CITY OF ALBUQUERQUE

Planning Department
Brennon Williams, Director

Mayor Timothy M. Keller

May 18, 2021

Ronald Bohannan, P.E.
Tierra West, LLC
5571 Midway Park Place NE
Albuquerque, NM 87109

RE: Southern Tire Mart
Grading and Drainage Plan
Engineer's Stamp Date: 03/29/21
Hydrology File: K09D047

Dear Mr. Bohannan:

Based upon the information provided in your submittal received 05/04/2021, the Grading & Drainage Plan is approved for Building Permit, Work Order, and for action by the DRB on Site Plan for Building Permit.

Please attach a copy of this approved plan in the construction sets for Building Permit processing along with a copy of this letter. Prior to approval in support of Permanent Release of Occupancy by Hydrology, Engineer Certification per the DPM checklist will be required.

As a reminder, if the project total area of disturbance (including the staging area and any work within the adjacent Right-of-Way) is 1 acre or more, then an Erosion and Sediment Control (ESC) Plan and Owner's certified Notice of Intent (NOI) is required to be submitted to the Stormwater Quality Engineer (Doug Hughes, PE, jhughes@cabq.gov, 924-3420) 14 days prior to any earth disturbance.

If you have any questions, please contact me at 924-3995 or rbrissette@cabq.gov.

Sincerely,

Renée C. Brissette

Renée C. Brissette, P.E. CFM
Senior Engineer, Hydrology
Planning Department

PO Box 1293

Albuquerque

NM 87103

www.cabq.gov

City of Albuquerque

Planning Department

Development & Building Services Division

DRAINAGE AND TRANSPORTATION INFORMATION SHEET (REV 6/2018)

Project Title: Southern Tire Mart Building Permit #: _____ Hydrology File #: _____

DRB#: _____ EPC#: _____ Work Order#: _____

Legal Description: Tract 2B Avalon Subdivision Unit 5 Albuquerque NM

City Address: Daytona Rd NW Albuquerque NM

Applicant: Tierra West, LLC Contact: Vince Carrica

Address: 5571 Midway Park Place NE Albuquerque NM 87109

Phone#: 505-858-3100 Fax#: 505-858-1118 E-mail: vcarrica@tierrawestllc.com

Other Contact: _____ Contact: _____

Address: _____

Phone#: _____ Fax#: _____ E-mail: _____

TYPE OF DEVELOPMENT: ____ PLAT (# of lots) ____ RESIDENCE __X__ DRB SITE ___ ADMIN SITE

IS THIS A RESUBMITTAL? ____ Yes __X__ No

DEPARTMENT ____ TRANSPORTATION __X__ HYDROLOGY/DRAINAGE

Check all that Apply:

TYPE OF SUBMITTAL:

____ ENGINEER/ARCHITECT CERTIFICATION
____ PAD CERTIFICATION
____ CONCEPTUAL G & D PLAN
__X__ GRADING PLAN
__X__ DRAINAGE REPORT
____ DRAINAGE MASTER PLAN
____ FLOODPLAIN DEVELOPMENT PERMIT APPLIC
____ ELEVATION CERTIFICATE
____ CLOMR/LOMR
____ TRAFFIC CIRCULATION LAYOUT (TCL)
____ TRAFFIC IMPACT STUDY (TIS)
____ STREET LIGHT LAYOUT
____ OTHER (SPECIFY) _____
____ PRE-DESIGN MEETING?

TYPE OF APPROVAL/ACCEPTANCE SOUGHT:

__X__ BUILDING PERMIT APPROVAL
____ CERTIFICATE OF OCCUPANCY

____ PRELIMINARY PLAT APPROVAL
____ SITE PLAN FOR SUB'D APPROVAL
__X__ SITE PLAN FOR BLDG. PERMIT APPROVAL
____ FINAL PLAT APPROVAL

____ SIA/ RELEASE OF FINANCIAL GUARANTEE
____ FOUNDATION PERMIT APPROVAL
____ GRADING PERMIT APPROVAL
____ SO-19 APPROVAL
____ PAVING PERMIT APPROVAL
____ GRADING/ PAD CERTIFICATION
____ WORK ORDER APPROVAL
____ CLOMR/LOMR
____ FLOODPLAIN DEVELOPMENT PERMIT
____ OTHER (SPECIFY) _____

DATE SUBMITTED: 3/30/2021 By: Vince Carrica

COA STAFF: ELECTRONIC SUBMITTAL RECEIVED: _____

FEE PAID: _____

DRAINAGE REPORT

For

TRACT 2B AVALON SUBDIVISION UNIT 5
ALBUQUERQUE, NEW MEXICO

Prepared by

Tierra West, LLC
5571 Midway Park Place NE
Albuquerque, New Mexico 87109

Prepared for

Southern Tire Mart
Albuquerque, NM

March 29, 2021

City of Albuquerque
Planning Department
Development Review Services
HYDROLOGY SECTION
APPROVED
DATE: 05/18/21
BY: Renée C. Brissette
HydroTrans # K09D047

THE APPROVAL OF THESE PLANS/REPORT SHALL NOT BE CONSTRUED TO PERMIT VIOLATIONS OF ANY CITY ORDINANCE OR STATE LAW, AND SHALL NOT PREVENT THE CITY OF ALBUQUERQUE FROM REQUIRING CORRECTION, OR ERROR OR DIMENSIONS IN PLANS, SPECIFICATIONS, OR CONSTRUCTIONS. SUCH APPROVED PLANS SHALL NOT BE CHANGED, MODIFIED OR ALTERED WITHOUT AUTHORIZATION.

RONALD R BOHANNAN, PE #7868

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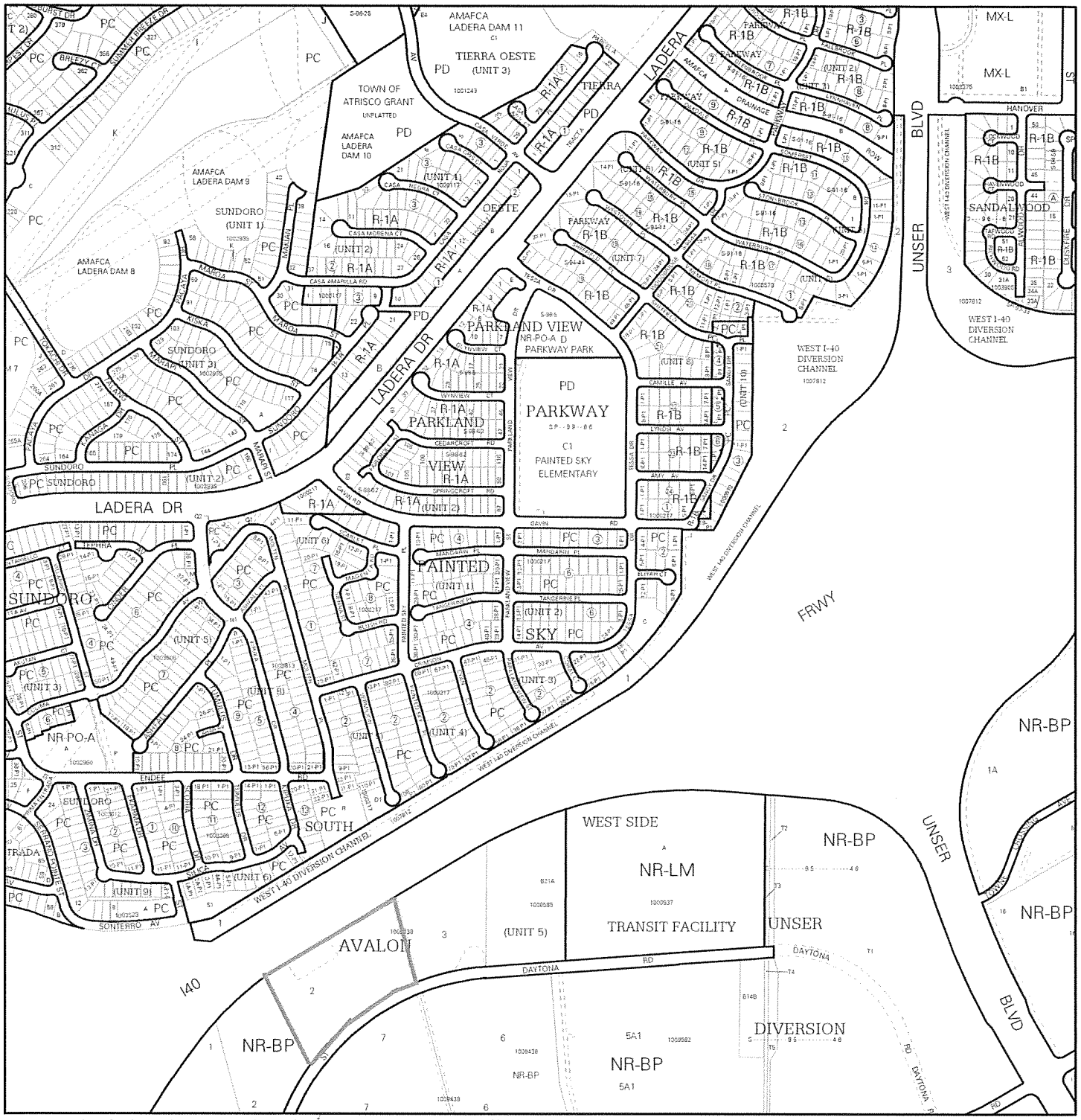


