CONDITIONS WITHOUT THE I-40 INTERCEPTOR IN PLACE
*S AND THE WEST SIDE TRANSIT FACILITY FULLY DEVELOPED.
*S
*S NOTES: UNSER DIVERSION POND NO. 5 IS USED IN EXISTING CONDITIONS.
*S        THE OUTLET RATING CURVES WERE ALSO BASED UPON TAILWATER
*S        ELEVATIONS SHOWN ON THE UNSER DIVERSION (AS-BUILT)
*S        CONSTRUCTION PLAN SET (PLANS BY CHUCK EASTERLING),
*S        DRAINAGE BASINS SOUTH OF INTERSTATE 40 (WITH THE EXCEPTION
*S        OF THE WEST SIDE TRANSIT FACILITY AREA) WERE TAKEN FROM
*S        THE "MASTER DRAINAGE PLAN FOR THE ATRISCO BUSINESS PARK"
*S        REVISED OCTOBER 1993, PREPARED BY EASTERLING & ASSOCIATES, INC.
*S DATE: 11/21/00
START                                                                                                    TIME=      .00
RAINFALL  TYPE= 2                                                                                        RAIN24=   1.770
*S BEGIN OFFSITE WATERSHED NORTH OF INTERSTATE 40 - EXISTING CONDITION
COMPUTE NM HYD          0-1E   -     3        .02420        3.43        .108       .08362    1.550     .221 PER IMP=     .00
COMPUTE NM HYD          0-2E   -     4        .04220        4.48        .188       .08362    1.600     .166 PER IMP=     .00
ADD HYD                 AP-1   3& 4  10       .06640        7.65        .296       .08361    1.550     .180
ROUTE                   AP-1R  10    2        .06640        5.45        .296       .08362    1.700     .128
COMPUTE NM HYD          0-3E   -     3        .01640        2.72        .086       .09818    1.550     .259 PER IMP=    1.00
ADD HYD              PARTAP-2  2& 3  10       .08280        7.02        .382       .08650    1.650     .132
COMPUTE NM HYD      AP-7&0-5E  -     3        .10970       13.54        .660       .11274    1.600     .193 PER IMP=    2.00
DIVIDE HYD          PART_AP-7    3   98       .10970       13.54        .660       .11274    1.600     .193
                    PART_AP-7  AND   99       .00000         .00        .000       .00000    -.050     .000
ROUTE                   AP-7R  99    2        .00000         .00        .000       .00000    -.050     .000
ADD HYD                 AP-2   10& 2  10       .08280        7.02        .382       .08650    1.650     .132
ROUTE                   AP-2R  10    2        .08280        6.98        .382       .08651    1.700     .132
COMPUTE NM HYD          I40-3  -     3        .00190        1.40        .060       .59326    1.500    1.154 PER IMP=   35.00
ADD HYD                 AP-3   2& 3  20       .08470        7.72        .442       .09786    1.650     .142
COMPUTE NM HYD      AP-4&0-4E  -     3        .00940        1.75        .057       .11274    1.550     .292 PER IMP=    2.00
ROUTE                   AP-4R    3   2        .00940        1.84        .057       .11280    1.550     .306
COMPUTE NM HYD          I40-4  -     3        .00120         .89        .038       .59326    1.500    1.156 PER IMP=   35.00
ADD HYD                 AP-5   2& 3  10       .01060        2.64        .094       .16710    1.550     .389
ADD HYD              AP-3+AP-5 20&10  10       .09530        9.60        .537       .10556    1.600     .157
ROUTE                PARTAP-6  10    12       .09530        9.61        .537       .10557    1.600     .158
ROUTE                AP-7R-CUL  98    2        .10970       13.58        .660       .11274    1.600     .193
COMPUTE NM HYD          I40-2  -     3        .00310        2.28        .098       .59326    1.500    1.152 PER IMP=   35.00

                                FROM  TO                    PEAK          RUNOFF                TIME TO   CFS        PAGE =   2
                        HYDROGRAPH  ID   ID        AREA   DISCHARGE       VOLUME      RUNOFF      PEAK     PER
COMMAND       IDENTIFICATION  NO.  NO.      (SQ MI)    (CFS)        (AC-FT)     (INCHES)    (HOURS)  ACRE     NOTATION

ADD HYD                 AP-8   2& 3  20       .11280       15.19        .758       .12594    1.600     .210
COMPUTE NM HYD          0-6E   -     3        .07320        6.49        .326       .08362    1.650     .138 PER IMP=     .00
COMPUTE NM HYD          0-7E   -     4        .05230        7.41        .233       .08362    1.550     .221 PER IMP=     .00
ADD HYD                 AP-9   3& 4  10       .12550       13.19        .560       .08362    1.600     .164
ROUTE                   AP-9R  10    2        .12550       13.27        .560       .08362    1.600     .165
COMPUTE NM HYD          0-8E   -     3        .00990       14.68        .698      1.32131    1.500    2.318 PER IMP=   85.00
ADD HYD                 AP-10  2& 3  10       .13540       26.66       1.257       .17411    1.550     .308
ROUTE                   AP-10R 10    2        .13540       21.80       1.257       .17411    1.650     .252
COMPUTE NM HYD          0-9E   -     3        .03480        8.15        .290       .15643    1.500     .366 PER IMP=    5.00
ADD HYD                 AP-11  2& 3  10       .17020       27.81       1.548       .17049    1.600     .255
ROUTE                   AP-11R 10    2        .17020       27.65       1.548       .17050    1.600     .254
COMPUTE NM HYD          I40-1  -     3        .00750        5.52        .237       .59326    1.500    1.150 PER IMP=   35.00
ADD HYD                 AP-12  2& 3  30       .17770       31.52       1.785       .18833    1.600     .277
ROUTE                   AP-8R  20    22       .11280       15.03        .758       .12595    1.650     .208
COMPUTE NM HYD          130.70 -     1        .01696        3.27        .092       .10184    1.550     .301 PER IMP=     .00
COMPUTE NM HYD          130.90 -     2        .00954        1.84        .052       .10184    1.550     .301 PER IMP=     .00
ADD HYD                 130.91 1& 2  1        .02650        5.11        .144       .10183    1.550     .301
ROUTE                   AP-12R 30    3        .17770       30.80       1.785       .18834    1.650     .271
ADD HYD                 130.72 1& 3  4        .20420       34.10       1.929       .17711    1.650     .261
ADD HYD                 AP-13  4&22  2        .31700       49.13       2.686       .15890    1.650     .242
ROUTE                   AP-13R   2   3        .31700       44.96       2.687       .15890    1.700     .222
COMPUTE NM HYD          130.80 -     1        .03010        4.52        .134       .08362    1.550     .235 PER IMP=     .00
ADD HYD                 130.81 3& 1  10       .34710       47.44       2.821       .15237    1.700     .214
COMPUTE NM HYD            P-5  -     3        .00290         .55        .017       .11274    1.550     .294 PER IMP=    2.00
ADD HYD                 AP-15UB 10& 3  11       .35000       47.71       2.837       .15199    1.700     .213
COMPUTE NM HYD          120.50 -     3        .00520         .82        .023       .08362    1.550     .246 PER IMP=     .00
ADD HYD                 AP-6   3&12  10       .10050       10.32        .560       .10443    1.600     .160
ROUTE                   AP-6R  10    2        .10050       10.45        .560       .10444    1.650     .162
```

COMPUTE NM HYD	A-1D	-	3	.02400	39.69	1.802	1.40796	1.500	2.584	PER IMP=	90.00
ADD HYD	AP-6A	2& 3	10	.12450	43.13	2.362	.35571	1.500	.541		
COMPUTE NM HYD	AP-6B&A-2D	-	3	.01250	18.04	.801	1.20080	1.500	2.256	PER IMP=	76.00
COMPUTE NM HYD	120.5A	-	4	.00200	.32	.009	.08362	1.550	.247	PER IMP=	.00
ADD HYD	AP-6B+120.5A	3& 4	20	.01450	18.35	.809	1.04667	1.500	1.977		
ADD HYD	AP-6A+	10&20	10	.13900	61.48	3.171	.42779	1.500	.691		
COMPUTE NM HYD	P-6	-	3	.00430	.81	.026	.11274	1.550	.293	PER IMP=	2.00
ADD HYD	AP-14	10& 3	18	.14330	62.27	3.196	.41823	1.500	.679		
ROUTE RESERVOIR	AP-14R	18	2	.14330	7.34	3.196	.41822	2.250	.080	AC-FT=	1.944
ROUTE	POND6ROUTE36	2	12	.14330	7.34	3.196	.41822	2.250	.080		
ROUTE	POND6ROUTE48	12	22	.14330	7.34	3.196	.41822	2.250	.080		
DIVIDE HYD	AP-15	11	17	.35000	51.05	3.036	.16263	1.700	.228		
	130.83	AND	16	.35000	3.34	.199	.01064	1.700	.015		
ROUTE RESERVOIR	AP-15R	17	5	.35000	23.22	3.036	.16263	2.050	.104	AC-FT=	1.289
ROUTE	P-5R	5	11	.35000	23.50	3.036	.16263	2.050	.105		
ADD HYD	POND-5R&6R	11&22	10	.49330	30.72	6.232	.23688	2.050	.097		
COMPUTE NM HYD	P-4	-	3	.00360	.68	.022	.11274	1.550	.293	PER IMP=	2.00
ADD HYD	AP-15A	10& 3	10	.49690	30.79	6.253	.23594	2.050	.097		
ROUTE RESERVOIR	AP-15AR	10	5	.49690	8.13	6.168	.23275	5.150	.026	AC-FT=	2.797
ROUTE	P-4R	5	12	.49690	8.13	6.168	.23275	5.150	.026		
COMPUTE NM HYD	AP-16	-	1	.01960	4.43	.124	.11841	1.550	.353	PER IMP=	.00
ROUTE	140.11	1	22	.01960	3.49	.124	.11845	1.600	.278		
ROUTE	140.12	22	15	.01960	3.47	.124	.11845	1.650	.276		
COMPUTE NM HYD	140.20	-	1	.08920	16.45	.462	.09721	1.550	.288	PER IMP=	.00

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION	
ADD HYD	AP-17UB	15& 1	2	.10880	18.28	.586	.10103	1.550	.262		
DIVIDE HYD	AP-17	2	17	.10880	19.74	.633	.10911	1.550	.283		
	140.23	AND	16	.10880	1.46	.047	.00808	1.550	.021		
ADD HYD	140.24	17&12	2	.60570	19.81	6.801	.21054	1.550	.051		
COMPUTE NM HYD	P-3	-	3	.00270	.51	.016	.11274	1.550	.294	PER IMP=	2.00
ADD HYD	AP-18	2& 3	10	.60840	20.32	6.817	.21008	1.550	.052		
ROUTE RESERVOIR	AP-18R	10	4	.60840	6.77	6.693	.20627	7.800	.017	AC-FT=	1.568
ROUTE	P-3R	4	2	.60840	6.77	6.693	.20626	7.850	.017		
COMPUTE NM HYD	P-2	-	3	.00360	.68	.022	.11274	1.550	.293	PER IMP=	2.00
ADD HYD	AP-18A	2& 3	10	.61200	6.77	6.713	.20568	7.850	.017		
ROUTE RESERVOIR	AP-18AR	10	5	.61200	4.76	5.481	.16793	12.500	.012	AC-FT=	2.899
ROUTE	P-2R	5	10	.61200	4.76	5.481	.16791	12.500	.012		
COMPUTE NM HYD	AP-19UB	-	1	.15250	16.02	.753	.09263	1.650	.164	PER IMP=	.00
DIVIDE HYD	AP-19	1	17	.15250	16.82	.791	.09726	1.650	.172		
	150.12	AND	16	.15250	.80	.038	.00463	1.650	.008		
ADD HYD	150.13	10&17	10	.76450	16.83	6.272	.15382	1.650	.034		
COMPUTE NM HYD	P-1	-	3	.00290	.55	.017	.11274	1.550	.294	PER IMP=	2.00
ADD HYD	AP-20	10& 3	10	.76740	17.28	6.288	.15364	1.600	.035		
ROUTE RESERVOIR	150.14	10	5	.76740	4.38	6.150	.15026	14.650	.009	AC-FT=	.672
COMPUTE NM HYD	AP-21&G-1D	-	3	.02290	3.61	.102	.08362	1.550	.246	PER IMP=	.00
COMPUTE NM HYD	AP-22&G-2D	-	4	.00310	4.90	.215	1.30254	1.500	2.469	PER IMP=	82.00
ROUTE	AP-22R	4	12	.00310	4.76	.215	1.30270	1.500	2.398		
COMPUTE NM HYD	AP-23&G-3D	-	5	.03510	4.10	.157	.08362	1.600	.183	PER IMP=	.00
COMPUTE NM HYD	G-4D	-	6	.00330	5.08	.220	1.24983	1.500	2.407	PER IMP=	78.00
ADD HYD	AP-24	12& 6	10	.00640	9.84	.435	1.27528	1.500	2.403		

FINISH

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994 RUN DATE (MON/DAY/YR) =01/29/2001
INPUT FILE = WTF100EP.IN USER NO.= C_ANDRSN.I01

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION

*S EXISTING CONDITIONS, WITHOUT I-40 INTERCEPTOR IN PLACE AND
*S WITH THE WEST SIDE TRANSIT FACILITY FULLY DEVELOPED
*S 100 YEAR, 24 HOUR STORM EVENT
*S
*S FILE NAME: WTF100EP.IN
*S
*S THIS MODEL ANALYZES THE UNSER DIVERSION PONDS WITH THE EXISTING POND
*S OUTLET WORKS AND THE PROPOSED POND GRADING.
*S
*S MASTER DRAINAGE PLAN FOR THE WEST SIDE TRANSIT FACILITY
*S
*S THIS AHYMO FILE ANALYZES THE UNSER DIVERSION (DETENTION PONDS) ASSUMING
*S EXISTING CONDITIONS WITHOUT THE I-40 INTERCEPTOR IN PLACE
*S AND THE WEST SIDE TRANSIT FACILITY FULLY DEVELOPED.
*S
*S NOTES: UNSER DIVERSION POND NO. 5 IS USED IN EXISTING CONDITIONS.
*S THE OUTLET RATING CURVES WERE ALSO BASED UPON TAILWATER
*S ELEVATIONS SHOWN ON THE UNSER DIVERSION (AS-BUILT)
*S CONSTRUCTION PLAN SET (PLANS BY CHUCK EASTERLING).
*S DRAINAGE BASINS SOUTH OF INTERSTATE 40 (WITH THE EXCEPTION
*S OF THE WEST SIDE TRANSIT FACILITY AREA) WERE TAKEN FROM
*S THE "MASTER DRAINAGE PLAN FOR THE ATRISCO BUSINESS PARK"
*S REVISED OCTOBER 1993, PREPARED BY EASTERLING & ASSOCIATES, INC.
*S DATE: 11/21/00
START TIME= .00
RAINFALL TYPE= 2 RAIN24= 2.660
*S BEGIN OFFSITE WATERSHED NORTH OF INTERSTATE 40 - EXISTING CONDITION

COMPUTE NM HYD	O-1E	-	3	.02420	18.06	.567	.43936	1.550	1.166	PER IMP= .00
COMPUTE NM HYD	O-2E	-	4	.04220	24.34	.989	.43936	1.600	.901	PER IMP= .00
ADD HYD	AP-1	3& 4	10	.06640	41.88	1.556	.43936	1.550	.986	
ROUTE	AP-1R	10	2	.06640	36.11	1.556	.43938	1.650	.850	
COMPUTE NM HYD	O-3E	-	3	.01640	13.10	.402	.45924	1.500	1.248	PER IMP= 1.00
ADD HYD	PARTAP-2	2& 3	10	.08280	44.73	1.958	.44330	1.650	.844	
COMPUTE NM HYD	AP-7&O-5E	-	3	.10970	63.37	2.803	.47912	1.600	.903	PER IMP= 2.00
DIVIDE HYD	PART_AP-7	3	98	.08594	32.00	2.196	.47911	1.500	.582	
	PART_AP-7	AND	99	.02376	31.37	.607	.47911	1.600	2.062	
ROUTE	AP-7R	99	2	.02376	31.64	.608	.47944	1.600	2.081	
ADD HYD	AP-2	10& 2	10	.10656	76.00	2.565	.45135	1.600	1.114	
ROUTE	AP-2R	10	2	.10656	79.83	2.565	.45136	1.600	1.171	
COMPUTE NM HYD	I40-3	-	3	.00190	2.86	.115	1.13512	1.500	2.350	PER IMP= 35.00
ADD HYD	AP-3	2& 3	20	.10846	81.89	2.680	.46333	1.600	1.180	
COMPUTE NM HYD	AP-4&O-4E	-	3	.00940	8.11	.240	.47912	1.500	1.348	PER IMP= 2.00
ROUTE	AP-4R	3	2	.00940	8.17	.240	.47921	1.500	1.358	
COMPUTE NM HYD	I40-4	-	3	.00120	1.81	.073	1.13512	1.500	2.357	PER IMP= 35.00
ADD HYD	AP-5	2& 3	10	.01060	9.98	.313	.55337	1.500	1.471	
ADD HYD	AP-3+AP-5	20&10	10	.11906	89.90	2.993	.47135	1.600	1.180	
ROUTE	PARTAP-6	10	12	.11906	87.97	2.993	.47135	1.650	1.154	
ROUTE	AP-7R-CUL	98	2	.08594	33.23	2.196	.47912	1.500	.604	
COMPUTE NM HYD	I40-2	-	3	.00310	4.65	.188	1.13512	1.500	2.346	PER IMP= 35.00

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION

ADD HYD	AP-8	2& 3	20	.08904	37.88	2.384	.50195	1.500	.665	
COMPUTE NM HYD	O-6E	-	3	.07320	35.69	1.715	.43936	1.600	.762	PER IMP= .00
COMPUTE NM HYD	O-7E	-	4	.05230	39.03	1.226	.43936	1.550	1.166	PER IMP= .00
ADD HYD	AP-9	3& 4	10	.12550	71.47	2.941	.43936	1.550	.890	
ROUTE	AP-9R	10	2	.12550	70.89	2.941	.43937	1.600	.883	
COMPUTE NM HYD	O-8E	-	3	.00990	23.00	1.124	2.12906	1.500	3.630	PER IMP= 85.00
ADD HYD	AP-10	2& 3	10	.13540	92.59	4.065	.56290	1.550	1.068	
ROUTE	AP-10R	10	2	.13540	89.10	4.065	.56291	1.600	1.028	
COMPUTE NM HYD	O-9E	-	3	.03480	31.99	1.000	.53875	1.500	1.436	PER IMP= 5.00
ADD HYD	AP-11	2& 3	10	.17020	114.14	5.065	.55797	1.600	1.048	
ROUTE	AP-11R	10	2	.17020	114.79	5.065	.55797	1.600	1.054	
COMPUTE NM HYD	I40-1	-	3	.00750	11.24	.454	1.13512	1.500	2.342	PER IMP= 35.00
ADD HYD	AP-12	2& 3	30	.17770	122.86	5.519	.58232	1.600	1.080	
ROUTE	AP-8R	20	22	.08904	38.28	2.384	.50196	1.550	.672	
COMPUTE NM HYD	130.70	-	1	.01696	15.02	.425	.46988	1.500	1.384	PER IMP= .00
COMPUTE NM HYD	130.90	-	2	.00954	8.45	.239	.46988	1.500	1.385	PER IMP= .00
ADD HYD	130.91	1& 2	1	.02650	23.48	.664	.46988	1.500	1.384	
ROUTE	AP-12R	30	3	.17770	121.65	5.519	.58233	1.600	1.070	
ADD HYD	130.72	1& 3	4	.20420	140.42	6.183	.56773	1.600	1.074	
ADD HYD	AP-13	4&22	2	.29324	176.26	8.567	.54776	1.600	.939	
ROUTE	AP-13R	2	3	.29324	172.47	8.567	.54776	1.650	.919	
COMPUTE NM HYD	130.80	-	1	.03010	23.38	.705	.43936	1.550	1.214	PER IMP= .00
ADD HYD	130.81	3& 1	10	.32334	188.04	9.272	.53767	1.650	.909	
COMPUTE NM HYD	P-5	-	3	.00290	2.51	.074	.47912	1.500	1.353	PER IMP= 2.00
ADD HYD	AP-15UB	10& 3	11	.32624	189.56	9.345	.53710	1.650	.908	
COMPUTE NM HYD	120.50	-	3	.00520	4.28	.122	.43936	1.500	1.287	PER IMP= .00
ADD HYD	AP-6	3&12	10	.12426	90.59	3.115	.47001	1.650	1.139	
ROUTE	AP-6R	10	2	.12426	91.47	3.115	.47001	1.650	1.150	

COMPUTE NM HYD	A-1D	-	3	.02400	61.30	2.881	2.25115	1.500	3.991	PER IMP=	90.00
ADD HYD	AP-6A	2& 3	10	.14826	126.03	5.996	.75833	1.650	1.328		
COMPUTE NM HYD	AP-6B&A-2D	-	3	.01250	29.07	1.312	1.96827	1.500	3.634	PER IMP=	76.00
COMPUTE NM HYD	120.5A	-	4	.00200	1.65	.047	.43936	1.500	1.290	PER IMP=	.00
ADD HYD	AP-6B+120.5A	3& 4	20	.01450	30.72	1.359	1.75736	1.500	3.310		
ADD HYD	AP-6A+	10&20	10	.16276	146.46	7.355	.84732	1.600	1.406		
COMPUTE NM HYD	P-6	-	3	.00430	3.72	.110	.47912	1.500	1.350	PER IMP=	2.00
ADD HYD	AP-14	10& 3	18	.16706	149.41	7.465	.83784	1.600	1.397		
ROUTE RESERVOIR	AP-14R	18	2	.16706	10.68	7.465	.83784	2.400	.100	AC-FT=	5.290
ROUTE	POND6ROUTE36	2	12	.16706	10.68	7.465	.83784	2.400	.100		
ROUTE	POND6ROUTE48	12	22	.16706	10.68	7.465	.83784	2.400	.100		
DIVIDE HYD	AP-15	11	17	.32624	202.83	9.999	.57470	1.650	.971		
	130.83	AND	16	.32624	13.27	.654	.03760	1.650	.064		
ROUTE RESERVOIR	AP-15R	17	5	.32624	130.19	9.999	.57470	1.850	.624	AC-FT=	2.645
ROUTE	P-5R	5	11	.32624	131.24	9.999	.57470	1.800	.629		
ADD HYD	POND-5R&6R	11&22	10	.49330	140.80	17.465	.66381	1.800	.446		
COMPUTE NM HYD	P-4	-	3	.00360	3.11	.092	.47912	1.500	1.352	PER IMP=	2.00
ADD HYD	AP-15A	10& 3	10	.49690	141.59	17.556	.66246	1.800	.445		
ROUTE RESERVOIR	AP-15AR	10	5	.49690	93.71	17.469	.65919	2.200	.295	AC-FT=	4.650
ROUTE	P-4R	5	12	.49690	94.25	17.469	.65918	2.200	.296		
COMPUTE NM HYD	AP-16	-	1	.01960	18.50	.522	.49901	1.500	1.475	PER IMP=	.00
ROUTE	140.11	1	22	.01960	16.56	.522	.49908	1.550	1.320		
ROUTE	140.12	22	15	.01960	17.13	.522	.49909	1.600	1.366		
COMPUTE NM HYD	140.20	-	1	.08920	78.50	2.196	.46166	1.500	1.375	PER IMP=	.00

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION	
ADD HYD	AP-17UB	15& 1	2	.10880	91.60	2.718	.46840	1.550	1.316		
DIVIDE HYD	AP-17	2	17	.10880	98.93	2.935	.50587	1.550	1.421		
	140.23	AND	16	.10880	7.33	.217	.03747	1.550	.105		
ADD HYD	140.24	17&12	2	.60570	101.32	20.405	.63164	2.100	.261		
COMPUTE NM HYD	P-3	-	3	.00270	2.34	.069	.47912	1.500	1.354	PER IMP=	2.00
ADD HYD	AP-18	2& 3	10	.60840	101.79	20.473	.63094	1.550	.261		
ROUTE RESERVOIR	AP-18R	10	4	.60840	88.61	20.300	.62562	2.350	.228	AC-FT=	2.803
ROUTE	P-3R	4	2	.60840	88.68	20.300	.62560	2.300	.228		
COMPUTE NM HYD	P-2	-	3	.00360	3.11	.092	.47912	1.500	1.352	PER IMP=	2.00
ADD HYD	AP-18A	2& 3	10	.61200	88.84	20.391	.62473	2.300	.227		
ROUTE RESERVOIR	AP-18AR	10	5	.61200	12.66	18.518	.56733	6.150	.032	AC-FT=	7.951
ROUTE	P-2R	5	10	.61200	12.66	18.516	.56727	6.150	.032		
COMPUTE NM HYD	AP-19UB	-	1	.15250	85.35	3.692	.45390	1.600	.874	PER IMP=	.00
DIVIDE HYD	AP-19	1	17	.15250	89.62	3.876	.47660	1.600	.918		
	150.12	AND	16	.15250	4.27	.185	.02270	1.600	.044		
ADD HYD	150.13	10&17	10	.76450	89.63	22.392	.54918	1.600	.183		
COMPUTE NM HYD	P-1	-	3	.00290	2.51	.074	.47912	1.500	1.353	PER IMP=	2.00
ADD HYD	AP-20	10& 3	10	.76740	91.62	22.465	.54890	1.600	.187		
ROUTE RESERVOIR	150.14	10	5	.76740	10.03	21.364	.52198	8.450	.020	AC-FT=	3.875
COMPUTE NM HYD	AP-21&G-1D	-	3	.02290	18.84	.537	.43936	1.500	1.286	PER IMP=	.00
COMPUTE NM HYD	AP-22&G-2D	-	4	.00310	7.72	.349	2.11028	1.500	3.893	PER IMP=	82.00
ROUTE	AP-22R	4	12	.00310	7.56	.349	2.11044	1.500	3.808		
COMPUTE NM HYD	AP-23&G-3D	-	5	.03510	22.33	.822	.43936	1.550	.994	PER IMP=	.00
COMPUTE NM HYD	G-4D	-	6	.00330	8.10	.359	2.03985	1.500	3.837	PER IMP=	78.00
ADD HYD	AP-24	12& 6	10	.00640	15.66	.708	2.07389	1.500	3.823		
FINISH											

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994 RUN DATE (MON/DAY/YR) =01/30/2001
INPUT FILE = WTF10PP.IN USER NO.= C_ANDRSN.I01

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 1 NOTATION
*S EXISTING CONDITIONS, WITHOUT I-40 INTERCEPTOR IN PLACE AND										
*S WITH THE WEST SIDE TRANSIT FACILITY FULLY DEVELOPED										
*S 10 YEAR, 24 HOUR STORM EVENT										
*S										
*S FILE NAME: WTF10PP.IN										
*S										
*S THIS MODEL ANALYZES THE UNSER DIVERSION PONDS WITH THE PROPOSED POND										
*S OUTLET WORKS AND THE PROPOSED POND GRADING.										
*S										
*S MASTER DRAINAGE PLAN FOR THE WEST SIDE TRANSIT FACILITY										
*S										
*S THIS AHYMO FILE ANALYZES THE UNSER DIVERSION (DETENTION PONDS) ASSUMING										
*S EXISTING CONDITIONS WITHOUT THE I-40 INTERCEPTOR IN PLACE										
*S AND THE WEST SIDE TRANSIT FACILITY FULLY DEVELOPED.										
*S										
*S NOTES: UNSER DIVERSION POND NO. 5 IS USED IN EXISTING CONDITIONS.										
*S THE OUTLET RATING CURVES WERE ALSO BASED UPON TAILWATER										
*S ELEVATIONS SHOWN ON THE UNSER DIVERSION (AS-BUILT)										
*S CONSTRUCTION PLAN SET (PLANS BY CHUCK EASTERLING).										
*S DRAINAGE BASINS SOUTH OF INTERSTATE 40 (WITH THE EXCEPTION										
*S OF THE WEST SIDE TRANSIT FACILITY AREA) WERE TAKEN FROM										
*S THE "MASTER DRAINAGE PLAN FOR THE ATRISCO BUSINESS PARK"										
*S REVISED OCTOBER 1993, PREPARED BY EASTERLING & ASSOCIATES, INC.										
