

City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

October 31, 2002

Diane Hoelzer, P.E. Mark Goodwin & Assoc. P.O. Box 90606 Albuquerque, New Mexico 87199

RE: CLIFFORD WEST CHURCH

(K-10/D37)

(8570 Saul Bell Rd)

(AKA: Fellowship Missionary Baptist Church) ENGINEERS CERTIFICATION FOR CERTIFICATE OF OCCUPANCY

ENGINEERS STAMP DATED 7/25/2001

ENGINEERS CERTIFICATION DATED 8/28/2002 Rev. 9/25/2002

Dear Diane:

Based upon the information provided in your Engineers Certification submittal dated 9/25/2002, the above referenced site is approved for a Permanent Certificate of Occupancy.

If I can be of further assistance, please contact me at 924-3981.

Sincerely,

Inesa A. Minter Teresa A. Martin

Hydrology Plan Checker

Development & Bldg. Ser. Division

Certificate of Occupancy Clerk, COA drainage file approval file

INSTRUCTIONS.



Brad L. Bingham

11/07/02 11:34 AM

To: Teresa A. Martin/PWD/CABQ@COA

CC:

Subject: Clifford West Church

Per my visual inspection of the final improvements required, this Site is in substantial compliance of the approved plans submitted by Mark Goodwin & Assoc and Thompson Engineering Consultants.



City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

July 23, 2001

Diane Hoelzer, P.E.
Mark Goodwin & Assoc.
P.O. Box 90606
Albuquerque, NM 87199

RE: CLIFFORD WEST CHURCH, Clifford West Business Park Unit 1 (K10-D37). Revised DRAINAGE REPORT, Revised GRADING AND DRAINAGE PLAN FOR SITE DEVELOPMENT PLAN FOR BUILDING PERMIT, GRADING PERMIT, AND BUILDING PERMIT APPROVALS. ENGINEER'S STAMP DATED JULY 12, 2001.

Dear Ms. Hoelzer:

Based on the information provided on your July 12, 2001 submittal, the above referenced project is approved for Site Development Plan for Building Permit, and for Grading and Building Permits.

Please attach a copy of this approved plan to the construction sets prior to sign-off by Hydrology.

Prior to Certificate of Occupancy approval, an Engineer's Certification per the DPM will be required.

If I can be of further assistance, please feel free to contact me at 924-3984.

Sincerely,

John P. Murray, P.E.

Hydrology

C

Terri Martin

DRAINAGE INFORMATION SHEET

PROJECT TITLE:	Clifford West Church	•	ZONE ATLAS	S#:	K 10 / D 37	
DRB#:	EPC#	·	WORK	ORDER#:		
LEGAL DESCRIPTION:	Lot 10, 11, 12, Block 2, Clifford We	est Business	Park Unit 1		/ \$	
CITY ADDRESS:						
ENGINEERING FIRM:	Mark Goodwin & Associates, PA		CONTACT:		•	
ADDRESS:	P.O. Box 90606, Albuquerque, NM 871	99	PHONE:	828-2200		
OWNER:			CONTACT:			
ADDRESS:			PHONE:	-		
ARCHITECT:	Paul and Associates	,	CONTACT:	Chad Young		
ADDRESS:	5620 Bullard Rd.#128, Tyler, Texas 75	703	PHONE:	903-581-8322	x 102	
SURVEYOR:	ALS		CONTACT:	TIM ALDRICH		
ADDRESS:	4109 MONTGOMERY BLVD.		PHONE:	884-1990		
CONTRACTOR:			CONTACT:	·	·	
ADDRESS:	•		PHONE:			
DRAINAGE F CONCEPTUA REPOSION CO	AL GRADING & DRAINAGE PLAN LAN (revised 7-12-01) ONTROL S CERTIFICATION	X	S. DEV. PLAN IS. DEV. PLAN IS SECTOR PLAN FINAL PLAT AFFOUNDATION BUILDING PER	PLAT APRROVER SUB'D APPROVAL PERMIT APPROVAL MIT APPROVA	PROVAL RMIT APPROVAL OVAL	
PRE-DESIGN MEETI	INIC.			MIT APPROVA	NCY APPROVAL	
YES			PAVING PERM			
NO			S.A.D. DRAINA			
COPY PROV			DRAINAGE REQUIREMENTS			
 	JUL 1 2 2001		OTHER		•	
•	HYDROLOGY SECTION	· · · · · · · · · · · · · · · · · · ·	RELEASE OF F	FINANCIAL GUA	ARANTY	
	——————————————————————————————————————					
	*	•	TRAFFIC CIRC	ULATION LAY	DUT	

BY:

DIANE HOELZER, RE



D. Mark Goodwin & Associates, P.A. Consulting Engineers

P.O. BOX 90606, ALBUQUERQUE, NM 87199 (505) 828-2200 FAX 797-9539

e-mail: dmg@swcp.com

July 12, 2001

Mr. John Murray, PE Hydrology Reviewer City of Albuquerque PO Box 1293 Albuquerque, NM 87103

Re: Request Site Development Plan for Building Permit Approval, Building Permit Approval, Grading Permit Approval for Clifford West Church Revised Grading and Drainage Plan, Engineers Stamp 7-12-01 (K10/D37)

Dear Mr. Murray;

In response to comments received through the building permit review process and DRB, there have been revisions to the Site Plan that required a revision to the original grading and drainage plan. The basic drainage plan concept remains unchanged. The grading plan was revised in response to elimination of parking area at the east end of the site, elimination of the driveway around the church building on the north and west side and minor shifts in the building location and driveway entrance roads.

Please call me if you have any questions.

Sincerely,

MARK GOODWIN & ASSOCIATES, P.A.

Diane Hoelzer, PE Senior Engineer

DLH/dlh f:\\clifford.chu\hyd_1.ltr JUL 1 2 2001
HYDROLOGY SECTION

CLIFFORD WEST CHURCH

Supplemental Drainage Information:

AHYMO Printouts

Orifice and Parking Area Pond Volume Calculations

HEC-2 outfall Channel Calculations



July 12, 2001

<u>M</u>

D. Mark Goodwin & Associates, P.A. Consulting Engineers

P.O. BOX 90606, ALBUQUERQUE, NM 87199 (505) 828-2200 FAX 797-9539 e-mail: dmg@swcp.com

PROJECT CLIFF	DRD WEST CHURCH
SUBJECT_HYD	
BY	DLH DATE 3-5-01
CHECKED	DATE
	SHEETOF_5_

SUBASINI 1

Q =
$$C \cdot a \cdot \sqrt{2gh}$$

 $0.63 = 0.6(a) \cdot \sqrt{64.4(36.5 - 30.25)}$
 $a = 0.52345F = .228^{\#}$ 2.75 $^{\#}$
 $a = .0525175F(use design)$

QOUT	Stor.	Elev.
0	0	30.25
0.28	.0009	31.50
0.33	.00118	32.0
0.42	.00173	33.0
0.49	.00228	34.0
0.55	.0028	35.0
.606	.05479	36.0
0.63	.09886	36.5

D. Mark Goodwin & Associates, P.A. Consulting Engineers

P.O. BOX 90606, ALBUQUERQUE, NM 87199 FAX 797-9539 (505) 828-2200 e-mail: dmg@swcp.com

SUBJECT Pondina Avea CHECKED. SHEET_2_OF_5_

5B1 Revise Ponding Area

35.0-36.0

<u>0.85+0</u> (24)(60)

0.85+0.(50)+.25+0.(17.2)=2(30)=351.00

0.8570(60) + 0.2570(27.8)7 = 2(30) = 434.63

1397.62 OF = 0.03209AF

=.0:00229AF

-.034385AF

36.0-36.5

0.5 + 0.3(24)(60) = 576 CF

0.25(70)(30) = 525 CF

0.25 (75) (30)=562.5 CF

1663.5 CF = .038189AF

Swele estimate (50)(10)(0.2) = 100 CF = .0022956AF

TOTAL = ,07257 AF @ 36.5

7 .			
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	1		1
	<u> 1 1 </u>		7)_

D. Mark Goodwin & Associates, P.A. Consulting Engineers

P.O. BOX 90606, ALBUQUEI (505) 828-2200 e-mail: dmg@swcp.com

RQUE,	NM	87199	
FA	X 79	7-9539	

PROJECT SUBJECT. DATE CHECKED. SHEET_3_OF_5

Qallow (Lot 10) = 0.63cfs Qallow (10+11+12)= 2.05 cfs

Lot 10 A = 34785 SF = 0.7986AC

4 = 560745F = 1.2873 AC

113017 SF Lot 12 A = 56943SF = 1,3072AC,

· Lot 10 Pervious Area

325'x9' 2925

20×8' 160

(95'X10')/2 475

50'x4' 200

720 18'XC10'

30'x3' 90

97'X16' 1392 600

20'x30'

185'x2' 370

25'x20' 500

7432 21%

TOTAL 147,802

Pervious 18762

Lot 11+12 Pervious Area = 1/330

40 x8 320

300×9 2700

40×7 280

40'x8' 320

40'X5 200 460xZ

20'x15 300'xZ' 300

(20×10)/2 100

42x25' (050

80×5' 400

110'x7' 770

80'xs' 400

451x241 1080

20 ×15' 300

1235×201) 350

40×10' 400

40'x8' 320

40xB 320

20 X15 300

40 x 9 360

40×8 320

40x8 320

320



D. Mark Goodwin & Associates, P.A. Consulting Engineers

P.O. BOX 90606, ALBUQUERQUE, NM 87199 (505) 828-2200 FAX 797-9539 e-mail: dmg@swcp.com

PROJECT <u>Clifford West Church</u>
SUBJECT <u>Hydrology - Area Calcs</u>
BY _____ DLH DATE <u>Z-9-01</u>
CHECKED _____ DATE ____
SHEET 4 OF 5

6180.6CF

=141887AF

SB Z

OUTFLOW	STORAGE	ELEVATION
	0	27.0
0.63	.03946	27.5
1.25	.07891	28.0
2.05	. 220741	28.5

27-28 1.2+6.4 (20) = 76 CF 6.4+13.5 (22) = 219CF 8.8+14.08 (40) = 457.6 CF 3.2+1.6 (40) = 96.2 CF 23+64.05 (23) = 1001CF 18+22.6 (20) = 406.2c= 38.25+14 (20) = 522.5 UF 9+1,56 (20) = 105,6 CF 23+7.15(30) = 452.2 CF5+3.1(25) = 101.25 CF 3437.55CF = .07891AF ORIFICE CALCS. Q= C·a·Vzgh

2.05cfs = 0.6(a) 12(32.2)1.25[Area = 0.38085q.ft. $0.76 \times 0.5'$ OPENING

28'-28.5 6+16.5 (20) = 225 16.5+27(20) = 435 11.5 (40) = 460 12+16 (40) = 560 32+61,5 (23)=1075,2 11.5(20)=230 45+28 (20) = 730 24+7.5 (ZO)=315 22 (30) 5 = 330 4360.2 CF 8.75+17.5 (42) = 551.2 17.5+15 (40)= 450 16.7+7.5 (Z2) = 266.2 8(20) = 160 1820.4 CF

TOTAL = . 220797

<u>M</u>

D. Mark Goodwin & Associates, P.A. Consulting Engineers

P.O. BOX 90606, ALBUQUERQUE, NM 87199 (505) 828-2200 FAX 797-9539 e-mail: dmg@swcp.com

PROJECT CITTO	nd West Church
SUBJECT HYA	pology Calcs DLH DATE Z-9-01
BY	DCH DATE 2-9-01
CHECKED	DATE
	SHFFT 5 OF 5