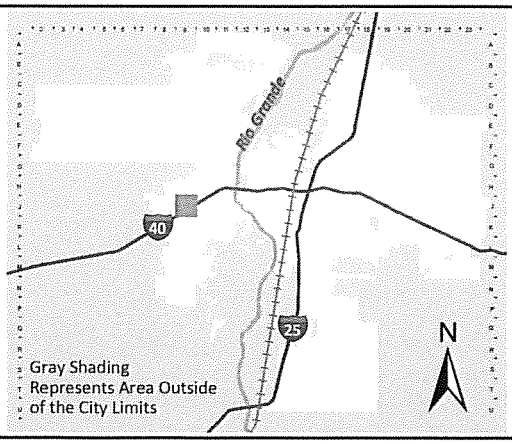
For more details about the Integrated Development Ordinance visit: http://www.cabq.gov/planning/codes-policies-regulations/integrated-development-ordinance

IDO Zone Atlas May 2018

Albuquerque Geographic Information System

IDO Zoning information as of May 17, 2018

The Zone Districts and Overlay Zones are established by the Integrated Development Ordinance (IDO).



Zone Atlas Page:

J-09-Z

- Easement
- Escarpment
- Petroglyph National Monument
- Areas Outside of City Limits
- Airport Protection Overlay (APO) Zone
- Character Protection Overlay (CPO) Zone
- Historic Protection Overlay (HPO) Zone
- View Protection Overlay (VPO) Zone

Feet: 0 250 500 1,000

LOCATION

The proposed commercial development is located off Daytona Rd south of Interstate 40 and west of Unser Blvd in southwest Albuquerque. It is comprised of approximately 5.8255 acres zoned NRBP. This report represents a drainage management and grading plan for approval by the City of Albuquerque, for grading and Building Permit submittal.

DRAINAGE BASIN DESIGNATION

The drainage basins for proposed conditions are as indicated on the BASIN MAP included in this report. The site is broken into ten onsite drainage basins.

EXISTING DRAINGE CONDITIONS

The site is currently vacant with the exception of a roadway turnaround. It is a part of Master Drainage Report for Westpoint 40 (Avalon Subdivision Unit 5) by BHI dated July 2019. It drains predominantly northwest to southeast and is elevated approximately twenty feet above the developed property to the east. An existing drainage way and desilting pond exists in the northeast corner of the site that channels runoff from the Interstate to an existing storm drain in the adjacent property. Runoff from north of the upland is captured in the AMAFCA North I-40 Diversion system. That diversion system removes the bulk of the contributing area to the two arroyos that run diagonally through the site from northwest to south east. Runoff from the existing site is conveyed to the Unser Diversion Pond system via street flow and an existing storm drain in Daytona Rd.

FIRM MAP

AMAFCA completed a LOMR to adjust the FEMA mapping for this area (LOMR 17-06-0267P Effective 11/28/2016). The site is no longer located in a designated Flood Hazard Zone Map No. 35001C0328J dated 11/4/2016.

National Flood Hazard Layer FIRMette

FEMA

Feet 1:6,000

0 250 500 1,000 1,500 2,000

Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

SPECIAL FLOOD HAZARD AREAS	Without Base Flood Elevation (BFE) Zone A, V, A99
	With BFE or Depth
	Regulatory Floodway Zone AE, AO, AH, VE, AR
OTHER AREAS OF FLOOD HAZARD	0.2% Annual Chance Flood Hazard, Area of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile Zone X
	Future Conditions 1% Annual Chance Flood Hazard Zone X
	Area with Reduced Flood Risk due to Levee. See Notes. Zone X
	Area with Flood Risk due to Levee Zone D
OTHER AREAS	NO SCREEN Area of Minimal Flood Hazard Zone X
	Effective LOMRs
	Area of Undetermined Flood Hazard Zone
GENERAL STRUCTURES	Channel, Culvert, or Storm Sewer
	Levee, Dike, or Floodwall
OTHER FEATURES	20.2 / 17.5 Cross Sections with 1% Annual Chance / Water Surface Elevation
	Coastal Transect
	Base Flood Elevation Line (BFE)
	Limit of Study
	Jurisdiction Boundary
	Coastal Transect Baseline
	Profile Baseline
	Hydrographic Feature
MAP PANELS	Digital Data Available
	No Digital Data Available
	Unmapped

N

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The base map shown complies with FEMA's base map accuracy standards

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on 3/27/2018 at 10:17:25 AM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: base map imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.

DESIGN-CRITERIA

The drainage plan presented in this report was prepared in accordance with the City of Albuquerque Drainage Ordinances and Chapter 22 of the Development Process Manual DPM. The hydrological analysis is based on the 100-year frequency, 6-hour duration storm, as Represented in Section 22, Part A, Hydrology, of the Development Process Manual. The plan will also include retention of the first flush in on-site landscaped areas. See attached Weighted E Table for excess precipitation values calculated for this site.

DEVELOPED-DRAINAGE CONDITIONS

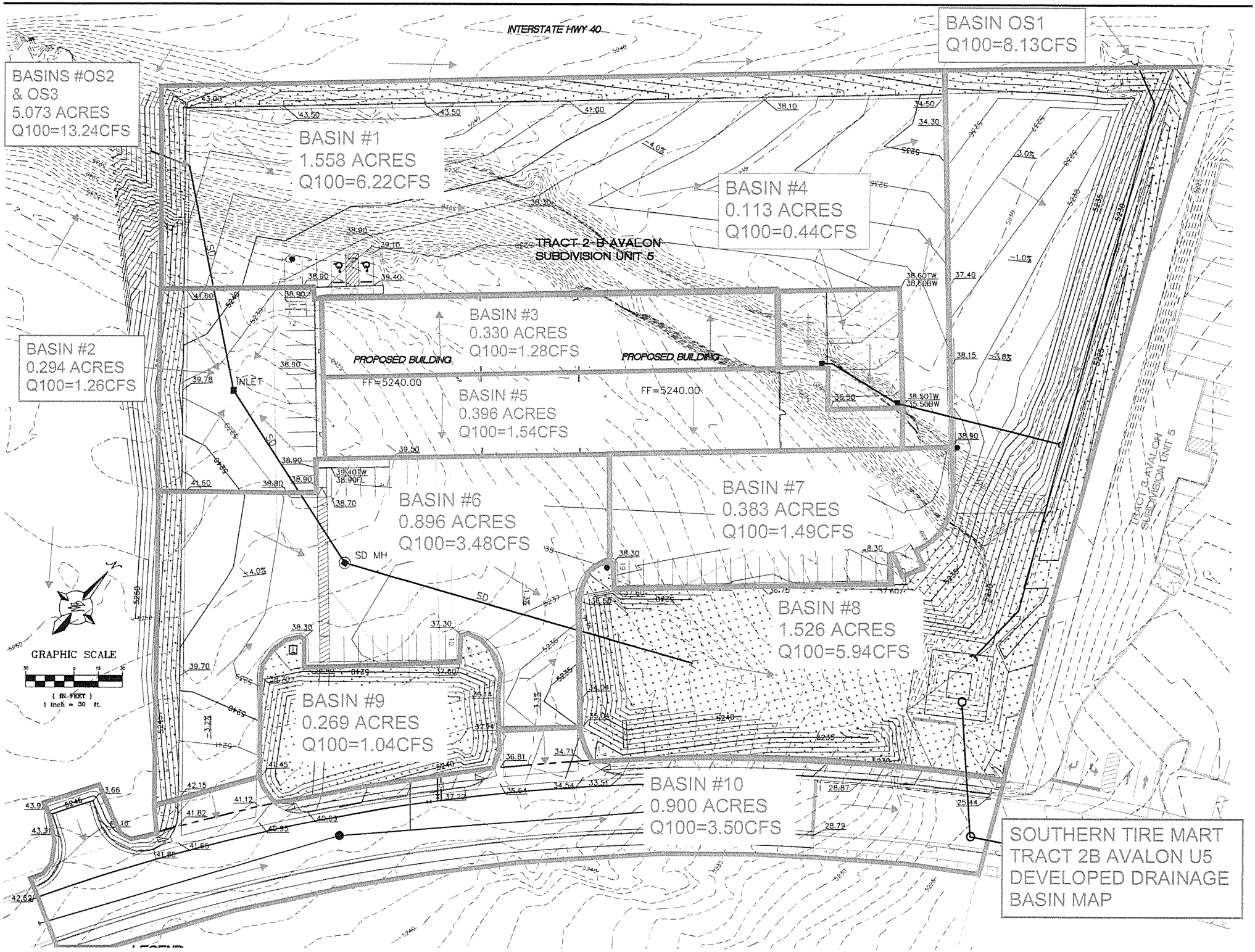
The site is proposed to be developed with a single user, Southern Tire Mart. In coordination with the landowner to the west, a drainage culvert will be constructed to intercept undeveloped upland flows to the west prior to them entering the site. Also, no offsite drainage will enter the site from the north, east and south with the exception of the runoff from a portion of the adjacent Interstate 40 right of way, which will be conveyed in a storm drain culvert from the northeast corner of the site to the proposed water quality pond on site. The onsite water quality pond will free discharge to Daytona roadway and existing storm drain and will be conveyed to the Unser Diversion Pond system located east of the site. Flows will be conveyed through the site via surface flows and a small onsite storm drains. Runoff will then flow to a shallow first flush retention pond before existing the site.