*S DATE: 11/21/00										
START										TIME= .00
RAINFALL TYPE= 2										RAIN24= 1.770
*S BEGIN OFFSITE WATERSHED NORTH OF INTERSTATE 40 - EXISTING CONDITION										
COMPUTE NM HYD	O-1E	-	3	.02420	3.43	.108	.08362	1.550	.221	PER IMP= .00
COMPUTE NM HYD	O-2E	-	4	.04220	4.48	.188	.08362	1.600	.166	PER IMP= .00
ADD HYD	AP-1	3& 4	10	.06640	7.65	.296	.08361	1.550	.180	
ROUTE	AP-1R	10	2	.06640	5.45	.296	.08362	1.700	.128	
COMPUTE NM HYD	O-3E	-	3	.01640	2.72	.086	.09818	1.550	.259	PER IMP= 1.00
ADD HYD	PARTAP-2	2& 3	10	.08280	7.02	.382	.08650	1.650	.132	
COMPUTE NM HYD	AP-7&O-5E	-	3	.10970	13.54	.660	.11274	1.600	.193	PER IMP= 2.00
DIVIDE HYD	PART_AP-7	3	98	.10970	13.54	.660	.11274	1.600	.193	
	PART_AP-7	AND	99	.00000	.00	.000	.00000	-.050	.000	
ROUTE	AP-7R	99	2	.00000	.00	.000	.00000	-.050	.000	
ADD HYD	AP-2	10& 2	10	.08280	7.02	.382	.08650	1.650	.132	
ROUTE	AP-2R	10	2	.08280	6.98	.382	.08651	1.700	.132	
COMPUTE NM HYD	I40-3	-	3	.00190	1.40	.060	.59326	1.500	1.154	PER IMP= 35.00
ADD HYD	AP-3	2& 3	20	.08470	7.72	.442	.09786	1.650	.142	
COMPUTE NM HYD	AP-4&O-4E	-	3	.00940	1.75	.057	.11274	1.550	.292	PER IMP= 2.00
ROUTE	AP-4R	3	2	.00940	1.84	.057	.11280	1.550	.306	
COMPUTE NM HYD	I40-4	-	3	.00120	.89	.038	.59326	1.500	1.156	PER IMP= 35.00
ADD HYD	AP-5	2& 3	10	.01060	2.64	.094	.16710	1.550	.389	
ADD HYD	AP-3+AP-5	20&10	10	.09530	9.60	.537	.10556	1.600	.157	
ROUTE	PARTAP-6	10	12	.09530	9.61	.537	.10557	1.600	.158	
ROUTE	AP-7R-CUL	98	2	.10970	13.58	.660	.11274	1.600	.193	
COMPUTE NM HYD	I40-2	-	3	.00310	2.28	.098	.59326	1.500	1.152	PER IMP= 35.00

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 2 NOTATION
ADD HYD	AP-8	2& 3	20	.11280	15.19	.758	.12594	1.600	.210	
COMPUTE NM HYD	O-6E	-	3	.07320	6.49	.326	.08362	1.650	.138	PER IMP= .00
COMPUTE NM HYD	O-7E	-	4	.05230	7.41	.233	.08362	1.550	.221	PER IMP= .00
ADD HYD	AP-9	3& 4	10	.12550	13.19	.560	.08362	1.600	.164	
ROUTE	AP-9R	10	2	.12550	13.27	.560	.08362	1.600	.165	
COMPUTE NM HYD	O-8E	-	3	.00990	14.68	.698	1.32131	1.500	2.318	PER IMP= 85.00
ADD HYD	AP-10	2& 3	10	.13540	26.66	1.257	.17411	1.550	.308	
ROUTE	AP-10R	10	2	.13540	21.80	1.257	.17411	1.650	.252	
COMPUTE NM HYD	O-9E	-	3	.03480	8.15	.290	.15643	1.500	.366	PER IMP= 5.00
ADD HYD	AP-11	2& 3	10	.17020	27.81	1.548	.17049	1.600	.255	
ROUTE	AP-11R	10	2	.17020	27.65	1.548	.17050	1.600	.254	
COMPUTE NM HYD	I40-1	-	3	.00750	5.52	.237	.59326	1.500	1.150	PER IMP= 35.00
ADD HYD	AP-12	2& 3	30	.17770	31.52	1.785	.18833	1.600	.277	
ROUTE	AP-8R	20	22	.11280	15.03	.758	.12595	1.650	.208	
COMPUTE NM HYD	130.70	-	1	.01696	3.27	.092	.10184	1.550	.301	PER IMP= .00
COMPUTE NM HYD	130.90	-	2	.00954	1.84	.052	.10184	1.550	.301	PER IMP= .00
ADD HYD	130.91	1& 2	1	.02650	5.11	.144	.10183	1.550	.301	
ROUTE	AP-12R	30	3	.17770	30.80	1.785	.18834	1.650	.271	
ADD HYD	130.72	1& 3	4	.20420	34.10	1.929	.17711	1.650	.261	
ADD HYD	AP-13	4&22	2	.31700	49.13	2.686	.15890	1.650	.242	
ROUTE	AP-13R	2	3	.31700	44.96	2.687	.15890	1.700	.222	
COMPUTE NM HYD	130.80	-	1	.03010	4.52	.134	.08362	1.550	.235	PER IMP= .00
ADD HYD	130.81	3& 1	10	.34710	47.44	2.821	.15237	1.700	.214	
COMPUTE NM HYD	P-5	-	3	.00290	.55	.017	.11274	1.550	.294	PER IMP= 2.00
ADD HYD	AP-15UB	10& 3	11	.35000	47.71	2.837	.15199	1.700	.213	
COMPUTE NM HYD	120.50	-	3	.00520	.82	.023	.08362	1.550	.246	PER IMP= .00
ADD HYD	AP-6	3&12	10	.10050	10.32	.560	.10443	1.600	.160	
ROUTE	AP-6R	10	2	.10050	10.45	.560	.10444	1.650	.162	

COMPUTE NM HYD	A-1D	-	3	.02400	39.69	1.802	1.40796	1.500	2.584	PER IMP=	90.00
ADD HYD	AP-6A	2& 3	10	.12450	43.13	2.362	.35571	1.500	.541		
COMPUTE NM HYD	AP-6B&A-2D	-	3	.01250	18.04	.801	1.20080	1.500	2.256	PER IMP=	76.00
COMPUTE NM HYD	120.5A	-	4	.00200	.32	.009	.08362	1.550	.247	PER IMP=	.00
ADD HYD	AP-6B+120.5A	3& 4	20	.01450	18.35	.809	1.04667	1.500	1.977		
ADD HYD	AP-6A+	10&20	10	.13900	61.48	3.171	.42779	1.500	.691		
COMPUTE NM HYD	P-6	-	3	.00430	.81	.026	.11274	1.550	.293	PER IMP=	2.00
ADD HYD	AP-14	10& 3	18	.14330	62.27	3.196	.41823	1.500	.679		
ROUTE RESERVOIR	AP-14R	18	2	.14330	19.45	3.196	.41823	2.000	.212	AC-FT=	1.412
ROUTE	POND6ROUTE36	2	12	.14330	19.44	3.196	.41822	2.000	.212		
ROUTE	POND6ROUTE48	12	22	.14330	19.44	3.196	.41822	2.000	.212		
DIVIDE HYD	AP-15	11	17	.35000	51.05	3.036	.16263	1.700	.228		
	130.83	AND	16	.35000	3.34	.199	.01064	1.700	.015		
ROUTE RESERVOIR	AP-15R	17	5	.35000	23.22	3.036	.16263	2.050	.104	AC-FT=	1.289
ROUTE	P-5R	5	11	.35000	23.50	3.036	.16263	2.050	.105		
ADD HYD	POND-5R&6R	11&22	10	.49330	42.93	6.232	.23688	2.050	.136		
COMPUTE NM HYD	P-4	-	3	.00360	.68	.022	.11274	1.550	.293	PER IMP=	2.00
ADD HYD	AP-15A	10& 3	10	.49690	43.01	6.253	.23594	2.050	.135		
ROUTE RESERVOIR	AP-15AR	10	5	.49690	29.74	6.172	.23291	2.400	.094	AC-FT=	1.014
ROUTE	P-4R	5	12	.49690	29.74	6.172	.23291	2.400	.094		
COMPUTE NM HYD	AP-16	-	1	.01960	4.43	.124	.11841	1.550	.353	PER IMP=	.00
ROUTE	140.11	1	22	.01960	3.49	.124	.11845	1.600	.278		
ROUTE	140.12	22	15	.01960	3.47	.124	.11845	1.650	.276		
COMPUTE NM HYD	140.20	-	1	.08920	16.45	.462	.09721	1.550	.288	PER IMP=	.00

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION	
ADD HYD	AP-17UB	15& 1	2	.10880	18.28	.586	.10103	1.550	.262		
DIVIDE HYD	AP-17	2	17	.10880	19.74	.633	.10911	1.550	.283		
	140.23	AND	16	.10880	1.46	.047	.00808	1.550	.021		
ADD HYD	140.24	17&12	2	.60570	30.91	6.805	.21067	2.400	.080		
COMPUTE NM HYD	P-3	-	3	.00270	.51	.016	.11274	1.550	.294	PER IMP=	2.00
ADD HYD	AP-18	2& 3	10	.60840	30.93	6.821	.21021	2.400	.079		
ROUTE RESERVOIR	AP-18R	10	4	.60840	26.20	6.752	.20808	2.850	.067	AC-FT=	1.137
ROUTE	P-3R	4	2	.60840	26.20	6.752	.20808	2.850	.067		
COMPUTE NM HYD	P-2	-	3	.00360	.68	.022	.11274	1.550	.293	PER IMP=	2.00
ADD HYD	AP-18A	2& 3	10	.61200	26.21	6.773	.20750	2.850	.067		
ROUTE RESERVOIR	AP-18AR	10	5	.61200	18.42	5.724	.17538	3.900	.047	AC-FT=	2.373
ROUTE	P-2R	5	10	.61200	18.42	5.724	.17537	3.900	.047		
COMPUTE NM HYD	AP-19UB	-	1	.15250	16.02	.753	.09263	1.650	.164	PER IMP=	.00
DIVIDE HYD	AP-19	1	17	.15250	16.82	.791	.09726	1.650	.172		
	150.12	AND	16	.15250	.80	.038	.00463	1.650	.008		
ADD HYD	150.13	10&17	10	.76450	18.72	6.515	.15979	3.900	.038		
COMPUTE NM HYD	P-1	-	3	.00290	.55	.017	.11274	1.550	.294	PER IMP=	2.00
ADD HYD	AP-20	10& 3	10	.76740	18.72	6.532	.15959	3.900	.038		
ROUTE RESERVOIR	150.14	10	5	.76740	17.12	6.481	.15834	4.550	.035	AC-FT=	.874
COMPUTE NM HYD	AP-21&G-1D	-	3	.02290	3.67	.102	.08362	1.550	.251	PER IMP=	.00
COMPUTE NM HYD	AP-22&G-2D	-	4	.00310	4.90	.215	1.30254	1.500	2.469	PER IMP=	82.00
ROUTE	AP-22R	4	12	.00310	4.76	.215	1.30270	1.500	2.398		
COMPUTE NM HYD	AP-23&G-3D	-	5	.03510	4.10	.157	.08362	1.600	.183	PER IMP=	.00
COMPUTE NM HYD	G-4D	-	6	.00330	5.08	.220	1.24983	1.500	2.407	PER IMP=	78.00
ADD HYD	AP-24	12& 6	10	.00640	9.84	.435	1.27528	1.500	2.403		

FINISH

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994 RUN DATE (MON/DAY/YR) =01/29/2001
INPUT FILE = WTF100PP.IN USER NO.= C_ANDRSN.I01

PAGE = 1

*S EXISTING CONDITIONS, WITHOUT I-40 INTERCEPTOR IN PLACE AND
*S WITH THE WEST SIDE TRANSIT FACILITY FULLY DEVELOPED
*S 100 YEAR, 24 HOUR STORM EVENT
*S
*S FILE NAME: WTF100PP.IN
*S
*S THIS MODEL ANALYZES THE UNSER DIVERSION PONDS WITH THE PROPOSED POND
*S OUTLET WORKS AND THE PROPOSED POND GRADING.
*S
*S MASTER DRAINAGE PLAN FOR THE WEST SIDE TRANSIT FACILITY
*S
*S THIS AHYMO FILE ANALYZES THE UNSER DIVERSION (DETENTION PONDS) ASSUMING
*S EXISTING CONDITIONS WITHOUT THE I-40 INTERCEPTOR IN PLACE
*S AND THE WEST SIDE TRANSIT FACILITY FULLY DEVELOPED.
*S
*S NOTES: UNSER DIVERSION POND NO. 5 IS USED IN EXISTING CONDITIONS.
*S THE OUTLET RATING CURVES WERE ALSO BASED UPON TAILWATER
*S ELEVATIONS SHOWN ON THE UNSER DIVERSION (AS-BUILT)
*S CONSTRUCTION PLAN SET (PLANS BY CHUCK EASTERLING).
*S DRAINAGE BASINS SOUTH OF INTERSTATE 40 (WITH THE EXCEPTION
*S OF THE WEST SIDE TRANSIT FACILITY AREA) WERE TAKEN FROM
*S THE "MASTER DRAINAGE PLAN FOR THE ATRISCO BUSINESS PARK"
*S REVISED OCTOBER 1993, PREPARED BY EASTERLING & ASSOCIATES, INC.
*S DATE: 11/21/00

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION
START										TIME= .00
RAINFALL TYPE= 2										RAIN24= 2.660
*S BEGIN OFFSITE WATERSHED NORTH OF INTERSTATE 40 - EXISTING CONDITION										
COMPUTE NM HYD	0-1E	-	3	.02420	18.06	.567	.43936	1.550	1.166	PER IMP= .00
COMPUTE NM HYD	0-2E	-	4	.04220	24.34	.989	.43936	1.600	.901	PER IMP= .00
ADD HYD	AP-1	3& 4	10	.06640	41.88	1.556	.43936	1.550	.986	
ROUTE	AP-1R	10	2	.06640	36.11	1.556	.43938	1.650	.850	
COMPUTE NM HYD	0-3E	-	3	.01640	13.10	.402	.45924	1.500	1.248	PER IMP= 1.00
ADD HYD	PARTAP-2	2& 3	10	.08280	44.73	1.958	.44330	1.650	.844	
COMPUTE NM HYD	AP-7&O-5E	-	3	.10970	63.37	2.803	.47912	1.600	.903	PER IMP= 2.00
DIVIDE HYD	PART_AP-7	3	98	.08594	32.00	2.196	.47911	1.500	.582	
	PART_AP-7	AND	99	.02376	31.37	.607	.47911	1.600	2.062	
ROUTE	AP-7R	99	2	.02376	31.64	.608	.47944	1.600	2.081	
ADD HYD	AP-2	10& 2	10	.10656	76.00	2.565	.45135	1.600	1.114	
ROUTE	AP-2R	10	2	.10656	79.83	2.565	.45136	1.600	1.171	
COMPUTE NM HYD	I40-3	-	3	.00190	2.86	.115	1.13512	1.500	2.350	PER IMP= 35.00
ADD HYD	AP-3	2& 3	20	.10846	81.89	2.680	.46333	1.600	1.180	
COMPUTE NM HYD	AP-4&O-4E	-	3	.00940	8.11	.240	.47912	1.500	1.348	PER IMP= 2.00
ROUTE	AP-4R	3	2	.00940	8.17	.240	.47921	1.500	1.358	
COMPUTE NM HYD	I40-4	-	3	.00120	1.81	.073	1.13512	1.500	2.357	PER IMP= 35.00
ADD HYD	AP-5	2& 3	10	.01060	9.98	.313	.55337	1.500	1.471	
ADD HYD	AP-3+AP-5	20&10	10	.11906	89.90	2.993	.47135	1.600	1.180	
ROUTE	PARTAP-6	10	12	.11906	87.97	2.993	.47135	1.650	1.154	
ROUTE	AP-7R-CUL	98	2	.08594	33.23	2.196	.47912	1.500	.604	
COMPUTE NM HYD	I40-2	-	3	.00310	4.65	.188	1.13512	1.500	2.346	PER IMP= 35.00

□

PAGE = 2

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION
ADD HYD	AP-8	2& 3	20	.08904	37.88	2.384	.50195	1.500	.665	
COMPUTE NM HYD	0-6E	-	3	.07320	35.69	1.715	.43936	1.600	.762	PER IMP= .00
COMPUTE NM HYD	0-7E	-	4	.05230	39.03	1.226	.43936	1.550	1.166	PER IMP= .00
ADD HYD	AP-9	3& 4	10	.12550	71.47	2.941	.43936	1.550	.890	
ROUTE	AP-9R	10	2	.12550	70.89	2.941	.43937	1.600	.883	
COMPUTE NM HYD	0-8E	-	3	.00990	23.00	1.124	2.12906	1.500	3.630	PER IMP= 85.00
ADD HYD	AP-10	2& 3	10	.13540	92.59	4.065	.56290	1.550	1.068	
ROUTE	AP-10R	10	2	.13540	89.10	4.065	.56291	1.600	1.028	
COMPUTE NM HYD	0-9E	-	3	.03480	31.99	1.000	.53875	1.500	1.436	PER IMP= 5.00
ADD HYD	AP-11	2& 3	10	.17020	114.14	5.065	.55797	1.600	1.048	
ROUTE	AP-11R	10	2	.17020	114.79	5.065	.55797	1.600	1.054	
COMPUTE NM HYD	I40-1	-	3	.00750	11.24	.454	1.13512	1.500	2.342	PER IMP= 35.00
ADD HYD	AP-12	2& 3	30	.17770	122.86	5.519	.58232	1.600	1.080	
ROUTE	AP-8R	20	22	.08904	38.28	2.384	.50196	1.550	.672	
COMPUTE NM HYD	130.70	-	1	.01696	15.02	.425	.46988	1.500	1.384	PER IMP= .00
COMPUTE NM HYD	130.90	-	2	.00954	8.45	.239	.46988	1.500	1.385	PER IMP= .00
ADD HYD	130.91	1& 2	1	.02650	23.48	.664	.46988	1.500	1.384	
ROUTE	AP-12R	30	3	.17770	121.65	5.519	.58233	1.600	1.070	
ADD HYD	130.72	1& 3	4	.20420	140.42	6.183	.56773	1.600	1.074	
ADD HYD	AP-13	4&22	2	.29324	176.26	8.567	.54776	1.600	.939	
ROUTE	AP-13R	2	3	.29324	172.47	8.567	.54776	1.650	.919	
COMPUTE NM HYD	130.80	-	1	.03010	23.38	.705	.43936	1.550	1.214	PER IMP= .00
ADD HYD	130.81	3& 1	10	.32334	188.04	9.272	.53767	1.650	.909	
COMPUTE NM HYD	P-5	-	3	.00290	2.51	.074	.47912	1.500	1.353	PER IMP= 2.00
ADD HYD	AP-15UB	10& 3	11	.32624	189.56	9.345	.53710	1.650	.908	
COMPUTE NM HYD	120.50	-	3	.00520	4.28	.122	.43936	1.500	1.287	PER IMP= .00
ADD HYD	AP-6	3&12	10	.12426	90.59	3.115	.47001	1.650	1.139	
ROUTE	AP-6R	10	2	.12426	91.47	3.115	.47001	1.650	1.150	

COMPUTE NM HYD	A-1D	-	3	.02400	61.30	2.881	2.25115	1.500	3.991	PER IMP=	90.00
ADD HYD	AP-6A	2& 3	10	.14826	126.03	5.996	.75833	1.650	1.328		
COMPUTE NM HYD	AP-6B&A-2D	-	3	.01250	29.07	1.312	1.96827	1.500	3.634	PER IMP=	76.00
COMPUTE NM HYD	120.5A	-	4	.00200	1.65	.047	.43936	1.500	1.290	PER IMP=	.00
ADD HYD	AP-6B+120.5A	3& 4	20	.01450	30.72	1.359	1.75736	1.500	3.310		
ADD HYD	AP-6A+	10&20	10	.16276	146.46	7.355	.84732	1.600	1.406		
COMPUTE NM HYD	P-6	-	3	.00430	3.72	.110	.47912	1.500	1.350	PER IMP=	2.00
ADD HYD	AP-14	10& 3	18	.16706	149.41	7.465	.83784	1.600	1.397		
ROUTE RESERVOIR	AP-14R	18	2	.16706	45.45	7.465	.83784	1.950	.425	AC-FT=	3.692
ROUTE	POND6ROUTE36	2	12	.16706	45.46	7.465	.83784	2.000	.425		
ROUTE	POND6ROUTE48	12	22	.16706	45.46	7.465	.83784	2.000	.425		
DIVIDE HYD	AP-15	11	17	.32624	202.83	9.999	.57470	1.650	.971		
	130.83	AND	16	.32624	13.27	.654	.03760	1.650	.064		
ROUTE RESERVOIR	AP-15R	17	5	.32624	130.19	9.999	.57470	1.850	.624	AC-FT=	2.645
ROUTE	P-5R	5	11	.32624	131.24	9.999	.57470	1.800	.629		
ADD HYD	POND-5R&6R	11&22	10	.49330	175.50	17.465	.66382	1.900	.556		
COMPUTE NM HYD	P-4	-	3	.00360	3.11	.092	.47912	1.500	1.352	PER IMP=	2.00
ADD HYD	AP-15A	10& 3	10	.49690	175.95	17.556	.66246	1.900	.553		
ROUTE RESERVOIR	AP-15AR	10	5	.49690	98.28	17.475	.65941	2.250	.309	AC-FT=	5.131
ROUTE	P-4R	5	12	.49690	98.21	17.475	.65941	2.300	.309		
COMPUTE NM HYD	AP-16	-	1	.01960	18.50	.522	.49901	1.500	1.475	PER IMP=	.00
ROUTE	140.11	1	22	.01960	16.56	.522	.49908	1.550	1.320		
ROUTE	140.12	22	15	.01960	17.13	.522	.49909	1.600	1.366		
COMPUTE NM HYD	140.20	-	1	.08920	78.50	2.196	.46166	1.500	1.375	PER IMP=	.00

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 3 NOTATION	
ADD HYD	AP-17UB	15& 1	2	.10880	91.60	2.718	.46840	1.550	1.316		
DIVIDE HYD	AP-17	2	17	.10880	98.93	2.935	.50587	1.550	1.421		
	140.23	AND	16	.10880	7.33	.217	.03747	1.550	.105		
ADD HYD	140.24	17&12	2	.60570	104.93	20.411	.63183	1.550	.271		
COMPUTE NM HYD	P-3	-	3	.00270	2.34	.069	.47912	1.500	1.354	PER IMP=	2.00
ADD HYD	AP-18	2& 3	10	.60840	107.19	20.479	.63113	1.550	.275		
ROUTE RESERVOIR	AP-18R	10	4	.60840	92.84	20.410	.62901	2.650	.238	AC-FT=	3.039
ROUTE	P-3R	4	2	.60840	92.83	20.410	.62901	2.650	.238		
COMPUTE NM HYD	P-2	-	3	.00360	3.11	.092	.47912	1.500	1.352	PER IMP=	2.00
ADD HYD	AP-18A	2& 3	10	.61200	92.90	20.502	.62811	2.650	.237		
ROUTE RESERVOIR	AP-18AR	10	5	.61200	48.57	19.452	.59597	3.700	.124	AC-FT=	7.496
ROUTE	P-2R	5	10	.61200	48.57	19.452	.59597	3.700	.124		
COMPUTE NM HYD	AP-19UB	-	1	.15250	85.35	3.692	.45390	1.600	.874	PER IMP=	.00
DIVIDE HYD	AP-19	1	17	.15250	89.62	3.876	.47660	1.600	.918		
	150.12	AND	16	.15250	4.27	.185	.02270	1.600	.044		
ADD HYD	150.13	10&17	10	.76450	89.64	23.329	.57216	1.600	.183		
COMPUTE NM HYD	P-1	-	3	.00290	2.51	.074	.47912	1.500	1.353	PER IMP=	2.00
ADD HYD	AP-20	10& 3	10	.76740	91.63	23.402	.57179	1.600	.187		
ROUTE RESERVOIR	150.14	10	5	.76740	48.42	23.351	.57053	4.200	.099	AC-FT=	2.311
COMPUTE NM HYD	AP-21&G-1D	-	3	.02290	19.35	.537	.43936	1.500	1.320	PER IMP=	.00
COMPUTE NM HYD	AP-22&G-2D	-	4	.00310	7.72	.349	2.11028	1.500	3.893	PER IMP=	82.00
ROUTE	AP-22R	4	12	.00310	7.56	.349	2.11044	1.500	3.808		
COMPUTE NM HYD	AP-23&G-3D	-	5	.03510	22.33	.822	.43936	1.550	.994	PER IMP=	.00
COMPUTE NM HYD	G-4D	-	6	.00330	8.10	.359	2.03985	1.500	3.837	PER IMP=	78.00
ADD HYD	AP-24	12& 6	10	.00640	15.66	.708	2.07389	1.500	3.823		

FINISH

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994 RUN DATE (MON/DAY/YR) =01/30/2001
INPUT FILE = INT10EE.IN USER NO.= C_ANDRSN.I01

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 1 NOTATION
*S DEVELOPED INTERIM CONDITIONS, WITHOUT I-40 INTERCEPTOR IN PLACE										
*S 10 YEAR, 24 HOUR STORM EVENT										
*S										
*S FILE NAME: INT10EE.IN										
*S										
*S THIS MODEL ANALYZES THE UNSER DIVERSION PONDS WITH THE EXISTING POND										
*S OUTLET WORKS AND THE EXISTING POND GRADING.										
*S										
*S MASTER DRAINAGE PLAN FOR THE WEST SIDE TRANSIT FACILITY										
*S THIS AHYMO FILE ANALYZES THE UNSER DIVERSION (DETENTION PONDS) ASSUMING										
*S DEVELOPED CONDITIONS SOUTH OF INTERSTATE 40 WITHOUT THE I-40 INTERCEPTOR										
*S IN PLACE. IN OTHER WORDS, FULLY DEVELOPED CONDITIONS SOUTH OF I-40										
*S INCLUDING EXISTING CONDITION OFFSITE FLOWS NORTH OF INTERSTATE 40.										
*S										
*S NOTES: UNSER DIVERSION POND NO. 5 IS USED IN DEVELOPED/INTERIM CONDITIONS.										
*S THE OUTLET RATING CURVES WERE ALSO BASED UPON TAILWATER										
*S ELEVATIONS SHOWN ON THE UNSER DIVERSION (AS-BUILT)										
*S CONSTRUCTION PLAN SET (PLANS BY CHUCK EASTERLING)										
*S										
*S DATE: 11/8/00										
START										TIME= .00
RAINFALL TYPE= 2										RAIN24= 1.770
*S BEGIN WATERSHED NORTH OF INTERSTATE 40										
COMPUTE NM HYD	O-1E	-	3	.02420	3.43	.108	.08362	1.550	.221	PER IMP= .00
COMPUTE NM HYD	O-2E	-	4	.04220	4.48	.188	.08362	1.600	.166	PER IMP= .00
ADD HYD	AP-1	3& 4	10	.06640	7.65	.296	.08361	1.550	.180	
ROUTE	AP-1R	10	2	.06640	5.45	.296	.08362	1.700	.128	
COMPUTE NM HYD	O-3E	-	3	.01640	2.72	.086	.09818	1.550	.259	PER IMP= 1.00
ADD HYD	PARTAP-2	2& 3	10	.08280	7.02	.382	.08650	1.650	.132	
COMPUTE NM HYD	AP-7&O-5E	-	3	.10970	13.54	.660	.11274	1.600	.193	PER IMP= 2.00
DIVIDE HYD	PART_AP-7	3	98	.10970	13.54	.660	.11274	1.600	.193	
	PART_AP-7	AND	99	.00000	.00	.000	.00000	-.050	.000	
ROUTE	AP-7R	99	2	.00000	.00	.000	.00000	-.050	.000	
ADD HYD	AP-2	10& 2	10	.08280	7.02	.382	.08650	1.650	.132	
ROUTE	AP-2R	10	2	.08280	6.98	.382	.08651	1.700	.132	
COMPUTE NM HYD	I40-3	-	3	.00190	1.40	.060	.59326	1.500	1.154	PER IMP= 35.00
ADD HYD	AP-3	2& 3	20	.08470	7.72	.442	.09786	1.650	.142	
COMPUTE NM HYD	AP-4&O-4E	-	3	.00940	1.75	.057	.11274	1.550	.292	PER IMP= 2.00
ROUTE	AP-4R	3	2	.00940	1.84	.057	.11280	1.550	.306	
COMPUTE NM HYD	I40-4	-	3	.00120	.89	.038	.59326	1.500	1.156	PER IMP= 35.00
ADD HYD	AP-5	2& 3	10	.01060	2.64	.094	.16710	1.550	.389	
ADD HYD	AP-3+AP-5	20&10	10	.09530	9.60	.537	.10556	1.600	.157	
ROUTE	AP-6	10	12	.09530	9.61	.537	.10557	1.600	.158	
ROUTE	AP-7R-CUL	98	2	.10970	13.58	.660	.11274	1.600	.193	
COMPUTE NM HYD	I40-2	-	3	.00310	2.28	.098	.59326	1.500	1.152	PER IMP= 35.00
ADD HYD	AP-8	2& 3	20	.11280	15.19	.758	.12594	1.600	.210	
COMPUTE NM HYD	O-6E	-	3	.07320	6.49	.326	.08362	1.650	.138	PER IMP= .00
COMPUTE NM HYD	O-7E	-	4	.05230	7.41	.233	.08362	1.550	.221	PER IMP= .00
ADD HYD	AP-9	3& 4	10	.12550	13.19	.560	.08362	1.600	.164	

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 2 NOTATION
ROUTE	AP-9R	10	2	.12550	13.27	.560	.08362	1.600	.165	
COMPUTE NM HYD	O-8E	-	3	.00990	14.68	.698	1.32131	1.500	2.318	PER IMP= 85.00
ADD HYD	AP-10	2& 3	10	.13540	26.66	1.257	.17411	1.550	.308	
ROUTE	AP-10R	10	2	.13540	21.80	1.257	.17411	1.650	.252	
COMPUTE NM HYD	O-9E	-	3	.03480	8.15	.290	.15643	1.500	.366	PER IMP= 5.00
ADD HYD	AP-11	2& 3	10	.17020	27.81	1.548	.17049	1.600	.255	
ROUTE	AP-11R	10	2	.17020	27.65	1.548	.17050	1.600	.254	
COMPUTE NM HYD	I40-1	-	3	.00750	5.52	.237	.59326	1.500	1.150	PER IMP= 35.00
ADD HYD	AP-12	2& 3	30	.17770	31.52	1.785	.18833	1.600	.277	
*S BEGIN WATERSHED SOUTH OF INTERSTATE 40										
ROUTE	AP-12R	30	2	.17770	31.40	1.785	.18834	1.600	.276	
ROUTE	AP-8R	20	22	.11280	15.07	.758	.12595	1.600	.209	
ADD HYD	AP-13	2&22	10	.29050	46.47	2.543	.16411	1.600	.250	
*S ROUTE FLOWS FROM AP-13 THRU PIPE TO UNSER DIVERSION POND NO. 5.										
*S IF FLOW FROM AP-13 WERE ROUTED TO TO UNSER DIVERSION POND NO. 6,										
*S POND NO. 6 WOULD OVERTOP. PLUS, THE EXISTING FLOW FROM THIS AREA										
*S DRAINS INTO POND NO. 5.										