SBZ continued

2.05 cfs=0.60(a) [2(32.2)1:14875

TTR = .3987 2TTR = 2,23838

$$Q = 1.486 (.3987) (.3166) .01 /2$$

SUMMARY TABLE

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994 INPUT FILE = C CHURCH.DAT RUN DATE (MON/DAY/YR) = 07/11/2001USER NO. = M_GOODWN. IO1

COMMAND I	HYDROGRAPH DENTIFICATION	FROM ID NO.	TO ID NO.	AREA (SQ MI)	PEAK DISCHARGE (CFS)	RUNOFF VOLUME (AC-FT)	RUNOFF (INCHES)	TIME TO PEAK (HOURS)	CFS PER ACRE	PAGE =	
START RAINFALL TYPE=				-		•			•	TIME= RAIN6=	.00
COMPUTE NM HYD	100.SB1	_	1	.00138	3.47	. 124	1.68355	1.499	3.928	PER IMP=	78.00
ROUTE RESERVOIR	POND. 100	1	3	.00138	. <i>63</i>	. 124	1.68333	2.098	. 709	AC-FT=	. 069
COMPUTE NM HYD	100.SB2	••	1	.00393	9.86	. <i>353</i>	1.68355	1.499	3.916	PER IMP=	78.00
ROUTE RESERVOIR FINISH	POND. 100	1	3	.00393	1.99	. 353	1.68348	2.065	. 792	AC-FT=	. 211

OUTPUT FILE

AHYMO PROGRAM (AHYM0194) - AMAFCA Hydrologic Model - January, 1994

RUN DATE (MON/DAY/YR) = 07/11/2001

START TIME (HR:MIN:SEC) = 15:57:50 USER NO. = M_GOODWN.I01

INPUT FILE = C_CHURCH.DAT

START

TIME=0.0

***** CHURCH IN THE CLIFFORD WEST BUSINESS PARK

***** LOT 10, 11, 12, BLOCK 2

***** 100-YEAR 6-HOUR STORM EVENT

***** FILE: C_CHURCH.DAT JULY 12, 2001 BY:DLH

RAINFALL

TYPE=1 RAIN QUARTER=0.0 IN

RAIN ONE=1.90 IN RAIN SIX=2.20 IN

RAIN DAY=0.0 IN DT=0.0333 HR

COMPUTED	6-HOUR	RAINFALL	DISTRIBU	JTION BAS	SED ON N	OAA ATLAS	2 -	PEAK	AT	1.40	HR.
DT =	.033300	HOURS	END T	CIME =	5.994	000 HOURS					
.0000	.0013	3 .0027	.0041	.0055	.0069	.0084					
.0099	.0115	.0131	.0147	.0164	.0181	.0199					
.0217	. 0236	.0255	.0275	.0296	.0317	.0339					
.0362	.0386	5 .0411	.0437	.0464	.0492	.0522					
.0553	. 0586	.0621	.0673	.0729	.0789	.0913					
.1196	.1634	. 2264	. 3124	. 4256	.5701	. 7500					
. 9698	1.1808	3 1.2673	1.3402	1.4049	1.4637	1.5178					
1.5681	1.6150	1.6590	1.7004	1.7395	1.7764	1.8113					
1.8444	1.8758	1.9056	1.9338	1.9607	1.9678	1.9733					
1.9786	1.9836	1.9884	1.9930	1.9974	2.0016	2.0056					
2.0095	2.0133	3 2.0170	2.0205	2.0240	2.0273	2.0306					
2.0337	2.0368	2.0398	2.0428	2.0456	2.0484	2.0512					
2.0538	2.0565	2.0590	2.0615	2.0640	2.0664	2.0688					
2.0712	2.0735	2.0757	2.0779	2.0801	2.0823	2.0844					
2.0865	2.0885	2.0906	2.0925	2.0945	2.0965	2.0984					
2.1003	2.1021	2.1039	2.1058	2.1076	2.1093	2.1111					

```
0.07537
             INFLOW
              (CFS)
                                 (AC-FT)
                                           (CFS)
               2.44
                        36.11
                        36.30
                                    .060
                        30.32
     4.80
                        30.32
                                    .000
    5.59
                        30.33
                                    .000
    6.39
                        30.26
                                    .000
 PEAK DISCHARGE =
                       .626 CFS - PEAK OCCURS AT HOUR
                                                         2.10
MAXIMUM WATER SURFACE ELEVATION =
                                       36.416
MAXIMUM STORAGE =
                    .0690 AC-FT
                                      INCREMENTAL TIME=
                                                               .033300HRS
****
**** SUB BASIN 2 EAST
***** Q (allowable) = 2.05 cfs
```

COMPUTE NM HYD ID=1 HYD NO=100.SB2 AREA=0.00393401 SQ MI PER A=0 PER B=22 PER C=0 PER D=78

TP=0.1333 HR MASS RAINFALL=-1

K = .072649HR TP =.133300HR K/TP RATIO = .545000SHAPE CONSTANT, N = 7.106420UNIT PEAK = 12.115CFS $UNIT\ VOLUME = .9984$ *526.28* P60 = 1.9000.003069 SQ MI AREA =IA = .10000 INCHES INF =.04000 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033300

K = .130761HR TP =.133300HR K/TP RATIO = .980950SHAPE CONSTANT, N = 3.599930UNIT PEAK = 2.1267 CFSUNIT VOLUME = .9938*327.55* P60 = 1.9000.000865 SQ MI IA = .50000 INCHES INF = 1.25000 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT =

PRINT HYD

ID=1 CODE=1

HYDROGRAPH FROM AREA 100.SB2

1.68355 INCHES .3532 ACRE-FEET PEAK DISCHARGE RATE = 9.86 CFS 1.499 HOURS BASIN AREA = '

•	
.00 .00 27.00 .000 .00	
.80 .00 27.00 .000 .00	
1.60 6.93 28.24 .148 1.64	
2.40 .40 28.36 .182 1.83	
3.20 .07 28.04 .090 1.31	
4.00 .04 27.43 .034 .54	
4.80 .04 27.17 .013 .21	
5.59 .05 27.08 .007 .11	
6.39 .00 27.05 .004 .06	
7.19 .00 27.02 .001 .02	
7.99 .00 27.01 .000 .01	
8.79 .00 27.00 .000 .00	
PEAK DISCHARGE = 1.993 CFS - PEAK OCCURS AT HOUR 2.06	
MAXIMUM WATER SURFACE ELEVATION = 28.465	
MAXIMUM STORAGE = .2108 AC-FT INCREMENTAL TIME= .033	300HRS

FINISH

NORMAL PROGRAM FINISH

END TIME (HR:MIN:SEC) = 15:57:51

```
* U.S. ARMY CORPS OF ENGINEERS
   WATER SURFACE PROFILES
                                                                                               THE HYDROLOGIC ENGINEERING CENTER
   VERSION OF SEPTEMBER 1988
                                                                                               609 SECOND STREET, SUITE D
   ERROR: 01,02
                                                                                              DAVIS, CALIFORNIA 95616-4687
   UPDATED: 4 APRIL
                        1989
                                                                                               (916) 756-1104, (916) 551-1748
                                 14:56:51
                          TIME
   RUN DATE
                                                              XXXXX
                                                    XXXXXXX
                                                                                 XXXXX
                                                                        XXXXX
                                                    XXXX
                                                                                XXXXXXX
                                                  XXXXXXX
                                                               XXXXX
THIS RUN EXECUTED 3/5/1
                             14:56:51
  HEC2 RELEASE DATED SEP 88 UPDATED APR 1989
  ERROR CORR - 01,02
  MODIFICATION -
            CLIFFORD WEST BUSINESS PARK
            CHURCH CHANNEL OUTFALL
            100-YEAR / 6-HOUR DESIGN STORM
                                                                                             FQ
                                                                                   WSEL
                                                                HVINS
                                                      METRIC
                                  IDIR
                                            STRT
                        NINV
    ICHECK
              INQ
J1
     VARIABLE CODES FOR SUMMARY PRINTOUT
     38
                             .017
                  .017
      .017
NC
                  2.05
                                       101.2
                                                                                                               101.2
                                                               100.6
                                                                            0.04
                                       100.1
       0.54
                   100
                             0.04
                                                                                  BANK ELEV
                                                                           OLOSS
                                       WSELK
                              CRIWS
                     CWSEL
    SECNO
            DEPTH
                                                                                LEFT/RIGHT
                                                                           TWA
                                       ALOB
                                                         AROB
                                                                  VOL
                                                ACH
                     QCH
                              QROB
            QLOB
                                                                                      SSTA
                                                                  WTN
                                                                           ELMIN
                                                         XNR
                                                XNCH
                                       XNL
   TIME
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                                                IDC
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                                                                  CORAR
                                                                           TOPWID
                                       ITRIAL
            XLOBL
                     XLCH
                              XLOBR
   SLOPE
 *PROF 1
          .100 CEHV=
                         .300
 CCHV=
 *SECNO 1.000
 2096 WSEL NOT GIVEN, AVG OF MAX, MIN USED
                                                   . 77
     1.00
                . 42
                        . 42
                                 . 51
                                           0.
```

THIS RUN EXECUTED 3/5/1 14:56:51

HEC2 RELEASE DATED SEP 88 UPDATED APR 1989

ERROR CORR - 01,02

MODIFICATION -

NOTE- ASTERISK (*) AT LEFT OF CROSS-SECTION NUMBER INDICATES MESSAGE IN SUMMARY OF ERRORS LIST

100-YEAR / 6-HOUR DE

SUMMARY PRINTOUT

SECNO	Q	CWSEL	CRIWS	VCH	TOPWID .	FRCH	EG
1.000	2.05	. 42	. 51	4.77	1.15	1.38	. 77
					•	•	-

3/ 5/ 1

14:56:51

2.1128 2.1145 2.1162 2.1179 2.1195 2.1211 2.1228 2.1259 2.1275 2.1290 2.1306 2.1321 2.1336 2.1365 2.1380 2.1394 2.1409 2.1423 2.1437 2.1465 2.1478 2.1492 2.1558 2.1571 2.1584 2.1596 2.1609 2.1646 2.1658 2.1670 2.1682 2.1694 2.1706 2.1730 2.1741 2.1753 2.1764 2.1776 2.1787 2.1809 2.1820 2.1831 2.1842 2.1853 2.1864 2.1885 2.1896 2.1906 2.1917 2.1927 2.1938 2.1948 2.1958 2.1968 2.1978 2.1988 2.1998 ***** ****************** ***** ***************** **** **** SUB BASIN 1 WEST ***** Q (allowable) = 0.63 cfs **** COMPUTE NM HYD ID=1 HYD NO=100.SB1 AREA=0.00137935 SQ MI PER A=0 PER B=22 PER C=0 PER D=78 TP=0.1333 HR MASS RAINFALL=-1 K = .072649HR TP =.133300HR K/TP RATIO = .545000SHAPE CONSTANT, N = 7.106420UNIT PEAK = 4.2477 CFSUNIT VOLUME = .9965526.28 P60 = 1.9000B = .001076 SQ MI IA = .10000 INCHESINF = .04000 INCHES PER HOURAREA =RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033300 K = .130761HR TP = .133300HR - K/TP RATIO = .980950SHAPE CONSTANT, N = 3.599930.74567 CFS $UNIT\ VOLUME = .9822$ B =*327.55* P60 = 1.9000.000303 SQ MI IA = .50000 INCHES INF = 1.25000 INCHES PER HOURAREA = RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033300

HYDROGRAPH FROM AREA 100.SB1

RUNOFF VOLUME = 1.68355 INCHES = .1239 ACRE-FEET
PEAK DISCHARGE RATE = 3.47 CFS AT 1.499 HOURS BASIN AREA = .0014 SQ. MI.