Refer to enclosed Weighted E computation spreadsheet for existing and developed. Storm drain capacities are listed in a table in the appendix.

SUMMARY

The proposed grading and drainage plan for the proposed development of the existing undeveloped property includes surface flows and an onsite storm drain to convey runoff to a water quality pond. The flows will be routed through first flush pond located in landscaped areas prior to the flows exiting the site to the Daytona right of way. Once in the right of way, flows will be routed to the Unser Diversion Pond system via street flow and an existing storm drain

channel located in Daytona Rd. The storm drain capacity downstream of the site is sufficient to carry the ultimate developed runoff of 52.6 cfs outlined in the I-40 South and Unser Diversion Mini DMP (see attached Plate 2 from the plan).


INTERSTATE HWY 40
BASIN OS1
Q100=8.13CFS
BASINS #OS2
& OS3
5.073 ACRES
Q100=13.24CFS
BASIN #1
1.558 ACRES
Q100=6.22CFS
BASIN #4
0.113 ACRES
Q100=0.44CFS
TRACT 2-B AVALON
SUBDIVISION UNIT 5
BASIN #2
0.294 ACRES
Q100=1.26CFS
BASIN #3
0.330 ACRES
Q100=1.28CFS
PROPOSED BUILDING
PROPOSED BUILDING
FF=5240.00
FF=5240.00
BASIN #5
0.396 ACRES
Q100=1.54CFS
INLET
SD
SD MH
BASIN #6
0.896 ACRES
Q100=3.48CFS
BASIN #7
0.383 ACRES
Q100=1.49CFS
BASIN #8
1.526 ACRES
Q100=5.94CFS
BASIN #9
0.269 ACRES
Q100=1.04CFS
BASIN #10
0.900 ACRES
Q100=3.50CFS
GRAPHIC SCALE
(IN FEET)
1 inch = 30 ft.
TRACT 3 AVALON
SUBDIVISION UNIT 5
SOUTHERN TIRE MART
TRACT 2B AVALON U5
DEVELOPED DRAINAGE
BASIN MAP

Southern Tire Mart Upland Basins

Write a description for your map.

Weighted E Method

Zone #1

Developed Basins

												100-Year		
Basin	Area (sf)	Area (acres)	Area (sq miles)	Treatment A		Treatment B		Treatment C		Treatment D		Weighted E (ac-ft)	Volume (ac-ft)	Flow cfs
				%	(acres)	%	(acres)	%	(acres)	%	(acres)			
1	67855.00	1.558	0.00243	0%	0	13%	0.203	0%	0	87%	1.355	1.801	0.234	6.33
2	12799.00	0.294	0.00046	0%	0	14%	0.041	0%	0	86%	0.253	1.788	0.044	1.19
3	14356.00	0.330	0.00051	0%	0	0%	0.000	0%	0	100%	0.330	1.970	0.054	1.44
4	4938.00	0.113	0.00018	0%	0	0%	0.000	0%	0	100%	0.113	1.970	0.019	0.50
5	17256.00	0.396	0.00062	0%	0	0%	0.000	0%	0	100%	0.396	1.970	0.065	1.73
6	39017.00	0.896	0.00140	0%	0	7%	0.063	0%	0	93%	0.833	1.879	0.140	3.77
7	16702.00	0.383	0.00060	0%	0	0%	0.000	0%	0	100%	0.383	1.970	0.063	1.68
8	66468.00	1.526	0.00238	0%	0	86%	1.312	14%	0.213625	0%	0.000	0.715	0.091	3.28
9	11698.00	0.269	0.00042	0%	0	100%	0.269	0%	0	0%	0.000	0.670	0.015	0.55
10	39192.00	0.900	0.00141	0%	0	0%	0.000	32%	0.287912	68%	0.612	1.656	0.124	3.50
OS-1	145926.00	3.350	0.00523	0%	0	83%	2.781	0%	0	17%	0.570	0.891	0.249	8.13
OS-2	163088.00	3.744	0.00585	0%	0	66%	2.471	0%	0	34%	1.273	1.112	0.347	10.58
OS-3	57891.00	1.329	0.00208	77%	1.023326	0%	0.000	0%	0	23%	0.306	0.792	0.088	2.66
Total	657186.00	15.087	0.02357								6.423		1.532	45.32

Equations:

FIRST FLUSH VOLUME = 9,793 CU.FT.

Weighted E = Ea*Aa + Eb*Ab + Ec*Ac + Ed*Ad / (Total Area)

Volume = Weighted D * Total Area

Flow = Qa * Aa + Qb * Ab + Qc * Ac + Qd * Ad

VOLUME CALCULATIONS

SOUTHERN TIRE MART

DETENTION POND W/ 1ST FLUSH RETENTION

Ab - Bottom Of The Pond Surface Area
At - Top Of The Pond Surface Area
D - Water Depth
Dt - Total Pond Depth
C - Change In Surface Area / Water Depth

Volume = $Ab * D + 0.5 * C * D^2$
C = (At - Ab) / Dt

Ab =	3,745.00	B.O.P.=	5222
At =	3,745.00	T.O.P. =	5227.00
Dt =	5.00		
C =	0.00		
B Elev. =	5,222.00		

ACTUAL ELEV.	DEPTH (FT)	VOLUME (AC-FT)	Q (CFS)
5222.00	0	0	0.000
5223.00	1.00	0.0960	0.000
5224.00	2.00	0.2272	0.000
5225.00	3.00	0.4030	0.000
5226.00	4.00	0.6406	0.000
5227.00	5.00	0.9682	5.090
5228.00	6.00	1.4080	6.017
5229.00	7.00	1.4080	10.421

Orifice Equation
Q = CA SQRT(2gH)