ROUTE	AP-13R	10	32	.29050	44.30	2.543	.16411	1.650	.238	
COMPUTE NM HYD	AP-13A&A-1D	-	3	.02570	36.06	1.551	1.13124	1.500	2.192	PER IMP= 69.00
ROUTE	AP-13AR	3	22	.02570	35.42	1.551	1.13126	1.500	2.164	
ROUTE	AP-6R	12	2	.09530	9.68	.537	.10557	1.600	.159	
ADD HYD	PARTAP-14	22& 2	10	.12100	42.03	2.087	.32341	1.550	.543	
COMPUTE NM HYD	A-2D	-	3	.02840	45.59	2.013	1.32889	1.500	2.508	PER IMP= 84.00
ADD HYD	AP-14	10& 3	10	.14940	85.56	4.100	.51455	1.500	.895	
ROUTE	AP-14R	10	2	.14940	85.01	4.100	.51455	1.550	.889	
COMPUTE NM HYD	A-3D	-	3	.02400	39.69	1.802	1.40796	1.500	2.584	PER IMP= 90.00
ADD HYD	AP-15	2& 3	10	.17340	121.81	5.902	.63820	1.500	1.098	
COMPUTE NM HYD	AP-16&A-4D	-	3	.01250	18.04	.801	1.20080	1.500	2.256	PER IMP= 76.00

ADD HYD	AP-15+AP-16	10& 3	10	.18590	139.86	6.703	.67603	1.500	1.176	
COMPUTE NM HYD	P-6	-	3	.00430	.81	.026	.11274	1.550	.293	PER IMP= 2.00
ADD HYD	AP-17	10& 3	10	.19020	140.65	6.728	.66322	1.500	1.155	
ROUTE RESERVOIR	AP-17R	10	2	.19020	10.85	6.728	.66322	2.300	.089	AC-FT= 4.349
ROUTE	POND6ROUTE36	2	12	.19020	10.85	6.728	.66322	2.300	.089	
ROUTE	POND6ROUTE48	12	22	.19020	10.85	6.728	.66322	2.300	.089	
COMPUTE NM HYD	P-5	-	3	.00290	.55	.017	.11274	1.550	.294	PER IMP= 2.00
ADD HYD	AP-18	3&32	10	.29340	44.66	2.559	.16354	1.650	.238	
ROUTE RESERVOIR	AP-18R	10	12	.29340	15.49	2.559	.16353	2.150	.082	AC-FT= 1.231
ROUTE	POND-5ROUTE4	12	32	.29340	15.68	2.559	.16354	2.150	.084	
ADD HYD	POND-5R&6R	32&22	20	.48360	26.47	9.287	.36006	2.150	.086	
COMPUTE NM HYD	AP-19&C-1D	-	3	.01150	14.28	.597	.97312	1.500	1.940	PER IMP= 57.00
ROUTE	AP-19R	3	2	.01150	13.58	.597	.97317	1.550	1.846	
COMPUTE NM HYD	C-2D	-	4	.05740	69.47	2.979	.97312	1.500	1.891	PER IMP= 57.00
ADD HYD	AP-20	2& 4	10	.06890	82.47	3.576	.97311	1.500	1.870	
COMPUTE NM HYD	P-4	-	3	.00360	.68	.022	.11274	1.550	.293	PER IMP= 2.00
ADD HYD	P-4+POND-5R&	20& 3	20	.48720	26.52	9.307	.35820	2.150	.085	
ADD HYD	AP-20A	10&20	30	.55610	93.37	12.883	.43438	1.500	.262	
ROUTE RESERVOIR	AP-20AR	30	2	.55610	23.45	12.796	.43143	2.450	.066	AC-FT= 3.540
ROUTE	POND-4R	2	12	.55610	23.46	12.795	.43142	2.450	.066	
COMPUTE NM HYD	P-3	-	3	.00270	.51	.016	.11274	1.550	.294	PER IMP= 2.00
ADD HYD	P-3+POND-4R	12& 3	10	.55880	23.48	12.811	.42985	2.450	.066	
COMPUTE NM HYD	AP-21&D-1D	-	3	.04680	58.08	2.429	.97312	1.500	1.939	PER IMP= 57.00
ADD HYD	AP-21A	10& 3	10	.60560	62.72	15.240	.47183	1.500	.162	

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION
ROUTE RESERVOIR	AP-21AR	10	2	.60560	23.96	15.034	.46548	2.650	.062	AC-FT= 2.136
COMPUTE NM HYD	AP-22&E-1D	-	3	.02820	39.44	1.741	1.15760	1.500	2.185	PER IMP= 71.00
ROUTE	AP-22R	3	12	.02820	38.12	1.741	1.15761	1.550	2.112	
COMPUTE NM HYD	E-2D	-	3	.01720	29.01	1.292	1.40796	1.500	2.635	PER IMP= 90.00
ADD HYD	AP-23	3&12	10	.04540	66.46	3.033	1.25243	1.500	2.287	
ROUTE	AP-23R	10	12	.04540	66.02	3.033	1.25245	1.550	2.272	
COMPUTE NM HYD	E-3D	-	3	.04250	52.74	2.206	.97312	1.500	1.939	PER IMP= 57.00
ADD HYD	AP-24	12& 3	10	.08790	115.92	5.238	1.11738	1.500	2.060	
ROUTE	AP-23R	10	12	.08790	116.53	5.238	1.11739	1.550	2.071	
COMPUTE NM HYD	E-4D	-	3	.00300	5.07	.225	1.40796	1.500	2.638	PER IMP= 90.00
ADD HYD	AP-25	12& 3	10	.09090	120.95	5.464	1.12697	1.550	2.079	
ADD HYD	AP-25+POND-3	2&10	10	.69650	124.65	20.498	.55181	1.550	.280	
COMPUTE NM HYD	P-2	-	3	.00360	.68	.022	.11274	1.550	.293	PER IMP= 2.00
ADD HYD	AP-25A	3&10	10	.70010	125.33	20.519	.54953	1.550	.280	
ROUTE RESERVOIR	AP-25AR	10	2	.70010	14.35	19.345	.51811	5.000	.032	AC-FT= 6.221
ROUTE	POND-2R	2	12	.70010	14.35	19.344	.51807	5.050	.032	
COMPUTE NM HYD	AP-26&F-1D	-	3	.03980	48.17	2.066	.97312	1.500	1.891	PER IMP= 57.00
COMPUTE NM HYD	P-1	-	4	.00290	.55	.017	.11274	1.550	.294	PER IMP= 2.00
ADD HYD	P-1+F-1D	3& 4	10	.04270	48.71	2.082	.91423	1.500	1.783	
ADD HYD	AP-26A	10&12	10	.74280	49.19	21.426	.54084	1.500	.103	
ROUTE RESERVOIR	AP-26AR	10	2	.74280	10.14	21.067	.53179	11.400	.021	AC-FT= 2.885
COMPUTE NM HYD	AP-27&G-1D	-	3	.02290	35.53	1.559	1.27619	1.500	2.424	PER IMP= 80.00
COMPUTE NM HYD	AP-28&G-2D	-	4	.00310	4.90	.215	1.30254	1.500	2.469	PER IMP= 82.00
ROUTE	AP-28R	4	12	.00310	4.76	.215	1.30270	1.500	2.398	
COMPUTE NM HYD	AP-29&G-3D	-	5	.03510	48.94	2.389	1.27619	1.500	2.178	PER IMP= 80.00
COMPUTE NM HYD	G-4D	-	6	.00330	5.08	.220	1.24983	1.500	2.407	PER IMP= 78.00
ADD HYD	AP-30	12& 6	10	.00640	9.84	.435	1.27528	1.500	2.403	

FINISH

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994 RUN DATE (MON/DAY/YR) =01/30/2001
INPUT FILE = INT100EE.IN USER NO.= C_ANDRSN.I01

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 1 NOTATION
*S DEVELOPED INTERIM CONDITIONS, WITHOUT I-40 INTERCEPTOR IN PLACE										
*S 100 YEAR, 24 HOUR STORM EVENT										
*S										
*S FILE NAME: INT100EE.IN										
*S										
*S THIS MODEL ANALYZES THE UNSER DIVERSION PONDS WITH THE EXISTING POND										
*S OUTLET WORKS AND THE EXISTING POND GRADING.										
*S										
*S MASTER DRAINAGE PLAN FOR THE WEST SIDE TRANSIT FACILITY										
*S THIS AHYMO FILE ANALYZES THE UNSER DIVERSION (DETENTION PONDS) ASSUMING										
*S DEVELOPED CONDITIONS SOUTH OF INTERSTATE 40 WITHOUT THE I-40 INTERCEPTOR										
*S IN PLACE. IN OTHER WORDS, FULLY DEVELOPED CONDITIONS SOUTH OF I-40										
*S INCLUDING EXISTING CONDITION OFFSITE FLOWS NORTH OF INTERSTATE 40.										
*S										
*S NOTES: UNSER DIVERSION POND NO. 5 IS USED IN DEVELOPED/INTERIM CONDITIONS.										
*S THE OUTLET RATING CURVES WERE ALSO BASED UPON TAILWATER										
*S ELEVATIONS SHOWN ON THE UNSER DIVERSION (AS-BUILT)										
*S CONSTRUCTION PLAN SET (PLANS BY CHUCK EASTERLING)										
*S										
*S DATE: 11/8/00										
START										TIME= .00
RAINFALL TYPE= 2										RAIN24= 2.660
*S BEGIN WATERSHED NORTH OF INTERSTATE 40										
COMPUTE NM HYD	O-1E	-	3	.02420	18.06	.567	.43936	1.550	1.166	PER IMP= .00
COMPUTE NM HYD	O-2E	-	4	.04220	24.34	.989	.43936	1.600	.901	PER IMP= .00
ADD HYD	AP-1	3& 4	10	.06640	41.88	1.556	.43936	1.550	.986	
ROUTE	AP-1R	10	2	.06640	36.11	1.556	.43938	1.650	.850	
COMPUTE NM HYD	O-3E	-	3	.01640	13.10	.402	.45924	1.500	1.248	PER IMP= 1.00
ADD HYD	PARTAP-2	2& 3	10	.08280	44.73	1.958	.44330	1.650	.844	
COMPUTE NM HYD	AP-7&O-5E	-	3	.10970	63.37	2.803	.47912	1.600	.903	PER IMP= 2.00
DIVIDE HYD	PART_AP-7	3	98	.08594	32.00	2.196	.47911	1.500	.582	
	PART_AP-7	AND	99	.02376	31.37	.607	.47911	1.600	2.062	
ROUTE	AP-7R	99	2	.02376	31.64	.608	.47944	1.600	2.081	
ADD HYD	AP-2	10& 2	10	.10656	76.00	2.565	.45135	1.600	1.114	
ROUTE	AP-2R	10	2	.10656	79.83	2.565	.45136	1.600	1.171	
COMPUTE NM HYD	I40-3	-	3	.00190	2.86	.115	1.13512	1.500	2.350	PER IMP= 35.00
ADD HYD	AP-3	2& 3	20	.10846	81.89	2.680	.46333	1.600	1.180	
COMPUTE NM HYD	AP-4&O-4E	-	3	.00940	8.11	.240	.47912	1.500	1.348	PER IMP= 2.00
ROUTE	AP-4R	3	2	.00940	8.17	.240	.47921	1.500	1.358	
COMPUTE NM HYD	I40-4	-	3	.00120	1.81	.073	1.13512	1.500	2.357	PER IMP= 35.00
ADD HYD	AP-5	2& 3	10	.01060	9.98	.313	.55337	1.500	1.471	
ADD HYD	AP-3+AP-5	20&10	10	.11906	89.90	2.993	.47135	1.600	1.180	
ROUTE	AP-6	10	12	.11906	87.97	2.993	.47135	1.650	1.154	
ROUTE	AP-7R-CUL	98	2	.08594	33.23	2.196	.47912	1.500	.604	
COMPUTE NM HYD	I40-2	-	3	.00310	4.65	.188	1.13512	1.500	2.346	PER IMP= 35.00
ADD HYD	AP-8	2& 3	20	.08904	37.88	2.384	.50195	1.500	.665	
COMPUTE NM HYD	O-6E	-	3	.07320	35.69	1.715	.43936	1.600	.762	PER IMP= .00
COMPUTE NM HYD	O-7E	-	4	.05230	39.03	1.226	.43936	1.550	1.166	PER IMP= .00
ADD HYD	AP-9	3& 4	10	.12550	71.47	2.941	.43936	1.550	.890	

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 2 NOTATION
ROUTE	AP-9R	10	2	.12550	70.89	2.941	.43937	1.600	.883	
COMPUTE NM HYD	O-8E	-	3	.00990	23.00	1.124	2.12906	1.500	3.630	PER IMP= 85.00
ADD HYD	AP-10	2& 3	10	.13540	92.59	4.065	.56290	1.550	1.068	
ROUTE	AP-10R	10	2	.13540	89.10	4.065	.56291	1.600	1.028	
COMPUTE NM HYD	O-9E	-	3	.03480	31.99	1.000	.53875	1.500	1.436	PER IMP= 5.00
ADD HYD	AP-11	2& 3	10	.17020	114.14	5.065	.55797	1.600	1.048	
ROUTE	AP-11R	10	2	.17020	114.79	5.065	.55797	1.600	1.054	
COMPUTE NM HYD	I40-1	-	3	.00750	11.24	.454	1.13512	1.500	2.342	PER IMP= 35.00
ADD HYD	AP-12	2& 3	30	.17770	122.86	5.519	.58232	1.600	1.080	
*S BEGIN WATERSHED SOUTH OF INTERSTATE 40										
ROUTE	AP-12R	30	2	.17770	123.23	5.519	.58233	1.600	1.084	
ROUTE	AP-8R	20	22	.08904	39.96	2.384	.50196	1.500	.701	
ADD HYD	AP-13	2&22	10	.26674	158.78	7.902	.55549	1.600	.930	
*S ROUTE FLOWS FROM AP-13 THRU PIPE TO UNSER DIVERSION POND NO. 5.										
*S IF FLOW FROM AP-13 WERE ROUTED TO TO UNSER DIVERSION POND NO. 6,										
*S POND NO. 6 WOULD OVERTOP. PLUS, THE EXISTING FLOW FROM THIS AREA										
*S DRAINS INTO POND NO. 5.										
ROUTE	AP-13R	10	32	.26674	154.05	7.902	.55550	1.650	.902	
COMPUTE NM HYD	AP-13A&A-1D	-	3	.02570	59.05	2.579	1.88137	1.500	3.590	PER IMP= 69.00
ROUTE	AP-13AR	3	22	.02570	58.26	2.579	1.88138	1.500	3.542	
ROUTE	AP-6R	12	2	.11906	88.44	2.993	.47135	1.650	1.161	
ADD HYD	PARTAP-14	22& 2	10	.14476	125.63	5.572	.72167	1.600	1.356	
COMPUTE NM HYD	A-2D	-	3	.02840	71.47	3.250	2.14550	1.500	3.932	PER IMP= 84.00
ADD HYD	AP-14	10& 3	10	.17316	175.97	8.822	.95518	1.550	1.588	
ROUTE	AP-14R	10	2	.17316	175.47	8.822	.95518	1.600	1.583	
COMPUTE NM HYD	A-3D	-	3	.02400	61.30	2.881	2.25115	1.500	3.991	PER IMP= 90.00
ADD HYD	AP-15	2& 3	10	.19716	229.58	11.703	1.11293	1.550	1.819	
COMPUTE NM HYD	AP-16&A-4D	-	3	.01250	29.07	1.312	1.96827	1.500	3.634	PER IMP= 76.00

ADD HYD	AP-15+AP-16	10& 3	10	.20966	255.16	13.015	1.16393	1.550	1.902		
COMPUTE NM HYD	P-6	-	3	.00430	3.72	.110	.47912	1.500	1.350	PER IMP=	2.00
ADD HYD	AP-17	10& 3	10	.21396	258.75	13.125	1.15016	1.550	1.890		
ROUTE RESERVOIR	AP-17R	10	2	.21396	82.33	13.125	1.15016	1.950	.601	AC-FT=	6.783
ROUTE	POND6ROUTE36	2	12	.21396	82.41	13.125	1.15016	1.950	.602		
ROUTE	POND6ROUTE48	12	22	.21396	83.00	13.125	1.15016	1.900	.606		
COMPUTE NM HYD	P-5	-	3	.00290	2.51	.074	.47912	1.500	1.353	PER IMP=	2.00
ADD HYD	AP-18	3&32	10	.26964	155.57	7.976	.55462	1.650	.901		
ROUTE RESERVOIR	AP-18R	10	12	.26964	113.38	7.976	.55462	1.800	.657	AC-FT=	2.044
ROUTE	POND-5ROUTE4	12	32	.26964	119.58	7.976	.55462	1.800	.693		
ADD HYD	POND-5R&6R	32&22	20	.48360	201.34	21.101	.81811	1.800	.651		
COMPUTE NM HYD	AP-19&C-1D	-	3	.01150	24.41	1.024	1.67006	1.500	3.317	PER IMP=	57.00
ROUTE	AP-19R	3	2	.01150	23.29	1.024	1.67010	1.550	3.164		
COMPUTE NM HYD	C-2D	-	4	.05740	118.78	5.113	1.67006	1.500	3.233	PER IMP=	57.00
ADD HYD	AP-20	2& 4	10	.06890	141.50	6.137	1.67005	1.500	3.209		
COMPUTE NM HYD	P-4	-	3	.00360	3.11	.092	.47912	1.500	1.352	PER IMP=	2.00
ADD HYD	P-4+POND-5R&	20& 3	20	.48720	202.13	21.192	.81559	1.800	.648		
ADD HYD	AP-20A	10&20	30	.55610	251.18	27.329	.92146	1.800	.706		
ROUTE RESERVOIR	AP-20AR	30	2	.55610	122.30	27.237	.91836	2.250	.344	AC-FT=	8.157
ROUTE	POND-4R	2	12	.55610	122.38	27.237	.91835	2.250	.344		
COMPUTE NM HYD	P-3	-	3	.00270	2.34	.069	.47912	1.500	1.354	PER IMP=	2.00
ADD HYD	P-3+POND-4R	12& 3	10	.55880	122.51	27.305	.91620	2.250	.343		
COMPUTE NM HYD	AP-21&D-1D	-	3	.04680	99.31	4.168	1.67006	1.500	3.316	PER IMP=	57.00
ADD HYD	AP-21A	10& 3	10	.60560	137.30	31.474	.97446	2.050	.354		

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION	
ROUTE RESERVOIR	AP-21AR	10	2	.60560	112.11	31.212	.96635	2.700	.289	AC-FT=	4.426
COMPUTE NM HYD	AP-22&E-1D	-	3	.02820	64.16	2.883	1.91659	1.500	3.555	PER IMP=	71.00
ROUTE	AP-22R	3	12	.02820	61.84	2.883	1.91660	1.550	3.427		
COMPUTE NM HYD	E-2D	-	3	.01720	44.80	2.065	2.25115	1.500	4.070	PER IMP=	90.00
ADD HYD	AP-23	3&12	10	.04540	106.21	4.948	2.04333	1.500	3.655		
ROUTE	AP-23R	10	12	.04540	104.90	4.948	2.04334	1.550	3.610		
COMPUTE NM HYD	E-3D	-	3	.04250	90.19	3.785	1.67006	1.500	3.316	PER IMP=	57.00
ADD HYD	AP-24	12& 3	10	.08790	191.75	8.733	1.86285	1.500	3.408		
ROUTE	AP-23R	10	12	.08790	190.60	8.733	1.86286	1.550	3.388		
COMPUTE NM HYD	E-4D	-	3	.00300	7.83	.360	2.25115	1.500	4.076	PER IMP=	90.00
ADD HYD	AP-25	12& 3	10	.09090	197.44	9.093	1.87566	1.550	3.394		
ADD HYD	AP-25+POND-3	2&10	10	.69650	203.90	40.305	1.08502	1.550	.457		
COMPUTE NM HYD	P-2	-	3	.00360	3.11	.092	.47912	1.500	1.352	PER IMP=	2.00
ADD HYD	AP-25A	3&10	10	.70010	206.90	40.397	1.08190	1.550	.462		
ROUTE RESERVOIR	AP-25AR	10	2	.70010	131.20	38.772	1.03839	2.200	.293	AC-FT=	7.367
ROUTE	POND-2R	2	12	.70010	132.28	38.770	1.03833	2.150	.295		
COMPUTE NM HYD	AP-26&F-1D	-	3	.03980	82.37	3.545	1.67006	1.500	3.234	PER IMP=	57.00
COMPUTE NM HYD	P-1	-	4	.00290	2.51	.074	.47912	1.500	1.353	PER IMP=	2.00
ADD HYD	P-1+F-1D	3& 4	10	.04270	84.88	3.618	1.58885	1.500	3.106		
ADD HYD	AP-26A	10&12	10	.74280	147.24	42.388	1.06998	2.050	.310		
ROUTE RESERVOIR	AP-26AR	10	2	.74280	113.87	41.053	1.03626	3.050	.240	AC-FT=	6.641
COMPUTE NM HYD	AP-27&G-1D	-	3	.02290	56.30	2.534	2.07506	1.500	3.841	PER IMP=	80.00
COMPUTE NM HYD	AP-28&G-2D	-	4	.00310	7.72	.349	2.11028	1.500	3.893	PER IMP=	82.00
ROUTE	AP-28R	4	12	.00310	7.56	.349	2.11044	1.500	3.808		
COMPUTE NM HYD	AP-29&G-3D	-	5	.03510	77.66	3.885	2.07506	1.500	3.457	PER IMP=	80.00
COMPUTE NM HYD	G-4D	-	6	.00330	8.10	.359	2.03985	1.500	3.837	PER IMP=	78.00
ADD HYD	AP-30	12& 6	10	.00640	15.66	.708	2.07389	1.500	3.823		

FINISH

```
AHYMO SUMMARY TABLE (AHYM0194) - AMAFCA Hydrologic Model - January, 1994          RUN DATE (MON/DAY/YR) =01/22/2001
INPUT FILE = INT10PE.IN                                                                 USER NO.= C_ANDRSN.I01

                                 FROM  TO                    PEAK        RUNOFF                TIME TO   CFS       PAGE =   1
                      HYDROGRAPH  ID    ID        AREA      DISCHARGE    VOLUME     RUNOFF     PEAK     PER
COMMAND       IDENTIFICATION     NO.   NO.      (SQ MI)     (CFS)       (AC-FT)   (INCHES)   (HOURS)  ACRE     NOTATION

*S DEVELOPED INTERIM CONDITIONS, WITHOUT I-40 INTERCEPTOR IN PLACE
*S 10 YEAR, 24 HOUR STORM EVENT
*S
*S FILE NAME: INT10PE.IN
*S
*S THIS MODEL ANALYZES THE UNSER DIVERSION PONDS WITH THE PROPOSED POND
*S OUTLET WORKS AND THE EXISTING POND GRADING.
*S
*S MASTER DRAINAGE PLAN FOR THE WEST SIDE TRANSIT FACILITY
*S THIS AHYMO FILE ANALYZES THE UNSER DIVERSION (DETENTION PONDS) ASSUMING
*S DEVELOPED CONDITIONS SOUTH OF INTERSTATE 40 WITHOUT THE I-40 INTERCEPTOR
*S IN PLACE. IN OTHER WORDS, FULLY DEVELOPED CONDITIONS SOUTH OF I-40
*S INCLUDING EXISTING CONDITION OFFSITE FLOWS NORTH OF INTERSTATE 40.
*S NOTES: UNSER DIVERSION POND NO. 5 IS USED IN DEVELOPED/INTERIM CONDITIONS.
*S        THE OUTLET WORKS FOR EACH UNSER DIVERSION POND HAS BEEN MODIFIED
*S        BY PLACING ADDITIONAL 10" OPENINGS AROUND EACH STRUCTURE EXCEPT
*S        AT UNSER DIVERSION POND NO. 5 (PER CHUCK EASTERLING'S REPORT).
*S        OUTLET RATING CURVES WERE ALSO BASED UPON TAILWATER ELEVATIONS
*S        SHOWN ON THE UNSER DIVERSION (AS-BUILT) CONSTRUCTION PLAN SET
*S        (PLANS BY CHUCK EASTERLING).
*S DATE: 11/8/00
START                                                                                                    TIME=      .00
RAINFALL  TYPE= 2                                                                                        RAIN24=   1.770
*S BEGIN WATERSHED NORTH OF INTERSTATE 40
COMPUTE NM HYD          O-1E   -     3        .02420        3.43        .108       .08362    1.550     .221 PER IMP=    .00
COMPUTE NM HYD          O-2E   -     4        .04220        4.48        .188       .08362    1.600     .166 PER IMP=    .00
ADD HYD                 AP-1   3& 4  10       .06640        7.65        .296       .08361    1.550     .180
ROUTE                   AP-1R  10    2        .06640        5.45        .296       .08362    1.700     .128
COMPUTE NM HYD          O-3E   -     3        .01640        2.72        .086       .09818    1.550     .259 PER IMP=   1.00
ADD HYD              PARTAP-2  2& 3  10       .08280        7.02        .382       .08650    1.650     .132
COMPUTE NM HYD      AP-7&O-5E  -     3        .10970       13.54        .660       .11274    1.600     .193 PER IMP=   2.00
DIVIDE HYD          PART_AP-7    3   98       .10970       13.54        .660       .11274    1.600     .193
                    PART_AP-7  AND   99       .00000         .00        .000       .00000    -.050     .000
ROUTE                   AP-7R  99    2        .00000         .00        .000       .00000    -.050     .000
ADD HYD                 AP-2   10& 2 10       .08280        7.02        .382       .08650    1.650     .132
ROUTE                   AP-2R  10    2        .08280        6.98        .382       .08651    1.700     .132
COMPUTE NM HYD          I40-3  -     3        .00190        1.40        .060       .59326    1.500    1.154 PER IMP=  35.00
ADD HYD                 AP-3   2& 3  20       .08470        7.72        .442       .09786    1.650     .142
COMPUTE NM HYD      AP-4&O-4E  -     3        .00940        1.75        .057       .11274    1.550     .292 PER IMP=   2.00
ROUTE                   AP-4R  3     2        .00940        1.84        .057       .11280    1.550     .306
COMPUTE NM HYD          I40-4  -     3        .00120         .89        .038       .59326    1.500    1.156 PER IMP=  35.00
ADD HYD                 AP-5   2& 3  10       .01060        2.64        .094       .16710    1.550     .389
ADD HYD             AP-3+AP-5  20&10 10       .09530        9.60        .537       .10556    1.600     .157
ROUTE                   AP-6   10    12       .09530        9.61        .537       .10557    1.600     .158
ROUTE                AP-7R-CUL 98    2        .10970       13.58        .660       .11274    1.600     .193
COMPUTE NM HYD          I40-2  -     3        .00310        2.28        .098       .59326    1.500    1.152 PER IMP=  35.00
ADD HYD                 AP-8   2& 3  20       .11280       15.19        .758       .12594    1.600     .210
COMPUTE NM HYD          O-6E   -     3        .07320        6.49        .326       .08362    1.650     .138 PER IMP=    .00
COMPUTE NM HYD          O-7E   -     4        .05230        7.41        .233       .08362    1.550     .221 PER IMP=    .00

                                 FROM  TO                    PEAK        RUNOFF                TIME TO   CFS       PAGE =   2
                      HYDROGRAPH  ID    ID        AREA      DISCHARGE    VOLUME     RUNOFF     PEAK     PER
COMMAND       IDENTIFICATION     NO.   NO.      (SQ MI)     (CFS)       (AC-FT)   (INCHES)   (HOURS)  ACRE     NOTATION

ADD HYD                 AP-9   3& 4  10       .12550       13.19        .560       .08362    1.600     .164
ROUTE                   AP-9R  10    2        .12550       13.27        .560       .08362    1.600     .165
COMPUTE NM HYD          O-8E   -     3        .00990       14.68        .698      1.32131    1.500    2.318 PER IMP=  85.00
ADD HYD                 AP-10  2& 3  10       .13540       26.66       1.257       .17411    1.550     .308
ROUTE                   AP-10R 10    2        .13540       21.80       1.257       .17411    1.650     .252
COMPUTE NM HYD          O-9E   -     3        .03480        8.15        .290       .15643    1.500     .366 PER IMP=   5.00
ADD HYD                 AP-11  2& 3  10       .17020       27.81       1.548       .17049    1.600     .255
ROUTE                   AP-11R 10    2        .17020       27.65       1.548       .17050    1.600     .254
COMPUTE NM HYD          I40-1  -     3        .00750        5.52        .237       .59326    1.500    1.150 PER IMP=  35.00
ADD HYD                 AP-12  2& 3  30       .17770       31.52       1.785       .18833    1.600     .277
*S BEGIN WATERSHED SOUTH OF INTERSTATE 40
ROUTE                   AP-12R 30    2        .17770       31.40       1.785       .18834    1.600     .276
ROUTE                   AP-8R  20    22       .11280       15.07        .758       .12595    1.600     .209
ADD HYD                 AP-13  2&22  10       .29050       46.47       2.543       .16411    1.600     .250
*S ROUTE FLOWS FROM AP-13 THRU PIPE TO UNSER DIVERSION POND NO. 5.
*S IF FLOW FROM AP-13 WERE ROUTED TO TO UNSER DIVERSION POND NO. 6,
*S POND NO. 6 WOULD OVERTOP. PLUS, THE EXISTING FLOW FROM THIS AREA
*S DRAINS INTO POND NO. 5.
ROUTE                   AP-13R 10    32       .29050       44.30       2.543       .16411    1.650     .238
COMPUTE NM HYD    AP-13A&A-1D  -     3        .02570       36.06       1.551      1.13124    1.500    2.192 PER IMP=  69.00
ROUTE                AP-13AR   3     22       .02570       35.42       1.551      1.13126    1.500    2.154
ROUTE                   AP-6R  12    2        .09530        9.68        .537       .10557    1.600     .159
ADD HYD              PARTAP-14 22& 2  10       .12100       42.03       2.087       .32341    1.550     .543
COMPUTE NM HYD          A-2D   -     3        .02840       45.59       2.013      1.32889    1.500    2.508 PER IMP=  84.00
ADD HYD                 AP-14  10& 3 10       .14940       85.56       4.100       .51455    1.500     .895
ROUTE                   AP-14R 10    2        .14940       85.01       4.100       .51455    1.550     .889
COMPUTE NM HYD          A-3D   -     3        .02400       39.69       1.802      1.40796    1.500    2.584 PER IMP=  90.00
ADD HYD                 AP-15  2& 3  10       .17340      121.81       5.902       .63820    1.500    1.098
```

COMPUTE NM HYD	AP-16&A-4D	-	3	.01250	18.04	.801	1.20080	1.500	2.256	PER IMP= 76.00
ADD HYD	AP-15+AP-16	10& 3	10	.18590	139.86	6.703	.67603	1.500	1.176	
COMPUTE NM HYD	P-6	-	3	.00430	.81	.026	.11274	1.550	.293	PER IMP= 2.00
ADD HYD	AP-17	10& 3	10	.19020	140.65	6.728	.66322	1.500	1.155	
ROUTE RESERVOIR	AP-17R	10	2	.19020	44.25	6.728	.66321	1.950	.364	AC-FT= 2.727
ROUTE	POND6ROUTE36	2	12	.19020	44.25	6.728	.66322	1.950	.364	
ROUTE	POND6ROUTE48	12	22	.19020	44.26	6.728	.66322	1.950	.364	
COMPUTE NM HYD	P-5	-	3	.00290	.55	.017	.11274	1.550	.294	PER IMP= 2.00
ADD HYD	AP-18	3&32	10	.29340	44.66	2.559	.16354	1.650	.238	
ROUTE RESERVOIR	AP-18R	10	12	.29340	15.49	2.559	.16353	2.150	.082	AC-FT= 1.231
ROUTE	POND-5ROUTE4	12	32	.29340	15.68	2.559	.16354	2.150	.084	
ADD HYD	POND-5R&6R	32&22	20	.48360	58.25	9.287	.36006	2.150	.188	
COMPUTE NM HYD	AP-19&C-1D	-	3	.01150	14.28	.597	.97312	1.500	1.940	PER IMP= 57.00
ROUTE	AP-19R	3	2	.01150	13.58	.597	.97317	1.550	1.846	
COMPUTE NM HYD	C-2D	-	4	.05740	69.47	2.979	.97312	1.500	1.891	PER IMP= 57.00
ADD HYD	AP-20	2& 4	10	.06890	82.47	3.576	.97311	1.500	1.870	
COMPUTE NM HYD	P-4	-	3	.00360	.68	.022	.11274	1.550	.293	PER IMP= 2.00
ADD HYD	P-4+POND-5R&	20& 3	20	.48720	58.30	9.307	.35820	2.150	.187	
ADD HYD	AP-20A	10&20	30	.55610	110.20	12.883	.43438	1.550	.310	
ROUTE RESERVOIR	AP-20AR	30	2	.55610	54.69	12.804	.43170	2.300	.154	AC-FT= 2.916
ROUTE	POND-4R	2	12	.55610	54.69	12.804	.43170	2.300	.154	
COMPUTE NM HYD	P-3	-	3	.00270	.51	.016	.11274	1.550	.294	PER IMP= 2.00
ADD HYD	P-3+POND-4R	12& 3	10	.55880	54.72	12.819	.43013	2.300	.153	
COMPUTE NM HYD	AP-21&D-1D	-	3	.04680	58.08	2.429	.97312	1.500	1.939	PER IMP= 57.00

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 3 NOTATION
ADD HYD	AP-21A	10& 3	10	.60560	83.36	15.248	.47209	1.550	.215	
ROUTE RESERVOIR	AP-21AR	10	2	.60560	60.70	15.179	.46996	2.150	.157	AC-FT= 2.184
COMPUTE NM HYD	AP-22&E-1D	-	3	.02820	39.44	1.741	1.15760	1.500	2.185	PER IMP= 71.00
ROUTE	AP-22R	3	12	.02820	38.12	1.741	1.15761	1.550	2.112	
COMPUTE NM HYD	E-2D	-	3	.01720	29.01	1.292	1.40796	1.500	2.635	PER IMP= 90.00
ADD HYD	AP-23	3&12	10	.04540	66.46	3.033	1.25243	1.500	2.287	
ROUTE	AP-23R	10	12	.04540	66.02	3.033	1.25245	1.550	2.272	
COMPUTE NM HYD	E-3D	-	3	.04250	52.74	2.206	.97312	1.500	1.939	PER IMP= 57.00
ADD HYD	AP-24	12& 3	10	.08790	115.92	5.238	1.11738	1.500	2.060	
ROUTE	AP-23R	10	12	.08790	116.53	5.238	1.11739	1.550	2.071	
COMPUTE NM HYD	E-4D	-	3	.00300	5.07	.225	1.40796	1.500	2.638	PER IMP= 90.00
ADD HYD	AP-25	12& 3	10	.09090	120.95	5.464	1.12697	1.550	2.079	
ADD HYD	AP-25+POND-3	2&10	10	.69650	142.13	20.643	.55571	1.550	.319	
COMPUTE NM HYD	P-2	-	3	.00360	.68	.022	.11274	1.550	.293	PER IMP= 2.00
ADD HYD	AP-25A	3&10	10	.70010	142.81	20.663	.55340	1.550	.319	
ROUTE RESERVOIR	AP-25AR	10	2	.70010	47.61	19.931	.53378	3.200	.106	AC-FT= 5.555
ROUTE	POND-2R	2	12	.70010	47.61	19.931	.53378	3.200	.106	
COMPUTE NM HYD	AP-26&F-1D	-	3	.03980	48.17	2.066	.97312	1.500	1.891	PER IMP= 57.00
COMPUTE NM HYD	P-1	-	4	.00290	.55	.017	.11274	1.550	.294	PER IMP= 2.00
ADD HYD	P-1+F-1D	3& 4	10	.04270	48.71	2.082	.91423	1.500	1.783	
ADD HYD	AP-26A	10&12	10	.74280	58.22	22.013	.55566	1.550	.122	
ROUTE RESERVOIR	AP-26AR	10	2	.74280	47.46	22.012	.55564	3.450	.100	AC-FT= 1.556
COMPUTE NM HYD	AP-27&G-1D	-	3	.02290	35.53	1.559	1.27619	1.500	2.424	PER IMP= 80.00
COMPUTE NM HYD	AP-28&G-2D	-	4	.00310	4.90	.215	1.30254	1.500	2.469	PER IMP= 82.00
ROUTE	AP-28R	4	12	.00310	4.76	.215	1.30270	1.500	2.398	
COMPUTE NM HYD	AP-29&G-3D	-	5	.03510	48.94	2.389	1.27619	1.500	2.178	PER IMP= 80.00
COMPUTE NM HYD	G-4D	-	6	.00330	5.08	.220	1.24983	1.500	2.407	PER IMP= 78.00
ADD HYD	AP-30	12& 6	10	.00640	9.84	.435	1.27528	1.500	2.403	

FINISH

AHYMO SUMMARY TABLE (AHYM0194) - AMAFCA Hydrologic Model - January, 1994 RUN DATE (MON/DAY/YR) =01/22/2001
INPUT FILE = INT100PE.IN USER NO.= C_ANDRSN.101

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 1 NOTATION
*$ DEVELOPED INTERIM CONDITIONS, WITHOUT I-40 INTERCEPTOR IN PLACE										
*$ 100 YEAR, 24 HOUR STORM EVENT										
*$										
*$ FILE NAME: INT100PE.IN										
*$										
*$ THIS MODEL ANALYZES THE UNSER DIVERSION PONDS WITH THE PROPOSED POND										
*$ OUTLET WORKS AND THE EXISTING POND GRADING.										