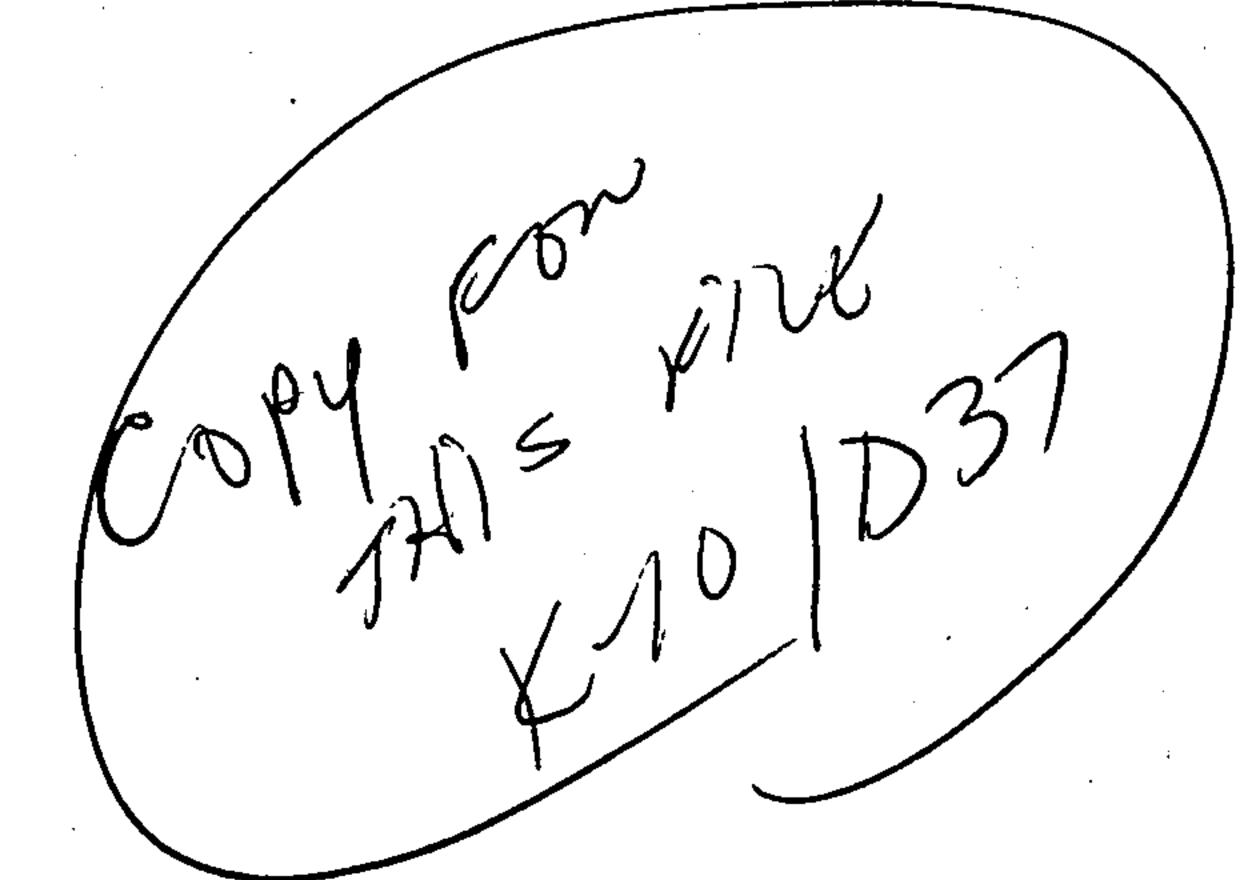
ID=1 CODE=1

PRINT HYD



City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103



April 25, 2002

David Thompson, PE
Thompson Engineering Consultants, Inc.
PO Box 15954
Rio Rancho, NM 87174

Re: Christian Apostolic Center Addendum to Grading and Drainage Plan Engineer's Stamp Dated 4-24-02, (K10/D40)

Dear Mr. Thompson,

Based on the information submitted on 4-24-02, the modification to the outlet of the detention pond is approved for Building Permit.

Prior to Certificate of Occupancy release, an Engineer Certification per the DPM checklist will be required.

If you have any questions, you can contact me at 924-3984.

1

Sincerely,

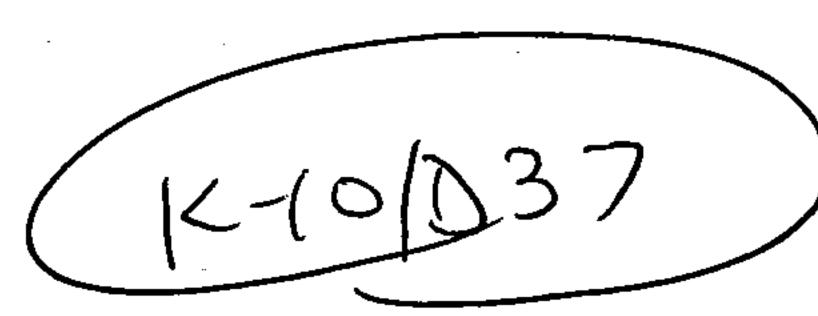
Leslie Romero

Engineering Associate, PWD

Development and Building Services

c: Diane Hoelzer, PE w/attachments
Terri Martin, Hydrology
File (2)

DRAINAGE INFORMATION SHEET



PROJECT TITLE: CHRISTIAN APOS' CENTER	TOLIC	ATLAS/DRAINAGE FILE# K10
LEGAL DESCRIPTION: Lots 6 & 7, Cliff CITY ADDRESS: Bluewater Road west ENGINEERING FIRM: THOMPSON ENGINEERING FIRM: THOMPSON ENDORESS: P.O. Box 15954, Rio Rancho, OWNER: CHRISTIAN APOSTOLIC CLADDRESS: 122 Broadway SE, Abq, NM	t of Unsea ENG. CO. NM 871 ENTER I 87102	r Blvd. NS. INC. CONTACT:DAVE THOMPSON 74 PHONE: 459-6933 CONTACT: Floyd Duran PHONE: 681-8208
ARCHITECT: Guadalupe Architects ADDRESS: 5961 Guadalupe Trail N SURVEYOR:		CONTACT: David Weatherman PHONE: 343-9305 CONTACT:
ADDRESS:		PHONE:
CONTRACTOR: NA		CONTACT: NA
ADDRESS: NA	PHONE	: NA
PRE-DESIGN MEETING: YES X NO COPY OF CONFERENCE RECAP SHEET PROVIDED		DRB NO. EPC NO. PROJECT NO.
TYPE OF SUBMITTAL:	CHECK	TYPE OF APPROVAL SOUGHT:
XX DRAINAGE REPORT		SECTOR PLAN APPROVAL
DRAINAGE PLAN		SKETCH PLAT APPROVAL
CONCEPTUAL GRADING & DRAINAGE PLAN	11-25	PRELIMINARY PLAT APPROVAL
XX GRADING PLAN	-911 	SITE DEVELOPMENT PLAN-APPROVAL
EROSION CONTROL PLAN		FINAL PLAT APPROVAL
ENGINEERS CERTIFICATION	<u>XX</u>	BUILDING PERMIT APPROVAL
FOUNDATION PERMIT		CERTIFICATE OF OCCUPANCY APPROVAL
	·	ROUGH GRADING PERMIT
		GRADING/PAVING PERMIT APPROVAL OTHER(SPECIFUP 2 2002
DATE SUBMITTED: 4-24-2002 BY: 4-24-2002 REV. 10/85		APIN SECTION

THOMPSON Engineering Consultants, Inc.

April 24, 2002

Mr. Brad Bingham, P. E.
Hydrology Development Section
Plaza del Sol – 2nd Floor West
600 2nd Street NW
Albuquerque, NM 87102

RE: REVISIONS TO THE APOSTOLOC ASSEMBLY GRADING AND DRAINAGE PLAN

Dear Mr. Bingham:

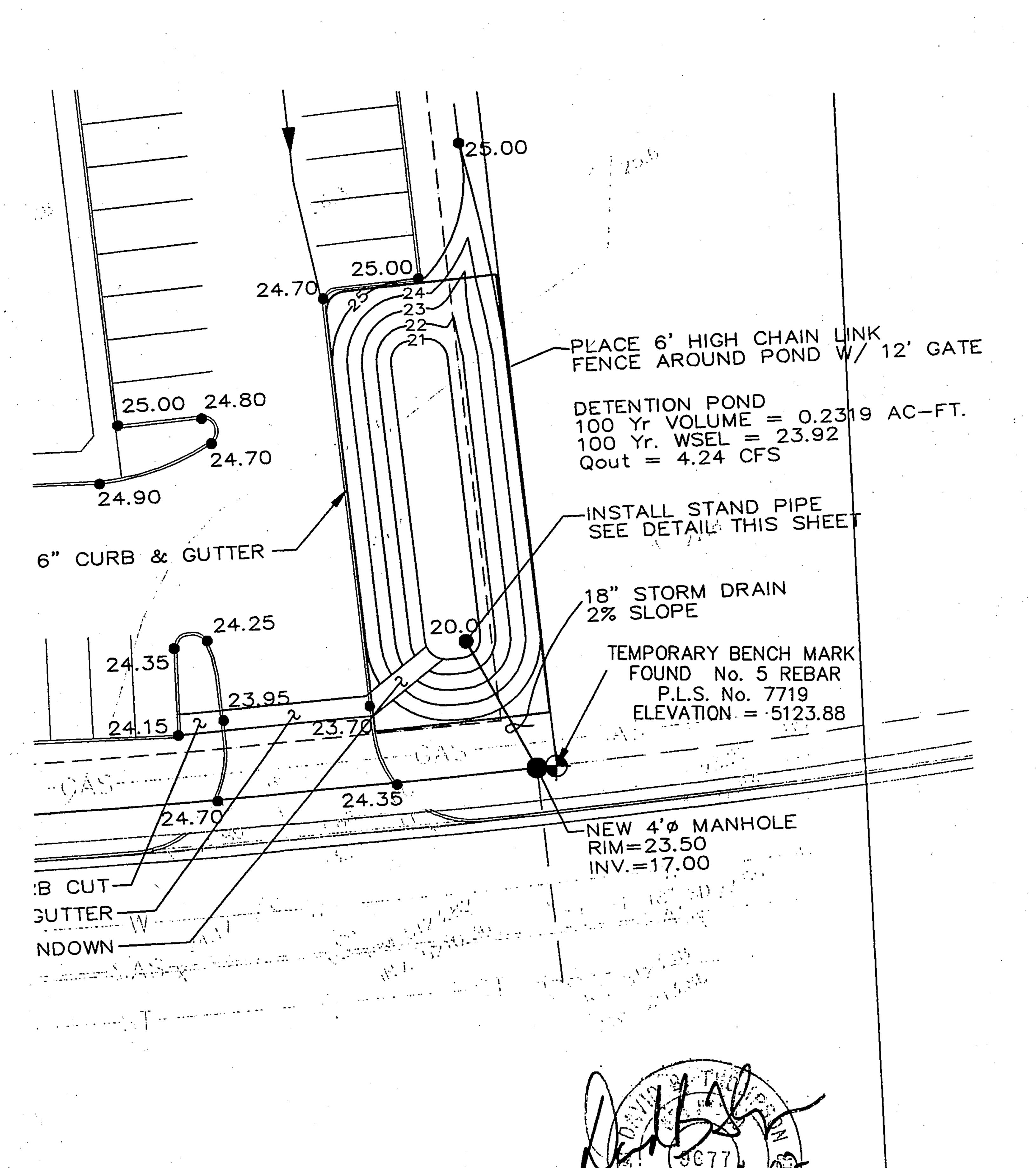
Enclosed are the revisions to the Apostolic Assembly (AA) Grading and Drainage plan to accommodate flows from the Clifford West Church (CWC) site to the north. I obtained the information from the CWC Grading and Drainage plan and input their AHYMO model into the AA AHYMO model. The standpipe was modified to allow more flow out of the detention pond on the AA site. The total flow out of the pond is 4.24 cfs, which is equal to the allowable discharge from the two sites. This was accomplished by modifying the size and number of holes in the standpipe to 5-3" holes with one-foot intervals. The detention pond volume was increased from 0.2264 acre-feet to 0.2319 acre-feet. The 100-year water surface elevation was changed from 23.85 to 23.92, which allows for 1.08 feet of freeboard.