C =	0.6	
Diameter (in)	18	
Area (ft^2)=	1.767145868	
g =	32.2	
H (Ft) =	Depth of water above center of orifice	
Q (CFS)=	Flow	

```
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           RUN DATE (MON/DAY/YR) = 03/29/2021
           START TIME (HR:MIN:SEC) = 14:29:02     USER NO.=
AHYMO_Temp_User:20122010
           INPUT FILE = C:\Users\Vince\Desktop\HYMO STM.txt


    ************************************************************
    *                  SOUTHERN TIRE MART                      *
    ************************************************************
    * 100-YEAR, 24-HR STORM  (UNDER PROPOSED CONDITIONS) W/ routing *
    ************************************************************
    START               TIME=0.0
    *
    *
    RAINFALL            TYPE=2 RAIN QUARTER=0.0 IN
                        RAIN ONE=1.87 IN RAIN SIX=2.20 IN
                        RAIN DAY=2.66 IN DT=0.05 HR

                   24-HOUR RAINFALL DIST. - BASED ON NOAA ATLAS 14 FOR CONVECTIVE
AREAS (NM & AZ) - D1
                   DT =   0.050000 HOURS       END TIME =    24.000002 HOURS
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                     0.0197  0.0264  0.0336  0.0412  0.0494  0.0578  0.0664
                     0.0753  0.0844  0.0946  0.1052  0.1168  0.1387  0.1657
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                    2.6280  2.6293  2.6306  2.6319  2.6332  2.6344  2.6357
                    2.6370  2.6383  2.6395  2.6408  2.6421  2.6434  2.6447
                    2.6459  2.6472  2.6485  2.6498  2.6510  2.6523  2.6536
                    2.6549  2.6562  2.6574  2.6587  2.6600

*
*
*BASIN 1
*
COMPUTE NM HYD      ID=1 HYD NO=100.1 AREA=0.00243 SQ MI
                    PER A=0.00 PER B=13.00 PER C=0.00 PER D=87.00
                    TP=-0.1333 HR MASS RAINFALL=-1
```

```
        K =  0.072649HR    TP =  0.133300HR    K/TP RATIO =  0.545000     SHAPE
CONSTANT, N =  7.106428
        UNIT PEAK =   8.3466    CFS   UNIT VOLUME =   0.9978       B =   526.28
 P60 = 1.8700
        AREA =     0.002114 SQ MI    IA =   0.10000 INCHES    INF =   0.04000
INCHES PER HOUR
        RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
0.050000


        K =  0.130992HR    TP =  0.133300HR    K/TP RATIO =  0.982685     SHAPE
CONSTANT, N =  3.593298
        UNIT PEAK =  0.77513    CFS   UNIT VOLUME =   0.9848       B =   327.08
 P60 = 1.8700
        AREA =     0.000316 SQ MI    IA =   0.50000 INCHES    INF =   1.25000
INCHES PER HOUR
        RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
0.050000

    PRINT HYD           ID=1 CODE=1

                                      PARTIAL HYDROGRAPH   100.10


       RUNOFF VOLUME =     2.21066 INCHES     =       0.2865 ACRE-FEET
       PEAK DISCHARGE RATE =       6.56 CFS  AT   1.500 HOURS   BASIN AREA =
0.0024 SQ. MI.



    *
    *
    *BASIN 2
    *
    COMPUTE NM HYD      ID=2 HYD NO=100.2 AREA=0.00046 SQ MI
                        PER A=0.00 PER B=14.00 PER C=0.00 PER D=86.00
                        TP=-0.1333 HR MASS RAINFALL=-1

        K =  0.072649HR    TP =  0.133300HR    K/TP RATIO =  0.545000     SHAPE
CONSTANT, N =  7.106428
        UNIT PEAK =   1.5618    CFS   UNIT VOLUME =   0.9928       B =   526.28
 P60 = 1.8700
        AREA =     0.000396 SQ MI    IA =   0.10000 INCHES    INF =   0.04000
INCHES PER HOUR
        RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
0.050000


        K =  0.130992HR    TP =  0.133300HR    K/TP RATIO =  0.982685     SHAPE
```

```
CONSTANT, N =  3.593298
         UNIT PEAK =  0.15802    CFS   UNIT VOLUME =   0.9156       B =   327.08
 P60 = 1.8700
         AREA =     0.000064 SQ MI    IA =   0.50000 INCHES    INF =   1.25000
INCHES PER HOUR
         RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
0.050000

    PRINT HYD           ID=2 CODE=1

                                      PARTIAL HYDROGRAPH   100.20


        RUNOFF VOLUME =     2.19489 INCHES     =       0.0538 ACRE-FEET
        PEAK DISCHARGE RATE =       1.25 CFS  AT   1.500 HOURS   BASIN AREA =
0.0005 SQ. MI.



    *
    *
    *BASIN 3
    *
    COMPUTE NM HYD      ID=3 HYD NO=100.3 AREA=0.00051 SQ MI
                        PER A=0.00 PER B=0.00 PER C=0.0 PER D=100.00
                        TP=-0.1333 HR MASS RAINFALL=-1

         K =  0.072649HR    TP =  0.133300HR    K/TP RATIO =  0.545000     SHAPE
CONSTANT, N =  7.106428
         UNIT PEAK =   2.0135    CFS   UNIT VOLUME =   0.9941       B =   526.28
 P60 = 1.8700
         AREA =     0.000510 SQ MI    IA =   0.10000 INCHES    INF =   0.04000
INCHES PER HOUR
         RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
0.050000

    PRINT HYD           ID=3 CODE=1

                                      PARTIAL HYDROGRAPH   100.30


        RUNOFF VOLUME =     2.41566 INCHES     =       0.0657 ACRE-FEET
        PEAK DISCHARGE RATE =       1.47 CFS  AT   1.500 HOURS   BASIN AREA =
0.0005 SQ. MI.



    *
    *
    *BASIN 4
```

```
    *
    COMPUTE NM HYD       ID=4 HYD NO=100.4 AREA=0.00018 SQ MI
                         PER A=0.00 PER B=0.00 PER C=0.0 PER D=100.00
                         TP=-0.1333 HR MASS RAINFALL=-1

         K =  0.072649HR    TP =  0.133300HR    K/TP RATIO =  0.545000     SHAPE
CONSTANT, N =  7.106428
         UNIT PEAK =  0.71065    CFS   UNIT VOLUME =   0.9832       B =   526.28
 P60 = 1.8700
         AREA =     0.000180 SQ MI    IA =   0.10000 INCHES    INF =   0.04000
INCHES PER HOUR
         RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
0.050000

    PRINT HYD            ID=4 CODE=1

                                       PARTIAL HYDROGRAPH   100.40


        RUNOFF VOLUME =     2.41566 INCHES      =       0.0232 ACRE-FEET
        PEAK DISCHARGE RATE =       0.52 CFS  AT   1.500 HOURS   BASIN AREA =
0.0002 SQ. MI.



    *
    *
    *BASIN 5
    *
    COMPUTE NM HYD       ID=5 HYD NO=100.5 AREA=0.00062 SQ MI
                         PER A=0.00 PER B=0.00 PER C=0.0 PER D=100.00
                         TP=-0.1333 HR MASS RAINFALL=-1

         K =  0.072649HR    TP =  0.133300HR    K/TP RATIO =  0.545000     SHAPE
CONSTANT, N =  7.106428
         UNIT PEAK =   2.4478    CFS   UNIT VOLUME =   0.9951       B =   526.28
 P60 = 1.8700
         AREA =     0.000620 SQ MI    IA =   0.10000 INCHES    INF =   0.04000
INCHES PER HOUR
         RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
0.050000

    PRINT HYD            ID=5 CODE=1

                                       PARTIAL HYDROGRAPH   100.50


        RUNOFF VOLUME =     2.41566 INCHES      =       0.0799 ACRE-FEET
        PEAK DISCHARGE RATE =       1.78 CFS  AT   1.500 HOURS   BASIN AREA =
0.0006 SQ. MI.
```

```
    *
    *
    *BASIN 6
    *
    COMPUTE NM HYD       ID=6 HYD NO=100.6 AREA=0.00140 SQ MI
                         PER A=0.00 PER B=7.00 PER C=0.00 PER D=93.00
                         TP=-0.1333 HR MASS RAINFALL=-1

         K =  0.072649HR    TP =  0.133300HR    K/TP RATIO =  0.545000     SHAPE
CONSTANT, N =  7.106428
         UNIT PEAK =   5.1404    CFS   UNIT VOLUME =   0.9971       B =   526.28
 P60 = 1.8700
         AREA =     0.001302 SQ MI    IA =   0.10000 INCHES    INF =   0.04000
INCHES PER HOUR
         RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
0.050000


         K =  0.130992HR    TP =  0.133300HR    K/TP RATIO =  0.982685     SHAPE
CONSTANT, N =  3.593298
         UNIT PEAK =  0.24047    CFS   UNIT VOLUME =   0.9497       B =   327.08
 P60 = 1.8700
         AREA =     0.000098 SQ MI    IA =   0.50000 INCHES    INF =   1.25000
INCHES PER HOUR
         RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
0.050000

    PRINT HYD            ID=6 CODE=1

                                       PARTIAL HYDROGRAPH   100.60


        RUNOFF VOLUME =     2.30527 INCHES     =       0.1721 ACRE-FEET
        PEAK DISCHARGE RATE =       3.89 CFS  AT   1.500 HOURS   BASIN AREA =
0.0014 SQ. MI.



    *
    *
    *BASIN 7
    *
    COMPUTE NM HYD       ID=7 HYD NO=100.7 AREA=0.00060 SQ MI
                         PER A=0.00 PER B=0.00 PER C=0.00 PER D=100.00
                         TP=-0.1333 HR MASS RAINFALL=-1

         K =  0.072649HR    TP =  0.133300HR    K/TP RATIO =  0.545000     SHAPE
```

```
CONSTANT, N =  7.106428
         UNIT PEAK =    2.3688    CFS    UNIT VOLUME =    0.9951       B =    526.28
 P60 = 1.8700
         AREA =     0.000600 SQ MI    IA =    0.10000 INCHES    INF =    0.04000
INCHES PER HOUR
         RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
0.050000