*$										
*$ MASTER DRAINAGE PLAN FOR THE WEST SIDE TRANSIT FACILITY										
*$ THIS AHYMO FILE ANALYZES THE UNSER DIVERSION (DETENTION PONDS) ASSUMING										
*$ DEVELOPED CONDITIONS SOUTH OF INTERSTATE 40 WITHOUT THE I-40 INTERCEPTOR										
*$ IN PLACE. IN OTHER WORDS, FULLY DEVELOPED CONDITIONS SOUTH OF I-40										
*$ INCLUDING EXISTING CONDITION OFFSITE FLOWS NORTH OF INTERSTATE 40.										
*$ NOTES: UNSER DIVERSION POND NO. 5 IS USED IN DEVELOPED/INTERIM CONDITIONS.										
*$ THE OUTLET WORKS FOR EACH UNSER DIVERSION POND HAS BEEN MODIFIED										
*$ BY PLACING ADDITIONAL 10" OPENINGS AROUND EACH STRUCTURE EXCEPT										
*$ AT UNSER DIVERSION POND NO. 5 (PER CHUCK EASTERLING'S REPORT).										
*$ OUTLET RATING CURVES WERE ALSO BASED UPON TAILWATER ELEVATIONS										
*$ SHOWN ON THE UNSER DIVERSION (AS-BUILT) CONSTRUCTION PLAN SET										
*$ (PLANS BY CHUCK EASTERLING).										
*$ DATE: 11/8/00										
START										TIME= .00
RAINFALL TYPE= 2										RAIN24= 2.660
*$ BEGIN WATERSHED NORTH OF INTERSTATE 40										
COMPUTE NM HYD	O-1E	-	3	.02420	18.06	.567	.43936	1.550	1.166	PER IMP= .00
COMPUTE NM HYD	O-2E	-	4	.04220	24.34	.989	.43936	1.600	.901	PER IMP= .00
ADD HYD	AP-1	3& 4	10	.06640	41.88	1.556	.43936	1.550	.986	
ROUTE	AP-1R	10	2	.06640	36.11	1.556	.43938	1.650	.850	
COMPUTE NM HYD	O-3E	-	3	.01640	13.10	.402	.45924	1.500	1.248	PER IMP= 1.00
ADD HYD	PARTAP-2	2& 3	10	.08280	44.73	1.958	.44330	1.650	.844	
COMPUTE NM HYD	AP-7&O-5E	-	3	.10970	63.37	2.803	.47912	1.600	.903	PER IMP= 2.00
DIVIDE HYD	PART_AP-7	3	98	.08594	32.00	2.196	.47911	1.500	.582	
	PART_AP-7	AND	99	.02376	31.37	.607	.47911	1.600	2.062	
ROUTE	AP-7R	99	2	.02376	31.64	.608	.47944	1.600	2.081	
ADD HYD	AP-2	10& 2	10	.10656	76.00	2.565	.45135	1.600	1.114	
ROUTE	AP-2R	10	2	.10656	79.83	2.565	.45136	1.600	1.171	
COMPUTE NM HYD	I40-3	-	3	.00190	2.86	.115	1.13512	1.500	2.350	PER IMP= 35.00
ADD HYD	AP-3	2& 3	20	.10846	81.89	2.680	.46333	1.600	1.180	
COMPUTE NM HYD	AP-4&O-4E	-	3	.00940	8.11	.240	.47912	1.500	1.348	PER IMP= 2.00
ROUTE	AP-4R	3	2	.00940	8.17	.240	.47921	1.500	1.358	
COMPUTE NM HYD	I40-4	-	3	.00120	1.81	.073	1.13512	1.500	2.357	PER IMP= 35.00
ADD HYD	AP-5	2& 3	10	.01060	9.98	.313	.55337	1.500	1.471	
ADD HYD	AP-3+AP-5	20&10	10	.11906	89.90	2.993	.47135	1.600	1.180	
ROUTE	AP-6	10	12	.11906	87.97	2.993	.47135	1.650	1.154	
ROUTE	AP-7R-CUL	98	2	.08594	33.23	2.196	.47912	1.500	.604	
COMPUTE NM HYD	I40-2	-	3	.00310	4.65	.188	1.13512	1.500	2.346	PER IMP= 35.00
ADD HYD	AP-8	2& 3	20	.08904	37.88	2.384	.50195	1.500	.665	
COMPUTE NM HYD	O-6E	-	3	.07320	35.69	1.715	.43936	1.600	.762	PER IMP= .00
COMPUTE NM HYD	O-7E	-	4	.05230	39.03	1.226	.43936	1.550	1.166	PER IMP= .00

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 2 NOTATION
ADD HYD	AP-9	3& 4	10	.12550	71.47	2.941	.43936	1.550	.890	
ROUTE	AP-9R	10	2	.12550	70.89	2.941	.43937	1.600	.883	
COMPUTE NM HYD	O-8E	-	3	.00990	23.00	1.124	2.12906	1.500	3.630	PER IMP= 85.00
ADD HYD	AP-10	2& 3	10	.13540	92.59	4.065	.56290	1.550	1.068	
ROUTE	AP-10R	10	2	.13540	89.10	4.065	.56291	1.600	1.028	
COMPUTE NM HYD	O-9E	-	3	.03480	31.99	1.000	.53875	1.500	1.436	PER IMP= 5.00
ADD HYD	AP-11	2& 3	10	.17020	114.14	5.065	.55797	1.600	1.048	
ROUTE	AP-11R	10	2	.17020	114.79	5.065	.55797	1.600	1.054	
COMPUTE NM HYD	I40-1	-	3	.00750	11.24	.454	1.13512	1.500	2.342	PER IMP= 35.00
ADD HYD	AP-12	2& 3	30	.17770	122.86	5.519	.58232	1.600	1.080	
*$ BEGIN WATERSHED SOUTH OF INTERSTATE 40										
ROUTE	AP-12R	30	2	.17770	123.23	5.519	.58233	1.600	1.084	
ROUTE	AP-8R	20	22	.08904	39.96	2.384	.50196	1.500	.701	
ADD HYD	AP-13	2&22	10	.26674	158.78	7.902	.55549	1.600	.930	
*$ ROUTE FLOWS FROM AP-13 THRU PIPE TO UNSER DIVERSION POND NO. 5.										
*$ IF FLOW FROM AP-13 WERE ROUTED TO TO UNSER DIVERSION POND NO. 6,										
*$ POND NO. 6 WOULD OVERTOP. PLUS, THE EXISTING FLOW FROM THIS AREA										
*$ DRAINS INTO POND NO. 5.										
ROUTE	AP-13R	10	32	.26674	154.05	7.902	.55550	1.650	.902	
COMPUTE NM HYD	AP-13A&A-1D	-	3	.02570	59.05	2.579	1.88137	1.500	3.590	PER IMP= 69.00
ROUTE	AP-13AR	3	22	.02570	58.26	2.579	1.88138	1.500	3.542	
ROUTE	AP-6R	12	2	.11906	88.44	2.993	.47135	1.650	1.161	
ADD HYD	PARTAP-14	22& 2	10	.14476	125.63	5.572	.72167	1.600	1.356	
COMPUTE NM HYD	A-2D	-	3	.02840	71.47	3.250	2.14550	1.500	3.932	PER IMP= 84.00
ADD HYD	AP-14	10& 3	10	.17316	175.97	8.822	.95518	1.550	1.588	
ROUTE	AP-14R	10	2	.17316	175.47	8.822	.95518	1.600	1.583	
COMPUTE NM HYD	A-3D	-	3	.02400	61.30	2.881	2.25115	1.500	3.991	PER IMP= 90.00
ADD HYD	AP-15	2& 3	10	.19716	229.58	11.703	1.11293	1.550	1.819	

COMPUTE NM HYD	AP-16&A-4D	-	3	.01250	29.07	1.312	1.96827	1.500	3.634	PER IMP=	76.00
ADD HYD	AP-15+AP-16	10& 3	10	.20966	255.16	13.015	1.16393	1.550	1.902		
COMPUTE NM HYD	P-6	-	3	.00430	3.72	.110	.47912	1.500	1.350	PER IMP=	2.00
ADD HYD	AP-17	10& 3	10	.21396	258.75	13.125	1.15016	1.550	1.890		
ROUTE RESERVOIR	AP-17R	10	2	.21396	78.24	13.125	1.15016	1.950	.571	AC-FT=	6.209
ROUTE	POND6ROUTE36	2	12	.21396	78.20	13.125	1.15016	2.000	.571		
ROUTE	POND6ROUTE48	12	22	.21396	78.26	13.125	1.15016	1.950	.571		
COMPUTE NM HYD	P-5	-	3	.00290	2.51	.074	.47912	1.500	1.353	PER IMP=	2.00
ADD HYD	AP-18	3&32	10	.26964	155.57	7.976	.55462	1.650	.901		
ROUTE RESERVOIR	AP-18R	10	12	.26964	113.38	7.976	.55462	1.800	.657	AC-FT=	2.044
ROUTE	POND-5ROUTE4	12	32	.26964	119.58	7.976	.55462	1.800	.693		
ADD HYD	POND-5R&6R	32&22	20	.48360	191.14	21.101	.81811	1.900	.618		
COMPUTE NM HYD	AP-19&C-1D	-	3	.01150	24.41	1.024	1.67006	1.500	3.317	PER IMP=	57.00
ROUTE	AP-19R	3	2	.01150	23.29	1.024	1.67010	1.550	3.164		
COMPUTE NM HYD	C-2D	-	4	.05740	118.78	5.113	1.67006	1.500	3.233	PER IMP=	57.00
ADD HYD	AP-20	2& 4	10	.06890	141.50	6.137	1.67005	1.500	3.209		
COMPUTE NM HYD	P-4	-	3	.00360	3.11	.092	.47912	1.500	1.352	PER IMP=	2.00
ADD HYD	P-4+POND-5R&	20& 3	20	.48720	191.58	21.192	.81559	1.900	.614		
ADD HYD	AP-20A	10&20	30	.55610	242.54	27.329	.92146	1.700	.681		
ROUTE RESERVOIR	AP-20AR	30	2	.55610	119.72	27.250	.91877	2.250	.336	AC-FT=	7.764
ROUTE	POND-4R	2	12	.55610	119.78	27.249	.91877	2.250	.337		
COMPUTE NM HYD	P-3	-	3	.00270	2.34	.069	.47912	1.500	1.354	PER IMP=	2.00
ADD HYD	P-3+POND-4R	12& 3	10	.55880	119.90	27.318	.91662	2.250	.335		
COMPUTE NM HYD	AP-21&D-1D	-	3	.04680	99.31	4.168	1.67006	1.500	3.316	PER IMP=	57.00

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION	
ADD HYD	AP-21A	10& 3	10	.60560	138.80	31.486	.97484	1.500	.358		
ROUTE RESERVOIR	AP-21AR	10	2	.60560	111.57	31.417	.97271	2.750	.288	AC-FT=	4.380
COMPUTE NM HYD	AP-22&E-1D	-	3	.02820	64.16	2.883	1.91659	1.500	3.555	PER IMP=	71.00
ROUTE	AP-22R	3	12	.02820	61.84	2.883	1.91660	1.550	3.427		
COMPUTE NM HYD	E-2D	-	3	.01720	44.80	2.065	2.25115	1.500	4.070	PER IMP=	90.00
ADD HYD	AP-23	3&12	10	.04540	106.21	4.948	2.04333	1.500	3.655		
ROUTE	AP-23R	10	12	.04540	104.90	4.948	2.04334	1.550	3.610		
COMPUTE NM HYD	E-3D	-	3	.04250	90.19	3.785	1.67006	1.500	3.316	PER IMP=	57.00
ADD HYD	AP-24	12& 3	10	.08790	191.75	8.733	1.86285	1.500	3.408		
ROUTE	AP-23R	10	12	.08790	190.60	8.733	1.86286	1.550	3.388		
COMPUTE NM HYD	E-4D	-	3	.00300	7.83	.360	2.25115	1.500	4.076	PER IMP=	90.00
ADD HYD	AP-25	12& 3	10	.09090	197.44	9.093	1.87566	1.550	3.394		
ADD HYD	AP-25+POND-3	2&10	10	.69650	231.20	40.510	1.09056	1.550	.519		
COMPUTE NM HYD	P-2	-	3	.00360	3.11	.092	.47912	1.500	1.352	PER IMP=	2.00
ADD HYD	AP-25A	3&10	10	.70010	234.21	40.602	1.08740	1.550	.523		
ROUTE RESERVOIR	AP-25AR	10	2	.70010	138.57	39.869	1.06777	2.150	.309	AC-FT=	7.131
ROUTE	POND-2R	2	12	.70010	138.89	39.869	1.06777	2.100	.310		
COMPUTE NM HYD	AP-26&F-1D	-	3	.03980	82.37	3.545	1.67006	1.500	3.234	PER IMP=	57.00
COMPUTE NM HYD	P-1	-	4	.00290	2.51	.074	.47912	1.500	1.353	PER IMP=	2.00
ADD HYD	P-1+F-1D	3& 4	10	.04270	84.88	3.618	1.58885	1.500	3.106		
ADD HYD	AP-26A	10&12	10	.74280	152.00	43.487	1.09772	2.100	.320		
ROUTE RESERVOIR	AP-26AR	10	2	.74280	111.47	43.487	1.09770	3.300	.234	AC-FT=	5.863
COMPUTE NM HYD	AP-27&G-1D	-	3	.02290	56.30	2.534	2.07506	1.500	3.841	PER IMP=	80.00
COMPUTE NM HYD	AP-28&G-2D	-	4	.00310	7.72	.349	2.11028	1.500	3.893	PER IMP=	82.00
ROUTE	AP-28R	4	12	.00310	7.56	.349	2.11044	1.500	3.808		
COMPUTE NM HYD	AP-29&G-3D	-	5	.03510	77.66	3.885	2.07506	1.500	3.457	PER IMP=	80.00
COMPUTE NM HYD	G-4D	-	6	.00330	8.10	.359	2.03985	1.500	3.837	PER IMP=	78.00
ADD HYD	AP-30	12& 6	10	.00640	15.66	.708	2.07389	1.500	3.823		

FINISH

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994 RUN DATE (MON/DAY/YR) =01/22/2001
INPUT FILE = INT10EP.IN USER NO.= C_ANDRSN.I01

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION
*S DEVELOPED INTERIM CONDITIONS, WITHOUT I-40 INTERCEPTOR IN PLACE										
*S 10 YEAR, 24 HOUR STORM EVENT										
*S										
*S FILE NAME: INT10EP.IN										
*S										
*S THIS MODEL ANALYZES THE UNSER DIVERSION PONDS WITH THE EXISTING POND										
*S OUTLET WORKS AND THE PROPOSED POND GRADING.										
*S										
*S MASTER DRAINAGE PLAN FOR THE WEST SIDE TRANSIT FACILITY										
*S THIS AHYMO FILE ANALYZES THE UNSER DIVERSION (DETENTION PONDS) ASSUMING										
*S DEVELOPED CONDITIONS SOUTH OF INTERSTATE 40 WITHOUT THE I-40 INTERCEPTOR										
*S IN PLACE. IN OTHER WORDS, FULLY DEVELOPED CONDITIONS SOUTH OF I-40										
*S INCLUDING EXISTING CONDITION OFFSITE FLOWS NORTH OF INTERSTATE 40.										
*S										
*S NOTES: UNSER DIVERSION POND NO. 5 IS USED IN DEVELOPED/INTERIM CONDITIONS.										
*S THE OUTLET RATING CURVES WERE ALSO BASED UPON TAILWATER										
*S ELEVATIONS SHOWN ON THE UNSER DIVERSION (AS-BUILT)										
*S CONSTRUCTION PLAN SET (PLANS BY CHUCK EASTERLING)										
*S										
*S DATE: 11/8/00										
START										TIME= .00
RAINFALL TYPE= 2										RAIN24= 1.770
*S BEGIN WATERSHED NORTH OF INTERSTATE 40										
COMPUTE NM HYD	O-1E	-	3	.02420	3.43	.108	.08362	1.550	.221	PER IMP= .00
COMPUTE NM HYD	O-2E	-	4	.04220	4.48	.188	.08362	1.600	.166	PER IMP= .00
ADD HYD	AP-1	3& 4	10	.06640	7.65	.296	.08361	1.550	.180	
ROUTE	AP-1R	10	2	.06640	5.45	.296	.08362	1.700	.128	
COMPUTE NM HYD	O-3E	-	3	.01640	2.72	.086	.09818	1.550	.259	PER IMP= 1.00
ADD HYD	PARTAP-2	2& 3	10	.08280	7.02	.382	.08650	1.650	.132	
COMPUTE NM HYD	AP-7&O-5E	-	3	.10970	13.54	.660	.11274	1.600	.193	PER IMP= 2.00
DIVIDE HYD	PART_AP-7	3	98	.10970	13.54	.660	.11274	1.600	.193	
	PART_AP-7	AND	99	.00000	.00	.000	.00000	-.050	.000	
ROUTE	AP-7R	99	2	.00000	.00	.000	.00000	-.050	.000	
ADD HYD	AP-2	10& 2	10	.08280	7.02	.382	.08650	1.650	.132	
ROUTE	AP-2R	10	2	.08280	6.98	.382	.08651	1.700	.132	
COMPUTE NM HYD	I40-3	-	3	.00190	1.40	.060	.59326	1.500	1.154	PER IMP= 35.00
ADD HYD	AP-3	2& 3	20	.08470	7.72	.442	.09786	1.650	.142	
COMPUTE NM HYD	AP-4&O-4E	-	3	.00940	1.75	.057	.11274	1.550	.292	PER IMP= 2.00
ROUTE	AP-4R	3	2	.00940	1.84	.057	.11280	1.550	.306	
COMPUTE NM HYD	I40-4	-	3	.00120	.89	.038	.59326	1.500	1.156	PER IMP= 35.00
ADD HYD	AP-5	2& 3	10	.01060	2.64	.094	.16710	1.550	.389	
ADD HYD	AP-3+AP-5	20&10	10	.09530	9.60	.537	.10556	1.600	.157	
ROUTE	AP-6	10	12	.09530	9.61	.537	.10557	1.600	.158	
ROUTE	AP-7R-CUL	98	2	.10970	13.58	.660	.11274	1.600	.193	
COMPUTE NM HYD	I40-2	-	3	.00310	2.28	.098	.59326	1.500	1.152	PER IMP= 35.00
ADD HYD	AP-8	2& 3	20	.11280	15.19	.758	.12594	1.600	.210	
COMPUTE NM HYD	O-6E	-	3	.07320	6.49	.326	.08362	1.650	.138	PER IMP= .00
COMPUTE NM HYD	O-7E	-	4	.05230	7.41	.233	.08362	1.550	.221	PER IMP= .00
ADD HYD	AP-9	3& 4	10	.12550	13.19	.560	.08362	1.600	.164	

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION
ROUTE	AP-9R	10	2	.12550	13.27	.560	.08362	1.600	.165	
COMPUTE NM HYD	O-8E	-	3	.00990	14.68	.698	1.32131	1.500	2.318	PER IMP= 85.00
ADD HYD	AP-10	2& 3	10	.13540	26.66	1.257	.17411	1.550	.308	
ROUTE	AP-10R	10	2	.13540	21.80	1.257	.17411	1.650	.252	
COMPUTE NM HYD	O-9E	-	3	.03480	8.15	.290	.15643	1.500	.366	PER IMP= 5.00
ADD HYD	AP-11	2& 3	10	.17020	27.81	1.548	.17049	1.600	.255	
ROUTE	AP-11R	10	2	.17020	27.65	1.548	.17050	1.600	.254	
COMPUTE NM HYD	I40-1	-	3	.00750	5.52	.237	.59326	1.500	1.150	PER IMP= 35.00
ADD HYD	AP-12	2& 3	30	.17770	31.52	1.785	.18833	1.600	.277	
*S BEGIN WATERSHED SOUTH OF INTERSTATE 40										
ROUTE	AP-12R	30	2	.17770	31.40	1.785	.18834	1.600	.276	
ROUTE	AP-8R	20	22	.11280	15.07	.758	.12595	1.600	.209	
ADD HYD	AP-13	2&22	10	.29050	46.47	2.543	.16411	1.600	.250	
*S ROUTE FLOWS FROM AP-13 THRU PIPE TO UNSER DIVERSION POND NO. 5.										
*S IF FLOW FROM AP-13 WERE ROUTED TO TO UNSER DIVERSION POND NO. 6,										
*S POND NO. 6 WOULD OVERTOP. PLUS, THE EXISTING FLOW FROM THIS AREA										
*S DRAINS INTO POND NO. 5.										
ROUTE	AP-13R	10	32	.29050	44.30	2.543	.16411	1.650	.238	
COMPUTE NM HYD	AP-13A&A-1D	-	3	.02570	36.06	1.551	1.13124	1.500	2.192	PER IMP= 69.00
ROUTE	AP-13AR	3	22	.02570	35.42	1.551	1.13126	1.500	2.154	
ROUTE	AP-6R	12	2	.09530	9.68	.537	.10557	1.600	.159	
ADD HYD	PARTAP-14	22& 2	10	.12100	42.03	2.087	.32341	1.550	.543	
COMPUTE NM HYD	A-2D	-	3	.02840	45.59	2.013	1.32889	1.500	2.508	PER IMP= 84.00
ADD HYD	AP-14	10& 3	10	.14940	85.56	4.100	.51455	1.500	.895	
ROUTE	AP-14R	10	2	.14940	85.01	4.100	.51455	1.550	.889	
COMPUTE NM HYD	A-3D	-	3	.02400	39.69	1.802	1.40796	1.500	2.584	PER IMP= 90.00
ADD HYD	AP-15	2& 3	10	.17340	121.81	5.902	.63820	1.500	1.098	
COMPUTE NM HYD	AP-16&A-4D	-	3	.01250	18.04	.801	1.20080	1.500	2.256	PER IMP= 76.00

ADD HYD	AP-15+AP-16	10& 3	10	.18590	139.86	6.703	.67603	1.500	1.176		
COMPUTE NM HYD	P-6	-	3	.00430	.81	.026	.11274	1.550	.293	PER IMP=	2.00
ADD HYD	AP-17	10& 3	10	.19020	140.65	6.728	.66322	1.500	1.155		
ROUTE RESERVOIR	AP-17R	10	2	.19020	9.95	6.728	.66321	2.300	.082	AC-FT=	4.422
ROUTE	POND6ROUTE36	2	12	.19020	9.94	6.728	.66321	2.300	.082		
ROUTE	POND6ROUTE48	12	22	.19020	9.95	6.728	.66321	2.350	.082		
COMPUTE NM HYD	P-5	-	3	.00290	.55	.017	.11274	1.550	.294	PER IMP=	2.00
ADD HYD	AP-18	3&32	10	.29340	44.66	2.559	.16354	1.650	.238		
ROUTE RESERVOIR	AP-18R	10	12	.29340	15.49	2.559	.16353	2.150	.082	AC-FT=	1.231
ROUTE	POND-5ROUTE4	12	32	.29340	15.68	2.559	.16354	2.150	.084		
ADD HYD	POND-5R&6R	32&22	20	.48360	25.56	9.287	.36006	2.150	.083		
COMPUTE NM HYD	AP-19&C-1D	-	3	.01150	14.28	.597	.97312	1.500	1.940	PER IMP=	57.00
ROUTE	AP-19R	3	2	.01150	13.58	.597	.97317	1.550	1.846		
COMPUTE NM HYD	C-2D	-	4	.05740	69.47	2.979	.97312	1.500	1.891	PER IMP=	57.00
ADD HYD	AP-20	2& 4	10	.06890	82.47	3.576	.97311	1.500	1.870		
COMPUTE NM HYD	P-4	-	3	.00360	.68	.022	.11274	1.550	.293	PER IMP=	2.00
ADD HYD	P-4+POND-5R&	20& 3	20	.48720	25.61	9.307	.35819	2.150	.082		
ADD HYD	AP-20A	10&20	30	.55610	93.15	12.883	.43438	1.500	.262		
ROUTE RESERVOIR	AP-20AR	30	2	.55610	22.02	12.793	.43135	2.500	.062	AC-FT=	3.519
ROUTE	POND-4R	2	12	.55610	22.03	12.793	.43134	2.500	.062		
COMPUTE NM HYD	P-3	-	3	.00270	.51	.016	.11274	1.550	.294	PER IMP=	2.00
ADD HYD	P-3+POND-4R	12& 3	10	.55880	22.05	12.808	.42977	2.500	.062		
COMPUTE NM HYD	AP-21&D-1D	-	3	.04680	58.08	2.429	.97312	1.500	1.939	PER IMP=	57.00
ADD HYD	AP-21A	10& 3	10	.60560	62.65	15.237	.47176	1.500	.162		

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 3 NOTATION	
ROUTE RESERVOIR	AP-21AR	10	2	.60560	22.59	15.020	.46504	2.650	.058	AC-FT=	2.122
COMPUTE NM HYD	AP-22&E-1D	-	3	.02820	39.44	1.741	1.15760	1.500	2.185	PER IMP=	71.00
ROUTE	AP-22R	3	12	.02820	38.12	1.741	1.15761	1.550	2.112		
COMPUTE NM HYD	E-2D	-	3	.01720	29.01	1.292	1.40796	1.500	2.635	PER IMP=	90.00
ADD HYD	AP-23	3&12	10	.04540	66.46	3.033	1.25243	1.500	2.287		
ROUTE	AP-23R	10	12	.04540	66.02	3.033	1.25245	1.550	2.272		
COMPUTE NM HYD	E-3D	-	3	.04250	52.74	2.206	.97312	1.500	1.939	PER IMP=	57.00
ADD HYD	AP-24	12& 3	10	.08790	115.92	5.238	1.11738	1.500	2.060		
ROUTE	AP-23R	10	12	.08790	116.53	5.238	1.11739	1.550	2.071		
COMPUTE NM HYD	E-4D	-	3	.00300	5.07	.225	1.40796	1.500	2.638	PER IMP=	90.00
ADD HYD	AP-25	12& 3	10	.09090	120.95	5.464	1.12697	1.550	2.079		
ADD HYD	AP-25+POND-3	2&10	10	.69650	124.64	20.484	.55143	1.550	.280		
COMPUTE NM HYD	P-2	-	3	.00360	.68	.022	.11274	1.550	.293	PER IMP=	2.00
ADD HYD	AP-25A	3&10	10	.70010	125.32	20.505	.54915	1.550	.280		
ROUTE RESERVOIR	AP-25AR	10	2	.70010	9.51	18.407	.49296	9.300	.021	AC-FT=	6.968
ROUTE	POND-2R	2	12	.70010	9.51	18.405	.49291	9.300	.021		
COMPUTE NM HYD	AP-26&F-1D	-	3	.03980	48.17	2.066	.97312	1.500	1.891	PER IMP=	57.00
COMPUTE NM HYD	P-1	-	4	.00290	.55	.017	.11274	1.550	.294	PER IMP=	2.00
ADD HYD	P-1+F-1D	3& 4	10	.04270	48.71	2.082	.91423	1.500	1.783		
ADD HYD	AP-26A	10&12	10	.74280	48.74	20.487	.51713	1.500	.103		
ROUTE RESERVOIR	AP-26AR	10	2	.74280	9.08	19.217	.48508	17.300	.019	AC-FT=	3.032
COMPUTE NM HYD	AP-27&G-1D	-	3	.02290	35.53	1.559	1.27619	1.500	2.424	PER IMP=	80.00
COMPUTE NM HYD	AP-28&G-2D	-	4	.00310	4.90	.215	1.30254	1.500	2.469	PER IMP=	82.00
ROUTE	AP-28R	4	12	.00310	4.76	.215	1.30270	1.500	2.398		
COMPUTE NM HYD	AP-29&G-3D	-	5	.03510	48.94	2.389	1.27619	1.500	2.178	PER IMP=	80.00
COMPUTE NM HYD	G-4D	-	6	.00330	5.08	.220	1.24983	1.500	2.407	PER IMP=	78.00
ADD HYD	AP-30	12& 6	10	.00640	9.84	.435	1.27528	1.500	2.403		

FINISH

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994 RUN DATE (MON/DAY/YR) =01/22/2001
INPUT FILE = INT100EP.IN USER NO.= C_ANDRSN.I01

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION
										PAGE = 1
*S DEVELOPED INTERIM CONDITIONS, WITHOUT I-40 INTERCEPTOR IN PLACE										
*S 100 YEAR, 24 HOUR STORM EVENT										
*S										
*S FILE NAME: INT100EP.IN										
*S										
*S THIS MODEL ANALYZES THE UNSER DIVERSION PONDS WITH THE EXISTING POND										
*S OUTLET WORKS AND THE PROPOSED POND GRADING.										
*S										
*S MASTER DRAINAGE PLAN FOR THE WEST SIDE TRANSIT FACILITY										
*S THIS AHYMO FILE ANALYZES THE UNSER DIVERSION (DETENTION PONDS) ASSUMING										
*S DEVELOPED CONDITIONS SOUTH OF INTERSTATE 40 WITHOUT THE I-40 INTERCEPTOR										
*S IN PLACE. IN OTHER WORDS, FULLY DEVELOPED CONDITIONS SOUTH OF I-40										
*S INCLUDING EXISTING CONDITION OFFSITE FLOWS NORTH OF INTERSTATE 40.										
*S										
*S NOTES: UNSER DIVERSION POND NO. 5 IS USED IN DEVELOPED/INTERIM CONDITIONS.										
