

# CITY OF ALBUQUERQUE

Planning Department  
David Campbell, Director



Mayor Timothy M. Keller

May 13, 2019

Vincent Carrica, P.E.  
Tierra West, LLC  
5571 Midway Park Place NE  
Albuquerque, NM, 87109

**RE: Maverik – Unser/Los Volcanes  
551 Silver Creek Rd. NW  
Grading and Drainage Plan & Drainage Report  
Engineer's Stamp Date: 05/02/19  
Hydrology File: K10D023D**

Dear Mr. Carrica:

Based upon the information provided in your resubmittal received 05/02/2019, the Grading & Drainage Plan and Drainage Report are approved for Building Permit and for action by the DRB on Site Plan for Building Permit.

PO Box 1293

Please attach a copy of this approved plan in the construction sets for Building Permit processing along with a copy of this letter. Prior to approval in support of Permanent Release of Occupancy by Hydrology, Engineer Certification per the DPM checklist will be required.

Albuquerque

If the project total area of disturbance (including the staging area and any work within the adjacent Right-of-Way) is 1 acre or more, then an Erosion and Sediment Control (ESC) Plan and Owner's certified Notice of Intent (NOI) is required to be submitted to the Stormwater Quality Engineer (Curtis Cherne, PE, [ccherne@cabq.gov](mailto:ccherne@cabq.gov), 924-3420) 14 days prior to any earth disturbance.

NM 87103

[www.cabq.gov](http://www.cabq.gov)

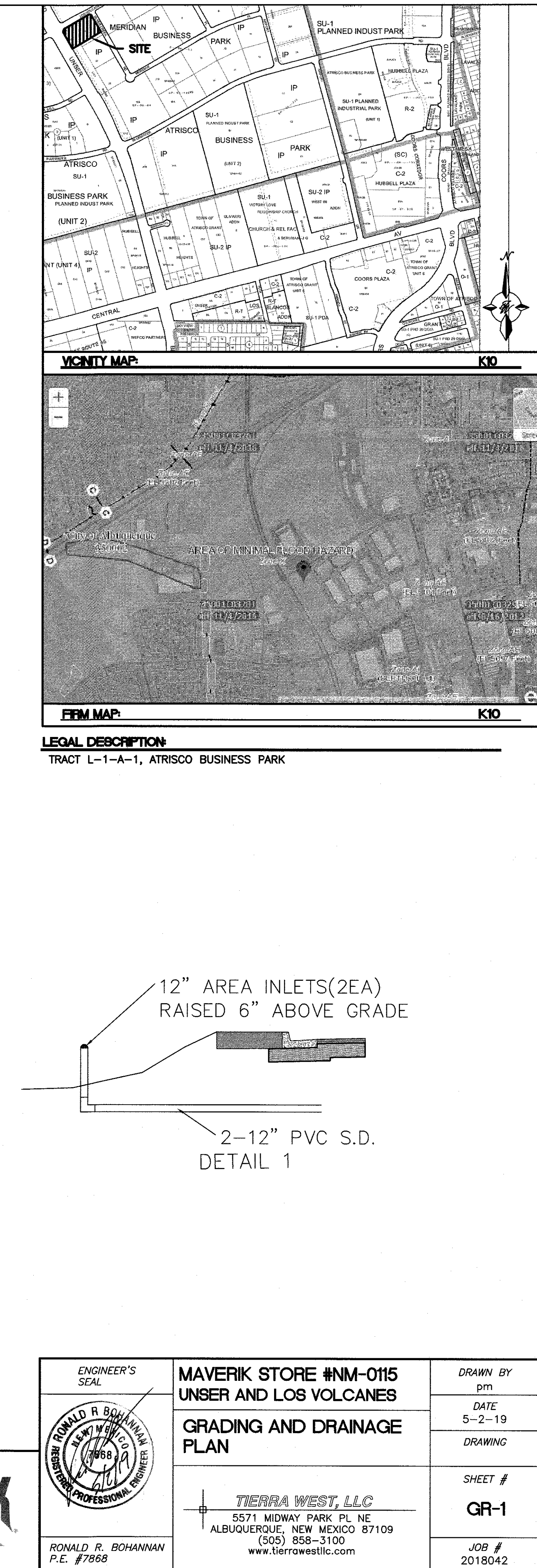
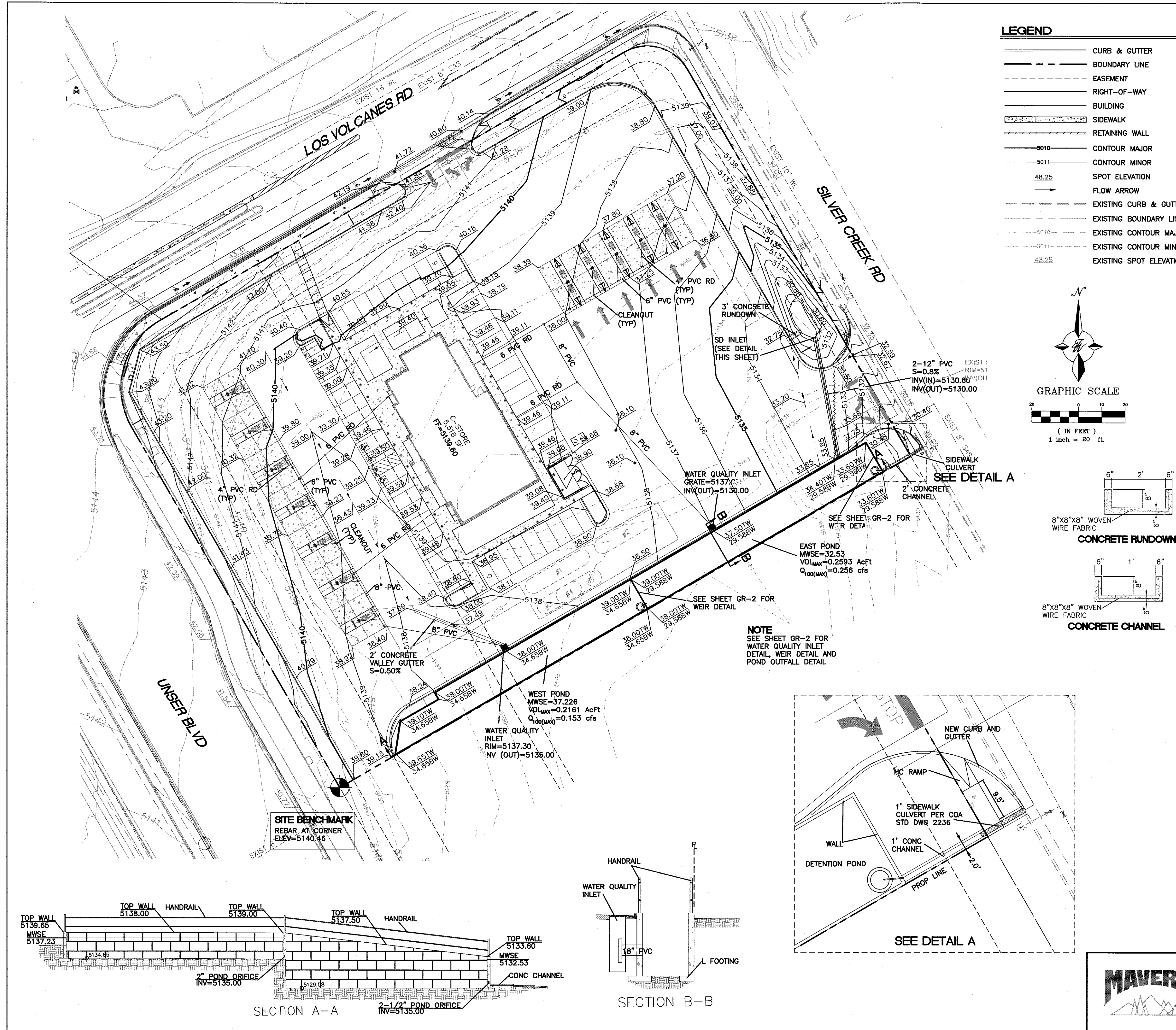
As a reminder, please provide a Drainage Covenant for the proposed detention ponds per Chapter 17 of the DPM prior to Permanent Release of Occupancy. Please submit this on the 4th floor of Plaza de Sol. A \$25 fee will be required.

If you have any questions, please contact me at 924-3995 or [rbrissette@cabq.gov](mailto:rbrissette@cabq.gov).