    PRINT HYD           ID=7 CODE=1

                                      PARTIAL HYDROGRAPH   100.70


        RUNOFF VOLUME =     2.41566 INCHES     =       0.0773 ACRE-FEET
        PEAK DISCHARGE RATE =       1.72 CFS  AT   1.500 HOURS   BASIN AREA =
0.0006 SQ. MI.



    *
    *
    *BASIN 8
    *
    COMPUTE NM HYD      ID=8 HYD NO=100.8 AREA=0.00238 SQ MI
                        PER A=0.00 PER B=86.00 PER C=14.0 PER D=0.00
                        TP=-0.1333 HR MASS RAINFALL=-1

         K =  0.127474HR    TP =  0.133300HR    K/TP RATIO =  0.956297     SHAPE
CONSTANT, N =  3.695096
         UNIT PEAK =    5.9689    CFS    UNIT VOLUME =    0.9992       B =    334.31
 P60 = 1.8700
         AREA =     0.002380 SQ MI    IA =    0.47900 INCHES    INF =    1.19120
INCHES PER HOUR
         RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
0.050000

    PRINT HYD           ID=8 CODE=1

                                      PARTIAL HYDROGRAPH   100.80


        RUNOFF VOLUME =     0.87123 INCHES     =       0.1106 ACRE-FEET
        PEAK DISCHARGE RATE =       3.84 CFS  AT   1.500 HOURS   BASIN AREA =
0.0024 SQ. MI.



    *
    *
    *BASIN 9
```

```
    *
    COMPUTE NM HYD       ID=9 HYD NO=100.9 AREA=0.00042 SQ MI
                         PER A=0.00 PER B=100.00 PER C=0.0 PER D=0.00
                         TP=-0.1333 HR MASS RAINFALL=-1

         K =  0.130992HR    TP =  0.133300HR    K/TP RATIO =  0.982685     SHAPE
CONSTANT, N =  3.593298
         UNIT PEAK =   1.0306    CFS   UNIT VOLUME =   0.9885       B =   327.08
 P60 = 1.8700
         AREA =     0.000420 SQ MI    IA =   0.50000 INCHES    INF =   1.25000
INCHES PER HOUR
         RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
0.050000

    PRINT HYD            ID=9 CODE=1

                                       PARTIAL HYDROGRAPH   100.90


        RUNOFF VOLUME =     0.83873 INCHES     =       0.0188 ACRE-FEET
        PEAK DISCHARGE RATE =       0.66 CFS  AT   1.500 HOURS   BASIN AREA =
0.0004 SQ. MI.



    *
    *
    *BASIN 10
    *
    COMPUTE NM HYD       ID=10 HYD NO=100.10 AREA=0.00141 SQ MI
                         PER A=0.00 PER B=0.00 PER C=32.0 PER D=68.00
                         TP=-0.1333 HR MASS RAINFALL=-1

         K =  0.072649HR    TP =  0.133300HR    K/TP RATIO =  0.545000     SHAPE
CONSTANT, N =  7.106428
         UNIT PEAK =   3.7854    CFS   UNIT VOLUME =   0.9966       B =   526.28
 P60 = 1.8700
         AREA =     0.000959 SQ MI    IA =   0.10000 INCHES    INF =   0.04000
INCHES PER HOUR
         RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
0.050000


         K =  0.105867HR    TP =  0.133300HR    K/TP RATIO =  0.794199     SHAPE
CONSTANT, N =  4.514592
         UNIT PEAK =   1.3138    CFS   UNIT VOLUME =   0.9928       B =   388.14
 P60 = 1.8700
         AREA =     0.000451 SQ MI    IA =   0.35000 INCHES    INF =   0.83000
INCHES PER HOUR
         RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
```

```
0.050000

    PRINT HYD           ID=9 CODE=1

                                        PARTIAL HYDROGRAPH   100.90


        RUNOFF VOLUME =     0.83873 INCHES     =       0.0188 ACRE-FEET
        PEAK DISCHARGE RATE =       0.66 CFS  AT   1.500 HOURS   BASIN AREA =
0.0004 SQ. MI.



    *
    *
    *BASIN OS-1
    *
    COMPUTE NM HYD      ID=11 HYD NO=100.11 AREA=0.00523 SQ MI
                        PER A=0.00 PER B=83.00 PER C=0.0 PER D=17.00
                        TP=-0.1333 HR MASS RAINFALL=-1

         K =  0.072649HR    TP =  0.133300HR    K/TP RATIO =  0.545000     SHAPE
CONSTANT, N =  7.106428
         UNIT PEAK =   3.5102    CFS   UNIT VOLUME =   0.9959       B =   526.28
 P60 = 1.8700
         AREA =     0.000889 SQ MI    IA =   0.10000 INCHES    INF =   0.04000
INCHES PER HOUR
         RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
0.050000


         K =  0.130992HR    TP =  0.133300HR    K/TP RATIO =  0.982685     SHAPE
CONSTANT, N =  3.593298
         UNIT PEAK =   10.651    CFS   UNIT VOLUME =   0.9999       B =   327.08
 P60 = 1.8700
         AREA =     0.004341 SQ MI    IA =   0.50000 INCHES    INF =   1.25000
INCHES PER HOUR
         RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
0.050000

    PRINT HYD           ID=11 CODE=1

                                        PARTIAL HYDROGRAPH   100.11


        RUNOFF VOLUME =     1.10681 INCHES     =       0.3087 ACRE-FEET
        PEAK DISCHARGE RATE =       9.28 CFS  AT   1.500 HOURS   BASIN AREA =
0.0052 SQ. MI.
```

```
    *
    *
    *BASIN OS-2
    *
    COMPUTE NM HYD      ID=12 HYD NO=100.12 AREA=0.00585 SQ MI
                        PER A=0.00 PER B=66.00 PER C=0.0 PER D=34.00
                        TP=-0.1333 HR MASS RAINFALL=-1

         K =  0.072649HR    TP =  0.133300HR    K/TP RATIO =  0.545000     SHAPE
CONSTANT, N =  7.106428
         UNIT PEAK =   7.8527    CFS   UNIT VOLUME =   0.9978       B =   526.28
 P60 = 1.8700
         AREA =     0.001989 SQ MI    IA =   0.10000 INCHES    INF =   0.04000
INCHES PER HOUR
         RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
0.050000


         K =  0.130992HR    TP =  0.133300HR    K/TP RATIO =  0.982685     SHAPE
CONSTANT, N =  3.593298
         UNIT PEAK =   9.4739    CFS   UNIT VOLUME =   0.9997       B =   327.08
 P60 = 1.8700
         AREA =     0.003861 SQ MI    IA =   0.50000 INCHES    INF =   1.25000
INCHES PER HOUR
         RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
0.050000

    PRINT HYD           ID=12 CODE=1

                                      PARTIAL HYDROGRAPH   100.12


        RUNOFF VOLUME =     1.37488 INCHES     =       0.4290 ACRE-FEET
        PEAK DISCHARGE RATE =      11.69 CFS  AT   1.500 HOURS   BASIN AREA =
0.0059 SQ. MI.



    *
    *
    *BASIN OS-3
    *
    COMPUTE NM HYD      ID=13 HYD NO=100.13 AREA=0.00208 SQ MI
                        PER A=77.00 PER B=0.00 PER C=0.0 PER D=23.00
                        TP=-0.1333 HR MASS RAINFALL=-1

         K =  0.072649HR    TP =  0.133300HR    K/TP RATIO =  0.545000     SHAPE
CONSTANT, N =  7.106428
         UNIT PEAK =   1.8887    CFS   UNIT VOLUME =   0.9941       B =   526.28
```

```
 P60 = 1.8700
         AREA =     0.000478 SQ MI    IA =   0.10000 INCHES    INF =   0.04000
INCHES PER HOUR
         RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
0.050000


         K =  0.163684HR    TP =  0.133300HR    K/TP RATIO =  1.227936     SHAPE
CONSTANT, N =  2.899626
         UNIT PEAK =   3.2866    CFS   UNIT VOLUME =   0.9943       B =   273.54
 P60 = 1.8700
         AREA =     0.001602 SQ MI    IA =   0.65000 INCHES    INF =   1.67000
INCHES PER HOUR
         RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =
0.050000

    PRINT HYD           ID=13 CODE=1

                                      PARTIAL HYDROGRAPH   100.13


        RUNOFF VOLUME =     1.03965 INCHES      =       0.1153 ACRE-FEET
        PEAK DISCHARGE RATE =       3.13 CFS  AT   1.500 HOURS   BASIN AREA =
0.0021 SQ. MI.



    *
    *
    ADD HYD                         ID=20 HYD NO=100.20 ID=1  ID=2

    ADD HYD                         ID=20 HYD NO=100.20 ID=20 ID=3

    ADD HYD                         ID=20 HYD NO=100.20 ID=20 ID=4

    ADD HYD                         ID=20 HYD NO=100.20 ID=20 ID=5

    ADD HYD                         ID=20 HYD NO=100.20 ID=20 ID=6

    ADD HYD                         ID=20 HYD NO=100.20 ID=20 ID=7

    ADD HYD                         ID=20 HYD NO=100.20 ID=20 ID=8

    ADD HYD                         ID=20 HYD NO=100.20 ID=20 ID=9

    ADD HYD                         ID=20 HYD NO=100.20 ID=20 ID=11

    ADD HYD                         ID=20 HYD NO=100.20 ID=20 ID=12

    ADD HYD                         ID=20 HYD NO=100.20 ID=20 ID=13
```

```
*
*
*ROUTE BASIN 1 THRU 9 & OS-1,OS-2 & OS3 THROUGH WATER QUALITY POND
*
*
ROUTE RESERVOIR     ID=55 HYD NO=200.1 INFLOW ID=20 CODE=24
                        OUTFLOW (CFS) STORAGE(AC-FT) ELEVATION(FT)
                                0.000          0.0000             22.00

                                0.010          0.0960             23.00

                                0.020          0.2272             24.00

                                0.030          0.4030             25.00

                                0.040          0.6406             26.00

                                5.090          0.9682             27.00

                                6.017          1.4080             28.00

                                10.421         1.4080             29.00
```