If you should have any questions please call me at 896-7996.

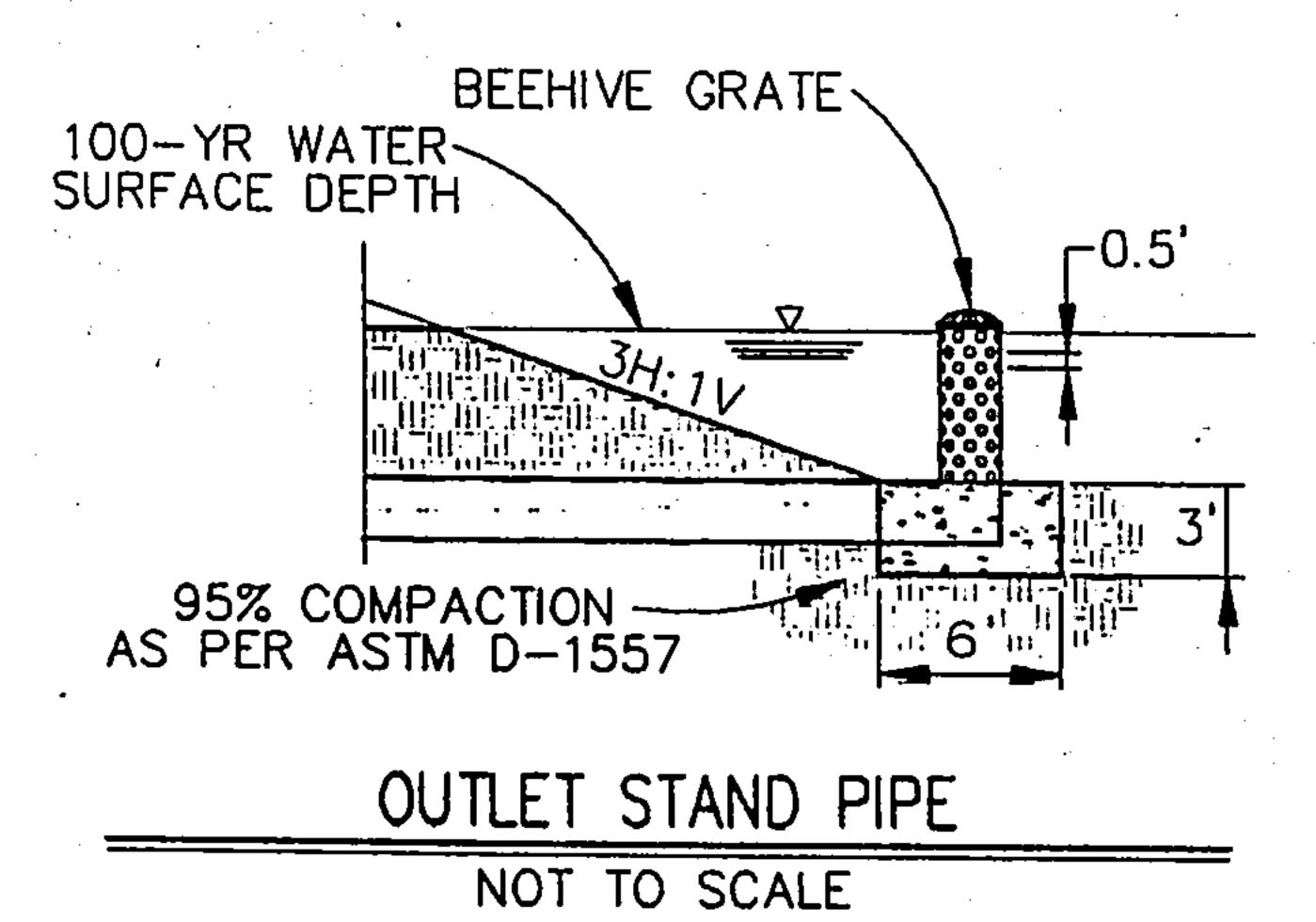
Sincerely,

David B. Thompson, P.E.

Enclosures



* 5-3" HOLES, EACH ROW, 1' INTERVALS



APOSTOLIC ASSEMBLY CHURCH RATING CURVE FOR RISER PIPE

Area for 1 - 3" dia. Hole

0.03

Area for 3 - 3" dia. Holes

0.10

Area for 5 - 3" dia. Holes

0.17

HEAD	Q	Q	Q	Q	Q	Q	Q	Q	Q	Q	TOTAL
(feet)	level 1	level 2	level 3	level 4	level 5	level 6	level 7	level 8	level 9	level 10	Q
0.00	·			• 1 L	in and the same of the same of					· · · · · · · · · · · · · · · · · · ·	0.00
0.25	0.41									•	0.41
0.50	0.58										0.58
0.75	0.71	-									0.71
1.00	0.82										0.82
1.25	0.92	0.41			e and a second of the second o		· • · • • <u>· · · · · · · · · · · · · · ·</u>		Tark are by to rap at be		1.33
1.50	1.00	0.58					•			•	1.58
1.75	1.09	0.71			•						1.80
2.00	1.16	0.82									1.98
2.25	1.23	0.92	0.41						reserved de la compresa del compresa de la compresa de la compresa del compresa de la compresa del la compresa del la compresa de la compresa del la compresa de la compresa del la compresa de la compresa de la compresa del la compresa de la compresa del	, 1964 7 4004 1 6 1 6 1 6 1 6 1 6 1 6 1 6 1 6 1 6 1	2.56
2.50	1.30	1.00	0.58	•			•				2.88
2.75	1.36	1.09	0.71								3.16
3.00	1.42	1.16	0.82								3.40
3.25	1.48	1.23	0.92	0.41							4.04
3.50	1.53	1.30	1.00	0.58		-	•				4.42
3.75	1.59	1.36	1.09	0.71				· .	-		4.74
4.00	1.64	1.42	1.16	0.82							4.74 5.04

```
AHYMO PROGRAM (AHYMO_97) - - Version: 1997.02c

RUN DATE (MON/DAY/YR) = 04/24/2002

START TIME (HR:MIN:SEC) = 10:10:53 USER NO.= AHYMO-I-9702a01000K21-AH
```

INPUT FILE = C:\Projects\APOSTO~1\AADHYD~1.TXT

APOSTOLIC CHURCH HYDROLOGIC MODEL--DEVELOPED CONDITIONS 4 MAY 2002

HYDROLOGIC MODEL FOR BASIN 100 100-YEAR, 24-HOUR STORM:

PRECIPITATION:

P60 = 1.87" P360 = 2.20" P1440 = 2.66"

START

TIME=0.0 HR PUNCH CODE=0

24-Hour rainfall distribution based on Montoyas Report

RAINFALL

TYPE=2 RAIN QUARTER=0.0 IN
RAIN ONE=1 77 IN DAIN

RAIN ONE=1.77 IN RAIN SIX=2.18 IN RAIN DAY=2.75 IN DT=0.05 HRS

COMPUTED 24-HOUR RAINFALL DISTRIBUTION BASED ON NOAA ATLAS 2 - PEAK AT 1.40 HR. .050000 HOURS DT =END TIME = 24.000000 HOURS .0000 .0039 .0080 .0122 .0166 .0210 .0257 .0305 .0355 .0408 .0462 .0519 .0578 .0641 .0707 .0777 .0851 .0930 .1014 .1105 .1203 .1278 .1360 .1599 .2164 .3134 .4635 .6797 .9753 1.2084 1.3137 1.4016 1.4786 1.5475 1.6099 1.6668 1.7190 1.7671 1.8114 1.8524 1.8903 1.8996 1.9082 1.9163 1.9239 1.9312 1.9380 1.9446 1.9509 1.9569 1.9627 1.9683 1.9737 1.9789 1.9840 1.9889 1.9937 1.9984 2.0029 2.0073 2.0117 2.0159 2.0200 2.0241 2.0280 2.0319 2.0357 2.0394 2.0431 2.0467 2.0502 2.0537 2.0571 2.0604 2.0638 2.0670 2.0702 2.0734 2.0765 2.0796 2.0826 2.0856 2.0885 2.0914 2.0943 2.0971 2.0999 2.1054 2.1081 2.1108 2.1027 2.1135 2.1161 2.1187 2.1212 2.1238 2.1263 2.1287 2.1312 2.1336 2.1360 2.1384 2.1408 2.1431 2.1454 2.1477 2.1500 2.1523 2.1545 2.1567 2.1589 2.1611 2.1633 2.1654 2.1676 2.1697 2.1718 2.1739 2.1759 2.1780 2.1800 2.1824 2.1848 2.1872 2.1895 2.1919 2.1942 2.1966 2.1989 2.2036 2.2059 2.2013 2.2082 2.2105 2.2128 2.2151 2.2173 2.2196 2.2219 2.2241 2.2264 2.2286 2.2309 2.2331 2.2353 2.2375 2.2397 2.2419 2.2441 2.2463 2.2485 2.2506 2.2528 2.2550 2.2571 2.2593 2.2614 2.2635 2.2656 2.2678 2.2699 2.2720 2.2741 2.2762 2.2783 2.2803 2.2824 2.2845 2.2866 2.2886 2.2907 2.2927 2.2947 2.2968 2.2988 2.3008 2.3028 2.3049 2.3069 2.3089 2.3109 2.3128 2.3148 2.3168 2.3188 2.3207 2.3227 2.3247 2.3266 2.3286 2.3305 2.3324 2.3344 2.3363 2.3382 2.3401 2.3420 2.3439 2.3458 2.3477 2.3496 2.3515 2.3534 2.3553 2.3571 2.3590 2.3609 2.3627 2.3646 2.3664 2.3683 2.3701 2.3719 2.3738 2.3756 2.3774 2.3792 2.3810 2.3828 2.3846 2.3864 2.3882 2.3900 2.3918 2.3936 2.3953 2.3971 2.3989 2.4006 2.4024 2.4042 2.4059 2.4077 2.4094 2.4111 2.4129 2.4146 2.4163 2.4180 2.4198 2.4215 2.4232 2.4249 2.4266 2.4283 2.4300 2.4317 2.4334 2.4350 2.4367 2.4384 2.4401 2.4417 2.4434 2.4450 2.4467 2.4484 2.4500 2.4516 2.4533 2.4549 2.4566 2.4582 2.4598 2.4614 2.4631 2.4647 2.4663 2.4679 2.4695 2.4711 2.4727 2.4743 2.4759 2.4775 2.4791 2.4807 2.4822 2.4838 2.4854 2.4870 2.4885 2.4901 2.4916 2.4932 2.4947 2.4963 2.4978 2.4994 2.5009 2.5025 2.5040 2.5055 2.5071 2.5086 2.5101 2.5116 2.5131 2.5146 2.5162 2.5177 2.5192 2.5207 2.5222 2.5237 2.5252 2.5266 2.5281 2.5296 2.5311 2.5326 2.5340 2.5355 2.5370 2.5384