*S THE OUTLET RATING CURVES WERE ALSO BASED UPON TAILWATER										
*S ELEVATIONS SHOWN ON THE UNSER DIVERSION (AS-BUILT)										
*S CONSTRUCTION PLAN SET (PLANS BY CHUCK EASTERLING)										
*S										
*S DATE: 11/8/00										
START										TIME= .00
RAINFALL TYPE= 2										RAIN24= 2.660
*S BEGIN WATERSHED NORTH OF INTERSTATE 40										
COMPUTE NM HYD	O-1E	-	3	.02420	18.06	.567	.43936	1.550	1.166	PER IMP= .00
COMPUTE NM HYD	O-2E	-	4	.04220	24.34	.989	.43936	1.600	.901	PER IMP= .00
ADD HYD	AP-1	3& 4	10	.06640	41.88	1.556	.43936	1.550	.986	
ROUTE	AP-1R	10	2	.06640	36.11	1.556	.43938	1.650	.850	
COMPUTE NM HYD	O-3E	-	3	.01640	13.10	.402	.45924	1.500	1.248	PER IMP= 1.00
ADD HYD	PARTAP-2	2& 3	10	.08280	44.73	1.958	.44330	1.650	.844	
COMPUTE NM HYD	AP-7&O-5E	-	3	.10970	63.37	2.803	.47912	1.600	.903	PER IMP= 2.00
DIVIDE HYD	PART_AP-7	3	98	.08594	32.00	2.196	.47911	1.500	.582	
	PART_AP-7	AND	99	.02376	31.37	.607	.47911	1.600	2.062	
ROUTE	AP-7R	99	2	.02376	31.64	.608	.47944	1.600	2.081	
ADD HYD	AP-2	10& 2	10	.10656	76.00	2.565	.45135	1.600	1.114	
ROUTE	AP-2R	10	2	.10656	79.83	2.565	.45136	1.600	1.171	
COMPUTE NM HYD	I40-3	-	3	.00190	2.86	.115	1.13512	1.500	2.350	PER IMP= 35.00
ADD HYD	AP-3	2& 3	20	.10846	81.89	2.680	.46333	1.600	1.180	
COMPUTE NM HYD	AP-4&O-4E	-	3	.00940	8.11	.240	.47912	1.500	1.348	PER IMP= 2.00
ROUTE	AP-4R	3	2	.00940	8.17	.240	.47921	1.500	1.358	
COMPUTE NM HYD	I40-4	-	3	.00120	1.81	.073	1.13512	1.500	2.357	PER IMP= 35.00
ADD HYD	AP-5	2& 3	10	.01060	9.98	.313	.55337	1.500	1.471	
ADD HYD	AP-3+AP-5	20&10	10	.11906	89.90	2.993	.47135	1.600	1.180	
ROUTE	AP-6	10	12	.11906	87.97	2.993	.47135	1.650	1.154	
ROUTE	AP-7R-CUL	98	2	.08594	33.23	2.196	.47912	1.500	.604	
COMPUTE NM HYD	I40-2	-	3	.00310	4.65	.188	1.13512	1.500	2.346	PER IMP= 35.00
ADD HYD	AP-8	2& 3	20	.08904	37.88	2.384	.50195	1.500	.665	
COMPUTE NM HYD	O-6E	-	3	.07320	35.69	1.715	.43936	1.600	.762	PER IMP= .00
COMPUTE NM HYD	O-7E	-	4	.05230	39.03	1.226	.43936	1.550	1.166	PER IMP= .00
ADD HYD	AP-9	3& 4	10	.12550	71.47	2.941	.43936	1.550	.890	

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION
										PAGE = 2
ROUTE	AP-9R	10	2	.12550	70.89	2.941	.43937	1.600	.883	
COMPUTE NM HYD	O-8E	-	3	.00990	23.00	1.124	2.12906	1.500	3.630	PER IMP= 85.00
ADD HYD	AP-10	2& 3	10	.13540	92.59	4.065	.56290	1.550	1.068	
ROUTE	AP-10R	10	2	.13540	89.10	4.065	.56291	1.600	1.028	
COMPUTE NM HYD	O-9E	-	3	.03480	31.99	1.000	.53875	1.500	1.436	PER IMP= 5.00
ADD HYD	AP-11	2& 3	10	.17020	114.14	5.065	.55797	1.600	1.048	
ROUTE	AP-11R	10	2	.17020	114.79	5.065	.55797	1.600	1.054	
COMPUTE NM HYD	I40-1	-	3	.00750	11.24	.454	1.13512	1.500	2.342	PER IMP= 35.00
ADD HYD	AP-12	2& 3	30	.17770	122.86	5.519	.58232	1.600	1.080	
*S BEGIN WATERSHED SOUTH OF INTERSTATE 40										
ROUTE	AP-12R	30	2	.17770	123.23	5.519	.58233	1.600	1.084	
ROUTE	AP-8R	20	22	.08904	39.96	2.384	.50196	1.500	.701	
ADD HYD	AP-13	2&22	10	.26674	158.78	7.902	.55549	1.600	.930	
*S ROUTE FLOWS FROM AP-13 THRU PIPE TO UNSER DIVERSION POND NO. 5.										
*S IF FLOW FROM AP-13 WERE ROUTED TO TO UNSER DIVERSION POND NO. 6,										
*S POND NO. 6 WOULD OVERTOP. PLUS, THE EXISTING FLOW FROM THIS AREA										
*S DRAINS INTO POND NO. 5.										
ROUTE	AP-13R	10	32	.26674	154.05	7.902	.55550	1.650	.902	
COMPUTE NM HYD	AP-13A&A-1D	-	3	.02570	59.05	2.579	1.88137	1.500	3.590	PER IMP= 69.00
ROUTE	AP-13AR	3	22	.02570	58.26	2.579	1.88138	1.500	3.542	
ROUTE	AP-6R	12	2	.11906	88.44	2.993	.47135	1.650	1.161	
ADD HYD	PARTAP-14	22& 2	10	.14476	125.63	5.572	.72167	1.600	1.356	
COMPUTE NM HYD	A-2D	-	3	.02840	71.47	3.250	2.14550	1.500	3.932	PER IMP= 84.00
ADD HYD	AP-14	10& 3	10	.17316	175.97	8.822	.95518	1.550	1.588	
ROUTE	AP-14R	10	2	.17316	175.47	8.822	.95518	1.600	1.583	
COMPUTE NM HYD	A-3D	-	3	.02400	61.30	2.881	2.25115	1.500	3.991	PER IMP= 90.00
ADD HYD	AP-15	2& 3	10	.19716	229.58	11.703	1.11293	1.550	1.819	
COMPUTE NM HYD	AP-16&A-4D	-	3	.01250	29.07	1.312	1.96827	1.500	3.634	PER IMP= 76.00

ADD HYD	AP-15+AP-16	10& 3	10	.20966	255.16	13.015	1.16393	1.550	1.902		
COMPUTE NM HYD	P-6	-	3	.00430	3.72	.110	.47912	1.500	1.350	PER IMP=	2.00
ADD HYD	AP-17	10& 3	10	.21396	258.75	13.125	1.15016	1.550	1.890		
ROUTE RESERVOIR	AP-17R	10	2	.21396	69.43	13.125	1.15015	2.000	.507	AC-FT=	7.626
ROUTE	POND6ROUTE36	2	12	.21396	69.45	13.125	1.15015	2.050	.507		
ROUTE	POND6ROUTE48	12	22	.21396	69.38	13.125	1.15015	2.050	.507		
COMPUTE NM HYD	P-5	-	3	.00290	2.51	.074	.47912	1.500	1.353	PER IMP=	2.00
ADD HYD	AP-18	3&32	10	.26964	155.57	7.976	.55462	1.650	.901		
ROUTE RESERVOIR	AP-18R	10	12	.26964	113.38	7.976	.55462	1.800	.657	AC-FT=	2.044
ROUTE	POND-5ROUTE4	12	32	.26964	119.58	7.976	.55462	1.800	.693		
ADD HYD	POND-5R&6R	32&22	20	.48360	176.49	21.101	.81811	1.900	.570		
COMPUTE NM HYD	AP-19&C-1D	-	3	.01150	24.41	1.024	1.67006	1.500	3.317	PER IMP=	57.00
ROUTE	AP-19R	3	2	.01150	23.29	1.024	1.67010	1.550	3.164		
COMPUTE NM HYD	C-2D	-	4	.05740	118.78	5.113	1.67006	1.500	3.233	PER IMP=	57.00
ADD HYD	AP-20	2& 4	10	.06890	141.50	6.137	1.67005	1.500	3.209		
COMPUTE NM HYD	P-4	-	3	.00360	3.11	.092	.47912	1.500	1.352	PER IMP=	2.00
ADD HYD	P-4+POND-5R&	20& 3	20	.48720	176.94	21.192	.81559	1.900	.567		
ADD HYD	AP-20A	10&20	30	.55610	214.97	27.329	.92145	1.900	.604		
ROUTE RESERVOIR	AP-20AR	30	2	.55610	116.55	27.232	.91819	2.200	.327	AC-FT=	7.281
ROUTE	POND-4R	2	12	.55610	116.61	27.232	.91818	2.250	.328		
COMPUTE NM HYD	P-3	-	3	.00270	2.34	.069	.47912	1.500	1.354	PER IMP=	2.00
ADD HYD	P-3+POND-4R	12& 3	10	.55880	116.74	27.300	.91604	2.250	.326		
COMPUTE NM HYD	AP-21&D-1D	-	3	.04680	99.31	4.168	1.67006	1.500	3.316	PER IMP=	57.00
ADD HYD	AP-21A	10& 3	10	.60560	132.28	31.469	.97430	2.050	.341		

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 3 NOTATION	
ROUTE RESERVOIR	AP-21AR	10	2	.60560	108.34	31.169	.96504	2.600	.280	AC-FT=	4.114
COMPUTE NM HYD	AP-22&E-1D	-	3	.02820	64.16	2.883	1.91659	1.500	3.555	PER IMP=	71.00
ROUTE	AP-22R	3	12	.02820	61.84	2.883	1.91660	1.550	3.427		
COMPUTE NM HYD	E-2D	-	3	.01720	44.80	2.065	2.25115	1.500	4.070	PER IMP=	90.00
ADD HYD	AP-23	3&12	10	.04540	106.21	4.948	2.04333	1.500	3.655		
ROUTE	AP-23R	10	12	.04540	104.90	4.948	2.04334	1.550	3.610		
COMPUTE NM HYD	E-3D	-	3	.04250	90.19	3.785	1.67006	1.500	3.316	PER IMP=	57.00
ADD HYD	AP-24	12& 3	10	.08790	191.75	8.733	1.86285	1.500	3.408		
ROUTE	AP-23R	10	12	.08790	190.60	8.733	1.86286	1.550	3.388		
COMPUTE NM HYD	E-4D	-	3	.00300	7.83	.360	2.25115	1.500	4.076	PER IMP=	90.00
ADD HYD	AP-25	12& 3	10	.09090	197.44	9.093	1.87566	1.550	3.394		
ADD HYD	AP-25+POND-3	2&10	10	.69650	203.89	40.263	1.08388	1.550	.457		
COMPUTE NM HYD	P-2	-	3	.00360	3.11	.092	.47912	1.500	1.352	PER IMP=	2.00
ADD HYD	AP-25A	3&10	10	.70010	206.90	40.354	1.08076	1.550	.462		
ROUTE RESERVOIR	AP-25AR	10	2	.70010	119.07	36.987	.99057	2.350	.266	AC-FT=	9.470
ROUTE	POND-2R	2	12	.70010	120.56	36.983	.99048	2.350	.269		
COMPUTE NM HYD	AP-26&F-1D	-	3	.03980	82.37	3.545	1.67006	1.500	3.234	PER IMP=	57.00
COMPUTE NM HYD	P-1	-	4	.00290	2.51	.074	.47912	1.500	1.353	PER IMP=	2.00
ADD HYD	P-1+F-1D	3& 4	10	.04270	84.88	3.618	1.58885	1.500	3.106		
ADD HYD	AP-26A	10&12	10	.74280	124.76	40.602	1.02488	2.250	.262		
ROUTE RESERVOIR	AP-26AR	10	2	.74280	103.22	37.292	.94132	3.150	.217	AC-FT=	7.967
COMPUTE NM HYD	AP-27&G-1D	-	3	.02290	56.30	2.534	2.07506	1.500	3.841	PER IMP=	80.00
COMPUTE NM HYD	AP-28&G-2D	-	4	.00310	7.72	.349	2.11028	1.500	3.893	PER IMP=	82.00
ROUTE	AP-28R	4	12	.00310	7.56	.349	2.11044	1.500	3.808		
COMPUTE NM HYD	AP-29&G-3D	-	5	.03510	77.66	3.885	2.07506	1.500	3.457	PER IMP=	80.00
COMPUTE NM HYD	G-4D	-	6	.00330	8.10	.359	2.03985	1.500	3.837	PER IMP=	78.00
ADD HYD	AP-30	12& 6	10	.00640	15.66	.708	2.07389	1.500	3.823		
FINISH											

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994 RUN DATE (MON/DAY/YR) =01/22/2001
INPUT FILE = INT10PP.IN USER NO.= C_ANDRSN.I01

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION
*S DEVELOPED INTERIM CONDITIONS, WITHOUT I-40 INTERCEPTOR IN PLACE										
*S 10 YEAR, 24 HOUR STORM EVENT										
*S										
*S FILE NAME: INT10PP.IN										
*S										
*S THIS MODEL ANALYZES THE UNSER DIVERSION PONDS WITH THE PROPOSED POND										
*S OUTLET WORKS AND THE PROPOSED POND GRADING.										
*S										
*S MASTER DRAINAGE PLAN FOR THE WEST SIDE TRANSIT FACILITY										
*S THIS AHYMO FILE ANALYZES THE UNSER DIVERSION (DETENTION PONDS) ASSUMING										
*S DEVELOPED CONDITIONS SOUTH OF INTERSTATE 40 WITHOUT THE I-40 INTERCEPTOR										
*S IN PLACE. IN OTHER WORDS, FULLY DEVELOPED CONDITIONS SOUTH OF I-40										
*S INCLUDING EXISTING CONDITION OFFSITE FLOWS NORTH OF INTERSTATE 40.										
*S										
*S NOTES: UNSER DIVERSION POND NO. 5 IS USED IN DEVELOPED/INTERIM CONDITIONS.										
*S THE OUTLET RATING CURVES WERE ALSO BASED UPON TAILWATER										
*S ELEVATIONS SHOWN ON THE UNSER DIVERSION (AS-BUILT)										
*S CONSTRUCTION PLAN SET (PLANS BY CHUCK EASTERLING)										
*S										
*S DATE: 11/8/00										
START										TIME= .00
RAINFALL TYPE= 2										RAIN24= 1.770
*S BEGIN WATERSHED NORTH OF INTERSTATE 40										
COMPUTE NM HYD	O-1E	-	3	.02420	3.43	.108	.08362	1.550	.221	PER IMP= .00
COMPUTE NM HYD	O-2E	-	4	.04220	4.48	.188	.08362	1.600	.166	PER IMP= .00
ADD HYD	AP-1	3& 4	10	.06640	7.65	.296	.08361	1.550	.180	
ROUTE	AP-1R	10	2	.06640	5.45	.296	.08362	1.700	.128	
COMPUTE NM HYD	O-3E	-	3	.01640	2.72	.086	.09818	1.550	.259	PER IMP= 1.00
ADD HYD	PARTAP-2	2& 3	10	.08280	7.02	.382	.08650	1.650	.132	
COMPUTE NM HYD	AP-7&O-5E	-	3	.10970	13.54	.660	.11274	1.600	.193	PER IMP= 2.00
DIVIDE HYD	PART_AP-7	3	98	.10970	13.54	.660	.11274	1.600	.193	
	PART_AP-7	AND	99	.00000	.00	.000	.00000	-.050	.000	
ROUTE	AP-7R	99	2	.00000	.00	.000	.00000	-.050	.000	
ADD HYD	AP-2	10& 2	10	.08280	7.02	.382	.08650	1.650	.132	
ROUTE	AP-2R	10	2	.08280	6.98	.382	.08651	1.700	.132	
COMPUTE NM HYD	I40-3	-	3	.00190	1.40	.060	.59326	1.500	1.154	PER IMP= 35.00
ADD HYD	AP-3	2& 3	20	.08470	7.72	.442	.09786	1.650	.142	
COMPUTE NM HYD	AP-4&O-4E	-	3	.00940	1.75	.057	.11274	1.550	.292	PER IMP= 2.00
ROUTE	AP-4R	3	2	.00940	1.84	.057	.11280	1.550	.306	
COMPUTE NM HYD	I40-4	-	3	.00120	.89	.038	.59326	1.500	1.156	PER IMP= 35.00
ADD HYD	AP-5	2& 3	10	.01060	2.64	.094	.16710	1.550	.389	
ADD HYD	AP-3+AP-5	20&10	10	.09530	9.60	.537	.10556	1.600	.157	
ROUTE	AP-6	10	12	.09530	9.61	.537	.10557	1.600	.158	
ROUTE	AP-7R-CUL	98	2	.10970	13.58	.660	.11274	1.600	.193	
COMPUTE NM HYD	I40-2	-	3	.00310	2.28	.098	.59326	1.500	1.152	PER IMP= 35.00
ADD HYD	AP-8	2& 3	20	.11280	15.19	.758	.12594	1.600	.210	
COMPUTE NM HYD	O-6E	-	3	.07320	6.49	.326	.08362	1.650	.138	PER IMP= .00
COMPUTE NM HYD	O-7E	-	4	.05230	7.41	.233	.08362	1.550	.221	PER IMP= .00
ADD HYD	AP-9	3& 4	10	.12550	13.19	.560	.08362	1.600	.164	

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION
ROUTE	AP-9R	10	2	.12550	13.27	.560	.08362	1.600	.165	
COMPUTE NM HYD	O-8E	-	3	.00990	14.68	.698	1.32131	1.500	2.318	PER IMP= 85.00
ADD HYD	AP-10	2& 3	10	.13540	26.66	1.257	.17411	1.550	.308	
ROUTE	AP-10R	10	2	.13540	21.80	1.257	.17411	1.650	.252	
COMPUTE NM HYD	O-9E	-	3	.03480	8.15	.290	.15643	1.500	.366	PER IMP= 5.00
ADD HYD	AP-11	2& 3	10	.17020	27.81	1.548	.17049	1.600	.255	
ROUTE	AP-11R	10	2	.17020	27.65	1.548	.17050	1.600	.254	
COMPUTE NM HYD	I40-1	-	3	.00750	5.52	.237	.59326	1.500	1.150	PER IMP= 35.00
ADD HYD	AP-12	2& 3	30	.17770	31.52	1.785	.18833	1.600	.277	
*S BEGIN WATERSHED SOUTH OF INTERSTATE 40										
ROUTE	AP-12R	30	2	.17770	31.40	1.785	.18834	1.600	.276	
ROUTE	AP-8R	20	22	.11280	15.07	.758	.12595	1.600	.209	
ADD HYD	AP-13	2&22	10	.29050	46.47	2.543	.16411	1.600	.250	
*S ROUTE FLOWS FROM AP-13 THRU PIPE TO UNSER DIVERSION POND NO. 5.										
*S IF FLOW FROM AP-13 WERE ROUTED TO TO UNSER DIVERSION POND NO. 6,										
*S POND NO. 6 WOULD OVERTOP. PLUS, THE EXISTING FLOW FROM THIS AREA										
*S DRAINS INTO POND NO. 5.										
ROUTE	AP-13R	10	32	.29050	44.30	2.543	.16411	1.650	.238	
COMPUTE NM HYD	AP-13A&A-1D	-	3	.02570	36.06	1.551	1.13124	1.500	2.192	PER IMP= 69.00
ROUTE	AP-13AR	3	22	.02570	35.42	1.551	1.13126	1.500	2.154	
ROUTE	AP-6R	12	2	.09530	9.68	.537	.10557	1.600	.159	
ADD HYD	PARTAP-14	22& 2	10	.12100	42.03	2.087	.32341	1.550	.543	
COMPUTE NM HYD	A-2D	-	3	.02840	45.59	2.013	1.32889	1.500	2.508	PER IMP= 84.00
ADD HYD	AP-14	10& 3	10	.14940	85.56	4.100	.51455	1.500	.895	
ROUTE	AP-14R	10	2	.14940	85.01	4.100	.51455	1.550	.889	
COMPUTE NM HYD	A-3D	-	3	.02400	39.69	1.802	1.40796	1.500	2.584	PER IMP= 90.00
ADD HYD	AP-15	2& 3	10	.17340	121.81	5.902	.63820	1.500	1.098	
COMPUTE NM HYD	AP-16&A-4D	-	3	.01250	18.04	.801	1.20080	1.500	2.256	PER IMP= 76.00

ADD HYD	AP-15+AP-16	10& 3	10	.18590	139.86	6.703	.67603	1.500	1.176	
COMPUTE NM HYD	P-6	-	3	.00430	.81	.026	.11274	1.550	.293	PER IMP= 2.00
ADD HYD	AP-17	10& 3	10	.19020	140.65	6.728	.66322	1.500	1.155	
ROUTE RESERVOIR	AP-17R	10	2	.19020	38.95	6.728	.66321	2.000	.320	AC-FT= 3.003
ROUTE	POND6ROUTE36	2	12	.19020	38.94	6.728	.66321	2.000	.320	
ROUTE	POND6ROUTE48	12	22	.19020	38.94	6.728	.66321	2.000	.320	
COMPUTE NM HYD	P-5	-	3	.00290	.55	.017	.11274	1.550	.294	PER IMP= 2.00
ADD HYD	AP-18	3&32	10	.29340	44.66	2.559	.16354	1.650	.238	
ROUTE RESERVOIR	AP-18R	10	12	.29340	15.49	2.559	.16353	2.150	.082	AC-FT= 1.231
ROUTE	POND-5ROUTE4	12	32	.29340	15.68	2.559	.16354	2.150	.084	
ADD HYD	POND-5R&6R	32&22	20	.48360	53.95	9.287	.36006	2.150	.174	
COMPUTE NM HYD	AP-19&C-1D	-	3	.01150	14.28	.597	.97312	1.500	1.940	PER IMP= 57.00
ROUTE	AP-19R	3	2	.01150	13.58	.597	.97317	1.550	1.846	
COMPUTE NM HYD	C-2D	-	4	.05740	69.47	2.979	.97312	1.500	1.891	PER IMP= 57.00
ADD HYD	AP-20	2& 4	10	.06890	82.47	3.576	.97311	1.500	1.870	
COMPUTE NM HYD	P-4	-	3	.00360	.68	.022	.11274	1.550	.293	PER IMP= 2.00
ADD HYD	P-4+POND-5R&	20& 3	20	.48720	54.00	9.307	.35819	2.150	.173	
ADD HYD	AP-20A	10&20	30	.55610	104.06	12.883	.43438	1.550	.292	
ROUTE RESERVOIR	AP-20AR	30	2	.55610	52.39	12.802	.43165	2.300	.147	AC-FT= 2.653
ROUTE	POND-4R	2	12	.55610	52.38	12.802	.43165	2.300	.147	
COMPUTE NM HYD	P-3	-	3	.00270	.51	.016	.11274	1.550	.294	PER IMP= 2.00
ADD HYD	P-3+POND-4R	12& 3	10	.55880	52.41	12.817	.43008	2.300	.147	
COMPUTE NM HYD	AP-21&D-1D	-	3	.04680	58.08	2.429	.97312	1.500	1.939	PER IMP= 57.00
ADD HYD	AP-21A	10& 3	10	.60560	82.19	15.246	.47204	1.550	.212	

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION
										PAGE = 3
ROUTE RESERVOIR	AP-21AR	10	2	.60560	58.50	15.177	.46991	2.150	.151	AC-FT= 2.163
COMPUTE NM HYD	AP-22&E-1D	-	3	.02820	39.44	1.741	1.15760	1.500	2.185	PER IMP= 71.00
ROUTE	AP-22R	3	12	.02820	38.12	1.741	1.15761	1.550	2.112	
COMPUTE NM HYD	E-2D	-	3	.01720	29.01	1.292	1.40796	1.500	2.635	PER IMP= 90.00
ADD HYD	AP-23	3&12	10	.04540	66.46	3.033	1.25243	1.500	2.287	
ROUTE	AP-23R	10	12	.04540	66.02	3.033	1.25245	1.550	2.272	
COMPUTE NM HYD	E-3D	-	3	.04250	52.74	2.206	.97312	1.500	1.939	PER IMP= 57.00
ADD HYD	AP-24	12& 3	10	.08790	115.92	5.238	1.11738	1.500	2.060	
ROUTE	AP-23R	10	12	.08790	116.53	5.238	1.11739	1.550	2.071	
COMPUTE NM HYD	E-4D	-	3	.00300	5.07	.225	1.40796	1.500	2.638	PER IMP= 90.00
ADD HYD	AP-25	12& 3	10	.09090	120.95	5.464	1.12697	1.550	2.079	
ADD HYD	AP-25+POND-3	2&10	10	.69650	141.83	20.641	.55566	1.550	.318	
COMPUTE NM HYD	P-2	-	3	.00360	.68	.022	.11274	1.550	.293	PER IMP= 2.00
ADD HYD	AP-25A	3&10	10	.70010	142.50	20.662	.55336	1.550	.318	
ROUTE RESERVOIR	AP-25AR	10	2	.70010	43.12	19.612	.52524	3.300	.096	AC-FT= 6.128
ROUTE	POND-2R	2	12	.70010	43.12	19.612	.52524	3.300	.096	
COMPUTE NM HYD	AP-26&F-1D	-	3	.03980	48.17	2.066	.97312	1.500	1.891	PER IMP= 57.00
COMPUTE NM HYD	P-1	-	4	.00290	.55	.017	.11274	1.550	.294	PER IMP= 2.00
ADD HYD	P-1+F-1D	3& 4	10	.04270	48.71	2.082	.91423	1.500	1.783	
ADD HYD	AP-26A	10&12	10	.74280	49.69	21.694	.54760	1.600	.105	
ROUTE RESERVOIR	AP-26AR	10	2	.74280	42.51	21.642	.54628	3.850	.089	AC-FT= 2.024
COMPUTE NM HYD	AP-27&G-1D	-	3	.02290	35.53	1.559	1.27619	1.500	2.424	PER IMP= 80.00
COMPUTE NM HYD	AP-28&G-2D	-	4	.00310	4.90	.215	1.30254	1.500	2.469	PER IMP= 82.00
ROUTE	AP-28R	4	12	.00310	4.76	.215	1.30270	1.500	2.398	
COMPUTE NM HYD	AP-29&G-3D	-	5	.03510	48.94	2.389	1.27619	1.500	2.178	PER IMP= 80.00
COMPUTE NM HYD	G-4D	-	6	.00330	5.08	.220	1.24983	1.500	2.407	PER IMP= 78.00
ADD HYD	AP-30	12& 6	10	.00640	9.84	.435	1.27528	1.500	2.403	

FINISH

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994 RUN DATE (MON/DAY/YR) =01/22/2001
INPUT FILE = INT100PP.IN USER NO.= C_ANDRSN.101

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION
										PAGE = 1
*S DEVELOPED INTERIM CONDITIONS, WITHOUT I-40 INTERCEPTOR IN PLACE										
*S 100 YEAR, 24 HOUR STORM EVENT										
*S										
*S FILE NAME: INT100PP.IN										
*S										
*S THIS MODEL ANALYZES THE UNSER DIVERSION PONDS WITH THE PROPOSED POND										
*S OUTLET WORKS AND THE PROPOSED POND GRADING.										
*S										
*S MASTER DRAINAGE PLAN FOR THE WEST SIDE TRANSIT FACILITY										
*S THIS AHYMO FILE ANALYZES THE UNSER DIVERSION (DETENTION PONDS) ASSUMING										
*S DEVELOPED CONDITIONS SOUTH OF INTERSTATE 40 WITHOUT THE I-40 INTERCEPTOR										
*S IN PLACE. IN OTHER WORDS, FULLY DEVELOPED CONDITIONS SOUTH OF I-40										
*S INCLUDING EXISTING CONDITION OFFSITE FLOWS NORTH OF INTERSTATE 40.										
*S										
*S NOTES: UNSER DIVERSION POND NO. 5 IS USED IN DEVELOPED/INTERIM CONDITIONS.										
*S THE OUTLET RATING CURVES WERE ALSO BASED UPON TAILWATER										
*S ELEVATIONS SHOWN ON THE UNSER DIVERSION (AS-BUILT)										
*S CONSTRUCTION PLAN SET (PLANS BY CHUCK EASTERLING)										
*S										
*S DATE: 11/8/00										
START										TIME= .00
RAINFALL TYPE= 2										RAIN24= 2.660
*S BEGIN WATERSHED NORTH OF INTERSTATE 40										
COMPUTE NM HYD	O-1E	-	3	.02420	18.06	.567	.43936	1.550	1.166	PER IMP= .00
COMPUTE NM HYD	O-2E	-	4	.04220	24.34	.989	.43936	1.600	.901	PER IMP= .00
ADD HYD	AP-1	3& 4	10	.06640	41.88	1.556	.43936	1.550	.986	
ROUTE	AP-1R	10	2	.06640	36.11	1.556	.43938	1.650	.850	
COMPUTE NM HYD	O-3E	-	3	.01640	13.10	.402	.45924	1.500	1.248	PER IMP= 1.00
ADD HYD	PARTAP-2	2& 3	10	.08280	44.73	1.958	.44330	1.650	.844	
COMPUTE NM HYD	AP-7&O-5E	-	3	.10970	63.37	2.803	.47912	1.600	.903	PER IMP= 2.00
DIVIDE HYD	PART_AP-7	3	98	.08594	32.00	2.196	.47911	1.500	.582	
	PART_AP-7	AND	99	.02376	31.37	.607	.47911	1.600	2.062	
ROUTE	AP-7R	99	2	.02376	31.64	.608	.47944	1.600	2.081	
ADD HYD	AP-2	10& 2	10	.10656	76.00	2.565	.45135	1.600	1.114	
ROUTE	AP-2R	10	2	.10656	79.83	2.565	.45136	1.600	1.171	
COMPUTE NM HYD	I40-3	-	3	.00190	2.86	.115	1.13512	1.500	2.350	PER IMP= 35.00
ADD HYD	AP-3	2& 3	20	.10846	81.89	2.680	.46333	1.600	1.180	
COMPUTE NM HYD	AP-4&O-4E	-	3	.00940	8.11	.240	.47912	1.500	1.348	PER IMP= 2.00
ROUTE	AP-4R	3	2	.00940	8.17	.240	.47921	1.500	1.358	
COMPUTE NM HYD	I40-4	-	3	.00120	1.81	.073	1.13512	1.500	2.357	PER IMP= 35.00
ADD HYD	AP-5	2& 3	10	.01060	9.98	.313	.55337	1.500	1.471	
ADD HYD	AP-3+AP-5	20&10	10	.11906	89.90	2.993	.47135	1.600	1.180	
ROUTE	AP-6	10	12	.11906	87.97	2.993	.47135	1.650	1.154	
ROUTE	AP-7R-CUL	98	2	.08594	33.23	2.196	.47912	1.500	.604	
COMPUTE NM HYD	I40-2	-	3	.00310	4.65	.188	1.13512	1.500	2.346	PER IMP= 35.00
ADD HYD	AP-8	2& 3	20	.08904	37.88	2.384	.50195	1.500	.665	
COMPUTE NM HYD	O-6E	-	3	.07320	35.69	1.715	.43936	1.500	.762	PER IMP= .00
COMPUTE NM HYD	O-7E	-	4	.05230	39.03	1.226	.43936	1.550	1.166	PER IMP= .00
ADD HYD	AP-9	3& 4	10	.12550	71.47	2.941	.43936	1.550	.890	

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION
										PAGE = 2
ROUTE	AP-9R	10	2	.12550	70.89	2.941	.43937	1.600	.883	
COMPUTE NM HYD	O-8E	-	3	.00990	23.00	1.124	2.12906	1.500	3.630	PER IMP= 85.00
ADD HYD	AP-10	2& 3	10	.13540	92.59	4.065	.56290	1.550	1.068	
ROUTE	AP-10R	10	2	.13540	89.10	4.065	.56291	1.600	1.028	
COMPUTE NM HYD	O-9E	-	3	.03480	31.99	1.000	.53875	1.500	1.436	PER IMP= 5.00
ADD HYD	AP-11	2& 3	10	.17020	114.14	5.065	.55797	1.600	1.048	
ROUTE	AP-11R	10	2	.17020	114.79	5.065	.55797	1.600	1.054	
COMPUTE NM HYD	I40-1	-	3	.00750	11.24	.454	1.13512	1.500	2.342	PER IMP= 35.00
ADD HYD	AP-12	2& 3	30	.17770	122.86	5.519	.58232	1.600	1.080	
*S BEGIN WATERSHED SOUTH OF INTERSTATE 40										
ROUTE	AP-12R	30	2	.17770	123.23	5.519	.58233	1.600	1.084	
ROUTE	AP-8R	20	22	.08904	39.96	2.384	.50196	1.500	.701	
ADD HYD	AP-13	2&22	10	.26674	158.78	7.902	.55549	1.600	.930	
*S ROUTE FLOWS FROM AP-13 THRU PIPE TO UNSER DIVERSION POND NO. 5.										