* * * * * * * * * * * * * * * * *

TIME (HRS)	INFLOW (CFS)	ELEV (FEET)	VOLUME (AC-FT)	OUTFLOW (CFS)
0.00	0.00	22.00	0.000	0.00
1.20	4.10	22.49	0.047	0.00
2.40	1.51	27.29	1.098	5.36
3.60	0.07	26.36	0.757	1.84
4.80	0.08	26.08	0.667	0.45
6.00	0.14	26.03	0.650	0.19
7.20	0.15	26.02	0.648	0.16
8.40	0.15	26.02	0.648	0.15
9.60	0.15	26.02	0.648	0.15
10.80	0.15	26.02	0.648	0.15
12.00	0.15	26.02	0.648	0.15
13.20	0.15	26.02	0.648	0.15
14.40	0.15	26.02	0.648	0.15
15.60	0.15	26.02	0.648	0.15
16.80	0.15	26.02	0.648	0.15
18.00	0.15	26.02	0.648	0.15
19.20	0.15	26.02	0.648	0.15
20.40	0.15	26.02	0.648	0.15
21.60	0.15	26.02	0.648	0.15
22.80	0.15	26.02	0.648	0.15

24.00	0.15	26.02	0.648	0.15
25.20	0.00	26.00	0.641	0.04
26.40	0.00	25.98	0.637	0.04
27.60	0.00	25.97	0.633	0.04
28.80	0.00	25.95	0.629	0.04
30.00	0.00	25.93	0.625	0.04
31.20	0.00	25.92	0.621	0.04
32.40	0.00	25.90	0.617	0.04
33.60	0.00	25.88	0.613	0.04
34.80	0.00	25.87	0.609	0.04
36.00	0.00	25.85	0.606	0.04
37.20	0.00	25.84	0.602	0.04
38.40	0.00	25.82	0.598	0.04
39.60	0.00	25.80	0.594	0.04
40.80	0.00	25.79	0.590	0.04
42.00	0.00	25.77	0.587	0.04
43.20	0.00	25.76	0.583	0.04
44.40	0.00	25.74	0.579	0.04
45.60	0.00	25.73	0.576	0.04
46.80	0.00	25.71	0.572	0.04
48.00	0.00	25.70	0.568	0.04
49.20	0.00	25.68	0.565	0.04
50.40	0.00	25.66	0.561	0.04
51.60	0.00	25.65	0.557	0.04
52.80	0.00	25.63	0.554	0.04
54.00	0.00	25.62	0.550	0.04
55.20	0.00	25.60	0.546	0.04
56.40	0.00	25.59	0.543	0.04
57.60	0.00	25.57	0.539	0.04
58.80	0.00	25.56	0.536	0.04
60.00	0.00	25.54	0.532	0.04
61.20	0.00	25.53	0.529	0.04
62.40	0.00	25.51	0.525	0.04
63.60	0.00	25.50	0.522	0.04
64.80	0.00	25.49	0.518	0.03
66.00	0.00	25.47	0.515	0.03

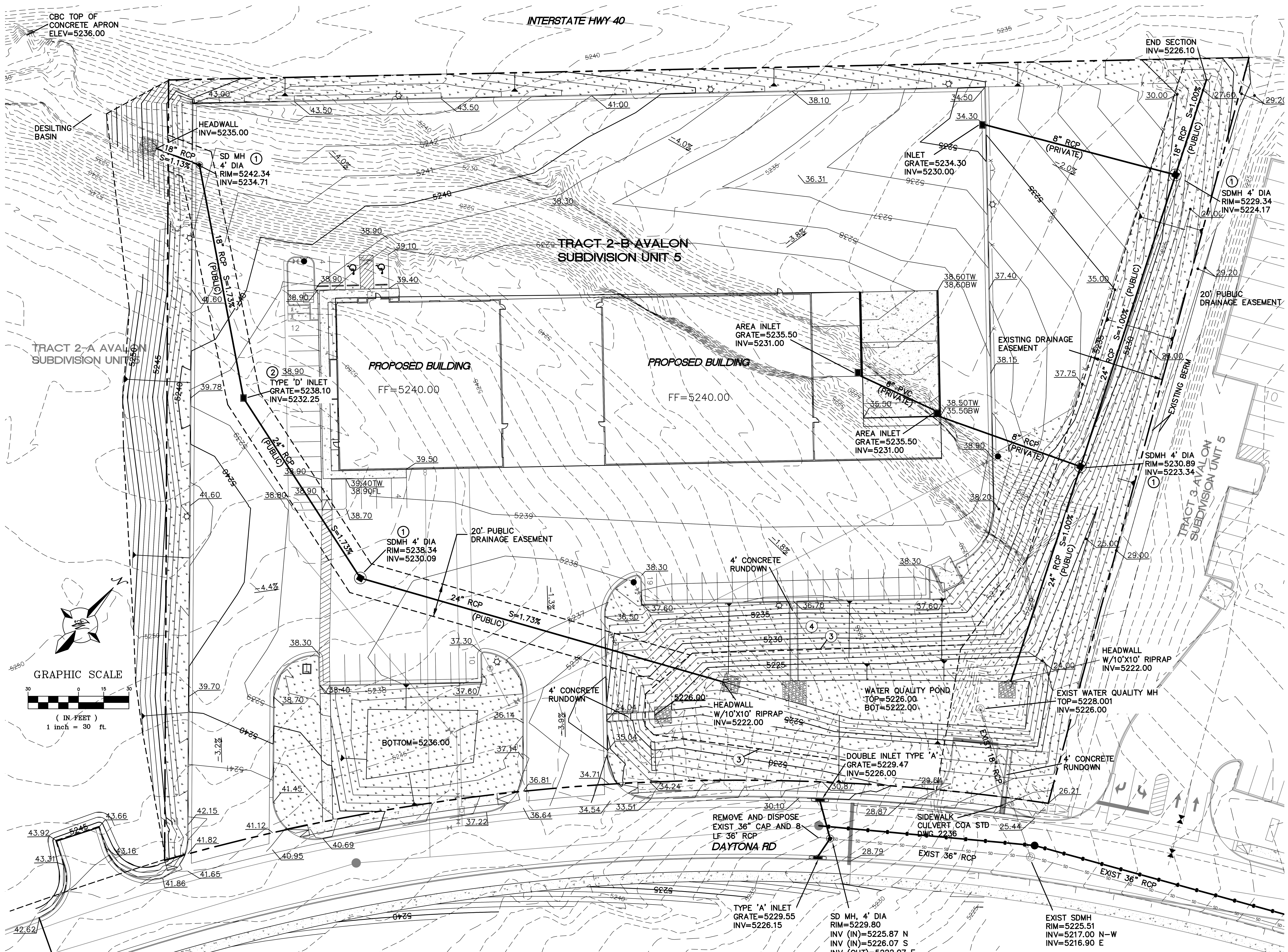
TIME (HRS)	INFLOW (CFS)	ELEV (FEET)	VOLUME (AC-FT)	OUTFLOW (CFS)
67.20	0.00	25.46	0.511	0.03
68.40	0.00	25.44	0.508	0.03
69.60	0.00	25.43	0.505	0.03
70.80	0.00	25.41	0.501	0.03
72.00	0.00	25.40	0.498	0.03
73.20	0.00	25.39	0.494	0.03
74.40	0.00	25.37	0.491	0.03
75.60	0.00	25.36	0.488	0.03
76.80	0.00	25.34	0.484	0.03
78.00	0.00	25.33	0.481	0.03