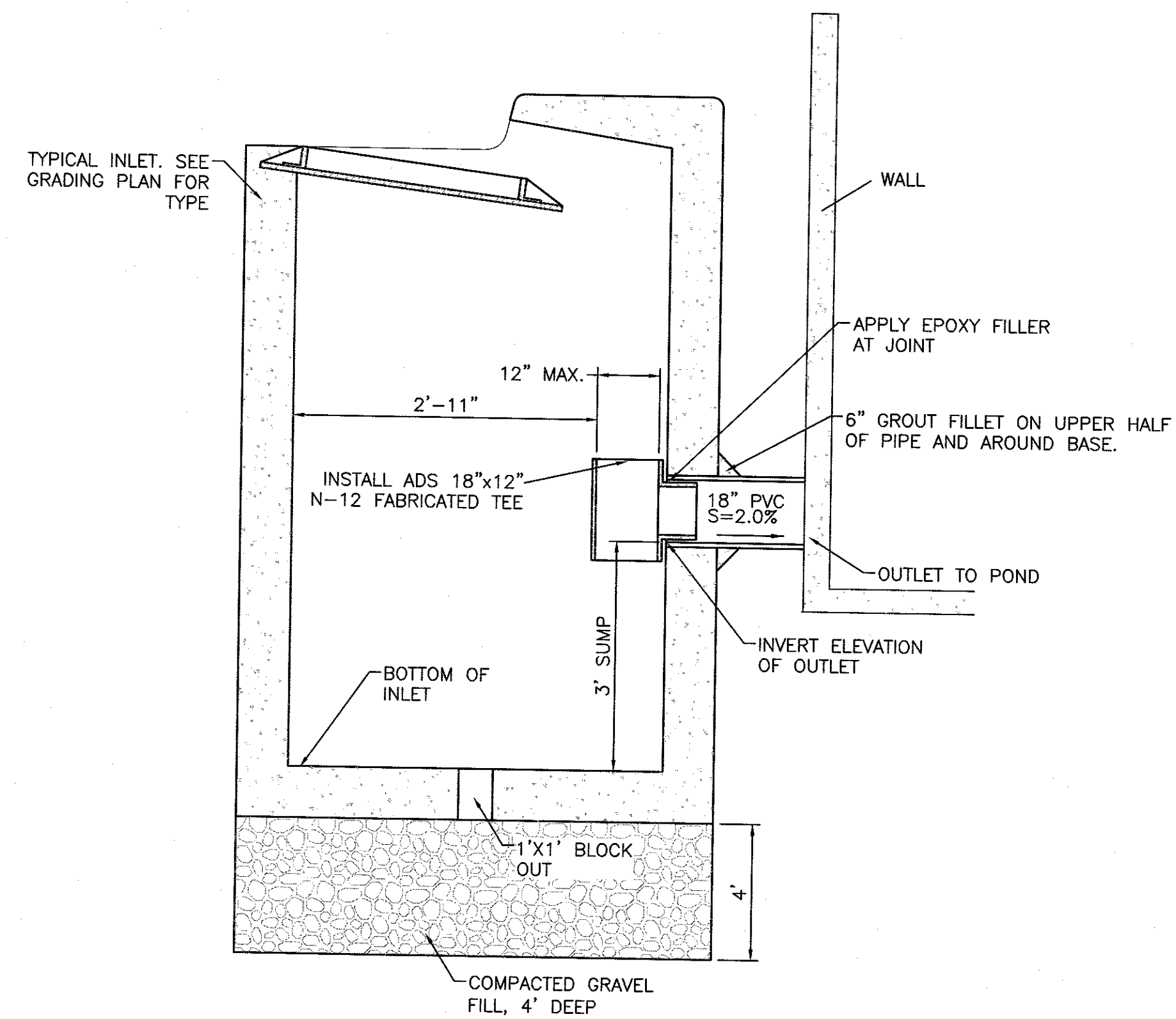
Sincerely,

Renée C. Brissette, P.E. CFM  
Senior Engineer, Hydrology  
Planning Department

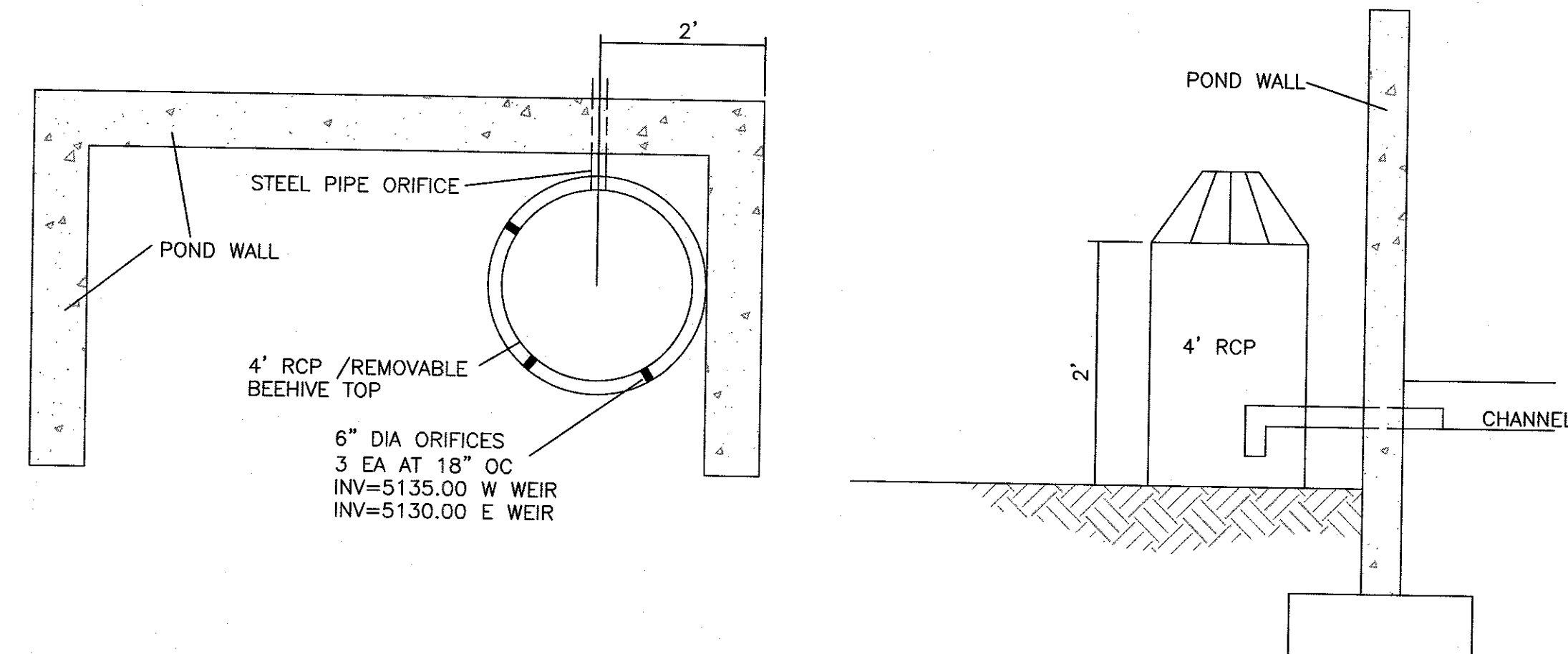




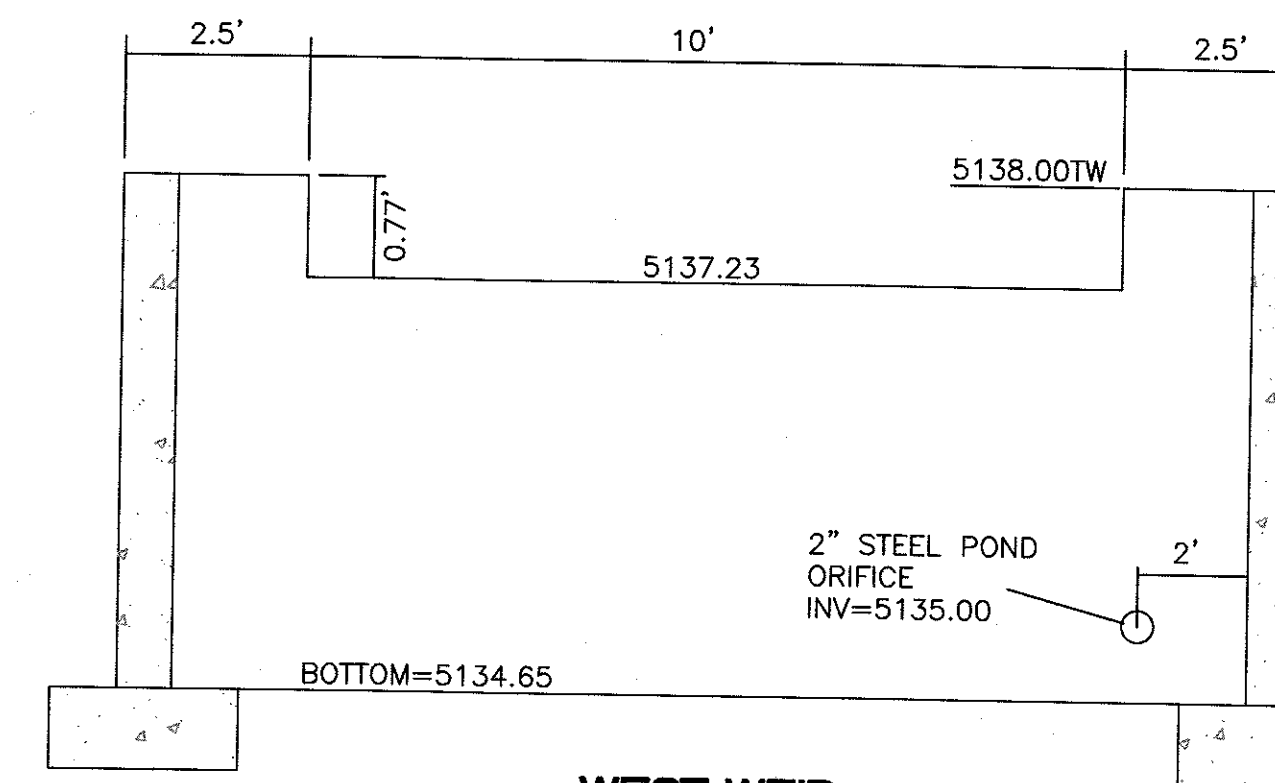




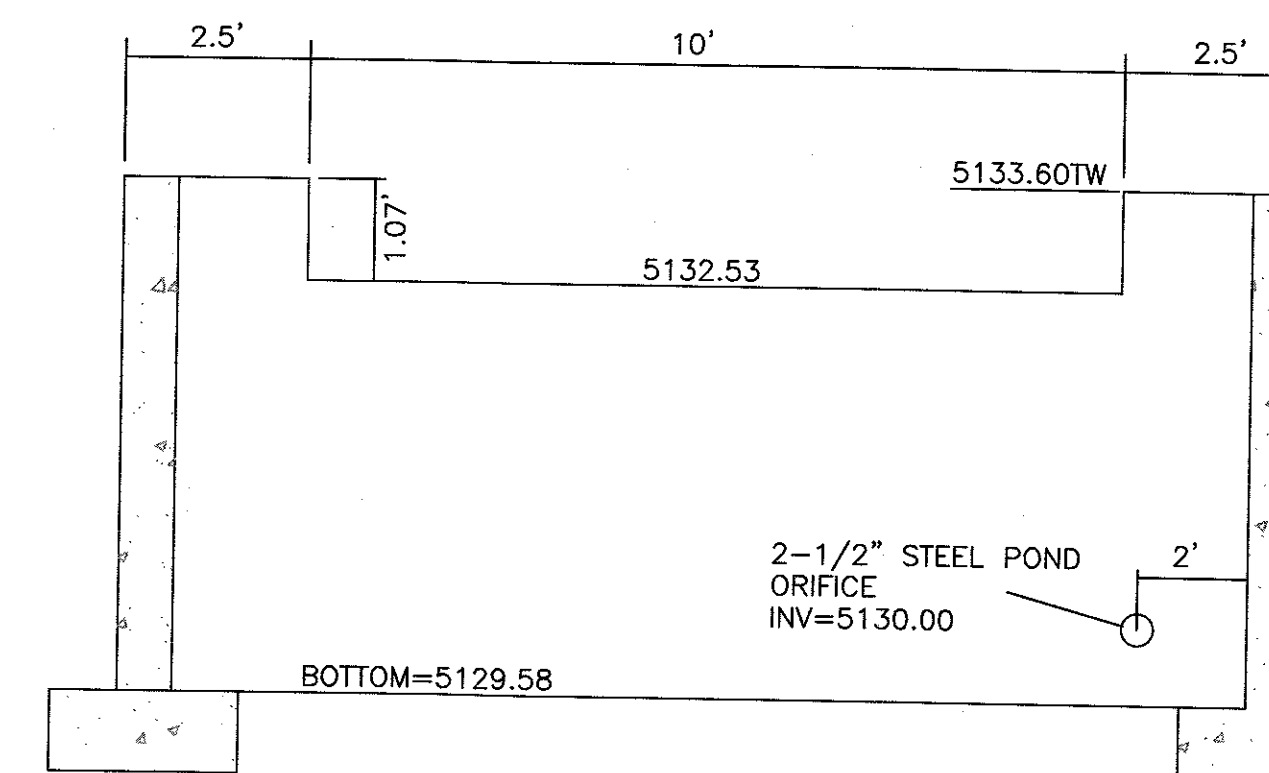
**TYPICAL WATER QUALITY INLET DETAIL**  
NTS



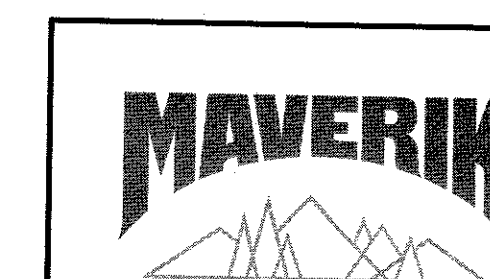
**POND OUTFALL DETAIL**  
NTS

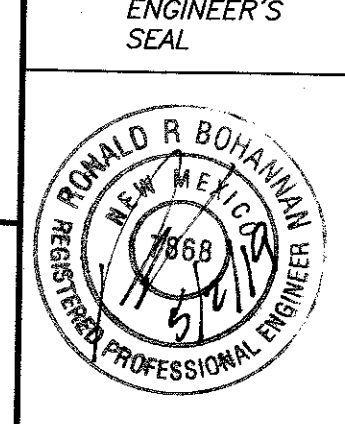
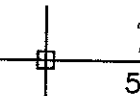


**WEST WEIR**  
NTS



**EAST WEIR**  
NTS



 RONALD R. BOHANNAN P.E. #7868	<b>MAVERIK STORE #NM-0115</b> <b>UNSER AND LOS VOLCANES</b>		DRAWN BY pm
	<b>GRADING DETAILS</b>		DATE 5-2-19
	 <b>TERRA WEST, LLC</b> 5571 MIDWAY PARK PL NE ALBUQUERQUE, NEW MEXICO 87109 (505) 858-5100 www.tierrowestllc.com		SHEET # <b>GR-2</b>