*S IF FLOW FROM AP-13 WERE ROUTED TO TO UNSER DIVERSION POND NO. 6,										
*S POND NO. 6 WOULD OVERTOP. PLUS, THE EXISTING FLOW FROM THIS AREA										
*S DRAINS INTO POND NO. 5.										
ROUTE	AP-13R	10	32	.26674	154.05	7.902	.55550	1.650	.902	
COMPUTE NM HYD	AP-13A&A-1D	-	3	.02570	59.05	2.579	1.88137	1.500	3.590	PER IMP= 69.00
ROUTE	AP-13AR	3	22	.02570	58.26	2.579	1.88138	1.500	3.542	
ROUTE	AP-6R	12	2	.11906	88.44	2.993	.47135	1.650	1.161	
ADD HYD	PARTAP-14	22& 2	10	.14476	125.63	5.572	.72167	1.600	1.356	
COMPUTE NM HYD	A-2D	-	3	.02840	71.47	3.250	2.14550	1.500	3.932	PER IMP= 84.00
ADD HYD	AP-14	10& 3	10	.17316	175.97	8.822	.95518	1.550	1.588	
ROUTE	AP-14R	10	2	.17316	175.47	8.822	.95518	1.600	1.583	
COMPUTE NM HYD	A-3D	-	3	.02400	61.30	2.881	2.25115	1.500	3.991	PER IMP= 90.00
ADD HYD	AP-15	2& 3	10	.19716	229.58	11.703	1.11293	1.550	1.819	
COMPUTE NM HYD	AP-16&A-4D	-	3	.01250	29.07	1.312	1.96827	1.500	3.634	PER IMP= 76.00

ADD HYD	AP-15+AP-16	10& 3	10	.20966	255.16	13.015	1.16393	1.550	1.902		
COMPUTE NM HYD	P-6	-	3	.00430	3.72	.110	.47912	1.500	1.350	PER IMP=	2.00
ADD HYD	AP-17	10& 3	10	.21396	258.75	13.125	1.15016	1.550	1.890		
ROUTE RESERVOIR	AP-17R	10	2	.21396	62.62	13.125	1.15015	2.050	.457	AC-FT=	6.765
ROUTE	POND6ROUTE36	2	12	.21396	62.63	13.125	1.15015	2.050	.457		
ROUTE	POND6ROUTE48	12	22	.21396	62.58	13.125	1.15015	2.050	.457		
COMPUTE NM HYD	P-5	-	3	.00290	2.51	.074	.47912	1.500	1.353	PER IMP=	2.00
ADD HYD	AP-18	3&32	10	.26964	155.57	7.976	.55462	1.650	.901		
ROUTE RESERVOIR	AP-18R	10	12	.26964	113.38	7.976	.55462	1.800	.657	AC-FT=	2.044
ROUTE	POND-5ROUTE4	12	32	.26964	119.58	7.976	.55462	1.800	.693		
ADD HYD	POND-5R&6R	32&22	20	.48360	174.55	21.101	.81811	1.800	.564		
COMPUTE NM HYD	AP-19&C-1D	-	3	.01150	24.41	1.024	1.67006	1.500	3.317	PER IMP=	57.00
ROUTE	AP-19R	3	2	.01150	23.29	1.024	1.67010	1.550	3.164		
COMPUTE NM HYD	C-2D	-	4	.05740	118.78	5.113	1.67006	1.500	3.233	PER IMP=	57.00
ADD HYD	AP-20	2& 4	10	.06890	141.50	6.137	1.67005	1.500	3.209		
COMPUTE NM HYD	P-4	-	3	.00360	3.11	.092	.47912	1.500	1.352	PER IMP=	2.00
ADD HYD	P-4+POND-5R&	20& 3	20	.48720	175.34	21.192	.81559	1.800	.562		
ADD HYD	AP-20A	10&20	30	.55610	237.86	27.329	.92145	1.700	.668		
ROUTE RESERVOIR	AP-20AR	30	2	.55610	115.67	27.248	.91871	2.200	.325	AC-FT=	7.165
ROUTE	POND-4R	2	12	.55610	115.65	27.248	.91871	2.200	.325		
COMPUTE NM HYD	P-3	-	3	.00270	2.34	.069	.47912	1.500	1.354	PER IMP=	2.00
ADD HYD	P-3+POND-4R	12& 3	10	.55880	115.79	27.316	.91656	2.200	.324		
COMPUTE NM HYD	AP-21&D-1D	-	3	.04680	99.31	4.168	1.67006	1.500	3.316	PER IMP=	57.00
ADD HYD	AP-21A	10& 3	10	.60560	138.09	31.484	.97479	1.500	.356		

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 3 NOTATION	
ROUTE RESERVOIR	AP-21AR	10	2	.60560	108.88	31.415	.97265	2.700	.281	AC-FT=	4.159
COMPUTE NM HYD	AP-22&E-1D	-	3	.02820	64.16	2.883	1.91659	1.500	3.555	PER IMP=	71.00
ROUTE	AP-22R	3	12	.02820	61.84	2.883	1.91660	1.550	3.427		
COMPUTE NM HYD	E-2D	-	3	.01720	44.80	2.065	2.25115	1.500	4.070	PER IMP=	90.00
ADD HYD	AP-23	3&12	10	.04540	106.21	4.948	2.04333	1.500	3.655		
ROUTE	AP-23R	10	12	.04540	104.90	4.948	2.04334	1.550	3.610		
COMPUTE NM HYD	E-3D	-	3	.04250	90.19	3.785	1.67006	1.500	3.316	PER IMP=	57.00
ADD HYD	AP-24	12& 3	10	.08790	191.75	8.733	1.86285	1.500	3.408		
ROUTE	AP-23R	10	12	.08790	190.60	8.733	1.86286	1.550	3.388		
COMPUTE NM HYD	E-4D	-	3	.00300	7.83	.360	2.25115	1.500	4.076	PER IMP=	90.00
ADD HYD	AP-25	12& 3	10	.09090	197.44	9.093	1.87566	1.550	3.394		
ADD HYD	AP-25+POND-3	2&10	10	.69650	231.02	40.509	1.09050	1.550	.518		
COMPUTE NM HYD	P-2	-	3	.00360	3.11	.092	.47912	1.500	1.352	PER IMP=	2.00
ADD HYD	AP-25A	3&10	10	.70010	234.02	40.600	1.08735	1.550	.522		
ROUTE RESERVOIR	AP-25AR	10	2	.70010	115.88	39.549	1.05921	2.500	.259	AC-FT=	9.274
ROUTE	POND-2R	2	12	.70010	115.91	39.549	1.05921	2.550	.259		
COMPUTE NM HYD	AP-26&F-1D	-	3	.03980	82.37	3.545	1.67006	1.500	3.234	PER IMP=	57.00
COMPUTE NM HYD	P-1	-	4	.00290	2.51	.074	.47912	1.500	1.353	PER IMP=	2.00
ADD HYD	P-1+F-1D	3& 4	10	.04270	84.88	3.618	1.58885	1.500	3.106		
ADD HYD	AP-26A	10&12	10	.74280	118.94	43.168	1.08965	2.450	.250		
ROUTE RESERVOIR	AP-26AR	10	2	.74280	93.73	43.114	1.08830	3.800	.197	AC-FT=	7.046
COMPUTE NM HYD	AP-27&G-1D	-	3	.02290	56.30	2.534	2.07506	1.500	3.841	PER IMP=	80.00
COMPUTE NM HYD	AP-28&G-2D	-	4	.00310	7.72	.349	2.11028	1.500	3.893	PER IMP=	82.00
ROUTE	AP-28R	4	12	.00310	7.56	.349	2.11044	1.500	3.808		
COMPUTE NM HYD	AP-29&G-3D	-	5	.03510	77.66	3.885	2.07506	1.500	3.457	PER IMP=	80.00
COMPUTE NM HYD	G-4D	-	6	.00330	8.10	.359	2.03985	1.500	3.837	PER IMP=	78.00
ADD HYD	AP-30	12& 6	10	.00640	15.66	.708	2.07389	1.500	3.823		

FINISH

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994 RUN DATE (MON/DAY/YR) =01/25/2001
INPUT FILE = DEV10EE.IN USER NO.= C_ANDRSN.I01

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION	
*S DEVELOPED CONDITIONS WITH I-40 INTERCEPTOR IN PLACE											
*S 10 YEAR, 24 HOUR STORM EVENT											
*S											
*S FILE NAME: DEV10EE.IN											
*S											
*S THIS MODEL ANALYZES THE UNSER DIVERSION PONDS WITH THE EXISTING POND											
*S OUTLET WORKS AND THE EXISTING POND GRADING.											
*S											
*S MASTER DRAINAGE PLAN FOR THE WEST SIDE TRANSIT FACILITY											
*S											
*S THIS AHYMO FILE ANALYZES THE UNSER DIVERSION (DETENTION PONDS) ASSUMING											
*S FULLY DEVELOPED CONDITIONS WITH THE I-40 INTERCEPTOR IN PLACE.											
*S NOTES: UNSER DIVERSION POND NO. 5 IS NOT USED IN FULLY DEVELOPED											
*S CONDITION (PER CHUCK EASTERING'S REPORT).											
*S THE OUTLET RATING CURVES WERE ALSO BASED UPON TAILWATER											
*S ELEVATIONS SHOWN ON THE UNSER DIVERSION (AS-BUILT)											
*S CONSTRUCTION PLAN SET (PLANS BY CHUCK EASTERLING)											
*S DATE: 11/7/00											
START										TIME=	.00
RAINFALL TYPE= 2										RAIN24=	1.770
*S FOR WATERSHED NORTH OF INTERSTATE 40											
*S ALL FLOWS TO BE PICKED UP BY I-40 INTERCEPTOR											
*S BEGIN WATERSHED SOUTH OF INTERSTATE 40											
COMPUTE NM HYD	I40-1	-	3	.00750	5.52	.237	.59326	1.500	1.150	PER IMP=	35.00
ROUTE	I40-1R	3	2	.00750	5.30	.237	.59333	1.550	1.104		
COMPUTE NM HYD	I40-2	-	3	.00310	2.28	.098	.59326	1.500	1.152	PER IMP=	35.00
ROUTE	I40-2R	3	12	.00310	2.24	.098	.59339	1.500	1.129		
ADD HYD	I401R&I402R	2&12	10	.01060	7.47	.335	.59322	1.550	1.102		
COMPUTE NM HYD	A-1D	-	3	.02570	36.06	1.551	1.13124	1.500	2.192	PER IMP=	69.00
ADD HYD	AP-1	10& 3	10	.03630	43.48	1.886	.97412	1.500	1.872		
ROUTE	AP-1R	10	2	.03630	42.61	1.886	.97414	1.500	1.834		
COMPUTE NM HYD	I40-3	-	3	.00190	1.40	.060	.59326	1.500	1.154	PER IMP=	35.00
COMPUTE NM HYD	I40-4	-	4	.00120	.89	.038	.59326	1.500	1.156	PER IMP=	35.00
ADD HYD	I40-3&I40-4	3& 4	10	.00310	2.29	.098	.59310	1.500	1.155		
ROUTE	I403R&I404R	10	12	.00310	2.09	.098	.59338	1.550	1.051		
ADD HYD	PARTAP-2	2&12	10	.03940	44.43	1.984	.94415	1.500	1.762		
COMPUTE NM HYD	A-2D	-	3	.02840	45.59	2.013	1.32889	1.500	2.508	PER IMP=	84.00
ADD HYD	AP-2	10& 3	10	.06780	90.02	3.997	1.10531	1.500	2.075		
ROUTE	AP-2R	10	2	.06780	87.57	3.997	1.10532	1.500	2.018		
COMPUTE NM HYD	A-3D	-	3	.02400	39.69	1.802	1.40796	1.500	2.584	PER IMP=	90.00
ADD HYD	AP-3	2& 3	10	.09180	127.26	5.799	1.18443	1.500	2.166		
COMPUTE NM HYD	AP-4&A-4D	-	3	.01250	18.04	.801	1.20080	1.500	2.256	PER IMP=	76.00
ADD HYD	AP-3+AP-4	10& 3	10	.10430	145.30	6.599	1.18639	1.500	2.177		
COMPUTE NM HYD	P-6	-	3	.00430	.81	.026	.11274	1.550	.293	PER IMP=	2.00
ADD HYD	AP-5	10& 3	10	.10860	146.10	6.625	1.14374	1.500	2.102		
ROUTE RESERVOIR	POND-6R	10	2	.10860	10.76	6.625	1.14374	2.250	.155	AC-FT=	4.257
ROUTE	POND6ROUTE36	2	12	.10860	10.76	6.625	1.14374	2.250	.155		
ROUTE	POND6ROUTE48	12	22	.10860	10.76	6.625	1.14374	2.250	.155		
COMPUTE NM HYD	AP-6&C-1D	-	3	.01150	14.28	.597	.97312	1.500	1.940	PER IMP=	57.00

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION	
ROUTE	AP-6R	3	2	.01150	13.60	.597	.97316	1.550	1.847		
COMPUTE NM HYD	C-2D	-	3	.05740	69.47	2.979	.97312	1.500	1.891	PER IMP=	57.00
ADD HYD	AP-7	2& 3	10	.06890	82.45	3.576	.97311	1.500	1.870		
ADD HYD	PARTAP-8	10&22	20	.17750	88.81	10.200	1.07750	1.500	.782		
COMPUTE NM HYD	P-4	-	3	.00360	.68	.022	.11274	1.550	.293	PER IMP=	2.00
ADD HYD	AP-8	20& 3	10	.18110	89.47	10.221	1.05823	1.500	.772		
ROUTE RESERVOIR	POND-4	10	2	.18110	9.75	10.134	1.04922	4.000	.084	AC-FT=	3.295
ROUTE	POND-4R	2	12	.18110	9.75	10.134	1.04922	4.000	.084		
COMPUTE NM HYD	P-3	-	3	.00270	.51	.016	.11274	1.550	.294	PER IMP=	2.00
ADD HYD	P-3+POND-4R	12& 3	10	.18380	9.75	10.149	1.03536	4.000	.083		
COMPUTE NM HYD	AP-9&D-1D	-	3	.04680	58.08	2.429	.97312	1.500	1.939	PER IMP=	57.00
ADD HYD	AP-9A	10& 3	10	.23060	62.65	12.578	1.02273	1.500	.424		
ROUTE RESERVOIR	POND-3	10	2	.23060	9.88	12.383	1.00688	4.200	.067	AC-FT=	1.991
COMPUTE NM HYD	AP-10&E-1D	-	3	.02820	39.44	1.741	1.15760	1.500	2.185	PER IMP=	71.00
ROUTE	AP-10R	3	12	.02820	38.12	1.741	1.15761	1.550	2.112		
COMPUTE NM HYD	E-2D	-	3	.01720	29.01	1.292	1.40796	1.500	2.635	PER IMP=	90.00
ADD HYD	AP-11	3&12	10	.04540	66.46	3.033	1.25243	1.500	2.287		
ROUTE	AP-11R	10	12	.04540	66.02	3.033	1.25245	1.550	2.272		
COMPUTE NM HYD	E-3D	-	3	.04250	52.74	2.206	.97312	1.500	1.939	PER IMP=	57.00
ADD HYD	AP-12	12& 3	10	.08790	115.92	5.238	1.11738	1.500	2.060		
ROUTE	AP-11R	10	12	.08790	116.53	5.238	1.11739	1.550	2.071		
COMPUTE NM HYD	E-4D	-	3	.00300	5.07	.225	1.40796	1.500	2.638	PER IMP=	90.00
ADD HYD	AP-13	12& 3	10	.09090	120.95	5.464	1.12697	1.550	2.079		
ADD HYD	AP-13+POND-3	2&10	10	.32150	124.65	17.847	1.04083	1.550	.606		
COMPUTE NM HYD	P-2	-	3	.00360	1.80	.048	.24840	1.500	.783	PER IMP=	2.00
ADD HYD	AP-13A	3&10	10	.32510	126.37	17.894	1.03200	1.550	.607		
ROUTE RESERVOIR	POND-2	10	2	.32510	9.18	16.801	.96898	9.900	.044	AC-FT=	4.894
ROUTE	POND-2R	2	12	.32510	9.18	16.799	.96890	9.900	.044		

COMPUTE NM HYD	AP-14&F-1D	-	3	.03980	48.17	2.066	.97312	1.500	1.891	PER IMP=	57.00
COMPUTE NM HYD	P-1	-	4	.00290	.55	.017	.11274	1.550	.294	PER IMP=	2.00
ADD HYD	P-1+F-1D	3& 4	10	.04270	48.71	2.082	.91423	1.500	1.783		
ADD HYD	AP-14A	10&12	10	.36780	49.21	18.881	.96255	1.500	.209		
ROUTE RESERVOIR	POND-1	10	2	.36780	9.13	18.707	.95367	14.750	.039	AC-FT=	2.184
COMPUTE NM HYD	AP-15&G-1D	-	3	.02290	35.53	1.559	1.27619	1.500	2.424	PER IMP=	80.00
COMPUTE NM HYD	AP-16&G-2D	-	4	.00310	4.90	.215	1.30254	1.500	2.469	PER IMP=	82.00
ROUTE	AP-16R	4	12	.00310	4.76	.215	1.30270	1.500	2.398		
COMPUTE NM HYD	AP-17&G-3D	-	5	.03510	48.94	2.389	1.27619	1.500	2.178	PER IMP=	80.00
COMPUTE NM HYD	G-4D	-	6	.00330	5.08	.220	1.24983	1.500	2.407	PER IMP=	78.00
ADD HYD	AP-18	12& 6	10	.00640	9.84	.435	1.27528	1.500	2.403		
FINISH											

```
AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994          RUN DATE (MON/DAY/YR) =01/25/2001
INPUT FILE = DEV100EE.IN                                                          . USER NO.= C_ANDRSN.I01

                                FROM  TO                  PEAK        RUNOFF                 TIME TO   CFS       PAGE =   1
                     HYDROGRAPH  ID   ID       AREA      DISCHARGE    VOLUME      RUNOFF      PEAK     PER
COMMAND       IDENTIFICATION     NO.  NO.    (SQ MI)      (CFS)      (AC-FT)    (INCHES)    (HOURS)  ACRE     NOTATION

*S DEVELOPED CONDITIONS WITH I-40 INTERCEPTOR IN PLACE
*S 100 YEAR, 24 HOUR STORM EVENT
*S
*S FILE NAME: DEV100EE.IN
*S
*S THIS MODEL ANALYZES THE UNSER DIVERSION PONDS WITH THE EXISTING POND
*S OUTLET WORKS AND THE EXISTING POND GRADING.
*S
*S MASTER DRAINAGE PLAN FOR THE WEST SIDE TRANSIT FACILITY
*S
*S THIS AHYMO FILE ANALYZES THE UNSER DIVERSION (DETENTION PONDS) ASSUMING
*S FULLY DEVELOPED CONDITIONS WITH THE I-40 INTERCEPTOR IN PLACE.
*S NOTES: UNSER DIVERSION POND NO. 5 IS NOT USED IN FULLY DEVELOPED
*S        CONDITION (PER CHUCK EASTERING'S REPORT).
*S        THE OUTLET RATING CURVES WERE ALSO BASED UPON TAILWATER
*S        ELEVATIONS SHOWN ON THE UNSER DIVERSION (AS-BUILT)
*S        CONSTRUCTION PLAN SET (PLANS BY CHUCK EASTERLING)
*S DATE: 11/7/00
START                                                                                                    TIME=      .00
RAINFALL  TYPE= 2                                                                                        RAIN24=   2.660
*S FOR WATERSHED NORTH OF INTERSTATE 40
*S ALL FLOWS TO BE PICKED UP BY I-40 INTERCEPTOR
*S BEGIN WATERSHED SOUTH OF INTERSTATE 40
COMPUTE NM HYD          I40-1   -    3        .00750       11.24        .454      1.13512    1.500    2.342 PER IMP=  35.00
ROUTE                  I40-1R   3    2        .00750       10.83        .454      1.13518    1.550    2.256
COMPUTE NM HYD          I40-2   -    3        .00310        4.65        .188      1.13512    1.500    2.346 PER IMP=  35.00
ROUTE                  I40-2R   3   12        .00310        4.61        .188      1.13529    1.500    2.325
ADD HYD          I401R&I402R  2&12   10        .01060       15.34        .642      1.13508    1.500    2.261
COMPUTE NM HYD          A-1D   -    3        .02570       59.05       2.579      1.88137    1.500    3.590 PER IMP=  69.00
ADD HYD                AP-1 10& 3   10        .03630       74.38       3.220      1.66343    1.500    3.202
ROUTE                  AP-1R   10    2        .03630       73.06       3.220      1.66346    1.500    3.145
COMPUTE NM HYD          I40-3   -    3        .00190        2.86        .115      1.13512    1.500    2.350 PER IMP=  35.00
COMPUTE NM HYD          I40-4   -    4        .00120        1.81        .073      1.13512    1.500    2.357 PER IMP=  35.00
ADD HYD          I40-3&I40-4  3& 4   10        .00310        4.67        .188      1.13493    1.500    2.353
ROUTE            I403R&I404R   10   12        .00310        4.38        .188      1.13522    1.550    2.208
ADD HYD              PARTAP-2  2&12   10        .03940       76.99       3.408      1.62186    1.500    3.053
COMPUTE NM HYD          A-2D   -    3        .02840       71.47       3.250      2.14550    1.500    3.932 PER IMP=  84.00
ADD HYD                AP-2 10& 3   10        .06780      148.46       6.658      1.84120    1.500    3.421
ROUTE                  AP-2R   10    2        .06780      144.80       6.658      1.84122    1.500    3.337
COMPUTE NM HYD          A-3D   -    3        .02400       61.30       2.881      2.25115    1.500    3.991 PER IMP=  90.00
ADD HYD                AP-3  2& 3   10        .09180      206.10       9.539      1.94838    1.500    3.508
COMPUTE NM HYD      AP-4&A-4D   -    3        .01250       29.07       1.312      1.96827    1.500    3.634 PER IMP=  76.00
ADD HYD          AP-3+AP-4 10& 3   10        .10430      235.17      10.851      1.95076    1.500    3.523
COMPUTE NM HYD          P-6   -    3        .00430        3.72        .110       .47912    1.500    1.350 PER IMP=   2.00
ADD HYD                AP-5 10& 3   10        .10860      238.89      10.961      1.89248    1.500    3.437
ROUTE RESERVOIR      POND-6R   10    2        .10860       59.90      10.961      1.89248    2.000     .862 AC-FT=    5.918
ROUTE            POND6ROUTE36    2   12        .10860       60.01      10.961      1.89248    2.000     .863
ROUTE            POND6ROUTE48   12   22        .10860       59.78      10.961      1.89248    1.950     .860
COMPUTE NM HYD      AP-6&C-1D   -    3        .01150       24.41       1.024      1.67006    1.500    3.317 PER IMP=  57.00
□
                                FROM  TO                  PEAK        RUNOFF                 TIME TO   CFS       PAGE =   2
                     HYDROGRAPH  ID   ID       AREA      DISCHARGE    VOLUME      RUNOFF      PEAK     PER
COMMAND       IDENTIFICATION     NO.  NO.    (SQ MI)      (CFS)      (AC-FT)    (INCHES)    (HOURS)  ACRE     NOTATION

ROUTE                  AP-6R    3    2        .01150       23.38       1.024      1.67010    1.550    3.177
COMPUTE NM HYD          C-2D   -    3        .05740      118.78       5.113      1.67006    1.500    3.233 PER IMP=  57.00
ADD HYD                 AP-7  2& 3   10        .06890      141.62       6.137      1.67005    1.500    3.212
ADD HYD              PARTAP-8 10&22   20        .17750      149.74      17.098      1.80614    1.500    1.318
COMPUTE NM HYD          P-4   -    3        .00360        3.11        .092       .47912    1.500    1.352 PER IMP=   2.00
ADD HYD                AP-8 20& 3   10        .18110      152.86      17.190      1.77971    1.500    1.319
ROUTE RESERVOIR      POND-4   10    2        .18110       81.98      17.099      1.77033    2.100     .707 AC-FT=    4.377
ROUTE                POND-4R    2   12        .18110       82.19      17.099      1.77032    2.100     .709
COMPUTE NM HYD          P-3   -    3        .00270        2.34        .069       .47912    1.500    1.354 PER IMP=   2.00
ADD HYD          P-3+POND-4R 12& 3   10        .18380       82.38      17.167      1.75127    2.100     .700
COMPUTE NM HYD      AP-9&D-1D   -    3        .04680       99.31       4.168      1.67006    1.500    3.316 PER IMP=  57.00
ADD HYD                AP-9A 10& 3   10        .23060      107.39      21.336      1.73478    1.500     .728
ROUTE RESERVOIR      POND-3   10    2        .23060       88.48      21.088      1.71463    2.150     .600 AC-FT=    2.796
COMPUTE NM HYD     AP-10&E-1D   -    3        .02820       64.16       2.883      1.91659    1.500    3.555 PER IMP=  71.00
ROUTE                 AP-10R    3   12        .02820       61.84       2.883      1.91660    1.550    3.427
COMPUTE NM HYD          E-2D   -    3        .01720       44.80       2.065      2.25115    1.500    4.070 PER IMP=  90.00
ADD HYD                AP-11  3&12   10        .04540      106.21       4.948      2.04333    1.500    3.655
ROUTE                  AP-11R   10   12        .04540      104.90       4.948      2.04334    1.550    3.610
COMPUTE NM HYD          E-3D   -    3        .04250       90.19       3.785      1.67006    1.500    3.316 PER IMP=  57.00
ADD HYD                AP-12 12& 3   10        .08790      191.75       8.733      1.86285    1.500    3.408
ROUTE                  AP-11R   10   12        .08790      190.60       8.733      1.86286    1.550    3.388
COMPUTE NM HYD          E-4D   -    3        .00300        7.83        .360      2.25115    1.500    4.076 PER IMP=  90.00
ADD HYD                AP-13 12& 3   10        .09090      197.44       9.093      1.87566    1.550    3.394
ADD HYD          AP-13+POND-3  2&10   10        .32150      203.89      30.181      1.76016    1.550     .991
COMPUTE NM HYD          P-2   -    3        .00360        4.76        .135       .70157    1.500    2.066 PER IMP=   2.00
ADD HYD                AP-13A  3&10   10        .32510      208.38      30.315      1.74841    1.550    1.002
ROUTE RESERVOIR      POND-2   10    2        .32510      126.20      28.794      1.66070    2.100     .607 AC-FT=    7.153
ROUTE                POND-2R    2   12        .32510      129.46      28.792      1.66057    2.100     .622
```

COMPUTE NM HYD	AP-14&F-1D	-	3	.03980	82.37	3.545	1.67006	1.500	3.234	PER IMP=	57.00
COMPUTE NM HYD	P-1	-	4	.00290	2.51	.074	.47912	1.500	1.353	PER IMP=	2.00
ADD HYD	P-1+F-1D	3& 4	10	.04270	84.88	3.618	1.58885	1.500	3.106		
ADD HYD	AP-14A	10&12	10	.36780	142.57	32.410	1.65225	2.100	.606		
ROUTE RESERVOIR	POND-1	10	2	.36780	93.38	31.228	1.59195	2.450	.397	AC-FT=	5.776
COMPUTE NM HYD	AP-15&G-1D	-	3	.02290	56.30	2.534	2.07506	1.500	3.841	PER IMP=	80.00
COMPUTE NM HYD	AP-16&G-2D	-	4	.00310	7.72	.349	2.11028	1.500	3.893	PER IMP=	82.00
ROUTE	AP-16R	4	12	.00310	7.56	.349	2.11044	1.500	3.808		
COMPUTE NM HYD	AP-17&G-3D	-	5	.03510	77.66	3.885	2.07506	1.500	3.457	PER IMP=	80.00
COMPUTE NM HYD	G-4D	-	6	.00330	8.10	.359	2.03985	1.500	3.837	PER IMP=	78.00
ADD HYD	AP-18	12& 6	10	.00640	15.66	.708	2.07389	1.500	3.823		
FINISH											

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994 RUN DATE (MON/DAY/YR) =01/19/2001
INPUT FILE = DEV10PE.IN USER NO.= C_ANDRSN.I01

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 1 NOTATION
*S DEVELOPED CONDITIONS WITH I-40 INTERCEPTOR IN PLACE										
*S 10 YEAR, 24 HOUR STORM EVENT										
*S										
*S FILE NAME: DEV10PE.IN										
*S THIS MODEL ANALYZES THE UNSER DIVERSION PONDS WITH THE PROPOSED POND										
*S OUTLET WORKS AND THE EXISTING POND GRADING.										
*S MASTER DRAINAGE PLAN FOR THE WEST SIDE TRANSIT FACILITY										
*S THIS AHYMO FILE ANALYZES THE UNSER DIVERSION (DETENTION PONDS) ASSUMING										
*S FULLY DEVELOPED CONDITIONS WITH THE I-40 INTERCEPTOR IN PLACE.										
*S NOTES: UNSER DIVERSION POND NO. 5 IS NOT USED IN FULLY DEVELOPED										
*S CONDITION (PER CHUCK EASTERING'S REPORT).										