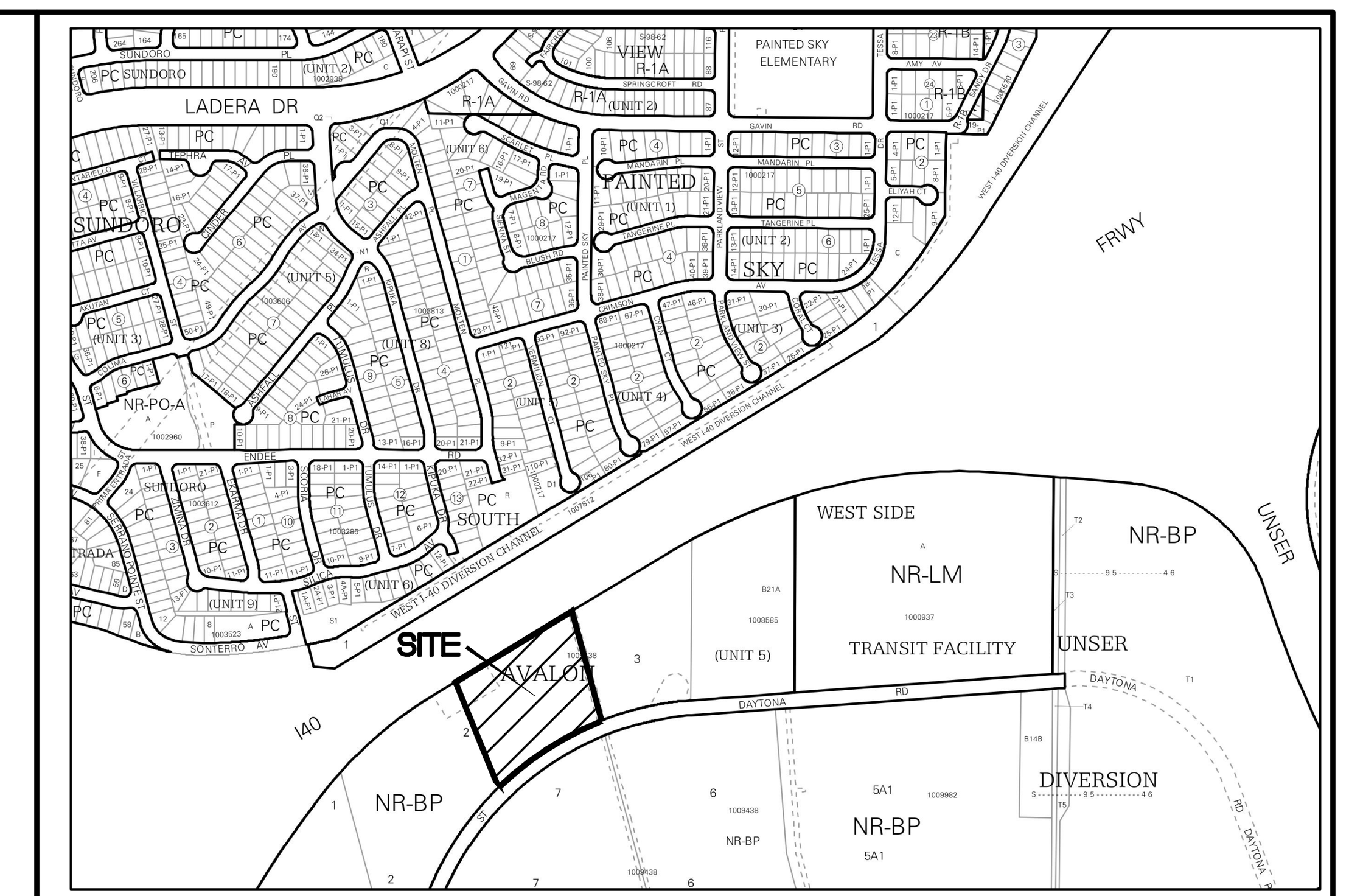
79.20	0.00	25.32	0.478	0.03
80.40	0.00	25.30	0.475	0.03
81.60	0.00	25.29	0.471	0.03
82.80	0.00	25.27	0.468	0.03
84.00	0.00	25.26	0.465	0.03
85.20	0.00	25.25	0.462	0.03
86.40	0.00	25.23	0.458	0.03
87.60	0.00	25.22	0.455	0.03
88.80	0.00	25.21	0.452	0.03
90.00	0.00	25.19	0.449	0.03
91.20	0.00	25.18	0.446	0.03
92.40	0.00	25.17	0.443	0.03
93.60	0.00	25.15	0.439	0.03
94.80	0.00	25.14	0.436	0.03
96.00	0.00	25.13	0.433	0.03
97.20	0.00	25.11	0.430	0.03
98.40	0.00	25.10	0.427	0.03
99.60	0.00	25.09	0.424	0.03
100.80	0.00	25.08	0.421	0.03
102.00	0.00	25.06	0.418	0.03
103.20	0.00	25.05	0.415	0.03
104.40	0.00	25.04	0.412	0.03
105.60	0.00	25.02	0.409	0.03
106.80	0.00	25.01	0.406	0.03
108.00	0.00	25.00	0.403	0.03
109.20	0.00	24.98	0.400	0.03
110.40	0.00	24.97	0.397	0.03
111.60	0.00	24.95	0.394	0.03
112.80	0.00	24.93	0.391	0.03
114.00	0.00	24.92	0.388	0.03
115.20	0.00	24.90	0.385	0.03
116.40	0.00	24.88	0.382	0.03
117.60	0.00	24.87	0.380	0.03
118.80	0.00	24.85	0.377	0.03
120.00	0.00	24.83	0.374	0.03
121.20	0.00	24.82	0.371	0.03
122.40	0.00	24.80	0.368	0.03
123.60	0.00	24.79	0.366	0.03
124.80	0.00	24.77	0.363	0.03
126.00	0.00	24.76	0.360	0.03
127.20	0.00	24.74	0.357	0.03
128.40	0.00	24.72	0.355	0.03
129.60	0.00	24.71	0.352	0.03
130.80	0.00	24.69	0.349	0.03
132.00	0.00	24.68	0.347	0.03
133.20	0.00	24.66	0.344	0.03

TIME (HRS)	INFLOW (CFS)	ELEV (FEET)	VOLUME (AC-FT)	OUTFLOW (CFS)