			JOB # 2018042

DRAINAGE REPORT

For

**551 Silver Creek Rd.  
ALBUQUERQUE, NEW MEXICO**


Prepared by

Tierra West, LLC  
5571 Midway Park Place NE  
Albuquerque, New Mexico 87109

Prepared for

Maverik  
Albuquerque, NM

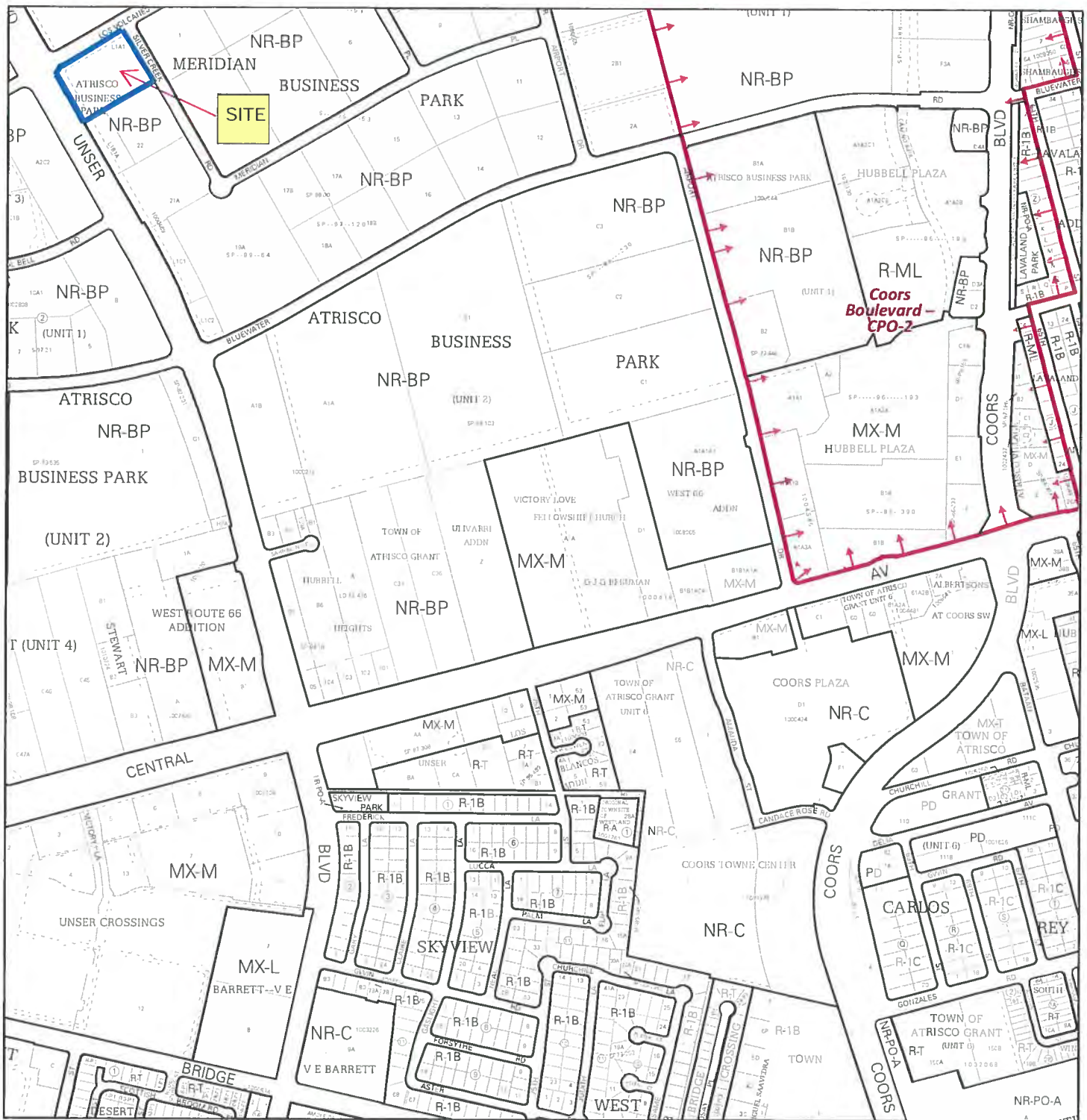
April 29, 2019



Vincent P. Carrica, PE #16212

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GRADING AND DRAINAGE PLAN .....	MAP POCKET



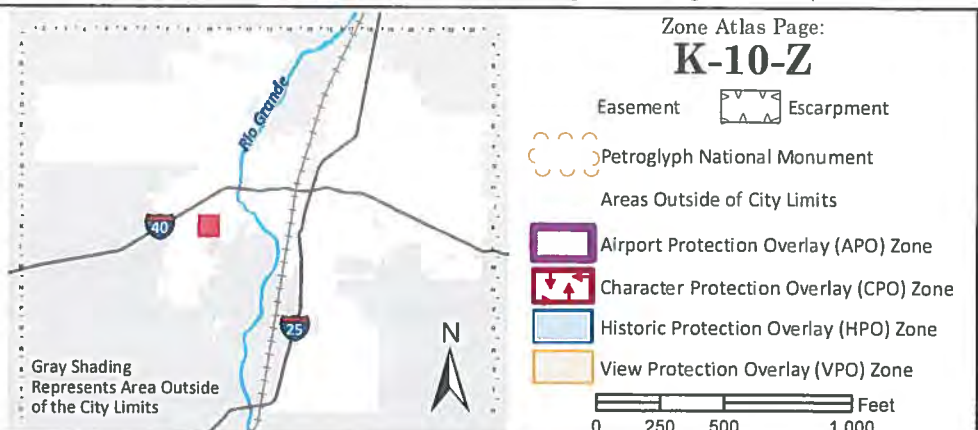
For more details about the Integrated Development Ordinance visit: <http://www.cabq.gov/planning/codes-policies-regulations/integrated-development-ordinance>

# IDO Zone Atlas May 2018



IDO Zoning information as of May 17, 2018

The Zone Districts and Overlay Zones  
are established by the  
Integrated Development Ordinance (IDO).