*S THE OUTLET WORKS FOR EACH UNSER DIVERSION POND HAS BEEN MODIFIED										
*S BY PLACING ADDITIONAL 10" OPENINGS AROUND EACH STRUCTURE (PER										
*S CHUCK EASTERING'S REPORT). OUTLET RATING CURVES WERE ALSO BASED										
*S UPON TAILWATER ELEVATIONS SHOWN ON THE UNSER DIVERSION (AS-BUILT)										
*S CONSTRUCTION PLAN SET (PLANS BY CHUCK EASTERLING)										
*S DATE: 11/7/00										
START										TIME= .00
RAINFALL TYPE= 2										RAIN24= 1.770
*S FOR WATERSHED NORTH OF INTERSTATE 40										
*S ALL FLOWS TO BE PICKED UP BY I-40 INTERCEPTOR										
*S BEGIN WATERSHED SOUTH OF INTERSTATE 40										
COMPUTE NM HYD	I40-1	-	3	.00750	5.52	.237	.59326	1.500	1.150	PER IMP= 35.00
ROUTE	I40-1R	3	2	.00750	5.30	.237	.59333	1.550	1.104	
COMPUTE NM HYD	I40-2	-	3	.00310	2.28	.098	.59326	1.500	1.152	PER IMP= 35.00
ROUTE	I40-2R	3	12	.00310	2.24	.098	.59339	1.500	1.129	
ADD HYD	I401R&I402R	2&12	10	.01060	7.47	.335	.59322	1.550	1.102	
COMPUTE NM HYD	A-1D	-	3	.02570	36.06	1.551	1.13124	1.500	2.192	PER IMP= 69.00
ADD HYD	AP-1	10& 3	10	.03630	43.48	1.886	.97412	1.500	1.872	
ROUTE	AP-1R	10	2	.03630	42.61	1.886	.97414	1.500	1.834	
COMPUTE NM HYD	I40-3	-	3	.00190	1.40	.060	.59326	1.500	1.154	PER IMP= 35.00
COMPUTE NM HYD	I40-4	-	4	.00120	.89	.038	.59326	1.500	1.156	PER IMP= 35.00
ADD HYD	I40-3&I40-4	3& 4	10	.00310	2.29	.098	.59310	1.500	1.155	
ROUTE	I403R&I404R	10	12	.00310	2.09	.098	.59338	1.550	1.051	
ADD HYD	PARTAP-2	2&12	10	.03940	44.43	1.984	.94415	1.500	1.762	
COMPUTE NM HYD	A-2D	-	3	.02840	45.59	2.013	1.32889	1.500	2.508	PER IMP= 84.00
ADD HYD	AP-2	10& 3	10	.06780	90.02	3.997	1.10531	1.500	2.075	
ROUTE	AP-2R	10	2	.06780	87.57	3.997	1.10532	1.500	2.018	
COMPUTE NM HYD	A-3D	-	3	.02400	39.69	1.802	1.40796	1.500	2.584	PER IMP= 90.00
ADD HYD	AP-3	2& 3	10	.09180	127.26	5.799	1.18443	1.500	2.166	
COMPUTE NM HYD	AP-4&A-4D	-	3	.01250	18.04	.801	1.20080	1.500	2.256	PER IMP= 76.00
ADD HYD	AP-3+AP-4	10& 3	10	.10430	145.30	6.599	1.18639	1.500	2.177	
COMPUTE NM HYD	P-6	-	3	.00430	.81	.026	.11274	1.550	.293	PER IMP= 2.00
ADD HYD	AP-5	10& 3	10	.10860	146.10	6.625	1.14374	1.500	2.102	
ROUTE RESERVOIR	POND-6R	10	2	.10860	43.69	6.625	1.14374	1.900	.629	AC-FT= 2.684
ROUTE	POND6ROUTE36	2	12	.10860	43.69	6.625	1.14374	1.900	.629	
ROUTE	POND6ROUTE48	12	22	.10860	43.68	6.625	1.14374	1.950	.628	
COMPUTE NM HYD	AP-6&C-1D	-	3	.01150	14.28	.597	.97312	1.500	1.940	PER IMP= 57.00
ROUTE	AP-6R	3	2	.01150	13.60	.597	.97316	1.550	1.847	

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE = 2 NOTATION
COMPUTE NM HYD	C-2D	-	3	.05740	69.47	2.979	.97312	1.500	1.891	PER IMP= 57.00
ADD HYD	AP-7	2& 3	10	.06890	82.45	3.576	.97311	1.500	1.870	
ADD HYD	PARTAP-8	10&22	20	.17750	105.83	10.200	1.07750	1.550	.932	
COMPUTE NM HYD	P-4	-	3	.00360	.68	.022	.11274	1.550	.293	PER IMP= 2.00
ADD HYD	AP-8	20& 3	10	.18110	106.51	10.221	1.05823	1.550	.919	
ROUTE RESERVOIR	POND-4	10	2	.18110	50.45	10.142	1.05000	2.150	.435	AC-FT= 2.433
ROUTE	POND-4R	2	12	.18110	50.45	10.142	1.05000	2.150	.435	
COMPUTE NM HYD	P-3	-	3	.00270	.51	.016	.11274	1.550	.294	PER IMP= 2.00
ADD HYD	P-3+POND-4R	12& 3	10	.18380	50.49	10.157	1.03614	2.150	.429	
COMPUTE NM HYD	AP-9&D-1D	-	3	.04680	58.08	2.429	.97312	1.500	1.939	PER IMP= 57.00
ADD HYD	AP-9A	10& 3	10	.23060	82.75	12.586	1.02335	1.550	.561	
ROUTE RESERVOIR	POND-3	10	2	.23060	57.95	12.517	1.01775	2.100	.393	AC-FT= 2.158
COMPUTE NM HYD	AP-10&E-1D	-	3	.02820	39.44	1.741	1.15760	1.500	2.185	PER IMP= 71.00
ROUTE	AP-10R	3	12	.02820	38.12	1.741	1.15761	1.550	2.112	
COMPUTE NM HYD	E-2D	-	3	.01720	29.01	1.292	1.40796	1.500	2.635	PER IMP= 90.00
ADD HYD	AP-11	3&12	10	.04540	66.46	3.033	1.25243	1.500	2.287	
ROUTE	AP-11R	10	12	.04540	66.02	3.033	1.25245	1.550	2.272	
COMPUTE NM HYD	E-3D	-	3	.04250	52.74	2.206	.97312	1.500	1.939	PER IMP= 57.00
ADD HYD	AP-12	12& 3	10	.08790	115.92	5.238	1.11738	1.500	2.060	
ROUTE	AP-11R	10	12	.08790	116.53	5.238	1.11739	1.550	2.071	
COMPUTE NM HYD	E-4D	-	3	.00300	5.07	.225	1.40796	1.500	2.638	PER IMP= 90.00
ADD HYD	AP-13	12& 3	10	.09090	120.95	5.464	1.12697	1.550	2.079	
ADD HYD	AP-13+POND-3	2&10	10	.32150	142.03	17.981	1.04863	1.550	.690	
COMPUTE NM HYD	P-2	-	3	.00360	1.80	.048	.24840	1.500	.783	PER IMP= 2.00
ADD HYD	AP-13A	3&10	10	.32510	143.76	18.027	1.03972	1.550	.691	
ROUTE RESERVOIR	POND-2	10	2	.32510	45.98	17.295	.99747	2.900	.221	AC-FT= 5.148
ROUTE	POND-2R	2	12	.32510	45.98	17.295	.99747	2.900	.221	
COMPUTE NM HYD	AP-14&F-1D	-	3	.03980	48.17	2.066	.97312	1.500	1.891	PER IMP= 57.00

COMPUTE NM HYD	P-1	-	4	.00290	.55	.017	.11274	1.550	.294	PER IMP=	2.00
ADD HYD	P-1+F-1D	3& 4	10	.04270	48.71	2.082	.91423	1.500	1.783		
ADD HYD	AP-14A	10&12	10	.36780	58.47	19.377	.98781	1.550	.248		
ROUTE RESERVOIR	POND-1	10	2	.36780	46.28	19.376	.98777	3.000	.197	AC-FT=	1.510
COMPUTE NM HYD	AP-15&G-1D	-	3	.02290	35.53	1.559	1.27619	1.500	2.424	PER IMP=	80.00
COMPUTE NM HYD	AP-16&G-2D	-	4	.00310	4.90	.215	1.30254	1.500	2.469	PER IMP=	82.00
ROUTE	AP-16R	4	12	.00310	4.76	.215	1.30270	1.500	2.398		
COMPUTE NM HYD	AP-17&G-3D	-	5	.03510	48.94	2.389	1.27619	1.500	2.178	PER IMP=	80.00
COMPUTE NM HYD	G-4D	-	6	.00330	5.08	.220	1.24983	1.500	2.407	PER IMP=	78.00
ADD HYD	AP-18	12& 6	10	.00640	9.84	.435	1.27528	1.500	2.403		
FINISH											

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994 RUN DATE (MON/DAY/YR) =01/25/2001
INPUT FILE = DEV100PE.IN USER NO.= C_ANDRSN.I01

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION
										PAGE = 1
*S DEVELOPED CONDITIONS WITH I-40 INTERCEPTOR IN PLACE										
*S 100 YEAR, 24 HOUR STORM EVENT										
*S										
*S FILE NAME: DEV100PE.IN										
*S THIS MODEL ANALYZES THE UNSER DIVERSION PONDS WITH THE PROPOSED POND										
*S OUTLET WORKS AND THE EXISTING POND GRADING.										
*S MASTER DRAINAGE PLAN FOR THE WEST SIDE TRANSIT FACILITY										
*S THIS AHYMO FILE ANALYZES THE UNSER DIVERSION (DETENTION PONDS) ASSUMING										
*S FULLY DEVELOPED CONDITIONS WITH THE I-40 INTERCEPTOR IN PLACE.										
*S NOTES: UNSER DIVERSION POND NO. 5 IS NOT USED IN FULLY DEVELOPED										
*S CONDITION (PER CHUCK EASTERING'S REPORT).										
*S THE OUTLET WORKS FOR EACH UNSER DIVERSION POND HAS BEEN MODIFIED										
*S BY PLACING ADDITIONAL 10" OPENINGS AROUND EACH STRUCTURE (PER										
*S CHUCK EASTERING'S REPORT). OUTLET RATING CURVES WERE ALSO BASED										
*S UPON TAILWATER ELEVATIONS SHOWN ON THE UNSER DIVERSION (AS-BUILT)										
*S CONSTRUCTION PLAN SET (PLANS BY CHUCK EASTERLING)										
*S DATE: 11/7/00										
START										TIME= .00
RAINFALL TYPE= 2										RAIN24= 2.660
*S FOR WATERSHED NORTH OF INTERSTATE 40										
*S ALL FLOWS TO BE PICKED UP BY I-40 INTERCEPTOR										
*S BEGIN WATERSHED SOUTH OF INTERSTATE 40										
COMPUTE NM HYD	I40-1	-	3	.00750	11.24	.454	1.13512	1.500	2.342	PER IMP= 35.00
ROUTE	I40-1R	3	2	.00750	10.83	.454	1.13518	1.550	2.256	
COMPUTE NM HYD	I40-2	-	3	.00310	4.65	.188	1.13512	1.500	2.346	PER IMP= 35.00
ROUTE	I40-2R	3	12	.00310	4.61	.188	1.13529	1.500	2.325	
ADD HYD	I401R&I402R	2&12	10	.01060	15.34	.642	1.13508	1.500	2.261	
COMPUTE NM HYD	A-1D	-	3	.02570	59.05	2.579	1.88137	1.500	3.590	PER IMP= 69.00
ADD HYD	AP-1	10& 3	10	.03630	74.38	3.220	1.66343	1.500	3.202	
ROUTE	AP-1R	10	2	.03630	73.06	3.220	1.66346	1.500	3.145	
COMPUTE NM HYD	I40-3	-	3	.00190	2.86	.115	1.13512	1.500	2.350	PER IMP= 35.00
COMPUTE NM HYD	I40-4	-	4	.00120	1.81	.073	1.13512	1.500	2.357	PER IMP= 35.00
ADD HYD	I40-3&I40-4	3& 4	10	.00310	4.67	.188	1.13493	1.500	2.353	
ROUTE	I403R&I404R	10	12	.00310	4.38	.188	1.13522	1.550	2.208	
ADD HYD	PARTAP-2	2&12	10	.03940	76.99	3.408	1.62186	1.500	3.053	
COMPUTE NM HYD	A-2D	-	3	.02840	71.47	3.250	2.14550	1.500	3.932	PER IMP= 84.00
ADD HYD	AP-2	10& 3	10	.06780	148.46	6.658	1.84120	1.500	3.421	
ROUTE	AP-2R	10	2	.06780	144.80	6.658	1.84122	1.500	3.337	
COMPUTE NM HYD	A-3D	-	3	.02400	61.30	2.881	2.25115	1.500	3.991	PER IMP= 90.00
ADD HYD	AP-3	2& 3	10	.09180	206.10	9.539	1.94838	1.500	3.508	
COMPUTE NM HYD	AP-4&A-4D	-	3	.01250	29.07	1.312	1.96827	1.500	3.634	PER IMP= 76.00
ADD HYD	AP-3+AP-4	10& 3	10	.10430	235.17	10.851	1.95076	1.500	3.523	
COMPUTE NM HYD	P-6	-	3	.00430	3.72	.110	.47912	1.500	1.350	PER IMP= 2.00
ADD HYD	AP-5	10& 3	10	.10860	238.89	10.961	1.89248	1.500	3.437	
ROUTE RESERVOIR	POND-6R	10	2	.10860	55.69	10.961	1.89248	2.000	.801	AC-FT= 4.965
ROUTE	POND6ROUTE36	2	12	.10860	55.69	10.961	1.89248	2.050	.801	
ROUTE	POND6ROUTE48	12	22	.10860	55.71	10.961	1.89248	2.050	.802	
COMPUTE NM HYD	AP-6&C-1D	-	3	.01150	24.41	1.024	1.67006	1.500	3.317	PER IMP= 57.00
ROUTE	AP-6R	3	2	.01150	23.38	1.024	1.67010	1.550	3.177	

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION
										PAGE = 2
COMPUTE NM HYD	C-2D	-	3	.05740	118.78	5.113	1.67006	1.500	3.233	PER IMP= 57.00
ADD HYD	AP-7	2& 3	10	.06890	141.62	6.137	1.67005	1.500	3.212	
ADD HYD	PARTAP-8	10&22	20	.17750	176.57	17.098	1.80614	1.550	1.554	
COMPUTE NM HYD	P-4	-	3	.00360	3.11	.092	.47912	1.500	1.352	PER IMP= 2.00
ADD HYD	AP-8	20& 3	10	.18110	179.57	17.190	1.77971	1.550	1.549	
ROUTE RESERVOIR	POND-4	10	2	.18110	82.38	17.110	1.77147	2.050	.711	AC-FT= 3.859
ROUTE	POND-4R	2	12	.18110	82.40	17.110	1.77147	2.050	.711	
COMPUTE NM HYD	P-3	-	3	.00270	2.34	.069	.47912	1.500	1.354	PER IMP= 2.00
ADD HYD	P-3+POND-4R	12& 3	10	.18380	82.62	17.178	1.75240	2.050	.702	
COMPUTE NM HYD	AP-9&D-1D	-	3	.04680	99.31	4.168	1.67006	1.500	3.316	PER IMP= 57.00
ADD HYD	AP-9A	10& 3	10	.23060	138.39	21.347	1.73569	1.500	.938	
ROUTE RESERVOIR	POND-3	10	2	.23060	91.01	21.278	1.73009	2.150	.617	AC-FT= 2.934
COMPUTE NM HYD	AP-10&E-1D	-	3	.02820	64.16	2.883	1.91659	1.500	3.555	PER IMP= 71.00
ROUTE	AP-10R	3	12	.02820	61.84	2.883	1.91660	1.550	3.427	
COMPUTE NM HYD	E-2D	-	3	.01720	44.80	2.065	2.25115	1.500	4.070	PER IMP= 90.00
ADD HYD	AP-11	3&12	10	.04540	106.21	4.948	2.04333	1.500	3.655	
ROUTE	AP-11R	10	12	.04540	104.90	4.948	2.04334	1.550	3.610	
COMPUTE NM HYD	E-3D	-	3	.04250	90.19	3.785	1.67006	1.500	3.316	PER IMP= 57.00
ADD HYD	AP-12	12& 3	10	.08790	191.75	8.733	1.86285	1.500	3.408	
ROUTE	AP-11R	10	12	.08790	190.60	8.733	1.86286	1.550	3.388	
COMPUTE NM HYD	E-4D	-	3	.00300	7.83	.360	2.25115	1.500	4.076	PER IMP= 90.00
ADD HYD	AP-13	12& 3	10	.09090	197.44	9.093	1.87566	1.550	3.394	
ADD HYD	AP-13+POND-3	2&10	10	.32150	231.12	30.371	1.77125	1.550	1.123	
COMPUTE NM HYD	P-2	-	3	.00360	4.76	.135	.70157	1.500	2.066	PER IMP= 2.00
ADD HYD	AP-13A	3&10	10	.32510	235.61	30.505	1.75938	1.550	1.132	
ROUTE RESERVOIR	POND-2	10	2	.32510	128.74	29.772	1.71710	2.100	.619	AC-FT= 7.056
ROUTE	POND-2R	2	12	.32510	129.54	29.772	1.71710	2.100	.623	
COMPUTE NM HYD	AP-14&F-1D	-	3	.03980	82.37	3.545	1.67006	1.500	3.234	PER IMP= 57.00

COMPUTE NM HYD	P-1	-	4	.00290	2.51	.074	.47912	1.500	1.353	PER IMP=	2.00
ADD HYD	P-1+F-1D	3& 4	10	.04270	84.88	3.618	1.58885	1.500	3.106		
ADD HYD	AP-14A	10&12	10	.36780	142.65	33.391	1.70221	2.100	.606		
ROUTE RESERVOIR	POND-1	10	2	.36780	82.17	33.390	1.70218	2.800	.349	AC-FT=	4.862
COMPUTE NM HYD	AP-15&G-1D	-	3	.02290	56.30	2.534	2.07506	1.500	3.841	PER IMP=	80.00
COMPUTE NM HYD	AP-16&G-2D	-	4	.00310	7.72	.349	2.11028	1.500	3.893	PER IMP=	82.00
ROUTE	AP-16R	4	12	.00310	7.56	.349	2.11044	1.500	3.808		
COMPUTE NM HYD	AP-17&G-3D	-	5	.03510	77.66	3.885	2.07506	1.500	3.457	PER IMP=	80.00
COMPUTE NM HYD	G-4D	-	6	.00330	8.10	.359	2.03985	1.500	3.837	PER IMP=	78.00
ADD HYD	AP-18	12& 6	10	.00640	15.66	.708	2.07389	1.500	3.823		
FINISH											

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994 RUN DATE (MON/DAY/YR) =01/25/2001
INPUT FILE = DEVP621.IN USER NO.= C_ANDRSN.I01

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION
										PAGE = 1
*S DEVELOPED CONDITIONS WITH I-40 INTERCEPTOR IN PLACE										
*S 100 YEAR, 24 HOUR STORM EVENT										
*S										
*S FILE NAME: DEVP621.IN										
*S										
*S THIS MODEL ANALYZES THE UNSER DIVERSION PONDS WITH THE EXISTING POND										
*S OUTLET WORKS AND THE EXISTING POND GRADING. IN THIS TRIAL, POND NO. 6,										
*S POND NO. 2 AND POND NO. 1 OUTLET WORKS HAVE BEEN MODIFIED TO REPRESENT										
*S PROPOSED OUTLET WORKS.										
*S										
*S MASTER DRAINAGE PLAN FOR THE WEST SIDE TRANSIT FACILITY										
*S										
*S THIS AHYMO FILE ANALYZES THE UNSER DIVERSION (DETENTION PONDS) ASSUMING										
*S FULLY DEVELOPED CONDITIONS WITH THE I-40 INTERCEPTOR IN PLACE.										
*S NOTES: UNSER DIVERSION POND NO. 5 IS NOT USED IN FULLY DEVELOPED										
*S CONDITION (PER CHUCK EASTERING'S REPORT).										
*S THE OUTLET RATING CURVES WERE ALSO BASED UPON TAILWATER										
*S ELEVATIONS SHOWN ON THE UNSER DIVERSION (AS-BUILT)										
*S CONSTRUCTION PLAN SET (PLANS BY CHUCK EASTERLING)										
*S DATE: 11/7/00										
START										TIME= .00
RAINFALL TYPE= 2										RAIN24= 2.660
*S FOR WATERSHED NORTH OF INTERSTATE 40										
*S ALL FLOWS TO BE PICKED UP BY I-40 INTERCEPTOR										
*S BEGIN WATERSHED SOUTH OF INTERSTATE 40										
COMPUTE NM HYD	I40-1	-	3	.00750	11.24	.454	1.13512	1.500	2.342	PER IMP= 35.00
ROUTE	I40-1R	3	2	.00750	10.83	.454	1.13518	1.550	2.256	
COMPUTE NM HYD	I40-2	-	3	.00310	4.65	.188	1.13512	1.500	2.346	PER IMP= 35.00
ROUTE	I40-2R	3	12	.00310	4.61	.188	1.13529	1.500	2.325	
ADD HYD	I401R&I402R	2&12	10	.01060	15.34	.642	1.13508	1.500	2.261	
COMPUTE NM HYD	A-1D	-	3	.02570	59.05	2.579	1.88137	1.500	3.590	PER IMP= 69.00
ADD HYD	AP-1	10& 3	10	.03630	74.38	3.220	1.66343	1.500	3.202	
ROUTE	AP-1R	10	2	.03630	73.06	3.220	1.66346	1.500	3.145	
COMPUTE NM HYD	I40-3	-	3	.00190	2.86	.115	1.13512	1.500	2.350	PER IMP= 35.00
COMPUTE NM HYD	I40-4	-	4	.00120	1.81	.073	1.13512	1.500	2.357	PER IMP= 35.00
ADD HYD	I40-3&I40-4	3& 4	10	.00310	4.67	.188	1.13493	1.500	2.353	
ROUTE	I403R&I404R	10	12	.00310	4.38	.188	1.13522	1.550	2.208	
ADD HYD	PARTAP-2	2&12	10	.03940	76.99	3.408	1.62186	1.500	3.053	
COMPUTE NM HYD	A-2D	-	3	.02840	71.47	3.250	2.14550	1.500	3.932	PER IMP= 84.00
ADD HYD	AP-2	10& 3	10	.06780	148.46	6.658	1.84120	1.500	3.421	
ROUTE	AP-2R	10	2	.06780	144.80	6.658	1.84122	1.500	3.337	
COMPUTE NM HYD	A-3D	-	3	.02400	61.30	2.881	2.25115	1.500	3.991	PER IMP= 90.00
ADD HYD	AP-3	2& 3	10	.09180	206.10	9.539	1.94838	1.500	3.508	
COMPUTE NM HYD	AP-4&A-4D	-	3	.01250	29.07	1.312	1.96827	1.500	3.634	PER IMP= 76.00
ADD HYD	AP-3+AP-4	10& 3	10	.10430	235.17	10.851	1.95076	1.500	3.523	
COMPUTE NM HYD	P-6	-	3	.00430	3.72	.110	.47912	1.500	1.350	PER IMP= 2.00
ADD HYD	AP-5	10& 3	10	.10860	238.89	10.961	1.89248	1.500	3.437	
ROUTE RESERVOIR	POND-6R	10	2	.10860	55.69	10.961	1.89248	2.000	.801	AC-FT= 4.965
ROUTE	POND6ROUTE36	2	12	.10860	55.69	10.961	1.89248	2.050	.801	

COMMAND	HYDROGRAPH IDENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	NOTATION
										PAGE = 2
ROUTE	POND6ROUTE48	12	22	.10860	55.71	10.961	1.89248	2.050	.802	
COMPUTE NM HYD	AP-6&C-1D	-	3	.01150	24.41	1.024	1.67006	1.500	3.317	PER IMP= 57.00
ROUTE	AP-6R	3	2	.01150	23.38	1.024	1.67010	1.550	3.177	
COMPUTE NM HYD	C-2D	-	3	.05740	118.78	5.113	1.67006	1.500	3.233	PER IMP= 57.00
ADD HYD	AP-7	2& 3	10	.06890	141.62	6.137	1.67005	1.500	3.212	
ADD HYD	PARTAP-8	10&22	20	.17750	176.57	17.098	1.80614	1.550	1.554	
COMPUTE NM HYD	P-4	-	3	.00360	3.11	.092	.47912	1.500	1.352	PER IMP= 2.00
ADD HYD	AP-8	20& 3	10	.18110	179.57	17.190	1.77971	1.550	1.549	
ROUTE RESERVOIR	POND-4	10	2	.18110	88.27	17.100	1.77039	2.000	.762	AC-FT= 4.464
ROUTE	POND-4R	2	12	.18110	88.20	17.100	1.77039	2.000	.761	
COMPUTE NM HYD	P-3	-	3	.00270	2.34	.069	.47912	1.500	1.354	PER IMP= 2.00
ADD HYD	P-3+POND-4R	12& 3	10	.18380	88.46	17.168	1.75134	1.950	.752	
COMPUTE NM HYD	AP-9&D-1D	-	3	.04680	99.31	4.168	1.67006	1.500	3.316	PER IMP= 57.00
ADD HYD	AP-9A	10& 3	10	.23060	111.94	21.336	1.73484	1.850	.758	
ROUTE RESERVOIR	POND-3	10	2	.23060	94.02	21.103	1.71590	2.150	.637	AC-FT= 3.115
COMPUTE NM HYD	AP-10&E-1D	-	3	.02820	64.16	2.883	1.91659	1.500	3.555	PER IMP= 71.00
ROUTE	AP-10R	3	12	.02820	61.84	2.883	1.91660	1.550	3.427	
COMPUTE NM HYD	E-2D	-	3	.01720	44.80	2.065	2.25115	1.500	4.070	PER IMP= 90.00
ADD HYD	AP-11	3&12	10	.04540	106.21	4.948	2.04333	1.500	3.655	
ROUTE	AP-11R	10	12	.04540	104.90	4.948	2.04334	1.550	3.610	
COMPUTE NM HYD	E-3D	-	3	.04250	90.19	3.785	1.67006	1.500	3.316	PER IMP= 57.00
ADD HYD	AP-12	12& 3	10	.08790	191.75	8.733	1.86285	1.500	3.408	
ROUTE	AP-11R	10	12	.08790	190.60	8.733	1.86286	1.550	3.388	
COMPUTE NM HYD	E-4D	-	3	.00300	7.83	.360	2.25115	1.500	4.076	PER IMP= 90.00
ADD HYD	AP-13	12& 3	10	.09090	197.44	9.093	1.87566	1.550	3.394	
ADD HYD	AP-13+POND-3	2&10	10	.32150	203.90	30.197	1.76107	1.550	.991	
COMPUTE NM HYD	P-2	-	3	.00360	4.76	.135	.70157	1.500	2.066	PER IMP= 2.00
ADD HYD	AP-13A	3&10	10	.32510	208.39	30.331	1.74931	1.550	1.002	

ROUTE RESERVOIR	POND-2	10	2	.32510	121.71	29.564	1.70508	2.150	.585	AC-FT=	7.003
ROUTE	POND-2R	2	12	.32510	121.90	29.563	1.70504	2.200	.586		
COMPUTE NM HYD	AP-14&F-1D	-	3	.03980	82.37	3.545	1.67006	1.500	3.234	PER IMP=	57.00
COMPUTE NM HYD	P-1	-	4	.00290	2.51	.074	.47912	1.500	1.353	PER IMP=	2.00
ADD HYD	P-1+F-1D	3& 4	10	.04270	84.88	3.618	1.58885	1.500	3.106		
ADD HYD	AP-14A	10&12	10	.36780	130.86	33.181	1.69155	2.150	.556		
ROUTE RESERVOIR	POND-1	10	2	.36780	78.98	33.161	1.69049	2.700	.336	AC-FT=	4.399
COMPUTE NM HYD	AP-15&G-1D	-	3	.02290	56.30	2.534	2.07506	1.500	3.841	PER IMP=	80.00
COMPUTE NM HYD	AP-16&G-2D	-	4	.00310	7.72	.349	2.11028	1.500	3.893	PER IMP=	82.00
ROUTE	AP-16R	4	12	.00310	7.56	.349	2.11044	1.500	3.808		
COMPUTE NM HYD	AP-17&G-3D	-	5	.03510	77.66	3.885	2.07506	1.500	3.457	PER IMP=	80.00
COMPUTE NM HYD	G-4D	-	6	.00330	8.10	.359	2.03985	1.500	3.837	PER IMP=	78.00
ADD HYD	AP-18	12& 6	10	.00640	15.66	.708	2.07389	1.500	3.823		
FINISH											

APPENDIX D

DAYTONA ROAD HYDRAULIC ANALYSIS

MASTER DRAINAGE PLAN – WEST SIDE TRANSIT FACILITY

DAYTONA ROAD HYDRAULIC ANALYSIS

Given: Fully Developed Conditions.
I-40 Interceptor is in place.
100yr.-24hr. storm event
Daytona Rd. is not classified as an arterial or collector street or roadway.
No Roadways intersecting Daytona Road are classified as arterial or collectors.
Thus, the 10 year storm event was not included as part of this analysis.
Street capacity calculations and storm drain pipe hydraulic calculations are attached.

Analysis:

Analysis begins at the west end of Daytona Rd.

Total flow from basin I40-1 (Interstate 40 median drain) through culvert under I-40 = 11cfs. This 11cfs will be routed in a storm drain pipe in Daytona Rd. to 90th St.

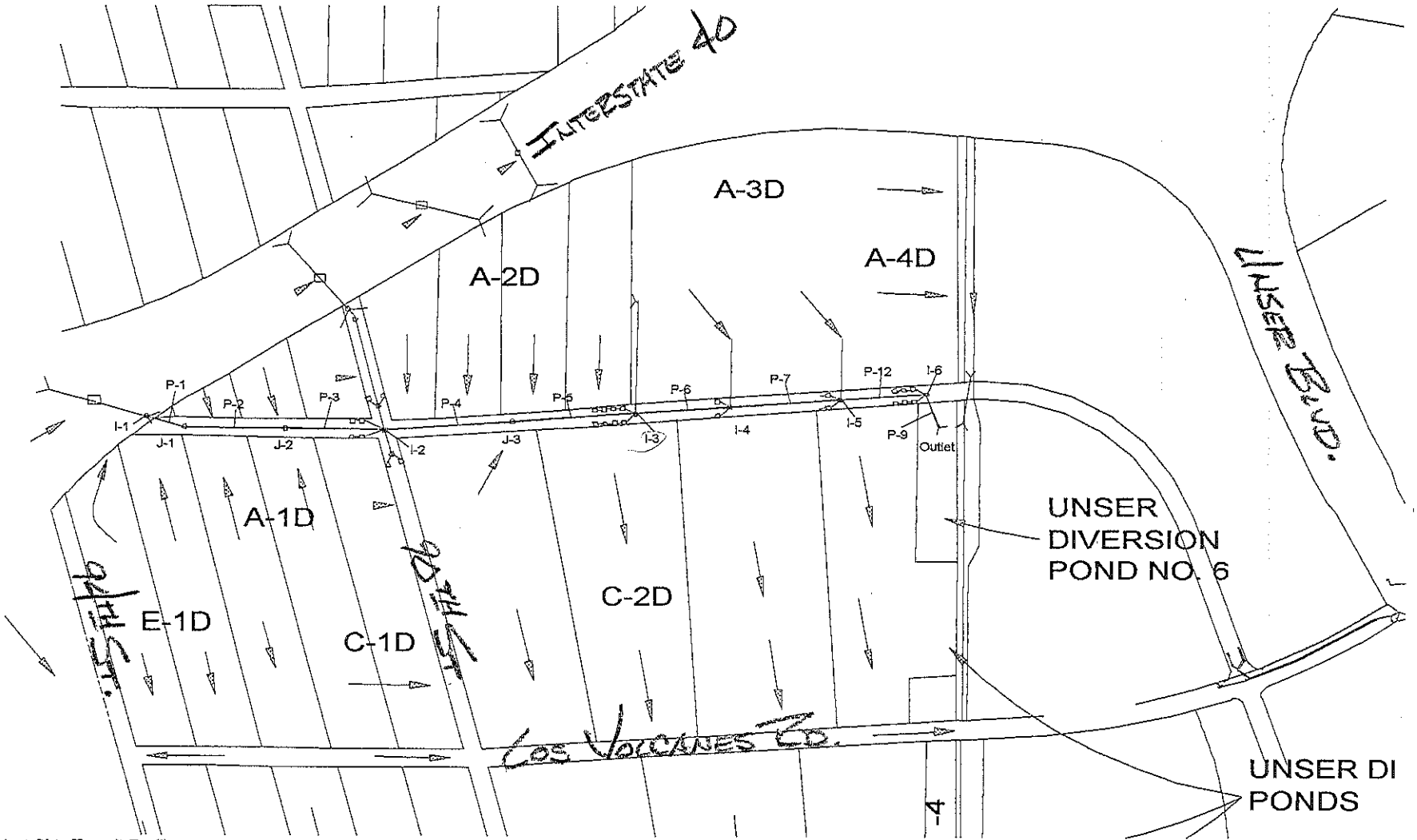
Total Flow at Daytona Rd. and 90th St. (AP-1) =74cfs.
