

LEGEND:

51.0 EXISTING SPOT ELEVATION

51.00 NEW SPOT ELEVATION

51 EXISTING CONTOUR

51 NEW CONTOUR

SWALE

51.0 ✓ VERIFIED ELEVATION

54.0 AS BUILT ELEVATION

BASIN BOUNDARY

PROPERTY LINE

FL FLOW LINE

GND GROUND

INV INVERT

TA TOP OF ASPHALT

TC TOP OF CURB

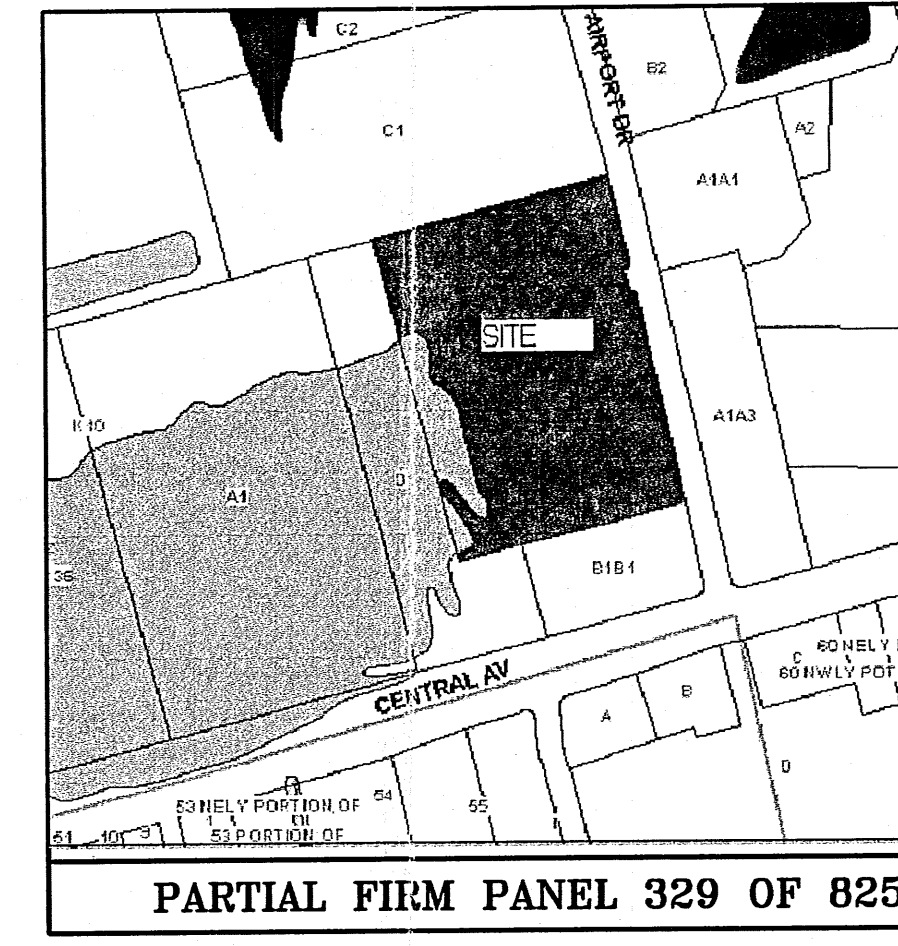
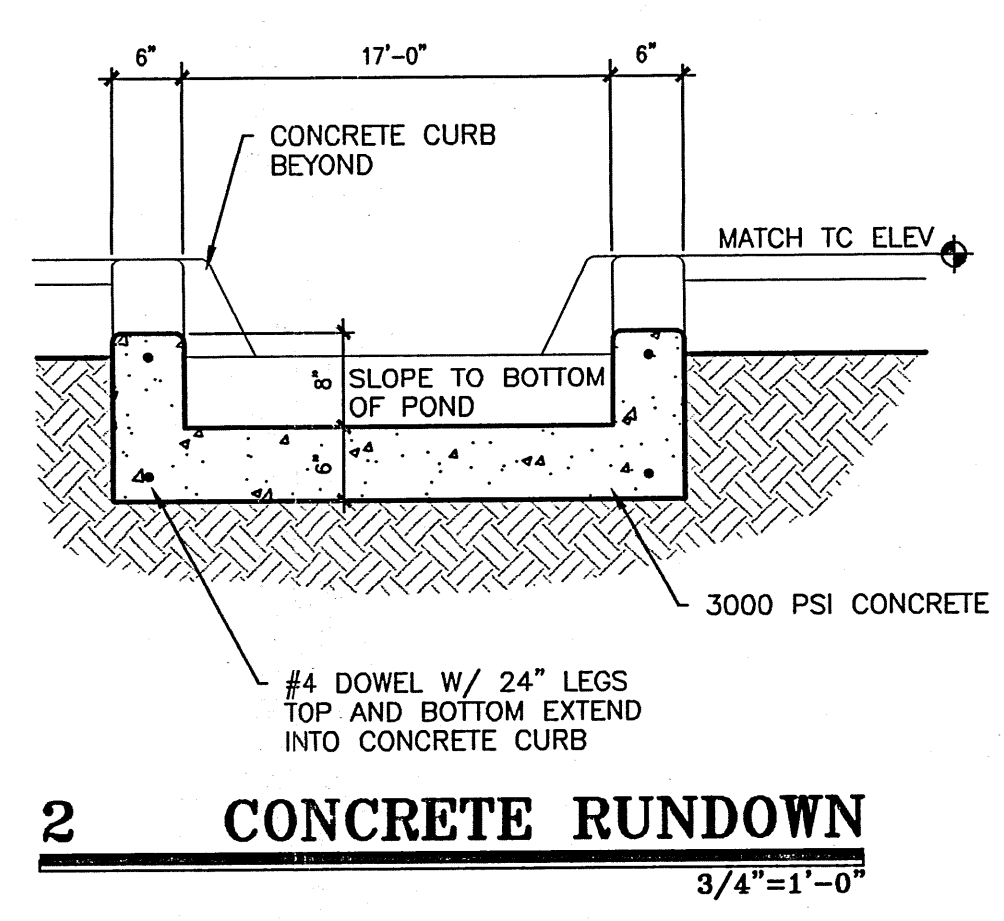
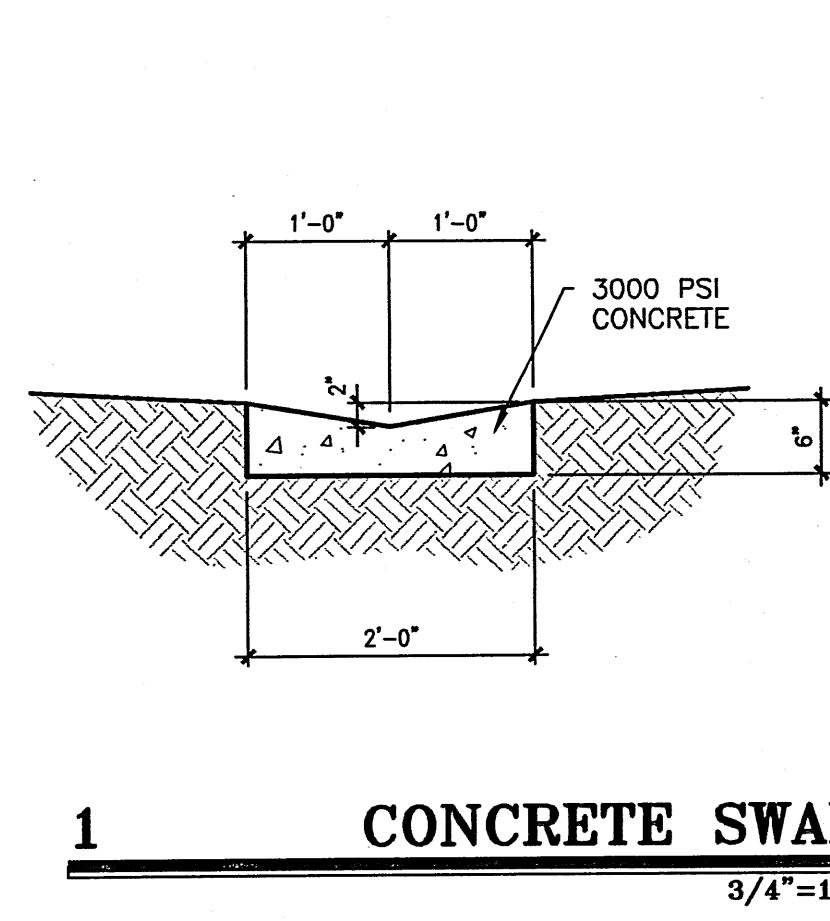
TG TOP OF GRATE