2.5399 2.5414 2.5428 2.5443 2.5457 2.5472 2.5486

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                        2.6576
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                2.7157
                        2.7169
                                2.7181
                                        2.7192
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                                        2.7273
                                               2.7285
2.7296 2.7308
                2.7319
                        2.7330
                                2.7342
                                        2.7353
                                               2.7365
2.7376 2.7387 2.7399
                        2.7410
                               2.7421 2.7433 2.7444
2.7455 2.7466 2.7478
                        2.7489
                                2.7500
```

LENGTH=600 FT SLOPE=0.02 K=3

LCODE=1 NK=1 ISLOPE=0

TC AND TP COMPUTED BY UPLAND/LAG TIME PROCEDURE

SCS UPLAND METHOD FACTORS

COMPUTE LT TP

LENGTH (FT) SLOPE (FT/FT) COMPOSITE K SHEET FLOW PORTION .000000 .0000 SHALLOW FLOW PORTION .000000 .0000 CHANNEL FLOW PORTION 600.0 .020000 3.0000 TOTAL BASIN 600.0 .020000 3.0000

TIME OF CONCENTRATION (HRS) = .0393 TIME TO PEAK (HRS) = .0262 LAG TIME (HRS) = .0295

TIME TO PEAK COMPUTED TO BE LESS THAN 0.133333 HOUR MINIMUM VALUE.

REVISED VALUES: TIME OF CONCENTRATION (HRS) = .2000 TIME TO PEAK (HRS) = .1333 LAG TIME

(HRS) = .1500

COMPUTE NM HYD ID=1 HYD NO=1000 DA=.00393401 SQ MI %A=0 %B=22 %C=0 %D=78

TP=0.0 HR
MASS RAINFALL=-1

TIME TO PEAK (hrs) = .1333

K = .072666HR TP = .133333HR K/TP RATIO = .545000 SHAPE CONSTANT, N = 7.106420 UNIT PEAK = 12.112 CFS UNIT VOLUME = .9983 B = 526.28 P60 = 1.7700 AREA = .003069 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .050000

K = .132784HR TP = .133333HR K/TP RATIO = .995885 SHAPE CONSTANT, N = 3.545063 UNIT PEAK = 2.1006 CFS UNIT VOLUME = .9947 B = 323.60 P60 = 1.7700 AREA = .000865 SQ MI IA = .50000 INCHES INF = 1.25000 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .050000

PRINT HYD ID=1 CODE=1

HYDROGRAPH FROM AREA 1000.00

RUNOFF VOLUME = 2.09300 INCHES = .4391 ACRE-FEET
PEAK DISCHARGE RATE = 9.07 CFS AT 1.500 HOURS BASIN AREA = .0039 SQ. MI.

ROUTE BASIN 1000 THRU DETENTION ROUTE RESERVOIR ID=2 HYD=1000.5 INFLOW ID=1 CODE=5 OUTFLOW (CFS) STORAGE (AC FT) ELEV (FT) 0.63 0.03946 1.25 0.07891 2.05 0.22079 TIME INFLOW **ELEV** VOLUME OUTFLOW (HRS) (CFS) (FEET) (AC-FT) (CFS) € .00 .00 .00 .000 .00 .25 .00 .00 .000 .00 .50 .00 .00 000 በበ .90 .67 .51 .38 .11 .10

.50	.00	.00	.000	.00	•
.75 1.00	.00 .09	.00	.000	.00	
1.25	.99	.00	.000	, .00	
1.50	9.07	.08	.006	.10	
1.75		1.05	.093	1.33	
2.00	3.49	1.36	.180	1.82	
	2.08	1.42	.197	1.92	
2.25	. 62	1.37	185	1.85	
2.50	.31	1.28	.157	1.69	•
2.75	.18	1.18	.129	1.53	•
3.00	.12	1.08	.102	1.38	
3.25	.09	. 97	.077	1.22	
3.50	.08	.72	.057	.90	
3.75	.07	.54	.042	. 67	
4.00	.07	.40	.032	.51	
4.25	.07	.30	.024	.38	
4.50	.07	.23	.018	.29	
4.75	.07	.18	.014	.23	
5.00	.07	.15	.012	.19	
5.25	.07	.12	.010	.15	
5.50	.07	.10	.008	.13	
5.75	.08	.09	.007	.11	
6.00	.08	.08	.007	.10	
6.25	.09	.08	.006	.10	
6.50	.09	.08	.006	.10	
6.75	.09	.08	.006	.10	
7.00	.09	.07	.006	.09	
7.25	.09	.07	.006	.09	
7.50	.09	.07	.006	.09	
7.75	.09	.07	.006	.09	
8.00	.08	.07	.006	.09	
8.25	.08	.07	.005	.09	
8.50	.08	.07	.005	.09	
8.75	.08	.07	.005		
9.00	.08	.07	.005	.08	
9.25	.08	.07	.005	.08	· · · · · · · · · · · · · · · · · · ·
9.50	.08	.06		.08	
9.75	08		.005	.08	
10.00	.08	.06	.005	.08	
10.25		.06	.005	.08	
10.23	.07	.06	.005	.08	
	.07	.06	.005	.08	
10.75	.07	.06	.005	.08	•
11.00	.07	.06	.005	.07	
11.25	.07	.06	.005	.07	¬.
11.50	.07	.06	.005	.07	
11.75	.07	.06	.005	.07	
12.00	.07	.06	.004	.07	
12.25	.07	.06	.004	.07	
12.50	.07	.05	.004	.07	
12.75	.07	.05	.004	.07	
13.00	.07	.05	.004	.07	
13.25	.06	.05	.004	.07	
13.50	.06	.05	.004	.07	·
13.75	.06	.05	.004	.07	
m Then				-	
TIME	INFLOW	ELEV	VOLUME	OUTFLOW	
(HRS)	(CFS)	(FEET)	(AC-FT)	(CFS)	
14 00	0.0		.		•
14.00	.06	.05	.004	.06	
14.25	.06	.05	.004	.06	••
14.50	.06	.05	.004	.06	
14.75	.06	.05	.004	.06	
15.00	.06	.05	.004	.06	
15.25	.06	.05	.004	.06	
15.50	.06	.05	.004	.06	
15.75	.06	.05	.004	.06	
16.00	.06	.05	.004	.06	••

13.00	.00	.05	.004	.0/
13.75	.06	.05	.004	.07
TIME (HRS)	INFLOW (CFS)	ELEV (FEET)	VOLUME (AC-FT)	OUTFLOW (CFS)
14.00 14.25 14.50 14.75 15.00 15.25 15.50	.06 .06 .06 .06 .06	.05 .05 .05 .05 .05	.004 .004 .004 .004 .004 .004	.06 .06 .06 .06 .06
16.00	.06	.05	.004	.06

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16.25
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 PEAK DISCHARGE =
                           1.917 CFS - PEAK OCCURS AT HOUR
                                                                  2.05
 MAXIMUM WATER SURFACE ELEVATION =
                                              1.417
 MAXIMUM STORAGE =
                             .1972 AC-FT
                                                INCREMENTAL TIME=
                                                                         .050000HRS
PRINT HYD
                      ID=2 CODE≃1
                                             HYDROGRAPH FROM AREA 1000.50
    RUNOFF VOLUME =
                          2.09292 INCHES
                                                         .4391 ACRE-FEET
    PEAK DISCHARGE RATE =
                                   1.92 CFS AT
                                                   2.050 HOURS
                                                                   BASIN AREA =
*COMPUTE TIME TO PEAK USING UPLAND/LAG TIME
```

.0039 SQ. MI.

```
*S******
*S BASIN 100 - ONSITE
*S******
```

COMPUTE LT TP LCODE=1 NK=1 ISLOPE=0

LENGTH=600 FT SLOPE=0.02 K=3

TC AND TP COMPUTED BY UPLAND/LAG TIME PROCEDURE

SCS UPLAND METHOD FACTORS

· .	LENGTH (FT)	SLOPE (FT/FT)	COMPOSITE K
SHEET FLOW PORTION	.0	.000000	.0000
SHALLOW FLOW PORTION	.0	.000000	.0000
CHANNEL FLOW PORTION	600.0	.020000	3.0000
TOTAL BASIN	600.0	.020000	3.0000

TIME OF CONCENTRATION (HRS) = .0393 TIME TO PEAK (HRS) = .0262 LAG TIME (HRS) =

TIME TO PEAK COMPUTED TO BE LESS THAN 0.133333 HOUR MINIMUM VALUE. REVISED VALUES: TIME OF CONCENTRATION (HRS) = .2000 TIME TO PEAK (HRS) = .1333 LAG TIME (HRS) =.1500

ID=3 HYD NO=100 DA=.00430 SQ MI %A=0 %B=9 %C=9 %D=82

TP=0.0 HR
MASS RAINFALL=-1

TIME TO PEAK (hrs) = .1333

K = .072666HR TP = .133333HR K/TP RATIO = .545000 SHAPE CONSTANT, N = 7.106420 UNIT PEAK = 13.917 CFS UNIT VOLUME = .9983 B = 526.28 P60 = 1.7700 AREA = .003526 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .050000

K = .118775HR TP = .133333HR K/TP RATIO = .890811 SHAPE CONSTANT, N = 3.981133 UNIT PEAK = 2.0545 CFS UNIT VOLUME = .9953 B = 353.91 P60 = 1.7700 AREA = .000774 SQ MI IA = .42500 INCHES INF = 1.04000 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .050000

PRINT HYD

ID=3 CODE=1

PARTIAL HYDROGRAPH 100.00

RUNOFF VOLUME = 2.19373 INCHES = .5031 ACRE-FEET
PEAK DISCHARGE RATE = 10.35 CFS AT 1.500 HOURS BASIN AREA = .0043 SQ. MI.

*S COMBINE BASIIN 1000.5 WITH 100

ADD HYD PRINT HYD

6.00

.20

ID=4 HYD=100.5 ID I=3 ID II=2 ID=4 CODE=1

PARTIAL HYDROGRAPH 100.50

RUNOFF VOLUME = 2.14538 INCHES = .9421 ACRE-FEET
PEAK DISCHARGE RATE = 11.68 CFS AT 1.500 HOURS BASIN AREA = .0082 SQ. MI.

*S ROUTE BASIN 100.5 THRU DETENTION POND

ROUTE RESERVOIR ID=6 HYD=100.9 INFLOW ID=4 CODE=5 OUTFLOW (CFS) STORAGE (AC FT) ELEV (FT) 0.71 0.0230 1.71 0.0788 2.94 0.1551 4.36 0.2390

TIME INFLOW ELEV VOLUME OUTFLOW (HRS) (CFS) (FEET) (AC-FT) (CFS) .00 .00 .00 .000 .00 .25 .00 .00 .000 .00 .50 .00 .00 .000 .00 .75 .00 .00 .000 .00 1.00 .11 .01 .000 .01 1.25 1.24 .30 .007 .21 1.50 11.68 2.42 .111 2.23 1.75 5.83 3.71 .215 3.95 2.00 4.30 3.92 .232 4.24 2.25 2.55 3.73 .216 3.97 2.50 2.04 3.37 .186 2.75 1.73 3.04 .158 2.99 3.00 1.51 2.72 .134 2.60 3.25 1.32 2.45 .113 2.26 3.50 1.00 2.19 .093 1.95 3.75 .76 1.92 .074 1.63 4.00 .59 1.62 .058 1.33 4.25 .46 1.37 .044 1.08 4.50 .37 1.16 .032 .87 4.75 .31 .99 .023 .71 5.00 .26 .71 .016 .51 5.25 .23 .54 .012 .38 5.50 .21 .43 .010 .31 5.75 .37 .008 .26

.33

.007

007	006	007

26.25 .01 .000 26.50 .00 .01 .000 .00 PEAK DISCHARGE 4.240 CFS - PEAK OCCURS AT HOUR 2.00 MAXIMUM WATER SURFACE ELEVATION = 3.915

MAXIMUM STORAGE =

.2319 AC-FT

INCREMENTAL TIME=

PRINT HYD

ID=6 CODE=1

PARTIAL HYDROGRAPH 100.90

RUNOFF VOLUME = 2.14538 INCHES .9421 ACRE-FEET PEAK DISCHARGE RATE = 4.24 CFS 2.000 HOURS AΤ .0082 SQ. MI. BASIN AREA =

NORMAL PROGRAM FINISH

END TIME (HR:MIN:SEC) = 10:10:53



City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

Public Works Department Transportation Development Services Section

September 13, 2002

Ralph Walden, Reg. Architect Paul & Associates Inc. 5820 Old Bollard Rd., Suite 128 Tyler, Tx. 75703

Re:

Approval of Temporary Certificate of Occupancy (C.O.) for

Fellowship Missionary Baptist Church, [K-10 / D037]

8570 Saul Bell Rd. N.W.

Architect's Stamp Dated 09/13/02

Dear Mr. Walden:

Based on the information provided on your submittal dated September 13, 2002, the above referenced project is approved for a 30-day Temporary C.O. (Temp).