11cfs is already in the pipe (from the I40-1 basin).
The remaining 63cfs will be in street (It should be noted that a small amount of the 63 cfs will probably be in 90th St. both north and south of Daytona Rd.).
Street capacity of Daytona Rd. (assuming a roadway slope of 2%) = 95cfs (see attached street capacity calculation sheet).
It appears there is no need to install inlets at this intersection. But, if we look downstream to AP-2, we see that the flow = 148cfs, which is greater than the street capacity of 95cfs. Thus, we will pick up some of this flow at Daytona Rd. and 90th St.
Install 2 inlets on 90th St. north of Daytona Rd.
Install 2 inlets on 90th St. south of Daytona Rd.
Install 4 inlets on Daytona Rd. west of 90th St.
Thus, total number of inlets at Daytona Rd. = 8.
Assuming each inlet will collect 5cfs, a total of 40cfs will be picked up and put into the storm drain pipe in Daytona Rd.
In summary (at Daytona Rd. and 90th St.), Qpipe = 11cfs + 40cfs = 51cfs, Qstreet = 23cfs. (51+23 = 74✓)

Total flow at west boundary of the West Side Transit Facility (AP-2) = 148cfs.
At this same location, (AP-2) additional offsite flow from the Mirehaven Arroyo "B" will need to be collected. This additional flow = ~~91~~ 19 cfs (the additional 91cfs will be picked up in a storm drain pipe and transported to the storm drain pipe in Daytona Rd.).
Thus, just upstream of AP-2 there is 51cfs + ~~91~~ 19cfs = ~~142~~ 70cfs in the Daytona Rd storm drain pipe. The street flow in Daytona Rd. = 148cfs – 51cfs = 97cfs. Street Capacity = 95cfs. Inlets are required.
Install 8 inlets at AP-2. Again, assuming 5cfs will be picked up at each inlet, this will put an additional 40cfs into the Daytona Rd. storm drain pipe.
In summary as we leave AP-2,
Qpipe = ~~142~~ 70cfs + 40cfs = ~~182~~ 110cfs.
Qstreet = 148cfs – ~~91~~ 19cfs = ~~57~~ 129cfs.

Total flow at Daytona Rd. and Unser Diversion Pond No. 6 (AP-3) = 206cfs plus the offsite flow of ~~91~~ 19 cfs from the Mirehaven Arroyo "B".
All of the flow from the West Side Transit Facility (Basin A-3D, Q=61cfs) is proposed to be picked up in a storm drain pipe and carried to the storm drain pipe located in Daytona Rd.
Thus, approaching AP-3, Qpipe = ~~182~~ 110cfs + 61cfs = ~~243~~ 171cfs. Qstreet = 206cfs + ~~91~~ 19cfs - ~~243~~ 171cfs = ~~54~~ 54cfs.
All of this flow is required to be picked up and put into the Daytona Rd. storm drain pipe.
Thus, assuming 5cfs is picked per inlet, 54cfs/5cfs/inlet = 10.8 inlets.
To be conservative, 12 inlets should be placed at AP-3 to collect all of the 54cfs.
In summary as we leave AP-3,
→ Qpipe = 297cfs. => This pipe flow will be emptied into the Unser Diversion Pond No. 6.
Qstreet = 0cfs (171+54 = 225)

SEE ATTACHED STORM DRAIN PIPE CALCULATIONS FOR ALL PROPOSED STORM DRAIN PIPE SIZES

DAYTONA ROAD STORM DRAIN – PLAN VIEW



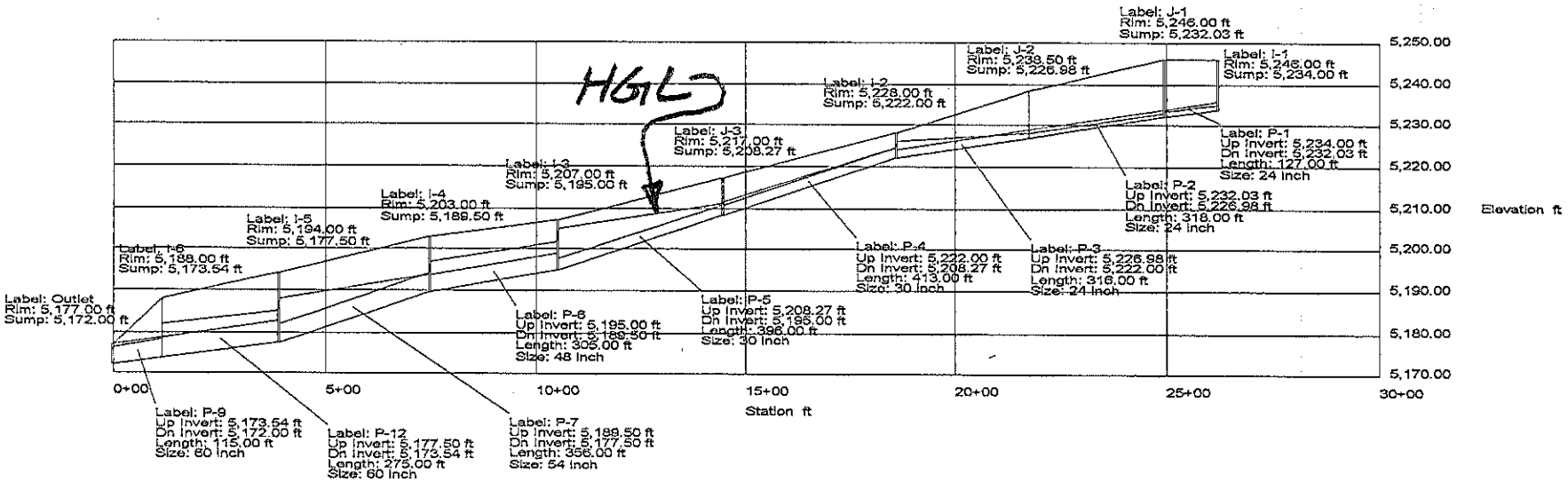
Project Title: West Side Transit Facility
o:\300\100300\draina~1\stormcad\devwoff.stm
01/31/01 01:16:44 PM

Smith Engineering
© Haestad Methods, Inc. 37 Brookside Road Waterbury, CT 06708 USA (203) 755-1666

Project Engineer: Smith Engineering Company
StormCAD v1.5 [158]
Page 1 of 1))

SHEET 2 OF 6

DAYTONA ROAD STORM DRAIN – PROFILE



Sheet 15 of 16

Project Title: West Side Transit Facility
o:\300\100300\draina~1\stormcad\devwoff.stm
01/31/01 01:34:29 PM

Smith Engineering
© Haestad Methods, Inc. 37 Brookside Road Waterbury, CT 06708 USA (203) 755-1666

Project Engineer: Smith Engineering Company
StormCAD v1.5 [158]
Page 1 of 1

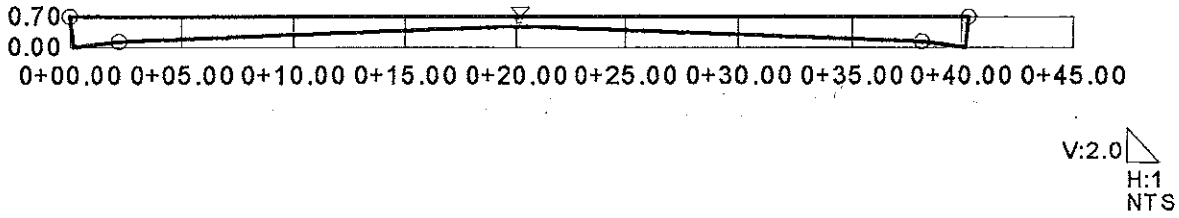
Downstream Node	Length (ft)	Section Size	Capacity (cfs)	Average Velocity (ft/s)	Upstream Invert Elevation (ft)	Dow El	Upstream Ground Elevation (ft)	Pipe	Roughness
J-1	127.00	24 inch	28.17	5.39	5,234.00	00	5,246.00	P-1	0.013
J-2	318.00	24 inch	28.51	5.58	5,232.03	50	5,246.00	P-2	0.013
I-2	316.00	24 inch	28.40	4.58	5,226.98	00	5,238.50	P-3	0.013
J-3	413.00	30 inch	74.78	10.57	5,222.00	00	5,228.00	P-4	0.013
I-3	396.00	30 inch	75.08	10.39	5,208.27	00	5,217.00	P-5	0.013
I-4	305.00	48 inch	192.88	14.48	5,195.00	00	5,207.00	P-6	0.013
I-5	356.00	54 inch	361.02	14.26	5,189.50	00	5,203.00	P-7	0.013
Outlet	115.00	60 inch	301.37	16.19	5,173.54	00	5,188.00	P-9	0.013
I-6	275.00	60 inch	312.51	13.90	5,177.50	00	5,194.00	P-12	0.013

DAYTONA IC

Master Drainage Plan for the West Side Transit Facility
Cross Section for Irregular Channel

Project Description	
Worksheet	40' STREET CAPACITY CALC → DAYTONA RD.
Flow Element	Irregular Channel
Method	Manning's Formula
Solve For	Discharge

Section Data	
Mannings Coefficient	0.018
Slope	2.0000 %
Water Surface Elevation	0.67 ft
Elevation Range	0.00 to 0.67
Discharge	95.38 cfs



Worksheet
Worksheet for Irregular Channel

Project Description	
Worksheet	40' STREET CAPACITY CALC
Flow Element	Irregular Channel
Method	Manning's Formula
Solve For	Discharge

DAYTONA RD.

Input Data		
Slope	2.0000	%
Water Surface Elevation	0.67	ft

Options	
Current Roughness Method	Improved Lotter's Method
Open Channel Weighting Metho	Improved Lotter's Method
Closed Channel Weighting Meth	Horton's Method

Results		
Mannings Coefficient	0.018	
Elevation Range	0.00 to 0.67	
Discharge	95.38	cfs
Flow Area	15.68	ft²
Wetted Perimeter	41.40	ft
Top Width	40.33	ft
Actual Depth	0.67	ft
Critical Elevation	0.84	ft
Critical Slope	0.6059	%
Velocity	6.08	ft/s
Velocity Head	0.57	ft
Specific Energy	1.24	ft
Froude Number	1.72	
Flow Type	Supercritical	

Roughness Segments

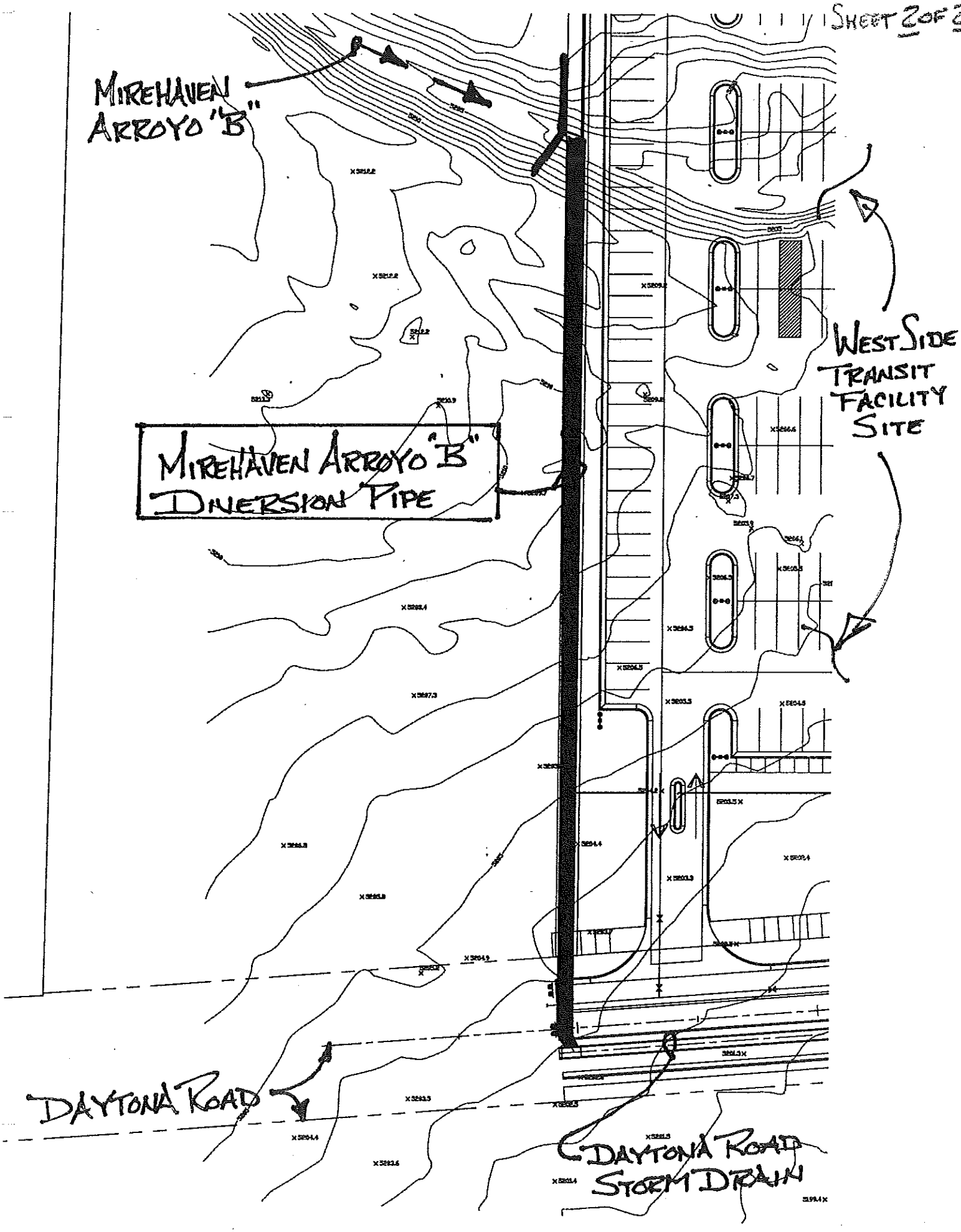
Start Station	End Station	Mannings Coefficient
0+00.00	0+02.17	0.013
0+02.17	0+38.17	0.020
0+38.17	0+40.33	0.013

Natural Channel Points

Station (ft)	Elevation (ft)
0+00.00	0.67
0+00.17	0.00
0+02.17	0.13
0+20.17	0.49
0+38.17	0.13
0+40.17	0.00
0+40.33	0.67

APPENDIX E

MIREHAVEN ARROYO "B" DIVERSION PIPE
HYDRAULIC ANALYSIS


MIREHAVEN ARROYO "B"
MIREHAVEN ARROYO "B" DIVERSION PIPE
WEST SIDE TRANSIT FACILITY SITE
DAYTONA ROAD
DAYTONA ROAD STORM DRAIN

APPENDIX F

UNSER DIVERSION EARTHERN CHANNEL
CULVERT PIPE UNDER DAYTONA ROAD
HYDRAULIC ANALYSIS

Culvert Calculator Report
U.D. Earthen Channel Under Daytona Rd.

Solve For: Headwater Elevation

Culvert Summary			
Allowable HW Elevation	5,180.0 ft	Headwater Depth/ Height	0.94
Computed Headwater Elevation	5,179.4 ft	Discharge	29.00 cfs
Inlet Control HW Elev	5,179.2 ft	Tailwater Elevation	5,176.7 ft
Outlet Control HW Elev	5,179.4 ft	Control Type	Entrance Control

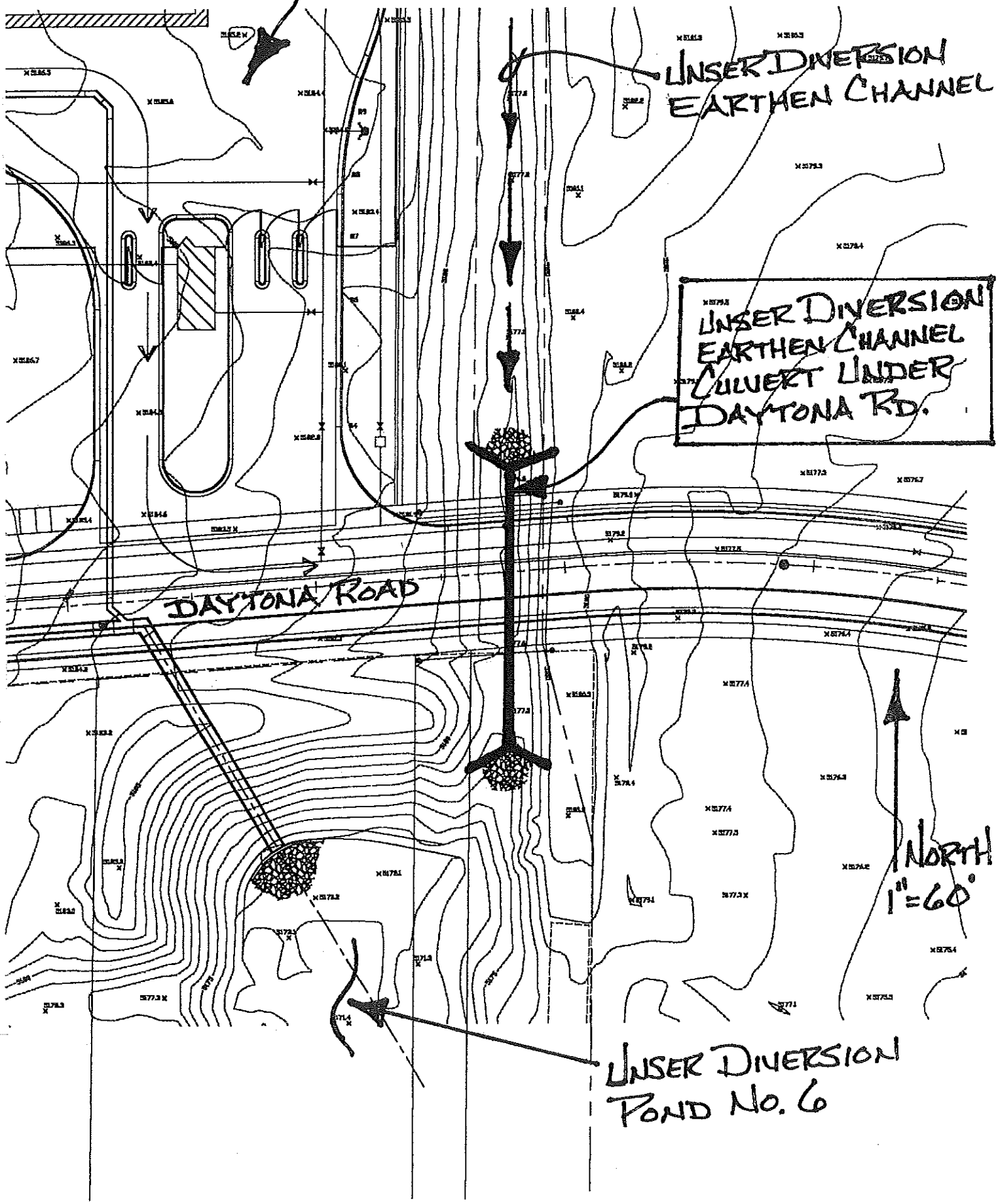
Grades			
Upstream Invert	5,176.6 ft	Downstream Invert	5,176.0 ft
Length	110.0 ft	Constructed Slope	0.5455 %

Hydraulic Profile			
Profile	S2	Depth, Downstream	1.7 ft
Slope Type	Steep	Normal Depth	1.7 ft
Flow Regime	Supercritical	Critical Depth	1.7 ft
Velocity Downstream	7.3 ft/s	Critical Slope	0.4624 %

Section			
Section Shape	Circular	Mannings Coefficient	0.013
Section Material	Concrete	Span	3.0 ft
Section Size	36 inch	Rise	3.0 ft
Number Sections	1		

Outlet Control Properties			
Outlet Control HW Elev	5,179.4 ft	Upstream Velocity Head	0.7 ft
Ke	0.50	Entrance Loss	0.4 ft

Inlet Control Properties			
Inlet Control HW Elev	5,179.2 ft	Flow Control	Unsubmerged
Inlet Type	Square edge w/headwall	Area Full	7 ft²
K	0.00980	HDS 5 Chart	1
M	2.00000	HDS 5 Scale	1
C	0.03980	Equation Form	1
Y	0.67000		


WEST SIDE TRANSIT FACILITY SITE
UNSER DIVERSION EARTHEN CHANNEL
UNSER DIVERSION EARTHEN CHANNEL CULVERT UNDER DAYTONA RD.
DAYTONA ROAD
NORTH
1"=60'
UNSER DIVERSION POND NO. 6

APPENDIX G

OLIVER ROSS ROAD AND LOS VOLCANES ROAD HYDRAULIC ANALYSIS

MASTER DRAINAGE PLAN – WEST SIDE TRANSIT FACILITY

OLIVER ROSS ROAD AND LOS VOLCANES ROAD HYDRAULIC ANALYSIS

Given: Fully Developed Conditions.
I-40 Interceptor is in place.
100yr.-24hr. storm event
Los Volcanes Rd. is not classified as an arterial or collector street or roadway.
Oliver Ross Rd. is not classified as an arterial or collector street or roadway
Thus, the 10 year storm event was not included as part of this analysis.
Street capacity calculations attached.
Assumption: Each inlet will collect 5cfs.

Analysis:

Analysis begins at the south end of Oliver Ross Rd., just north of Los Volcanes Rd.

Total flow generated from the Oliver Ross Basin (Basin G-2D and AP-16) = 8cfs. Oliver Ross Rd. capacity = 95cfs (see street capacity calculation sheet, attached). No inlets or storm drain pipe is required on Oliver Ross at Los Volcanes Rd. Let this flow enter Los Volcanes Rd.

Total flow in Los Volcanes just west of Unser Blvd. (AP-18) = 16 cfs. At this point, all flow in Los Volcanes will be on the South side of the roadway. The width of Los Volcanes at this location is approximately 48 feet. Capacity of Los Volcanes Rd. at this location = 68cfs. Two inlets currently exist on the south side of Los Volcanes Rd. just west of Unser Blvd. These two inlets will intercept 10cfs at this location. The remaining 6cfs will enter the west half/side of Unser Blvd. as street flow. The capacity of Unser Blvd will be discussed below.

Let's take a look at Unser Blvd. around the Los Volcanes Rd. intersection. According to the Master Drainage Plan for the Atrisco Business Park, October 1993 prepared by Easterling & Associates, Inc., the west half of Unser Blvd. is carrying 14cfs just north of Los Volcanes Rd. while the east half of Unser is carrying 13cfs. A total of 4 inlets currently exist on Unser Blvd. just north of Los Volcanes (two on the west side and two on the east side). Each inlet will intercept 5 cfs. Thus, looking at the west side of Unser, 10cfs will be picked up by inlets while 4cfs bypassed the two inlets and remain in the street just south of the two westerly inlets. When we add the 6cfs from Los Volcanes Rd., there will be 10cfs on the west half of Unser Rd. The capacity of the west half of Unser Rd. = 43cfs. -- OK. In summary, six total inlets exist at this intersection (4 on Unser Blvd and 2 on Los Volcanes Rd.) capturing a total of 30cfs. According to Easterling's report (mentioned above), the capacity of the 30" storm drain pipe in Unser Blvd. = 41cfs. – OK. Thus, no new storm drain infrastructure will be required at this intersection.

In summary (to this point), all of the flow in Oliver Ross Rd and Los Volcanes is accounted for. Now we need to look at basins G-1D and G-3D. According to the Master Plan for the Atrisco Business Park (mentioned above) the maximum runoff allowed from these two basins combined = 47cfs. Combined runoff from these two basins = 134cfs. Thus, detention of the runoff from these two basins is required in order to meet the allowable runoff rate of 47 cfs. The same property owner owns both tracts of land (G-1D and G-3D). At this time, it is difficult to determine how drainage of these two tracts will develop. The following is proposed at this time:

A detention pond will be constructed at the southeast corner of basin G-3D. Flows from Basin G-1D will be routed to basin G-3D and eventually to the detention Pond located within basin G-3D. A storm drain stub out will need to be installed (in the future) from the Unser Blvd storm drain to the southwest detention pond located within basin G-3D. This will meet the requirements laid out in the Master Drainage Plan for the Atrisco Business Park.

SEE ATTACHED STREET CAPACITY CALCULATIONS

Worksheet
Worksheet for Irregular Channel

Project Description	
Worksheet	40' STREET CAPACITY CALC
Flow Element	Irregular Channel
Method	Manning's Formula
Solve For	Discharge

→ OLIVET KASS RD.

Input Data		
Slope	2.0000	%
Water Surface Elevation	0.67	ft

Options	
Current Roughness Method	Improved Lotter's Method
Open Channel Weighting Metho	Improved Lotter's Method
Closed Channel Weighting Meth	Horton's Method

Results		
Mannings Coefficient	0.018	
Elevation Range	0.00 to 0.67	
Discharge	95.38	cfs
Flow Area	15.68	ft²
Wetted Perimeter	41.40	ft
Top Width	40.33	ft
Actual Depth	0.67	ft
Critical Elevation	0.84	ft
Critical Slope	0.6059	%
Velocity	6.08	ft/s
Velocity Head	0.57	ft
Specific Energy	1.24	ft
Froude Number	1.72	
Flow Type	Supercritical	

Roughness Segments

Start Station	End Station	Mannings Coefficient
0+00.00	0+02.17	0.013
0+02.17	0+38.17	0.020
0+38.17	0+40.33	0.013

Natural Channel Points

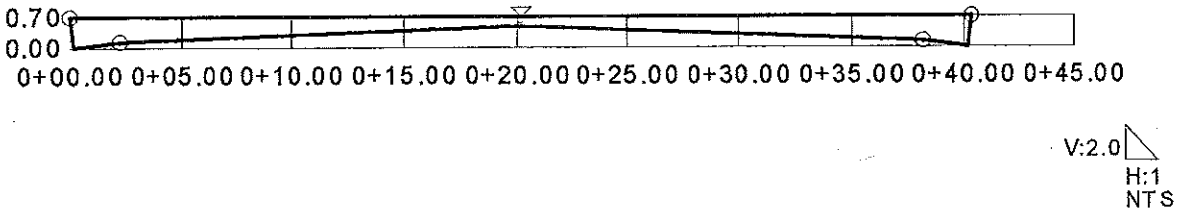
Station (ft)	Elevation (ft)
0+00.00	0.67
0+00.17	0.00
0+02.17	0.13
0+20.17	0.49
0+38.17	0.13
0+40.17	0.00
0+40.33	0.67

Master Drainage Plan for the West Side Transit Facility
Cross Section for Irregular Channel

Project Description	
Worksheet	40' STREET CAPACITY CALC
Flow Element	Irregular Channel
Method	Manning's Formula
Solve For	Discharge

OLIVER ROAD RD.

Section Data		
Mannings Coefficient	0.018	
Slope	2.0000	%
Water Surface Elevation	0.67	ft
Elevation Range	0.00 to 0.67	
Discharge	95.38	cfs



Worksheet
Worksheet for Irregular Channel

Project Description	
Worksheet	48' STREET CAPACITY CALC — LOS VOLCANES RD.
Flow Element	Irregular Channel
Method	Manning's Formula
Solve For	Discharge

Input Data		
Slope	1.0000	%
Water Surface Elevation	0.67	ft

Options	
Current Roughness Method	Improved Lotter's Method
Open Channel Weighting Metho	Improved Lotter's Method
Closed Channel Weighting Meth	Horton's Method

Results		
Mannings Coefficient	0.018	
Elevation Range	0.00 to 0.67	
Discharge	67.81	cfs
Flow Area	16.84	ft²
Wetted Perimeter	49.40	ft
Top Width	48.33	ft
Actual Depth	0.67	ft
Critical Elevation	0.72	ft
Critical Slope	0.6654	%
Velocity	4.03	ft/s
Velocity Head	0.25	ft
Specific Energy	0.92	ft
Froude Number	1.20	
Flow Type	Supercritical	

Roughness Segments

Start Station	End Station	Mannings Coefficient
0+00.00	0+02.17	0.013
0+02.17	0+46.17	0.020
0+46.17	0+48.33	0.013

Natural Channel Points

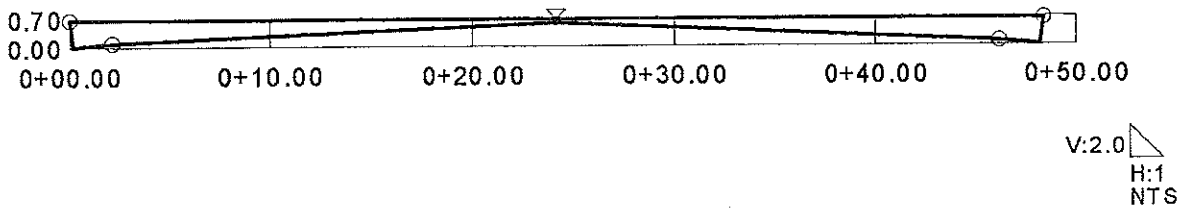
Station (ft)	Elevation (ft)
0+00.00	0.67
0+00.17	0.00
0+02.17	0.13
0+24.17	0.56
0+46.17	0.13
0+48.17	0.00
0+48.33	0.67

Cross Section
Cross Section for Irregular Channel

Project Description	
Worksheet	48' STREET CAPACITY CALC
Flow Element	Irregular Channel
Method	Manning's Formula
Solve For	Discharge

LOS VOLCANES RD.

Section Data		
Mannings Coefficient	0.018	
Slope	1.0000	%
Water Surface Elevation	0.67	ft
Elevation Range	0.00 to 0.67	
Discharge	67.81	cfs



Worksheet
Worksheet for Irregular Channel

Project Description	
Worksheet	ARTERIAL' STREET CAPACITY CALCULATION (→ UNSER BLVD (WEST SIDE)
Flow Element	Irregular Channel
Method	Manning's Formula
Solve For	Discharge

Input Data		
Slope	1.6000	%
Water Surface Elevation	0.67	ft

Options	
Current Roughness Method	Improved Lotter's Method
Open Channel Weighting Metho	Improved Lotter's Method
Closed Channel Weighting Meth	Horton's Method

Results		
Mannings Coefficient	0.018	
Elevation Range	0.00 to 1.05	
Discharge	42.65	cfs
Flow Area	8.37	ft²
Wetted Perimeter	24.81	ft
Top Width	24.21	ft
Actual Depth	0.67	ft
Critical Elevation	0.78	ft
Critical Slope	0.6298	%
Velocity	5.10	ft/s
Velocity Head	0.40	ft
Specific Energy	1.07	ft
Froude Number	1.53	
Flow Type	Supercritical	

Roughness Segments

Start Station	End Station	Mannings Coefficient
0+00.00	0+02.17	0.013
0+02.17	0+23.34	0.020
0+23.34	0+24.34	0.130

Natural Channel Points

Station (ft)	Elevation (ft)
0+00.00	0.67
0+00.17	0.00
0+02.17	0.13
0+23.34	0.55
0+24.17	0.57
0+24.34	1.05

Cross Section
Cross Section for Irregular Channel

Project Description	
Worksheet	ARTERIAL' STREET CAPACITY CALCULATION (→ UNSER BLVD. (WEST SIDE)
Flow Element	Irregular Channel
Method	Manning's Formula
Solve For	Discharge

Section Data		
Mannings Coefficient	0.018	
Slope	1.6000	%
Water Surface Elevation	0.67	ft
Elevation Range	0.00 to 1.05	
Discharge	42.65	cfs