TS TOP OF CONCRETE SLAB

TW TOP OF WALL

TBM TEMPORARY BENCH MARK

BENCH MARK:
ACS CONTROL STATION 5-K-10 LOCATED AT THE INTERSECTION OF COORS ROAD & CENTRAL AVENUE ELEVATION = 5093.733 FT.



GRADING AND DRAINAGE PLAN LOT A-1-A-1 & B-1-A-1

THE FOLLOWING ITEMS CONCERNING THE LOTS A-1-A-1 & B-1-A-1 WEST 66 ADDITION ATRISCO BUSINESS PARK, GRADING AND DRAINAGE PLAN ARE CONTAINED HEREON:

1. VICINITY MAP
2. GRADING PLAN
3. CALCULATIONS

THE PROPOSED IMPROVEMENTS, AS SHOWN BY THE VICINITY MAP, ARE LOCATED ON THE WEST SIDE OF AIRPORT DRIVE NW, NORTH OF CENTRAL AVENUE NW. LOT A-1-A-1 IS CURRENTLY DEVELOPED, LOT B-1-A-1 IS CURRENTLY UNDEVELOPED. THE EXISTING FLOW FROM LOTS A-1-A-1 AND B-1-A-1 ARE ROUTED TO THE EXISTING DETENTION POND LOCATED AT THE SOUTH EAST CORNER OF LOT A-1-A-1.

THE MASTER DRAINAGE PLAN FOR THIS SUBDIVISION WAS PREPARED BY EASTERLING AND ASSOCIATES. THE MASTER PLAN ESTABLISHED A DISCHARGE RATE OF 0.1 CFS/ACRE. THE EXISTING CONTROL OUTLET HAS A DISCHARGE RATE OF .67 CFS.

THE LANDS TO THE WEST AND TO NORTH SLOPE AWAY FROM THE SITE. THE SITE TO THE SOUTH HAS BEEN DEVELOPED WITH FLOWS DIRECTED TO AN ON SITE DETENTION POND PREVENTING OFF SITE FLOW FROM THAT DIRECTION, THEREFORE OFF SITE FLOWS ARE CONSIDERED INSIGNIFICANT.

THE GRADING PLAN SHOWS:

1. THE EXISTING AND PROPOSED GRADES, INDICATED BY SPOT ELEVATIONS AND CONTOURS AT 1'-0" INTERVALS.
2. CONTINUITY BETWEEN EXISTING AND PROPOSED ELEVATIONS.
3. THE LIMIT AND CHARACTER OF EXISTING IMPROVEMENTS, AND
4. THE LIMIT AND CHARACTER OF THE PROPOSED IMPROVEMENTS.

THE PROPOSED IMPROVEMENTS CONSIST OF A WAREHOUSE / OFFICE ADDITION WITH ASSOCIATED PARKING AND LANDSCAPING. THE EXISTING POND WAS SIZED TO ACCEPT THE RUNOFF FROM THE DEVELOPED LOT A-1-A-1 AND THE UNDEVELOPED CONDITIONS FOR LOTS B-1-A-1 AND B-1-B-1. THE RUNOFF FROM LOT B-1-B-1 NOW DRAINS TO A POND ON THAT SITE. THE PROPOSED RUNOFF FROM THE SUBJECT SITE WILL DRAIN TO THE EXISTING POND. THE POND SIZE WILL BE MODIFIED TO ACCOUNT FOR THE CHANGE IN REQUIRED PONDING VOLUME. THE EXISTING CONTROLLED DISCHARGE WAS SIZED FOR LOT A-1-A-1 ONLY. THE EXISTING CONTROLLED DISCHARGE WILL BE INCREASED TO ACCOUNT FOR THE AREA OF LOT A-1-B-1. THE EXISTING DISCHARGE IS 0.67 CFS. THE ALLOWABLE DISCHARGE RATE FOR THE COMBINED SITE IF 0.1 CFS/ACRE x 10.576 ACRES = 1.06 CFS. THE PROPOSED RUNOFF RATE IN 1.02 CFS.