A Temp has been issued allowing the submittal of an exact copy of the acceptable, approved TCL (Traffic Circulation Layout [Site Plan]) and Letter of Certification (including the word "certify"), to be completed within this time period. Another copy <u>closely</u> similar to the TCL is acceptable however, more time will be required to verify the copy before issuing the Final C.O. If a DRB Site Plan, with a signature block and appropriate signatures, is part of the City approved field set, a copy of that plan is required.

Once the site is again Certified to be in substantial compliance, and the Cert, for Transportation, has been resubmitted for evaluation and is approved, a Permanent (Final) C.O. will be issued. The Cert includes a Letter of Certification and the stamped, signed TCL. Either plan and/or the letter needs to be stamped by the designer, signed and dated for that Certification.

In addition to the Cert required, issues to be resolved within this time period are as follows: 1) Removal of all barricading required to separate vehicles/pedestrians from incomplete areas, 2) removal of all construction fencing and all equipment and refuse/compactors, in the traffic/pedestrian circulation areas and 3) removal of all landscape material (gravel, bark, etc.) from parking stalls. From the time of this Temp, up to Certification for Final C.O., make sure material is contained in single stall or stalls needed to hold landscape material, keeping surrounding area clean of this material, to relieve liability.

Submit the Cert along with the fully completed Drainage and Transportation Information Sheet (enclosed) to front counter personnel or mail it in for log-in and evaluation by Transportation. If a local representative is assigned to submit the Cert, make sure he/she has enough information to <u>FULLY</u> complete the D & T Information Sheet ("surveyor" and "contractor" are not so critical).

If you have any questions, please call me at 924-3620.

Sincerely,

Mike Zamora, Commercial Plan Checker

Development and Building Services

Plánning Department

c: Engineer
Hydrology file
Mike Zamora

DRAINAGE AND TRANSPORTATION INFORMATION SHEET

(REV. 1/11/2002)

PROJECT TITLE: FISLLOWShip Missionary BATIST CHURCE DRB #:	
DRB #:EPC#:EPC#:EPC#:	E MAP/DRG. FILE(#: /-/0/)3/ RK ORDER#:
	UEST BUISHESS PARK
ENGINEERING FIRM: Mark Godun 8/550cb. ADDRESS: CITY, STATE:	CONTACT: Jone Hole
OWNER: FELLOWSLIP MISSION APY BAPTIST CHURCH	ZIP CODE:
CITY, STATE: ALBUQUERQUE, NH 87/195	CONTACT:PHONE: ZIP CODE:
ARCHITECT: RALPH WALDERY ADDRESS: 5820 OLD BUILARD RD. 575 128	CONTACT:
CIT, STATE: 1916, 1613 25102	PHONE: ZIP CODE:
SURVEYOR: ADDRESS CITY, STATE:	CONTACT:PHONE:
CONTRACTOR:	ZIP CODE:
ADDRESS:CITY, STATE:	CONTACT: PHONE: ZIP CODE:
CHECK TYPE OF SUBMITTAL: DRAINAGE REPORT CHECK TYPE CHECK TYPE CHECK TYPE	E OF APPROVAL SOUGHT:
—— CONCEPTUAL GRADING & DRAINAGE PLAN ——— S. DE	FINANCIAL GUARANTEE RELEASE LIMINARY PLAT APPROVAL V. PLAN FOR SUB'D. APPROVAL V. PLAN FOR BLDG. PERMIT APPROVAL
—— ENGINEER'S CERTIFICATION (HYDROLOGY) —— SECT —— CLOMR/LOMR —— FINAL	OR PLAN APPROVAL L PLAT APPROVAL NDATION PERMIT APPROVAL
ENGINEERS CERTIFICATION (TCL) FNGINEERS CERTIFICATION (DCC) CERT	DING PERMIT APPROVAL IFICATE OF OCCUPANCY (PERM.) IFICATE OF OCCUPANCY (TEMP.)
PAVIN	VG PERMIT APPROVAL
	K ORDER AFFROYAL SED 3
WAS A PRE-DESIGN CONFERENCE ATTENDED: YES	HYDROLOGY 2002
NO COPY PROVIDED	HYDROLOGY SECTION
DATE SUBMITTED: 9/3/02 BY: MILLIUM:	
· Die Land	
Requests for approvals of Site Development Plans and/or Subdivision Plats sharper particular nature, location and scope of the proposed development defined more of the following levels of submittal may be required based on the following 1. Conceptual Grading and Drainage Plans Research	S the degree of drainage detail Oss s. *** **
1. Conceptual Grading and Drainage Plan: Required for approval of 2. Drainage Plans: Required for building permits, grading permits, pay 3. Drainage Report: Required for subdivisions containing more than tempore.	Site Development Plans groots at a contract of the state
9/13/02 - Cldm 30t=pp///s; -9/16 -	-Sout Lefter dated 9/13) - logg! Need for Final; This time, but Nonce

8820 OLD BULLARD RD., STE. 128 - TYLER, TEXAS 75703

Mr. Mike Zomonia City of Affriculture PO Hex 1293 Africanciae, New Mexico 87103

RE: Fellowship Missionary Baptist Church

Dear Mr. Zammuna

This letter is to certify the site improvements indicated on our permit drawing C 1.01 6/25/01 have been carried out in the field in a manner consistent with this drawing. Therefore please accept this letter as a certification of these improvements.

Thank you for your affention to this matter.

Sincereig

C: Melenia

SEP 1 3 2002

HYDROLOGY SECTION

09/13/2002

11 元 1835年 安徽



City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

April 5, 2001

Diane Hoelzer, P.E.
Mark Goodwin & Assoc.
P.O. Box 90606
Albuquerque, NM 87199

RE: CLIFFORD WEST CHURCH, Clifford West Business Park Unit 17 (K10-D37).

Supplemental Information (GRADING AND DRAINAGE PLAN) FOR SITE

DEVELOPMENT PLAN FOR BUILDING PERMIT APPROVAL. ENGINEER'S

STAMP DATED APRIL 4, 2001. G&D Plan Itself Stamped MARCH 8, 2001.

Dear Ms. Hhelzer:

Thank you for furnishing the design calculations for the Pond Volumes shown on the Grading and Drainage Plan. This clears the way for the approval of the Building Permit itself. .

This letter supercedes C.O.A. letter dated 3/29/01 subject as above.

If I can be of further assistance, please feel free to contact me at 924-3984.

Sincerely,

John P. Murray, P.E.

Hydrology

0

Terri Martin

DRAINAGE INFORMATION SHEET

PROJECT TITLE:	Clifford West Church		ZONE ATLA	S#:	K 10 / D 37
DRB#:	EPC#	WORK ORDER#:			
LEGAL DESCRIPTION:	Lot 10, 11, 12, Block 2, Clifford W	est Busir	ness Park Unit 1		
CITY ADDRESS:					
			00117107		
•	Mark Goodwin & Associates, PA		CONTACT:	•	· · · · · · · · · · · · · · · · · · ·
ADDRESS:	P.O. Box 90606, Albuquerque, NM 87	PHONE:	828-2200		
OWNER:			CONTACT:		<u>. </u>
ADDRESS:		•	PHONE:	·	- · · · · · · · · · · · · · · · · · · ·
ARCHITECT:	Paul and Associates		CONTACT:	Chad Young	<u> </u>
ADDRESS:	5620 Bullard Rd.#128, Tyler, Texas 7	5703	PHONE:	903-581-8322	
SURVEYOR:	ALS	<u></u>	CONTACT:	TIM ALDRICE	-
ADDRESS:	4109 MONTGOMERY BLVD.		PHONE:	884-1990	<u> </u>
CONTRACTOR:		CONTACT:			
ADDRESS:			PHONE:		
TYPE OF SUBMITTA	L:	СНЕ	CK TYPE OF APP	ROVAL SOUGI	HT:
X DRAINAGE F	REPORT (Supplemental information)		SKETCH PLAT	APRROVAL	
DRAINAGE F	PLAN		PRELIMINARY	PLAT APRRO	VAL
CONCEPTUA	AL GRADING & DRAINAGE PLAN	S. DEV. PLAN	S. DEV. PLAN FOR SUB'D APPRO		
GRADING PL	AN	S. DEV. PLAN FOR BLDG PERMIT APPROVA			
EROSION CO	ONTROL		SECTOR PLAN	N APPROVAL	
ENGINEER'S	CERTIFICATION		FINAL PLAT A	PPROVAL	
OTHER			FOUNDATION	PERMIT APPR	OVAL
EASEMENT	ACATION		BUILDING PEF	RMIT APPROVA	\L
		7	CERTIFICATIO	N OF OCCUPA	ANCY APPROVAL
PRE-DESIGN MEETI	NG:		GRADING PER	RMIT APPROVA	\L
YES			PAVING PERM	IIT APPROVAL	
X NO			S.A.D. DRAINA	GE REPORT	
COPY PROV	DED		– DRAINAGE RE	QUIREMENTS	
		·	OTHER		
			RELEASE OF	FINANCIAL GU	ARANTY
			TRAFFIC CIRC	ULATION LAY	TUC
				国 (C)[国]	WE
DATE SUBMITTED:	APRIL 4, 2001			APR 04	2001
BY: ////	ZED DE	 			CECTIONI
DIANE HOEL			1 1 1 X	DROLOGY	SECTION

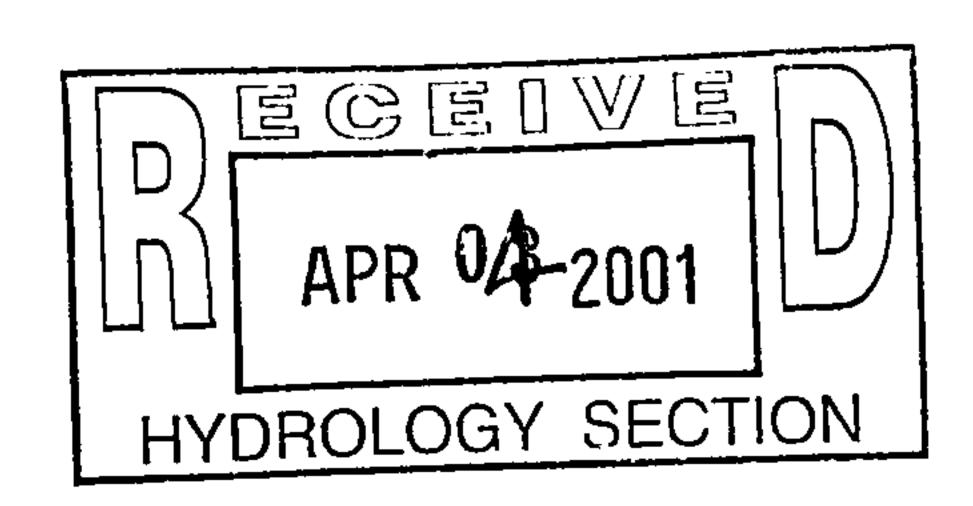
D. Mark Goodwin and Associates, P.A. Consulting Engineers

P.O. Box 90606 Albuquerque, NM 87199 (505) 828-2200 (505) 797-9539 fax e-mail: dmg@swcp.com

TO: <u>John Murray</u>, PE

LETTER OF TRANSMITTAL

We ar	Hydro One S	top Del Sol	PE	RE: CLIFFORD WEST CHURCH K10 / D37	
	Copies	Date		Description	
	1		Requested supple	emental drainage information	
			or your files s you requested	For your information For your comments	
			Pre-D	esign Meeting	
				for the Grading and Drainage Plan. Please calional information. Thanks,	ai
Projec	ct Engine	_	ne Hoelzer, PE		





ALBUQUERQUE, NEW MEXICO 87103

March 29, 2001

Diane Hoelzer, P.E. Mark Goodwin & Assoc. P.O. Box 90606 Albuquerque, NM 87199

CLIFFORD WEST CHURCH, Clifford West Business Park Unit 1 (K10-D37). RE: GRADING AND DRAINAGE PLAN FOR SITE DEVELOPMENT PLAN FOR BUILDING PERMIT APPROVAL. ENGINEER'S STAMP DATED MARCH 8, 2001.