## **LOCATION**

The proposed commercial development is located off Silver Creek Rd south of Interstate 40, east of Unser Blvd at the corner of Los Volcanes and Silver Creek Rd in southwest Albuquerque. It is comprised of approximately 3.06 acres zoned NR-BP. This report represents a drainage management and grading plan for approval by the City of Albuquerque, for Site Plan, grading and Building Permit submittal.

## **DRAINAGE BASIN DESIGNATION**

The drainage basins for proposed conditions are as indicated on the BASIN MAP included in this report. The site is broken into six onsite drainage basins and one upland offsite basin.

## **EXISTING DRAINAGE CONDITIONS**

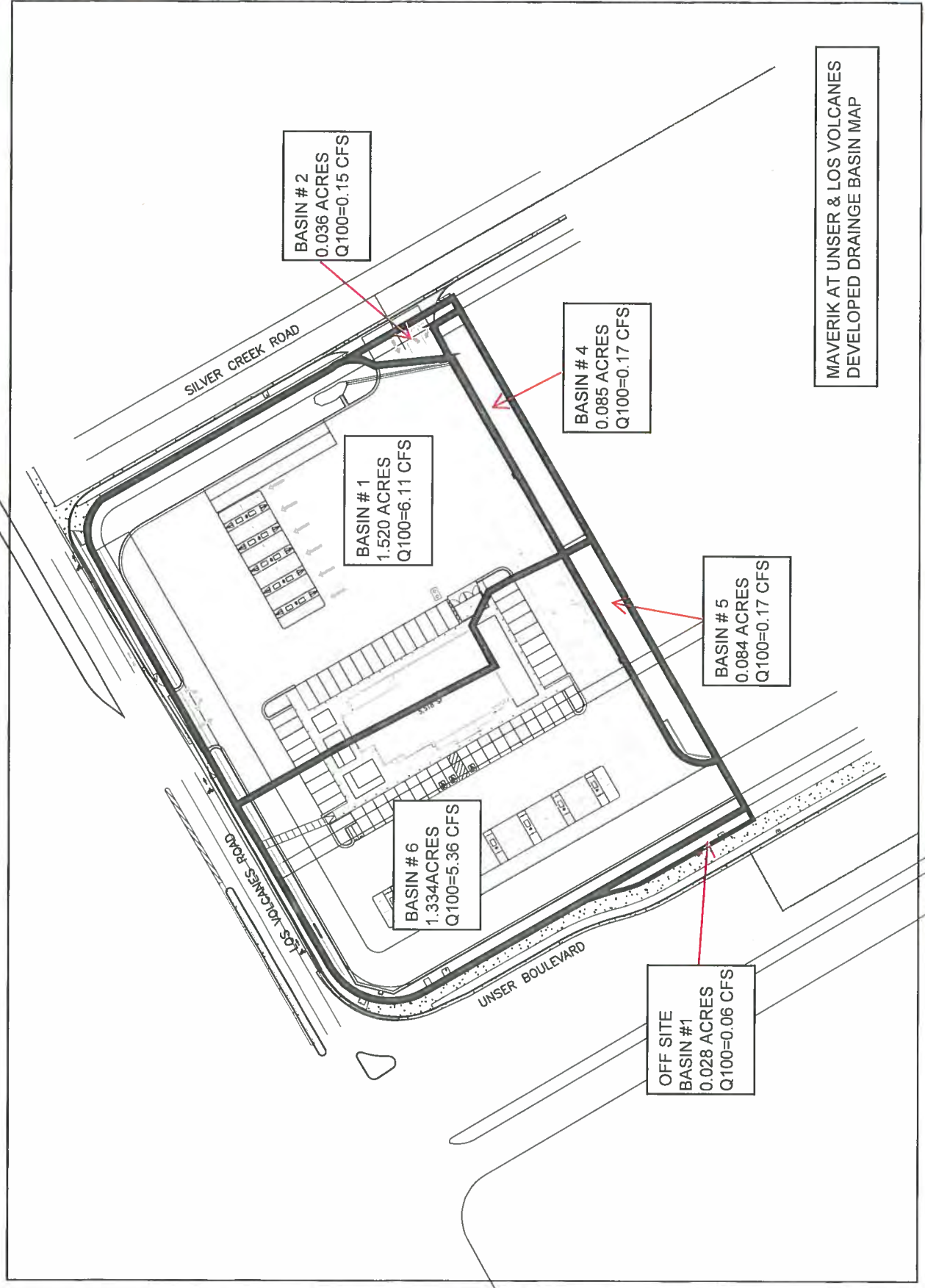
The site is currently vacant with an earthen detention pond constructed in the southeast corner of the site. It drains predominantly northwest to southeast. Runoff from a small upland basin that is within Unser Blvd right-of-way drains onto the site. This runoff is combined with the onsite runoff and routed through an existing detention pond before being released to Silver Creek Rd, which then drains to the south per the Atrisco Business Park Master Drainage Plan for fully developed conditions, dated February of 1992.

## **FIRM MAP**

The site is not located in a flood plain as is shown on designated Flood Hazard Zone Map No. 35001C0328J dated 11/4/2016.

## **DESIGN-CRITERIA**

The drainage plan presented in this report was prepared in accordance with the City of Albuquerque Drainage Ordinances and the Development Process Manual DPM. The hydrological analysis is based on the 100-year frequency, 6-hour duration storm. The plan will also include retention of the first flush in on-site drainage ponds. See attached Weighted E Table for excess precipitation values calculated for this site.



BASIN # 2  
0.036 ACRES  
Q100=0.15 CFS

BASIN # 4  
0.085 ACRES  
Q100=0.17 CFS

BASIN # 1  
1.520 ACRES  
Q100=6.11 CFS

BASIN # 5  
0.084 ACRES  
Q100=0.17 CFS

BASIN # 6  
1.334 ACRES  
Q100=5.36 CFS

OFF SITE  
BASIN #1  
0.028 ACRES  
Q100=0.06 CFS

MAVERIK AT UNSER & LOS VOLCANES  
DEVELOPED DRAINAGE BASIN MAP



# National Flood Hazard Layer FIRMette



35°52'7.93"N

106°43'53.05"W



USGS The National Map, Orthoimagery, Data refreshed October 2017  
35°45'58.49"N  
106°43'15.59"W

## Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

**SPECIAL FLOOD HAZARD AREAS**

- Without Base Flood Elevation (BFE)  
Zone AE, V, A99
- With BFE or Depth Zone AE, AO, AH, VE, AR
- Regulatory Floodway

**OTHER AREAS OF FLOOD HAZARD**

- 0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile Zone X
- Future Conditions 1% Annual Chance Flood Hazard Zone X
- Area with Reduced Flood Risk due to Levee, See Notes, Zone X
- Area with Flood Risk due to Levee Zone D

**OTHER AREAS**

- Area of Minimal Flood Hazard Zone X
- Effective LOMRs
- Area of Undetermined Flood Hazard Zone D
- Channel, Culvert, or Storm Sewer
- Levee, Dike, or Floodwall

**OTHER FEATURES**

- Cross Sections with 1% Annual Chance
- Water Surface Elevation
- Coastal Transect
- Base Flood Elevation Line (BFE)
- Limit of Study
- Jurisdiction Boundary
- Coastal Transect Baseline
- Profile Baseline
- Hydrographic Feature

**MAP PANELS**

- Digital Data Available
- No Digital Data Available
- Unmapped



The pin displayed on the map is an approximate point selected by the user and does not represent an authoritative property location.

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on **1/31/2019 at 6:28:05 PM** and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.

### **DEVELOPED-DRAINAGE CONDITIONS**

The site is proposed to be developed with a single user, Maverik C-Store. No offsite flows will enter the site with the exception of the small upland basin in the Unser Blvd right-of-way, which will continue to be routed through the subject site. Runoff from the site will be routed through two onsite drainage ponds and will then discharge to Silver Creek roadway with a controlled discharge rate equal to or less than the allowable 0.1 cfs per acre. This is in compliance with the Atrisco Business Park Master Drainage Plan for fully developed conditions dated February of 1992. The drainage ponds will retain the first flush retention volumes as required by the drainage ordinance.

Refer to enclosed Weighted E computation spreadsheet for developed runoff conditions. Storm drain capacities are listed in a table in the appendix.