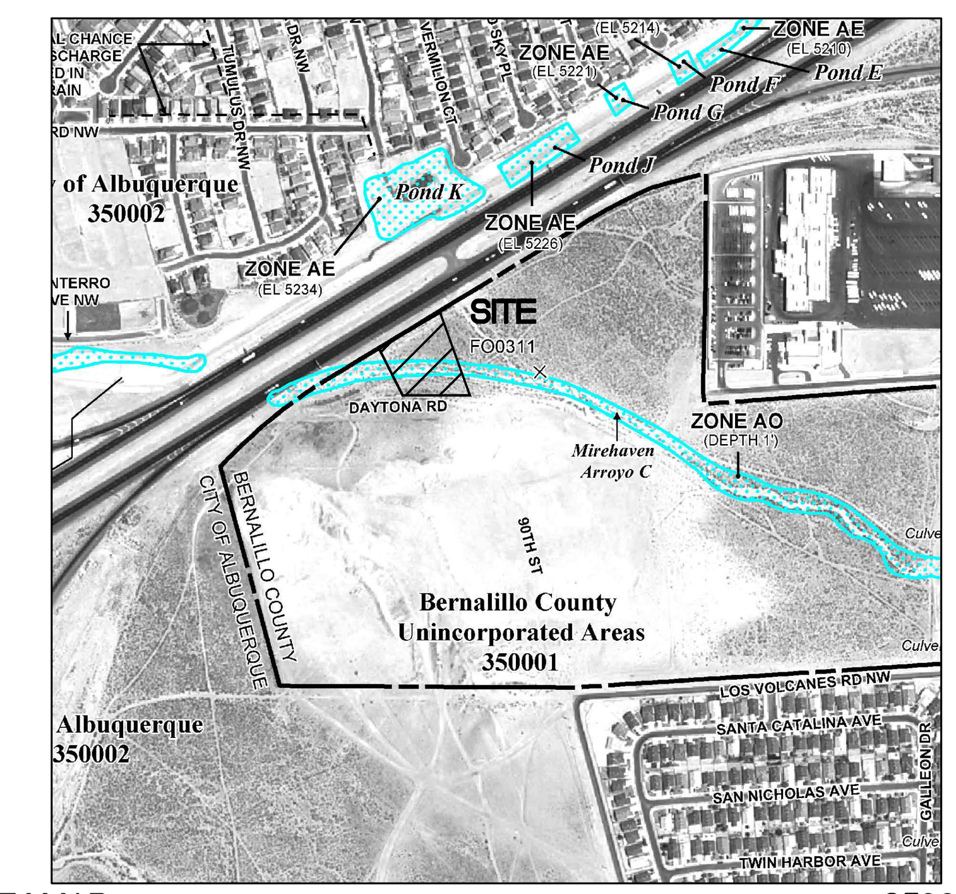
134.40	0.00	24.65	0.341	0.03
135.60	0.00	24.63	0.339	0.03
136.80	0.00	24.62	0.336	0.03
138.00	0.00	24.60	0.333	0.03
139.20	0.00	24.59	0.331	0.03
140.40	0.00	24.58	0.328	0.03
141.60	0.00	24.56	0.326	0.03
142.80	0.00	24.55	0.323	0.03
144.00	0.00	24.53	0.321	0.03
145.20	0.00	24.52	0.318	0.03
146.40	0.00	24.50	0.316	0.03
147.60	0.00	24.49	0.313	0.02
148.80	0.00	24.48	0.311	0.02
150.00	0.00	24.46	0.308	0.02
151.20	0.00	24.45	0.306	0.02
152.40	0.00	24.43	0.303	0.02
153.60	0.00	24.42	0.301	0.02
154.80	0.00	24.41	0.299	0.02
156.00	0.00	24.39	0.296	0.02
157.20	0.00	24.38	0.294	0.02
158.40	0.00	24.37	0.292	0.02
159.60	0.00	24.35	0.289	0.02
160.80	0.00	24.34	0.287	0.02
162.00	0.00	24.33	0.285	0.02
163.20	0.00	24.31	0.282	0.02
164.40	0.00	24.30	0.280	0.02
165.60	0.00	24.29	0.278	0.02
166.80	0.00	24.27	0.275	0.02
168.00	0.00	24.26	0.273	0.02
169.20	0.00	24.25	0.271	0.02
170.40	0.00	24.24	0.269	0.02
171.60	0.00	24.22	0.267	0.02
172.80	0.00	24.21	0.264	0.02
174.00	0.00	24.20	0.262	0.02
175.20	0.00	24.19	0.260	0.02
176.40	0.00	24.17	0.258	0.02
177.60	0.00	24.16	0.256	0.02
178.80	0.00	24.15	0.254	0.02
180.00	0.00	24.14	0.251	0.02
181.20	0.00	24.13	0.249	0.02
182.40	0.00	24.11	0.247	0.02
183.60	0.00	24.10	0.245	0.02
184.80	0.00	24.09	0.243	0.02
186.00	0.00	24.08	0.241	0.02
187.20	0.00	24.07	0.239	0.02
188.40	0.00	24.05	0.237	0.02
189.60	0.00	24.04	0.235	0.02
190.80	0.00	24.03	0.233	0.02
192.00	0.00	24.02	0.231	0.02
193.20	0.00	24.01	0.229	0.02

```
      194.40       0.00     24.00      0.227      0.02
      195.60       0.00     23.98      0.225      0.02
      196.80       0.00     23.97      0.223      0.02
      198.00       0.00     23.95      0.221      0.02
      199.20       0.00     23.94      0.219      0.02
    PEAK DISCHARGE =        5.539 CFS - PEAK OCCURS AT HOUR    2.00
    MAXIMUM WATER SURFACE ELEVATION =       27.484
    MAXIMUM STORAGE =        1.1810 AC-FT      INCREMENTAL TIME=    0.050000HRS

   *
   PRINT HYD           ID=55  CODE=1

                                     PARTIAL HYDROGRAPH   200.10


       RUNOFF VOLUME =     1.28866 INCHES     =       1.5230 ACRE-FEET
       PEAK DISCHARGE RATE =       5.54 CFS  AT   2.000 HOURS   BASIN AREA =
0.0222 SQ. MI.



   *
   *
   ADD HYD                         ID=60 HYD NO=100.60 ID=10 ID=55

   *
   *TOTAL RUNOFF FROM SITE AND FRONTING ROADWAY
   PRINT HYD           ID=60  CODE=1

                                     PARTIAL HYDROGRAPH   100.60


       RUNOFF VOLUME =     1.33061 INCHES     =       1.6726 ACRE-FEET
       PEAK DISCHARGE RATE =       7.19 CFS  AT   1.650 HOURS   BASIN AREA =
0.0236 SQ. MI.



   *
   *
   FINISH

       NORMAL PROGRAM FINISH          END TIME (HR:MIN:SEC) = 14:29:02
```





VICINITY MAP J-09Z



VICINITY MAP 35001C0328H

KEYED NOTES

1. SD MH 4' DIA PER COA STD DWG 2101
2. SINGLE TYPE 'D' STORM INLET PER COA STD DWG 2206
3. DRAINAGE EASEMENT
4. OERSIZE GRAVEL MULCH W/SEEDING

City of Albuquerque
Planning Department
Development Review Services
HYDROLOGY SECTION
APPROVED
DATE: 05/18/21
BY: [signature]
HydroTrans #: K09D047

THE APPROVAL OF THESE PLANS/REPORT SHALL NOT BE CONSTRUED TO PERMIT VIOLATIONS OF ANY CITY ORDINANCE OR STATE LAW, AND SHALL NOT PREVENT THE CITY OF ALBUQUERQUE FROM REQUIRING CORRECTION OR ERASURE OF ERRORS IN PLANS, SPECIFICATIONS, OR CONSTRUCTION. SUCH APPROVED PLANS SHALL NOT BE CHANGED, MODIFIED OR ALTERED WITHOUT AUTHORIZATION.

EROSION CONTROL NOTES:

1. CONTRACTOR IS RESPONSIBLE FOR OBTAINING A TOPSOIL DISTURBANCE PERMIT PRIOR TO BEGINNING WORK.
2. CONTRACTOR IS RESPONSIBLE FOR MAINTAINING RUN-OFF ON SITE DURING CONSTRUCTION.
3. CONTRACTOR IS RESPONSIBLE FOR CLEANING ALL SEDIMENT THAT GETS INTO EXISTING RIGHT-OF-WAY.
4. REPAIR OF DAMAGED FACILITIES AND CLEANUP OF SEDIMENT ACCUMULATIONS ON ADJACENT PROPERTIES AND IN PUBLIC FACILITIES IS THE RESPONSIBILITY OF THE CONTRACTOR.
5. ALL EXPOSED EARTH SURFACES MUST BE PROTECTED FROM WIND AND WATER EROSION PRIOR TO FINAL (CITY) ACCEPTANCE OF ANY PROJECT.
6. ALL SLOPES NOT STABILIZED AT THE END OF THE PROJECT SHALL BE STABILIZED IN ACCORDANCE WITH COA SPECS OR ¾" GRAVEL

LEGEND

— —	BOUNDARY LINE	— —	EXISTING CURB & GUTTER
	BUILDING		EXISTING BOUNDARY LINE
	SIDEWALK	—5010—	EXISTING CONTOUR MAJOR
—5010—	CONTOUR MAJOR	- - - -	EXISTING CONTOUR MINOR
—5011—	CONTOUR MINOR	☼	EXISTING LIGHT STANDARD
XXXX.XX	SPOT ELEVATION (FLOWLINE)		LANDSCAPE AREA
→	FLOW ARROW		RIPRAP (D50=9")

THE EXISTING SITE HAS BEEN PARTIALLY GRADED WITH A DRAINAGE SWALE ALONG THE EAST PROPERTY LINE. PROPOSED GRADING WILL INCLUDE REPLACING THE SWALE WITH A CULVERT PIPE. GRADING ALONG THE WEST PROPERTY LINE WILL INCLUDE GRADING BACK SLOPES INTO THE ADJACENT PROPERTY TO THE WEST. THE EXISTING ARROYO RUNNING THROUGH THE SITE FROM NORTHWEST TO SOUTHEAST WILL BE FILLED IN TO ACCOMPLISH REQUIRE DEVELOPED GRADES.
RUNOFF FROM THE UPLAND DRAINAGE AREA WILL BE CONVEYED TO THE EXISTING STORM DRAIN IN DAYTONA THROUGH A CULVERT PIPE AND ROUTED THROUGH THE PROPOSED WATER QUALITY POND.

CAUTION

ALL EXISTING UTILITIES SHOWN WERE OBTAINED FROM RESEARCH, AS-BUILTS, SURVEYS OR INFORMATION PROVIDED BY OTHERS. IT SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR TO CONDUCT ALL NECESSARY FIELD INVESTIGATIONS PRIOR TO AND INCLUDING ANY EXCAVATION, TO DETERMINE THE ACTUAL LOCATION OF UTILITIES AND OTHER IMPROVEMENTS, PRIOR TO STARTING THE WORK. ANY CHANGES FROM THIS PLAN SHALL BE COORDINATED WITH AND APPROVED BY THE ENGINEER.

ENGINEER'S SEAL
RONALD R. BOHANNAN
NEW MEXICO
7868
PROFESSIONAL ENGINEER
03/29/21
RONALD R. BOHANNAN
P.E. #7868

SOUTHERN TIRE MART
ALBUQUERQUE, NM

GRADING AND DRAINAGE PLAN

TIERRA WEST, LLC
5571 MIDWAY PARK PL NE
ALBUQUERQUE, NEW MEXICO 87109
(505) 858-3100
www.tierrawestllc.com

DRAWN BY	pm
DATE	5-18-21
DRAWING	2020031-GR
SHEET #	GR-1
JOB #	2020031