Dear Ms, Hoelzer:

Based on the information provided on your March 8, 2001 submittal, the above referenced project is approved for SITE DEVELOPMENT PLAN for Building Permit (for DRB action). This G&D Plan should be labeled "Conceptual."

Prior to the issuance of the Building Permit, furnish the necessary data and details for the design of the proposed system, e.g., sizing of the ponds, etc.

If I can be of further assistance, please feel free to contact me at 924-3984.

Sincerely,

John P. Murray, P.E.

Hydrology

Terri Martin

File

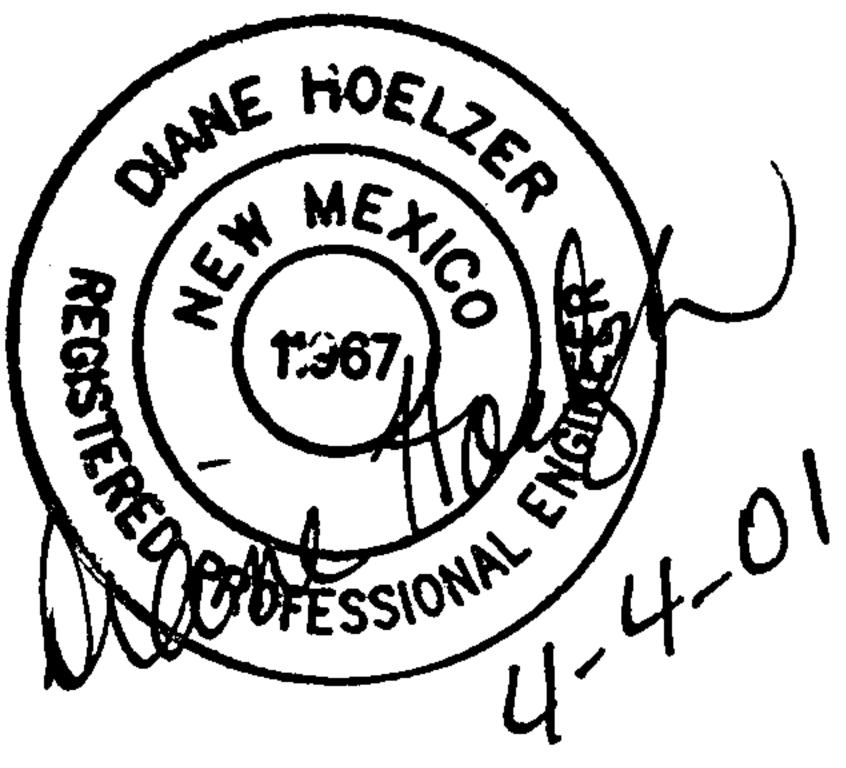
CLIFFORD WEST CHURCH

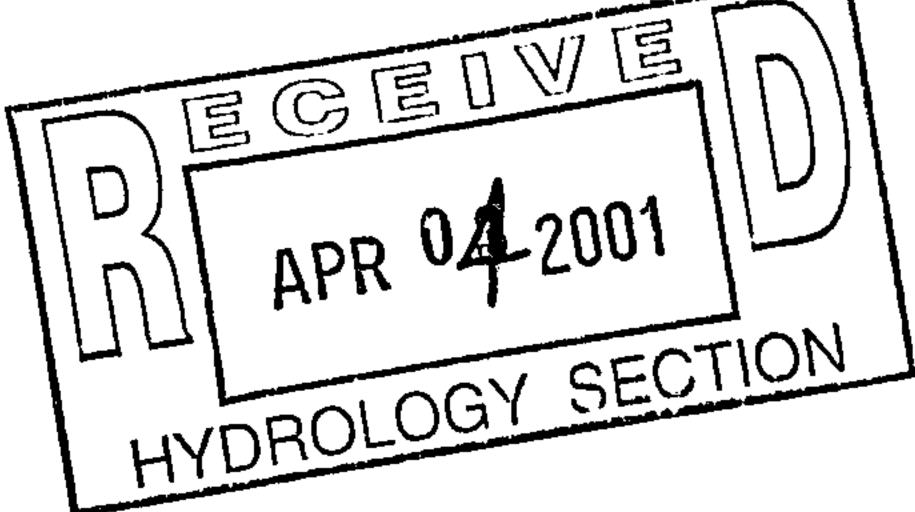
Supplemental Drainage Information:

AHYMO Input & Output

Orifice and Parking Area Pond Volume Calculations

HEC-2 Outfall channel Calculations





April 4, 2001

Mark Goodwin & Associates, PE

AHYMO SUMMARY TABLE (AHYMO194) - AMAFCA Hydrologic Model - January, 1994 INPUT FILE = $c_{church.dat}$

RUN DATE (MON/DAY/YR) = 03/05/2001USER NO. = M_GOODWN.I01

•	HYDROGRAPH	FROM ID	TO ID	AREA	PEAK DISCHARGE	RUNOFF VOLUME	RUNOFF	TIME TO PEAK	CFS PER	PAGE =	1
COMMAND	IDENTIFICATION	NO.	NO.	(SQ MI)	(CFS)	(AC-FT)	(INCHES)	(HOURS)	ACRE	NOTATI	ON
START	•									TIME=	.00
RAINFALL TYPE	= 1					•				RAIN6=	2.200
COMPUTE NM HYD	100.SB1	_	1	.00138	3.47	. 124	1.68355	1.499	3.928	PER IMP=	78.00
ROUTE RESERVOI	R POND. 100	1	3	.00138	. 61	. 124	1.68333	2.098	. 696	AC-FT=	.070
COMPUTE NM HYD	100.SB2	.	1	.00393	9.86	. 353	1.68355	1.499	3.916	PER IMP=	78.00
ROUTE RESERVOI	R POND. 100	1	3	.00393	1.99	. <i>353</i>	1.68348	2.065	. 792	AC-FT=	. 211
FINISH		-		-		•					

AHYMO PROGRAM (AHYMO194) - AMAFCA Hydrologic Model - January, 1994

RUN DATE (MON/DAY/YR) = 03/05/2001

START TIME (HR:MIN:SEC) = 11:44:19 USER NO. = M_GOODWN.I01

INPUT FILE = c church.dat

START

TIME=0.0

***** CHURCH IN THE CLIFFORD WEST BUSINESS PARK

***** LOT 10, 11, 12, BLOCK 2

***** 100-YEAR 6-HOUR STORM EVENT

***** FILE: C_CHURCH.DAT MARCH 5, 2001 BY:DLH

RAINFALL

TYPE=1 RAIN QUARTER=0.0 IN

RAIN ONE=1.90 IN RAIN SIX=2.20 IN RAIN DAY=0.0 IN DT=0.0333 HR

COMPUTED 6-HOUR RAINFALL DISTRIBUTION BASED ON NOAA ATLAS 2 - PEAK AT 1.40 HR.

DT =	.033300 н	OURS	END T	IME =	5.9940	00 HOURS
. 0000	.0013	.0027	.0041	. 0055	.0069	.0084
.0099	.0115	. 0131	.0147	.0164	.0181	.0199
. 0217	. 0236	. 0255	. 0275	. 0296	.0317	.0339
. 0362	.0386	.0411	.0437	.0464	. 0492	. 0522
. 0553	. 0586	.0621	.0673	.0729	.0789	.0913
. 1196	. 1634	. 2264	.3124	. 4256	.5701	. 7500
. 9698	1.1808	1.2673	1.3402	1.4049	1.4637	1.5178
1.5681	1.6150	1.6590	1.7004	1.7395	1.7764	1.8113
1.8444	1.8758	1.9056	1.9338	1.9607	1.9678	1.9733
1.9786	1.9836	1.9884	1.9930	1.9974	2.0016	2.0056
2.0095	2.0133	2.0170	2.0205	2.0240	2.0273	2.0306
2.0337	2.0368	2.0398	2.0428	2.0456	2.0484	2.0512
2.0538	2.0565	2.0590	2.0615	2.0640	2.0664	2.0688
2.0712	2.0735	2.0757	2.0779	2.0801	2.0823	2.0844
2.0865	2.0885	2.0906	2.0925	2.0945	2.0965	2.0984
2.1003	2.1021	2.1039	2.1058	2.1076	2.1093	2.1111
2.1128	2.1145	2.1162	2.1179	2.1195	2.1211	2.1228
2.1243	2.1259	2.1275	2.1290	2.1306	2.1321	2.1336
2.1351	2.1365	2.1380	2.1394	2.1409	2.1423	2.1437
2.1451	2.1465	2.1478	2.1492	2.1505	2.1519	2.1532

```
2.1545 2.1558 2.1571 2.1584 2.1596 2.1609 2.1621
              2.1634 2.1646 2.1658 2.1670 2.1682
              2.1718 2.1730 2.1741 2.1753 2.1764 2.1776 2.1787
              2.1798 2.1809 2.1820 2.1831 2.1842
              2.1875 2.1885 2.1896 2.1906 2.1917 2.1927 2.1938
              2.1948 2.1958 2.1968 2.1978 2.1988 2.1998
***** ***********************
***** ***** DEVELOPED CONDITIONS *****
***** ****************
**** SUB BASIN 1 WEST
***** Q (allowable) = 0.63 cfs
```

ID=1 HYD NO=100.SB1 AREA=0.00137935 SQ MI COMPUTE NM HYD PER A=0 PER B=22 PER C=0 PER D=78 TP=0.1333 HR MASS RAINFALL=-1

K = .072649 HR TP = .133300 HR K/TP RATIO = .545000 SHAPE CONSTANT, N = 7.106420 526.28 P60 = 1.9000UNIT PEAK = 4.2477 CFS UNIT VOLUME = .9965 B = $.001076 \ SQ \ MI \qquad IA = 10000 \ INCHES$ INF = .04000 INCHES PER HOURAREA = RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033300

K = .130761HR.74567 CFS UNIT VOLUME = .9822 B = $327.55 \cdot P60 = 1.9000$ $.000303 \ SQ \ MI \qquad IA = 0.50000 \ INCHES \qquad INF = 0.25000 \ INCHES \ PER HOUR$ AREA =RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033300