### **SUMMARY**

The proposed grading and drainage plan for the proposed development of the existing undeveloped property includes surface flows and an onsite storm drain to convey runoff to detention ponds before discharging to the Silver Creek Roadway at a controlled discharge rate of equal to or less than 0.1 cfs per acre.

# VOLUME CALCULATIONS

Maverik @ Unser & Los Volcanes

West Pond

Ab - Bottom Of The Pond Surface Area

At - Top Of The Pond Surface Area

D - Water Depth

Dt - Total Pond Depth

C - Change In Surface Area / Water Depth

$$\text{Volume} = \text{Ab} * \text{D} + 0.5 * \text{C} * \text{D}^2$$

$$\text{C} = (\text{At} - \text{Ab}) / \text{Dt}$$

$$\text{Ab} = 3,655.00 \text{ T.O.P.} = 5138$$

$$\text{At} = 3,655.00 \text{ B.O.P.} = 5135$$

$$\text{Dt} = 3.00$$

$$\text{C} = 0.00$$

$$\text{B Elev.} = 5,135.00$$

ACTUAL ELEV.	DEPTH (FT)	VOLUME (AC-FT)	Q (CFS)
5134.65	0	0	0
5135.00	0.35	0.0294	0.000
5135.50	0.50	0.0713	0.068
5136.00	1.00	0.1133	0.101
5136.50	1.50	0.1552	0.125
5137.00	2.00	0.1972	0.145
5137.50	2.50	0.2391	0.163
5138.00	3.00	0.2811	0.179
5138.50	3.50	0.3230	0.194

DI Invert

DI Rim

Emergency Overflow

Orifice Equation

$$Q = \text{CA} \text{ SQRT}(2gH)$$

$$\text{C} = 0.6$$

$$\text{Diameter (in)} = 2$$

$$\text{Area (ft}^2\text{)} = 0.021816616$$

$$g = 32.2$$

$$\text{H (Ft)} = \text{Depth of water above center of orifice}$$

$$\text{Q (CFS)} = \text{Flow}$$



# VOLUME CALCULATIONS

Maverik @ Unser & Los Volcanes

East Pond

Ab - Bottom Of The Pond Surface Area

At - Top Of The Pond Surface Area

D - Water Depth

Dt - Total Pond Depth

C - Change In Surface Area / Water Depth

$$\text{Volume} = \text{Ab} * \text{D} + 0.5 * \text{C} * \text{D}^2$$

$$\text{C} = (\text{At} - \text{Ab}) / \text{Dt}$$

$$\text{Ab} = 3,705.00 \quad \text{T.O.P.} = 5133$$

$$\text{At} = 3,705.00 \quad \text{B.O.P.} = 5129.58$$

$$\text{Dt} = 3.42$$

$$\text{C} = 0.00$$

$$\text{B Elev.} = 5,130.00$$

ACTUAL ELEV.	DEPTH (FT)	VOLUME (AC-FT)	Q (CFS)
5129.58	0	0	0.000
5130.00	0.42	0.0424	0.000
5130.50	0.50	0.0849	0.103
5131.00	1.00	0.1275	0.155
5131.50	1.50	0.1700	0.194
5132.00	2.00	0.2125	0.226
5132.50	2.50	0.2551	0.254
5133.00	3.00	0.2976	0.279
5133.50	3.50	0.3401	0.302

DI Invert

DI Rim

Emergency Overflow

Orifice Equation

$$Q = \text{CA} \sqrt{2gH}$$

$$\text{C} = 0.6$$

$$\text{Diameter (in)} = 2.5$$

$$\text{Area (ft}^2\text{)} = 0.034088462$$

$$g = 32.2$$

$$\text{H (Ft)} = \text{Depth of water above center of orifice}$$

$$\text{Q (CFS)} = \text{Flow}$$

Zone #1  
Developed Basins

Maverik @ Unser & Los Volcanes  
Weighted E Method

											100-Year			10-Year			2-Year			
Basin	Area (sf)	Area (acres)	Area (sq miles)	Treatment A		Treatment B		Treatment C		Treatment D		Weighted E (ac-ft)	Volume (ac-ft)	Flow cfs	Weighted E (ac-ft)	Volume (ac-ft)	Flow cfs	Weighted E (ac-ft)	Volume (ac-ft)	Flow cfs
				%	(acres)	%	(acres)	%	(acres)	%	(acres)									
1	66226.00	1.520	0.00238	0%	0	15%	0.228	0%	0	85%	1.292	1.775	0.225	6.11	1.087	0.138	3.91	0.614	0.078	2.19
2	1578.00	0.036	0.00006	0%	0	15%	0.005	0%	0	85%	0.031	1.775	0.005	0.15	1.087	0.003	0.09	0.614	0.002	0.05
3	58095.00	1.334	0.00208	0%	0	15%	0.200	0%	0	85%	1.134	1.775	0.197	5.36	1.087	0.121	3.43	0.614	0.068	1.92
4	3705.00	0.085	0.00013	0%	0	100%	0.085	0%	0	0%	0.000	0.670	0.005	0.17	0.220	0.002	0.06	0.010	0.000	0.00
5	3655.00	0.084	0.00013	0%	0	100%	0.084	0%	0	0%	0.000	0.670	0.005	0.17	0.220	0.002	0.06	0.010	0.000	0.00
OS-1	1236.00	0.028	0.00004	0%	0	100%	0.028	0%	0	0%	0.000	0.670	0.002	0.06	0.220	0.001	0.02	0.010	0.000	0.00
Total	134495.00	3.088	0.00482										0.439	12.02		0.265	7.58		0.148	4.17

Equations:

Weighted E = Ea\*Aa + Eb\*Ab + Ec\*Ac + Ed\*Ad / (Total Area)

Volume = Weighted D \* Total Area

Flow = Qa \* Aa + Qb \* Ab + Qc \* Ac + Qd \* Ad



TIERRA WEST, LLC

Project MAVERIK @ UNSER / Los Volcans Date 3-29-19

Project No. WEIR CALCS

Meeting Purpose \_\_\_\_\_ Sheet No 1 of \_\_\_\_\_

Attendees \_\_\_\_\_

### OVERFLOW WEIRS

- BROAD CRESTED WEIR  $Q = 1.6 L H^{3/2} \Rightarrow H = \left( \frac{Q}{1.6 L} \right)^{2/3}$

WEST OVERFLOW:

$L = 10 \text{ ft}$

$Q = 6 \text{ cfs}$

$$H_{REQ} = \left( \frac{6}{1.6(10)} \right)^{2/3} = 0.52 \text{ ft} \leq H_{PROV.} = 0.77 \text{ ft} \checkmark$$

EAST OVERFLOW

$L = 10 \text{ ft}$

$Q = 12 \text{ cfs}$

$$H_{REQ} = \left( \frac{12}{1.6(10)} \right)^{2/3} = 0.83 \text{ ft} \leq H_{PROV.} = 1.07 \text{ ft} \checkmark$$

### 3ft RUN DOWN

$$Q = \frac{1.49}{n} A R^{2/3} S^{1/2}$$

$$L = 6 \text{ ft} \quad W = 3 \text{ ft} \quad S = \frac{32.75 - 30.60}{6} = 35.83\%$$

$$Q = \frac{1.49}{0.13} (2.01) (0.46)^{2/3} (36)^{1/2}$$

$$n = 0.13 \quad H = 0.67 \text{ ft}$$

$$A = 2.01 \text{ ft}^2 \quad WP = 4.34 \text{ ft} \quad R = \frac{2.01}{4.34} = 0.46$$

$$Q_{PROV.} = 82.4 \text{ cfs} \geq Q_{REQ.} = 6.11 \text{ cfs} \checkmark$$

### SIDE WALK CULVERT, 12"

$$Q = 1.6 L H^{3/2}$$

$$L = 1 \text{ ft} \quad H = 0.5 \text{ ft}$$

$$Q_{PROV.} = 1.6(1)(0.5)^{3/2} = 0.57 \text{ cfs} \geq Q_{REQ} = 0.30 \text{ cfs} \checkmark$$

### 2ft CHANNEL

$$Q = 1.6 L H^{3/2}$$

$$L = 2 \text{ ft} \quad H = 0.5 \text{ ft}$$

$$Q_{PROV.} = 1.6(2)(0.5)^{3/2} = 1.14 \text{ cfs} \geq Q_{REQ} = 0.30 \text{ cfs} \checkmark$$