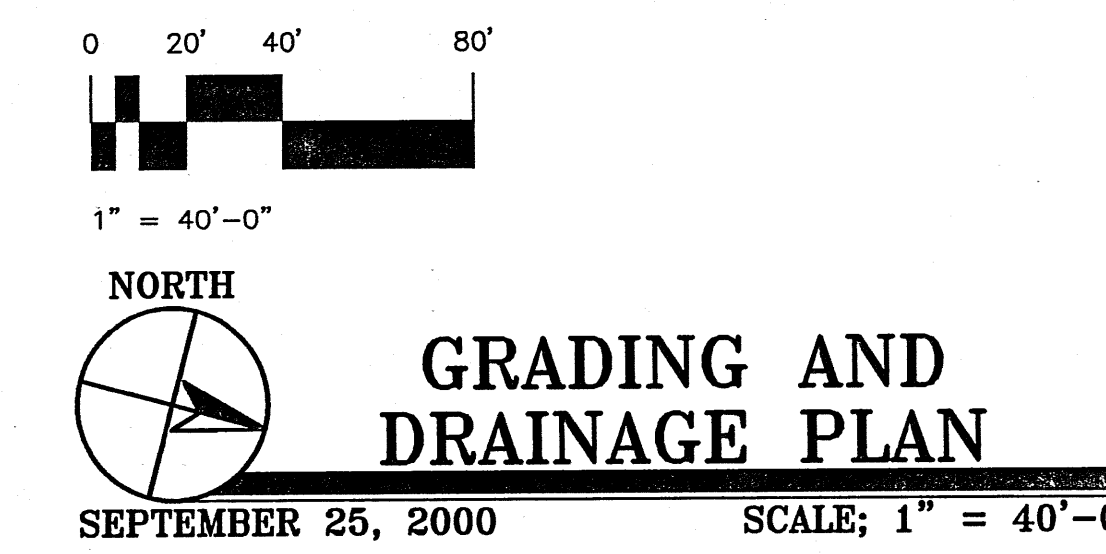
THE CALCULATIONS BELOW ANALYZE THE EXISTING AND PROPOSED CONDITIONS FOR THE 6-HOUR, 100 YEAR RAINFALL EVENT. THE ANALYSIS IS IN ACCORDANCE WITH THE CITY OF ALBUQUERQUE DEVELOPMENT PROCESS MANUAL VOLUME II, AS SHOWN BY THESE CALCULATIONS, THE RATE AND VOLUME OF RUNOFF WILL INCREASE, BUT THE POND(S) WITH CONTROLLED OUTLETS WILL MITIGATE THE INCREASES. THIS PLAN IS IN CONFORMANCE WITH THE MASTER DRAINAGE PLAN.

CALCULATIONS
PRECIPITATION ZONE = 1
TOTAL SITE AREA = 10.576 ACRES

EXISTING CONDITIONS
LAND TREATMENT A=68% B=2% C=0% D=31%
 $E = 0.44(0.68) + 0.67(0.02) + 1.97(0.31) = 0.91$ INCHES
 $V = 0.91(10.576) / 12 = 0.803$ ACRE FEET
 $Q = [1.29(0.68) + 2.03(0.02) + 4.37(0.31)] 10.576 = 23.73$ CFS

DEVELOPED CONDITIONS
LAND TREATMENT A=45% B=4% C=0% D=52%
 $E = 0.44(0.45) + 0.67(0.03) + 1.97(0.52) = 1.24$ INCHES
 $V = 1.24(10.576) / 12 = 1.091$ ACRE FEET
 $Q = [1.29(0.45) + 2.03(0.03) + 4.37(0.52)] 10.576 = 30.73$ CFS
INCREASE IN VOLUME OF RUNOFF = 1.091 - 0.803 = 0.287 ACRE FT
INCREASE IN RATE OF RUNOFF = 30.73 - 23.73 = 7.00 CFS

POND VOLUME
 $T = 0.2$ HR
 $2.107 E At / Qp - 0.25 Ad / At = 0.217$ HR
 $T = (0.7 Tc) + ((1.6 - Ad / At)) / 12 = 0.230$ HR
 $V_{required} = 46,850$ CF
VOLUME @ ELEV 5090.5
 $V = [0.5(39460 + 24540) * 1.5] = 48,000$ CF



ORIFICES CALCULATIONS

WATER SURFACE ELEVATION	92.5
EXISTING 1/4" HOLE INV	92.5
EXISTING DISCHARGE RATE	0.61 CFS
PROPOSED DISCHARGE RATE	1.02 CFS

$Q = k A \sqrt{2gh}$
 $h = (92.5 - 92.5) - 0.25 / 2 = 0.215$ FT
 $\Delta Q = 1.02 - 0.61 = 0.41$ CFS

$Q = k A \sqrt{2gh}$
 $\Delta A = 2(A_1 - A_2) = 123$ in²
 $A_1 = 1/2 \Delta A + A_2 = 1/2(123) + 123 = 624$ in²
 $DIA = 2.35$ in USE (2) 3/4" HOLES

Engineer's Certification for Permanent Certificate of Occupancy
Zanios Foods Addition - Tract A-1-A-1 & A-1-B-1

I hereby certify that I have inspected the site grading and drainage improvements and that they have been completed in substantial compliance with the approved grading and drainage plan; and are expected to function as intended. Proposed contours have not been revised to reflect the as-constructed information and should be considered approximate. Spot elevations which have not been revised should be considered approximate.

John Arthur Blessen 8-10-01
John Arthur Blessen, PE date
NM PE # 13481

CLAUDIO VIGIL ARCHITECTS

ZANIOS FOODS
WAREHOUSE ADDITION PHASE III
221 AIRPORT ROAD, N.W.
ALBUQUERQUE, NEW MEXICO

SHEET C-1

PROJECT NUMBER 00000

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