hymoMaverik.txt

```
*****
*                               *
*      Maverik @ UNSER & LOS VOLCANES      *
*                               *
*****
* 100-YEAR, 24-HR STORM  (UNDER PROPOSED CONDITIONS) W/ routing *
*****
START                TIME=0.0
*
*
RAINFALL              TYPE=2 RAIN QUARTER=0.0 IN
                     RAIN ONE=1.87 IN RAIN SIX=2.20 IN
                     RAIN DAY=2.66 IN DT=0.05 HR
*
*
*BASIN 1
*
COMPUTE NM HYD        ID=1 HYD NO=100.1 AREA=0.00238 SQ MI
                     PER A=0.00 PER B=15.00 PER C=0.00 PER D=85.00
                     TP=-0.1333 HR MASS RAINFALL=-1
PRINT HYD             ID=1 CODE=1
*
*
*BASIN 2
*
COMPUTE NM HYD        ID=2 HYD NO=100.2 AREA=0.00006 SQ MI
                     PER A=0.00 PER B=15.00 PER C=0.00 PER D=85.00
                     TP=-0.1333 HR MASS RAINFALL=-1
PRINT HYD             ID=2 CODE=1
*
*
*BASIN 3
*
COMPUTE NM HYD        ID=3 HYD NO=100.3 AREA=0.00208 SQ MI
                     PER A=0.00 PER B=15.00 PER C=0.0 PER D=85.00
                     TP=-0.1333 HR MASS RAINFALL=-1
PRINT HYD             ID=3 CODE=1
*
*
*BASIN 4
*
COMPUTE NM HYD        ID=4 HYD NO=100.4 AREA=0.00013 SQ MI
                     PER A=0.00 PER B=100.00 PER C=0.0 PER D=0.00
                     TP=-0.1333 HR MASS RAINFALL=-1
PRINT HYD             ID=4 CODE=1
*
*
*BASIN 5
*
COMPUTE NM HYD        ID=5 HYD NO=100.5 AREA=0.00013 SQ MI
```

```

                                hymoMaverik.txt
PER A=0.00 PER B=100.00 PER C=0.0 PER D=0.00
TP=-0.1333 HR MASS RAINFALL=-1
PRINT HYD                      ID=5 CODE=1
*
*
ADD HYD                        ID=20 HYD NO=100.20 ID=3 ID=5
*
*
*BASIN OS-1 (OFFSITE BASIN)
*
COMPUTE NM HYD                ID=7 HYD NO=100.7 AREA=0.00004 SQ MI
PER A=0.00 PER B=100.00 PER C=0.00 PER D=0.00
TP=-0.1333 HR MASS RAINFALL=-1
PRINT HYD                      ID=7 CODE=1
*
*
ADD HYD                        ID=20 HYD NO=100.20 ID=3 ID=5
ADD HYD                        ID=21 HYD NO=100.21 ID=20 ID=7
*
*
*ROUTE BASIN 3, 5 & OS-1 THROUGH DETENTION WEST POND
*
*
ROUTE RESERVOIR                ID=55 HYD NO=200.1 INFLOW ID=21 CODE=24
                                OUTFLOW (CFS) STORAGE(AC-FT) ELEVATION(FT)
                                0.000          0.0294          35.00
                                0.068          0.0713          35.50
                                0.101          0.1133          36.00
                                0.125          0.1552          36.50
                                0.145          0.1972          37.00
                                0.163          0.2391          37.50
                                0.179          0.2811          38.00
                                0.194          0.3230          38.50
*
PRINT HYD                      ID=55 CODE=1
*
*
ADD HYD                        ID=22 HYD NO=100.22 ID=1 ID=4
ADD HYD                        ID=24 HYD NO=100.24 ID=22 ID=55
*
*
*ROUTE BASIN 1 & 4 AND OUTFLOW FROM WEST POND THROUGH DETENTION EAST POND
*
*
ROUTE RESERVOIR                ID=56 HYD NO=200.2 INFLOW ID=24 CODE=24
                                OUTFLOW (CFS) STORAGE(AC-FT) ELEVATION(FT)
                                0.000          0.0424          30.00
                                0.103          0.0849          30.50

```

	hymoMaverik.txt		
	0.155	0.1275	31.00
	0.194	0.1700	31.50
	0.226	0.2125	32.00
	0.254	0.2551	32.50
	0.279	0.2976	33.00
	0.302	0.3401	33.50

\*  
 PRINT HYD ID=56 CODE=1  
 \*  
 \*  
 ADD HYD ID=57 HYD NO=100.57 ID=2 ID=56  
 \*  
 \*  
 PRINT HYD ID=57 CODE=1  
 \*  
 \*  
 FINISH





AHYMO PROGRAM SUMMARY TABLE (AHYMO-S4)  
 Rel: 01a RUN DATE (MON/DAY/YR) =03/19/2019  
 INPUT FILE = C:\Users\Vince\Desktop\hymoMaverik.txt  
 USER NO.= AHYMO\_Temp\_User:20122010

- Ver. S4.01a,

TIME TO	CFS	FROM	TO	PEAK	RUNOFF
PEAK	HYDROGRAPH	PAGE =	1	DISCHARGE	VOLUME
PER	IDENTIFICATION	ID	ID	(CFS)	(AC-FT)
COMMAND	ACRE	NO.	NO.	(SQ MI)	
(INCHES)	(HOURS)	NOTATION			
START					
	TIME=	0.00			
RAINFALL	TYPE= 2 NOAA 14				
	RAIN24=	2.660			
COMPUTE NM HYD	100.10	-	1	0.00238	0.277
2.17912 1.500	4.175 PER IMP=	85.00			
COMPUTE NM HYD	100.20	-	2	0.00006	0.007
2.17912 1.500	4.408 PER IMP=	85.00			
COMPUTE NM HYD	100.30	-	3	0.00208	0.242
2.17912 1.500	4.177 PER IMP=	85.00			
COMPUTE NM HYD	100.40	-	4	0.00013	0.006
0.83873 1.500	2.519 PER IMP=	0.00			
COMPUTE NM HYD	100.50	-	5	0.00013	0.006
0.83873 1.500	2.519 PER IMP=	0.00			
ADD HYD	100.20	3& 5	20	0.00221	0.248
2.10011 1.500	4.079				
COMPUTE NM HYD	100.70	-	7	0.00004	0.002
0.83873 1.500	2.765 PER IMP=	0.00			
ADD HYD	100.20	3& 5	20	0.00221	0.248
2.10011 1.500	4.079				
ADD HYD	100.21	20& 7	21	0.00225	0.249
2.07766 1.500	4.056				
ROUTE RESERVOIR	200.10	21	55	0.00225	0.249
2.07766 2.450	0.106 AC-FT=	0.216			
ADD HYD	100.22	1& 4	22	0.00251	0.282
2.10957 1.500	4.089				
ADD HYD	100.24	22&55	24	0.00476	0.531
2.09206 1.500	2.186				
ROUTE RESERVOIR	200.20	24	56	0.00476	0.531
2.09206 2.550	0.084 AC-FT=	0.259			
ADD HYD	100.57	2&56	57	0.00482	0.536
2.08613 1.550	0.109				
FINISH					





# AHYMOout11.txt

AHYMO PROGRAM (AHYMO-S4) - Version: S4.01a - Rel: 01a  
 RUN DATE (MON/DAY/YR) = 03/19/2019  
 START TIME (HR:MIN:SEC) = 08:24:59 USER NO.=  
 AHYMO\_Temp\_User:20122010  
 INPUT FILE = C:\Users\Vince\Desktop\hymoMaverik.txt

\*\*\*\*\*  
 \* Maverik @ UNSER & LOS VOLCANES \*  
 \*\*\*\*\*  
 \* 100-YEAR, 24-HR STORM (UNDER PROPOSED CONDITIONS) w/ routing \*  
 \*\*\*\*\*

START TIME=0.0

\*

\*

RAINFALL TYPE=2 RAIN QUARTER=0.0 IN  
 RAIN ONE=1.87 IN RAIN SIX=2.20 IN  
 RAIN DAY=2.66 IN DT=0.05 HR

24-HOUR RAINFALL DIST. - BASED ON NOAA ATLAS 14 FOR CONVECTIVE  
 AREAS (NM & AZ) - D1

DT =	0.050000 HOURS	END TIME =	24.000002 HOURS
0.0000	0.0022	0.0045	0.0069
0.0096	0.0123	0.0154	0.0197
0.0264	0.0336	0.0412	0.0494
0.0578	0.0664	0.0753	0.0844
0.0946	0.1052	0.1168	0.1387
0.1657	0.2020	0.2430	0.2937
0.3614	0.4375	0.5689	0.7733
1.1234	1.3695	1.5635	1.6610
1.7465	1.8079	1.8568	1.8994
1.9306	1.9592	1.9828	1.9979
2.0087	2.0183	2.0273	2.0352
2.0426	2.0499	2.0568	2.0625
2.0659	2.0692	2.0724	2.0754
2.0784	2.0813	2.0842	2.0870
2.0896	2.0923	2.0949	2.0974
2.0999	2.1023	2.1046	2.1069
2.1092	2.1115	2.1136	2.1158
2.1179	2.1199	2.1220	2.1240
2.1260	2.1280	2.1299	2.1318
2.1337	2.1356	2.1374	2.1392
2.1411	2.1428	2.1446	2.1463
2.1481	2.1498	2.1514	2.1531
2.1548	2.1564	2.1580	2.1596
2.1612	2.1628	2.1643	2.1658
2.1674	2.1689	2.1704	2.1718
2.1733	2.1747	2.1762	2.1776
2.1790	2.1804	2.1818	2.1832
2.1845	2.1859	2.1872	2.1885
2.1899	2.1912	2.1924	2.1937
2.1950	2.1963	2.1975	2.1988
2.2000	2.2013	2.2026	2.2038
2.2051	2.2064	2.2077	2.2089
2.2102	2.2115	2.2128	2.2141
2.2153	2.2166	2.2179	2.2192
2.2204	2.2217	2.2230	2.2243
2.2256	2.2268	2.2281	2.2294
2.2307	2.2319	2.2332	2.2345
2.2358	2.2371	2.2383	2.2396
2.2409	2.2422	2.2434	2.2447
2.2460	2.2473	2.2486	2.2498
2.2511	2.2524	2.2537	2.2549
2.2562	2.2575	2.2588	2.2601
2.2613	2.2626	2.2639	2.2652
2.2664	2.2677	2.2690	

AHYMOout11.txt

2.2703	2.2716	2.2728	2.2741	2.2754	2.2767	2.2779
2.2792	2.2805	2.2818	2.2831	2.2843	2.2856	2.2869
2.2882	2.2894	2.2907	2.2920	2.2933	2.2946	2.2958
2.2971	2.2984	2.2997	2.3009	2.3022	2.3035	2.3048
2.3061	2.3073	2.3086	2.3099	2.3112	2.3124	2.3137
2.3150	2.3163	2.3176	2.3188	2.3201	2.3214	2.3227
2.3239	2.3252	2.3265	2.3278	2.3291	2.3303	2.3316
2.3329	2.3342	2.3354	2.3367	2.3380	2.3393	2.3406
2.3418	2.3431	2.3444	2.3457	2.3469	2.3482	2.3495
2.3508	2.3521	2.3533	2.3546	2.3559	2.3572	2.3584
2.3597	2.3610	2.3623	2.3636	2.3648	2.3661	2.3674
2.3687	2.3699	2.3712	2.3725	2.3738	2.3750	2.3763
2.3776	2.3789	2.3802	2.3814	2.3827	2.3840	2.3853
2.3865	2.3878	2.3891	2.3904	2.3917	2.3929	2.3942
2.3955	2.3968	2.3980	2.3993	2.4006	2.4019	2.4032
2.4044	2.4057	2.4070	2.4083	2.4095	2.4108	2.4121
2.4134	2.4147	2.4159	2.4172	2.4185	2.4198	2.4210
2.4223	2.4236	2.4249	2.4262	2.4274	2.4287	2.4300
2.4313	2.4325	2.4338	2.4351	2.4364	2.4377	2.4389
2.4402	2.4415	2.4428	2.4440	2.4453	2.4466	2.4479
2.4492	2.4504	2.4517	2.4530	2.4543	2.4555	2.4568
2.4581	2.4594	2.4607	2.4619	2.4632	2.4645	2.4658
2.4670	2.4683	2.4696	2.4709	2.4722	2.4734	2.4747
2.4760	2.4773	2.4785	2.4798	2.4811	2.4824	2.4837
2.4849	2.4862	2.4875	2.4888	2.4900	2.4913	2.4926
2.4939	2.4952	2.4964	2.4977	2.4990	2.5003	2.5015
2.5028	2.5041	2.5054	2.5067	2.5079	2.5092	2.5105
2.5118	2.5130	2.5143	2.5156	2.5169	2.5182	2.5194
2.5207	2.5220	2.5233	2.5245	2.5258	2.5271	2.5284
2.5297	2.5309	2.5322	2.5335	2.5348	2.5360	2.5373
2.5386	2.5399	2.5412	2.5424	2.5437	2.5450	2.5463
2.5475	2.5488	2.5501	2.5514	2.5527	2.5539	2.5552
2.5565	2.5578	2.5590	2.5603	2.5616	2.5629	2.5642
2.5654	2.5667	2.5680	2.5693	2.5705	2.5718	2.5731
2.5744	2.5757	2.5769	2.5782	2.5795	2.5808	2.5820
2.5833	2.5846	2.5859	2.5872	2.5884	2.5897	2.5910
2.5923	2.5935	2.5948	2.5961	2.5974	2.5987	2.5999
2.6012	2.6025	2.6038	2.6050	2.6063	2.6076	2.6089
2.6102	2.6114	2.6127	2.6140	2.6153	2.6165	2.6178
2.6191	2.6204	2.6217	2.6229	2.6242	2.6255	2.6268
2.6280	2.6293	2.6306	2.6319	2.6332	2.6344	2.6357
2.6370	2.6383	2.6395	2.6408	2.6421	2.6434	2.6447
2.6459	2.6472	2.6485	2.6498	2.6510	2.6523	2.6536
2.6549	2.6562	2.6574	2.6587	2.6600		

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\*BASIN 1

AHYMOout11.txt

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COMPUTE NM HYD ID=1 HYD NO=100.1 AREA=0.00238 SQ MI  
PER A=0.00 PER B=15.00 PER C=0.00 PER D=85.00  
TP=-0.1333 HR MASS RAINFALL=-1

K = 0.072649HR TP = 0.133300HR K/TP RATIO = 0.545000 SHAPE  
CONSTANT, N = 7.106428  
UNIT PEAK = 7.9869 CFS UNIT VOLUME = 0.9978 B = 526.28  
P60 = 1.8700  
AREA = 0.002023 SQ MI IA = 0.10000 INCHES INF = 0.04000  
INCHES PER HOUR  
RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =  
0.050000

K = 0.130992HR TP = 0.133300HR K/TP RATIO = 0.982685 SHAPE  
CONSTANT, N = 3.593298  
UNIT PEAK = 0.87598 CFS UNIT VOLUME = 0.9867 B = 327.08  
P60 = 1.8700  
AREA = 0.000357 SQ MI IA = 0.50000 INCHES INF = 1.25000  
INCHES PER HOUR  
RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =  
0.050000

PRINT HYD ID=1 CODE=1

PARTIAL HYDROGRAPH 100.10

RUNOFF VOLUME = 2.17912 INCHES = 0.2766 ACRE-FEET  
PEAK DISCHARGE RATE = 6.36 CFS AT 1.500 HOURS BASIN AREA =  
0.0024 SQ. MI.

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\*BASIN 2

\*

COMPUTE NM HYD ID=2 HYD NO=100.2 AREA=0.00006 SQ MI  
PER A=0.00 PER B=15.00 PER C=0.00 PER D=85.00  
TP=-0.1333 HR MASS RAINFALL=-1

K = 0.072649HR TP = 0.133300HR K/TP RATIO = 0.545000 SHAPE  
CONSTANT, N = 7.106428  
UNIT PEAK = 0.20135 CFS UNIT VOLUME = 0.9490 B = 526.28  
P60 = 1.8700  
AREA = 0.000051 SQ MI IA = 0.10000 INCHES INF = 0.04000

INCHES PER HOUR

RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = 0.050000

K = 0.130992HR TP = 0.133300HR K/TP RATIO = 0.982685 SHAPE  
CONSTANT, N = 3.593298  
UNIT PEAK = 0.22084E-01CFS UNIT VOLUME = 0.8758 B = 327.08  
P60 = 1.8700  
AREA = 0.000009 SQ MI IA = 0.50000 INCHES INF = 1.25000  
INCHES PER HOUR

RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = 0.050000

PRINT HYD ID=2 CODE=1

PARTIAL HYDROGRAPH 100.20

RUNOFF VOLUME = 2.17912 INCHES = 0.0070 ACRE-FEET  
PEAK DISCHARGE RATE = 0.17 CFS AT 1.500 HOURS BASIN AREA = 0.0001 SQ. MI.

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\*  
\*BASIN 3  
\*

COMPUTE NM HYD ID=3 HYD NO=100.3 AREA=0.00208 SQ MI  
PER A=0.00 PER B=15.00 PER C=0.0 PER D=85.00  
TP=-0.1333 HR MASS RAINFALL=-1

K = 0.072649HR TP = 0.133300HR K/TP RATIO = 0.545000 SHAPE  
CONSTANT, N = 7.106428  
UNIT PEAK = 6.9802 CFS UNIT VOLUME = 0.9975 B = 526.28  
P60 = 1.8700  
AREA = 0.001768 SQ MI IA = 0.10000 INCHES INF = 0.04000  
INCHES PER HOUR

RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = 0.050000

K = 0.130992HR TP = 0.133300HR K/TP RATIO = 0.982685 SHAPE  
CONSTANT, N = 3.593298  
UNIT PEAK = 0.76556 CFS UNIT VOLUME = 0.9848 B = 327.08  
P60 = 1.8700  
AREA = 0.000312 SQ MI IA = 0.50000 INCHES INF = 1.25000



AHYMOout11.txt

INCHES PER HOUR

RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =  
0.050000

PRINT HYD

ID=3 CODE=1

PARTIAL HYDROGRAPH 100.30

RUNOFF VOLUME = 2.17912 INCHES = 0.2417 ACRE-FEET  
PEAK DISCHARGE RATE = 5.56 CFS AT 1.500 HOURS BASIN AREA =  
0.0021 SQ. MI.

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\*

\*BASIN 4

\*

COMPUTE NM HYD

ID=4 HYD NO=100.4 AREA=0.00013 SQ MI  
PER A=0.00 PER B=100.00 PER C=0.0 PER D=0.00  
TP=-0.1333 HR MASS RAINFALL=-1

K = 0.130992HR TP = 0.133300HR K/TP RATIO = 0.982685 SHAPE  
CONSTANT, N = 3.593298

UNIT PEAK = 0.31898 CFS UNIT VOLUME = 0.9613 B = 327.08  
P60 = 1.8700

AREA = 0.000130 SQ MI IA = 0.50000 INCHES INF = 1.25000  
INCHES PER HOUR

RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =  
0.050000

PRINT HYD

ID=4 CODE=1

PARTIAL HYDROGRAPH 100.40

RUNOFF VOLUME = 0.83873 INCHES = 0.0058 ACRE-FEET  
PEAK DISCHARGE RATE = 0.21 CFS AT 1.500 HOURS BASIN AREA =  
0.0001 SQ. MI.

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\*

\*BASIN 5

\*

COMPUTE NM HYD

ID=5 HYD NO=100.5 AREA=0.00013 SQ MI

AHYMOout11.txt  
PER A=0.00 PER B=100.00 PER C=0.0 PER D=0.00  
TP=-0.1333 HR MASS RAINFALL=-1

K = 0.130992HR TP = 0.133300HR K/TP RATIO = 0.982685 SHAPE  
CONSTANT, N = 3.593298  
UNIT PEAK = 0.31898 CFS UNIT VOLUME = 0.9613 B = 327.08  
P60 = 1.8700  
AREA = 0.000130 SQ MI IA = 0.50000 INCHES INF = 1.25000  
INCHES PER HOUR  
RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =  
0.050000

PRINT HYD ID=5 CODE=1

PARTIAL HYDROGRAPH 100.50

RUNOFF VOLUME = 0.83873 INCHES = 0.0058 ACRE-FEET  
PEAK DISCHARGE RATE = 0.21 CFS AT 1.500 HOURS BASIN AREA =  
0.0001 SQ. MI.

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ADD HYD ID=20 HYD NO=100.20 ID=3 ID=5

\*  
\*

\*BASIN OS-1 (OFFSITE BASIN)

\*

COMPUTE NM HYD ID=7 HYD NO=100.7 AREA=0.00004 SQ MI  
PER A=0.00 PER B=100.00 PER C=0.00 PER D=0.00  
TP=-0.1333 HR MASS RAINFALL=-1

K = 0.130992HR TP = 0.133300HR K/TP RATIO = 0.982685 SHAPE  
CONSTANT, N = 3.593298  
UNIT PEAK = 0.98149E-01CFS UNIT VOLUME = 0.8758 B = 327.08  
P60 = 1.8700  
AREA = 0.000040 SQ MI IA = 0.50000 INCHES INF = 1.25000  
INCHES PER HOUR  
RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =  
0.050000

PRINT HYD ID=7 CODE=1

PARTIAL HYDROGRAPH 100.70

AHYMOout11.txt

RUNOFF VOLUME = 0.83873 INCHES = 0.0018 ACRE-FEET  
 PEAK DISCHARGE RATE = 0.07 CFS AT 1.500 HOURS BASIN AREA =  
 0.0000 SQ. MI.

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ADD HYD ID=20 HYD NO=100.20 ID=3 ID=5

ADD HYD ID=21 HYD NO=100.21 ID=20 ID=7

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 \*

\*ROUTE BASIN 3, 5 & OS-1 THROUGH DETENTION WEST POND

\*  
 \*

ROUTE RESERVOIR	ID=55	HYD NO=200.1	INFLOW ID=21	CODE=24
	OUTFLOW (CFS)	STORAGE(AC-FT)	ELEVATION(FT)	
	0.000	0.0294	35.00	
	0.068	0.0713	35.50	
	0.101	0.1133	36.00	
	0.125	0.1552	36.50	
	0.145	0.1972	37.00	
	0.163	0.2391	37.50	
	0.179	0.2811	38.00	
	0.194	0.3230	38.50	

\* \* \* \* \*

TIME (HRS)	INFLOW (CFS)	ELEV (FEET)	VOLUME (AC-FT)	OUTFLOW (CFS)
0.00	0.00	35.00	0.029	0.00
1.20	0.80	35.11	0.038	0.01
2.40	0.19	37.23	0.216	0.15
3.60	0.01	37.09	0.205	0.15
4.80	0.02	36.93	0.192	0.14
6.00	0.03	36.80	0.180	0.14

AHYMOout11.txt

7.20	0.03	36.67	0.170	0.13
8.40	0.03	36.55	0.160	0.13
9.60	0.03	36.44	0.150	0.12
10.80	0.03	36.33	0.141	0.12
12.00	0.03	36.23	0.133	0.11
13.20	0.03	36.14	0.125	0.11
14.40	0.03	36.05	0.117	0.10
15.60	0.03	35.96	0.110	0.10
16.80	0.03	35.88	0.103	0.09
18.00	0.03	35.81	0.097	0.09
19.20	0.03	35.74	0.092	0.08
20.40	0.03	35.68	0.086	0.08
21.60	0.03	35.62	0.082	0.08
22.80	0.03	35.57	0.077	0.07
24.00	0.03	35.52	0.073	0.07
25.20	0.00	35.45	0.067	0.06
26.40	0.00	35.38	0.061	0.05
27.60	0.00	35.32	0.057	0.04
28.80	0.00	35.28	0.053	0.04
30.00	0.00	35.23	0.049	0.03
31.20	0.00	35.20	0.046	0.03
32.40	0.00	35.17	0.044	0.02
33.60	0.00	35.14	0.042	0.02
34.80	0.00	35.12	0.040	0.02
36.00	0.00	35.10	0.038	0.01
37.20	0.00	35.09	0.037	0.01
38.40	0.00	35.08	0.036	0.01
39.60	0.00	35.06	0.035	0.01
40.80	0.00	35.06	0.034	0.01
42.00	0.00	35.05	0.033	0.01
43.20	0.00	35.04	0.033	0.01
44.40	0.00	35.03	0.032	0.00

PEAK DISCHARGE = 0.153 CFS - PEAK OCCURS AT HOUR 2.45

MAXIMUM WATER SURFACE ELEVATION = 37.226

MAXIMUM STORAGE = 0.2161 AC-FT INCREMENTAL TIME= 0.050000HRS

\*

PRINT HYD ID=55 CODE=1

PARTIAL HYDROGRAPH 200.10

RUNOFF VOLUME = 2.07766 INCHES = 0.2493 ACRE-FEET

PEAK DISCHARGE RATE = 0.15 CFS AT 2.450 HOURS BASIN AREA =  
0.0023 SQ. MI.

AHYMOout11.txt

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ADD HYD ID=22 HYD NO=100.22 ID=1 ID=4

ADD HYD ID=24 HYD NO=100.24 ID=22 ID=55

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\*ROUTE BASIN 1 & 4 AND OUTFLOW FROM WEST POND THROUGH DETENTION EAST POND

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ROUTE RESERVOIR	ID=56	HYD NO=200.2	INFLOW	ID=24	CODE=24
	OUTFLOW (CFS)	STORAGE(AC-FT)	ELEVATION(FT)		
	0.000	0.0424	30.00		
	0.103	0.0849	30.50		
	0.155	0.1275	31.00		
	0.194	0.1700	31.50		
	0.226	0.2125	32.00		
	0.254	0.2551	32.50		
	0.279	0.2976	33.00		
	0.302	0.3401	33.50		

\* \* \* \* \*

TIME (HRS)	INFLOW (CFS)	ELEV (FEET)	VOLUME (AC-FT)	OUTFLOW (CFS)
0.00	0.00	30.00	0.042	0.00
1.20	0.93	30.12	0.053	0.03
2.40	0.37	32.54	0.259	0.26
3.60	0.16	32.47	0.253	0.25
4.80	0.16	32.37	0.244	0.25
6.00	0.17	32.28	0.236	0.24
7.20	0.17	32.19	0.229	0.24
8.40	0.16	32.11	0.222	0.23
9.60	0.16	32.02	0.215	0.23
10.80	0.15	31.94	0.207	0.22
12.00	0.15	31.86	0.200	0.22
13.20	0.14	31.77	0.193	0.21
14.40	0.14	31.69	0.186	0.21

AHYMOout11.txt

15.60	0.13	31.61	0.179	0.20
16.80	0.13	31.53	0.173	0.20
18.00	0.12	31.45	0.166	0.19
19.20	0.12	31.37	0.159	0.18
20.40	0.11	31.29	0.153	0.18
21.60	0.11	31.22	0.146	0.17
22.80	0.11	31.15	0.140	0.17
24.00	0.10	31.08	0.134	0.16
25.20	0.06	30.98	0.126	0.15
26.40	0.05	30.87	0.117	0.14
27.60	0.04	30.77	0.108	0.13
28.80	0.04	30.67	0.099	0.12
30.00	0.03	30.58	0.091	0.11
31.20	0.03	30.49	0.084	0.10
32.40	0.02	30.41	0.077	0.08
33.60	0.02	30.34	0.072	0.07
34.80	0.02	30.29	0.067	0.06
36.00	0.01	30.24	0.063	0.05
37.20	0.01	30.20	0.060	0.04
38.40	0.01	30.17	0.057	0.04
39.60	0.01	30.15	0.055	0.03
40.80	0.01	30.12	0.053	0.03
42.00	0.01	30.10	0.051	0.02
43.20	0.01	30.09	0.050	0.02
44.40	0.00	30.07	0.049	0.02
45.60	0.00	30.06	0.048	0.01
46.80	0.00	30.05	0.047	0.01
48.00	0.00	30.04	0.046	0.01
49.20	0.00	30.04	0.046	0.01
50.40	0.00	30.03	0.045	0.01
51.60	0.00	30.03	0.045	0.01
52.80	0.00	30.02	0.044	0.00

PEAK DISCHARGE = 0.256 CFS - PEAK OCCURS AT HOUR 2.55

MAXIMUM WATER SURFACE ELEVATION = 32.550

MAXIMUM STORAGE = 0.2593 AC-FT INCREMENTAL TIME= 0.050000HRS

\*

PRINT HYD ID=56 CODE=1

PARTIAL HYDROGRAPH 200.20

RUNOFF VOLUME = 2.09206 INCHES = 0.5311 ACRE-FEET

PEAK DISCHARGE RATE = 0.26 CFS AT 2.550 HOURS BASIN AREA = 0.0048 SQ. MI.



AHYMOout11.txt

\*

\*

ADD HYD

ID=57 HYD NO=100.57 ID=2 ID=56

\*

\*

PRINT HYD

ID=57 CODE=1

PARTIAL HYDROGRAPH 100.57

RUNOFF VOLUME = 2.08613 INCHES = 0.5363 ACRE-FEET

PEAK DISCHARGE RATE = 0.34 CFS AT 1.550 HOURS BASIN AREA =  
0.0048 SQ. MI.

\*

\*

FINISH

NORMAL PROGRAM FINISH

END TIME (HR:MIN:SEC) = 08:24:59