PRINT HYD

ID=1 CODE=1

HYDROGRAPH FROM AREA 100.SB1

1.68355 INCHES = .1239 ACRE-FEET RUNOFF VOLUME = PEAK DISCHARGE RATE = 3.47 CFS AT 1.499 HOURS BASIN AREA = .0014 SQ. MI.

```
****
***** ONSITE PARKING LOT PONDING AREA
**** Maximum Allowable Discharge = 0.63 cfs
                   ID=3 HYD=POND.100 INFLOW=1 CODE=24
ROUTE RESERVOIR
                    OUTFLOW (CFS) STORAGE (AC FT)
                                                   ELEV (FT)
                                    0.00000
                                                     30.25
                      . 28
                                    0.00090
                                                     31.50
                      .33
                                                     32.0
                                    0.00118
                      . 42
                                    0.00173
                                                     33.0
                      .49
                                    0.00228
                                                     34.0
                      . 55
                                                     35.0
                                    0.00280
                      . 606
                                    0.05479
                                                     36.0
```

* * * * * * * * * * * * *

VOLUME INFLOW (CFS) (HRS) (FEET) (AC-FT) .00 30.25 *35.82* 1.60 2.44 36.07 . 61 2.40 35.44 3.20 30.32 4.00 30.32 4.80 *30.33* 5.59 30.26 6.39

PEAK DISCHARGE = | .614 CFS - PEAK OCCURS AT HOUR 2.10

MAXIMUM WATER SURFACE ELEVATION = 36.169

MAXIMUM STORAGE = .0697 AC-FT INCREMENTAL TIME= .033300HRS

**** SUB BASIN 2 EAST

***** Q (allowable) = 2.05 cfs

PER A=0 PER B=22 PER C=0 PER D=78

TP=0.1333 HR MASS RAINFALL=-1

K = .072649 HR TP = .133300 HR K/TP RATIO = .545000 SHAPE CONSTANT, N = 7.106420 UNIT PEAK = 12.115 CFS UNIT VOLUME = .9984 B = 526.28 P60 = 1.9000 AREA = .003069 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033300

K = .130761 HR TP = .133300 HR K/TP RATIO = .980950 SHAPE CONSTANT, N = 3.599930 UNIT PEAK = 2.1267 CFS UNIT VOLUME = .9938 B = 327.55 P60 = 1.9000 AREA = .000865 SQ MI IA = .50000 INCHES INF = 1.25000 INCHES PER HOUR RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033300

PRINT HYD ID=1 CODE=1

HYDROGRAPH FROM AREA 100.SB2

RUNOFF VOLUME = 1.68355 INCHES = .3532 ACRE-FEET

PEAK DISCHARGE RATE = 9.86 CFS AT 1.499 HOURS BASIN AREA = .0039 SQ. MI.

**** ONSITE PARKING LOT PONDING AREA

									,	•
•				·		•				
•			•							· - .
•										

ROUTE RESERVOIR	R I	D=3 HYD=PON	D.100 INFLC	W=1 CODE=2	24		•			
		UTFLOW (CFS				•		-		
		0	0.00		7.0					
	•	0. <i>63</i>	0.03946		7. 5					
	•	1.25	0.07891	28	3.0					
		2.05	0.22079		3.5					
	INFLOW	ELEV	VOLUME	* * * OUTFLOW				•		
TIME						•				
TIME :	INFLOW (CFS)	ELEV (FEET)	VOLUME (AC-FT)	OUTFLOW (CFS)		•				
TIME (HRS)	INFLOW (CFS) .00	ELEV (FEET) 27.00	VOLUME (AC-FT)	OUTFLOW (CFS)						
TIME (HRS) .00 .80	INFLOW (CFS) . 00 . 00	ELEV (FEET) 27.00 27.00	VOLUME (AC-FT) . 000 . 000	OUTFLOW (CFS) .00 .00						
TIME (HRS)	INFLOW (CFS) .00	ELEV (FEET) 27.00	VOLUME (AC-FT) . 000 . 000 . 148	OUTFLOW (CFS) .00 .00 1.64						
TIME (HRS) .00 .80 1.60	INFLOW (CFS) . 00 . 00 6. 93	ELEV (FEET) 27.00 27.00 28.24	VOLUME (AC-FT) . 000 . 000 . 148 . 182	OUTFLOW (CFS) . 00 . 00 1. 64 1. 83						
TIME (HRS) .00 .80 1.60 2.40 3.20 4.00	INFLOW (CFS) . 00 . 00 6. 93 . 40	ELEV (FEET) 27.00 27.00 28.24 28.36	VOLUME (AC-FT) . 000 . 000 . 148	OUTFLOW (CFS) .00 .00 1.64						
TIME (HRS) .00 .80 1.60 2.40 3.20 4.00 4.80	INFLOW (CFS) . 00 . 00 . 93 . 40 . 07 . 04	ELEV (FEET) 27.00 27.00 28.24 28.36 28.36	VOLUME (AC-FT) . 000 . 000 . 148 . 182 . 090	OUTFLOW (CFS) .00 .00 1.64 1.83 1.31						
TIME (HRS) .00 .80 1.60 2.40 3.20 4.00 4.80 5.59	INFLOW (CFS) . 00 . 00 . 93 . 40 . 07 . 04 . 04 . 05	ELEV (FEET) 27.00 27.00 28.24 28.36 28.04 27.43 27.17 27.08	VOLUME (AC-FT) . 000 . 000 . 148 . 182 . 090 . 034	OUTFLOW (CFS) .00 .00 1.64 1.83 1.31 .54						
TIME (HRS) .00 .80 1.60 2.40 3.20 4.00 4.80 5.59 6.39	INFLOW (CFS) . 00 . 00 . 07 . 04 . 04 . 05 . 00	ELEV (FEET) 27.00 27.00 28.24 28.36 28.04 27.43 27.17 27.08 27.05	VOLUME (AC-FT) . 000 . 000 . 148 . 182 . 090 . 034 . 013 . 007 . 004	OUTFLOW (CFS) .00 .00 1.64 1.83 1.31 .54 .21						
TIME (HRS) .00 .80 1.60 2.40 3.20 4.00 4.80 5.59 6.39 7.19	INFLOW (CFS) . 00 . 00 . 07 . 04 . 04 . 05 . 00 . 00	ELEV (FEET) 27.00 27.00 28.24 28.36 28.04 27.43 27.17 27.08 27.05 27.05	VOLUME (AC-FT) . 000 . 000 . 148 . 182 . 090 . 034 . 013 . 007 . 004 . 001	OUTFLOW (CFS) .00 .00 1.64 1.83 1.31 .54 .21						
TIME (HRS) .00 .80 1.60 2.40 3.20 4.00 4.80 5.59 6.39	INFLOW (CFS) . 00 . 00 . 07 . 04 . 04 . 05 . 00	ELEV (FEET) 27.00 27.00 28.24 28.36 28.04 27.43 27.17 27.08 27.05	VOLUME (AC-FT) . 000 . 000 . 148 . 182 . 090 . 034 . 013 . 007 . 004	OUTFLOW (CFS) .00 .00 1.64 1.83 1.31 .54 .21 .11						

.2108 AC-FT

FINISH

NORMAL PROGRAM FINISH

MAXIMUM STORAGE =

END TIME (HR:MIN:SEC) = 11:44:19

<u>M</u>

D. Mark Goodwin & Associates, P.A. Consulting Engineers

P.O. BOX 90606, ALBUQUERQUE, NM 87199 (505) 828-2200 FAX 797-9539 e-mail: dmg@swcp.com

PROJECT CLIF	FORD WEST CHIRCH
	DROCALCS.
BY	DLH DATE 3-5-01
CHECKED	DATE
	SHEET_/OF_5_

SUBASIN

$$Q = C \cdot a \cdot \sqrt{2gh}$$
 $0.63 = 0.6(a) \cdot \sqrt{64.4(36.5 - 30.25)}$
 $a = 0.52345F = .228^{\#'} 2.75^{\#'}$
 $a = .0525175F(use design)$

Q OUT	Stor.	Elev.
0	0	30.25
0,28	.0009	31.50
0.33	.00118	32.0
0.42	.00173	33.0
0.49	.00228	34.0
0.55	.00Z8	35.0
.606	.05479	36.0
0.63	.09886	36.5



D. Mark Goodwin & Associates, P.A. Consulting Engineers

P.O. BOX 90606, ALBUQUERQUE, NM 87199 (505) 828-2200 FAX 797-9539 e-mail: dmg@swcp.com PROJECT CUFFORD WEST LHURCH
SUBJECT HYDRO CALCS

BY DUT DATE 3-5-01

CHECKED DATE

SHEET Z OF 5

VOLUME @ ELEV 36.0

$$0.3+0 (15) = 2.25 SF$$

$$0.85 \pm 1.15$$
 (15) = 15.0 sf

$$6.36 \pm .06(15) = 3.15 = 5$$

SUBASIN 1

$$.23+0 (22) = 2.53 5E$$

$$0.63+.85(22)=16.285$$

VOUNTE @ ETEN 36,50

$$0.8 + 0.5(15) = 9.75 SF$$

$$0.89 + 1.19 (15) = 15.65F$$

$$1.35 + 1.65 (15) = 22.50 SF$$

$$1.5 + 1.8(15) = 24.755F$$

$$0.86 + .56 (15) = 10.655F$$

$$0.73 + 0.5(22) - 13.535E$$

$$1.13 + 1.35 (22) = 27.28 sc$$

$$99+0.5(22) = 16.395F$$

YY

D. Mark Goodwin & Associates, P.A. Consulting Engineers

P.O. BOX 90606, ALBUQUERQUE, NM 87199 (505) 828-2200 FAX 797-9539

e-mail: dmg@swcp.com

PROJECT	·
SUBJECT	
BY	DATE
CHECKED	DATE
	SHEET 3 OF 5

Wallow (Lot 10) = 0.63cfs Qallow (Lo+11+12)= 2.05 cfs

LOT 10 A = 34785 SF = 0.7986AC

Lot 11 A = 560745F = 1.2873 AC

Lot 12 A = 56943 SF = 1,3072 Ac / 2,5945 Ac.

Lot 10 Pennous Area

325'x9' 2925

20'x8' 160 (95'X10')/2 475

50'x4' 200

18'xc/0' 720

30'x3' 90

97'X16' 1392

20'x30' 600

185'x2' 370

25'x20' 500

7432 21%

TOTAL 147,802

Pervious 18762

Lot 11+12 Pervois Area = 11330

40'x8' 320

40x7

300×9 2700 10%

40'x 8' 320

40'x5 200

460 KZ 300 300'xZ'

280

20/x15 100

(20×10)/2 100

42x25' (050)

80×5' 400

110'x7' 770

801×51 400

451x241 1080

20 ×15' 300

1235×201) 350

40×10' 400

40'x8' 320

40xB 320

20 X15 300.

40 x 9 360

40×8 320

40x8 320

320

Chad Young Paul of ASSOC Tyler Jenes

D. Mark Goodwin & Associates, P.A. Consulting Engineers

P.O. BOX 90606, ALBUQUERQUE, NM 87199 (505) 828-2200 FAX 797-9539 e-mail: dmg@swcp.com

CHECKED.

OUTFLOW	STORAGE	ELEVA-77.0N
	0	27.0
0.63	.03946	27.5
1.25	.07891	28,0
2.05	.220741	28.5

27-28 1.2+6.4 (20) = 76 CF 6.4 +13.5 (22) = 219CF 8.8+14.08 (40) = 457.6 CF 3.2+1.6 (40) = 96.2 CF 23+64.05 (23) = 1001CF 18+22.6 (20) = 406.20= 38.25+14 (20) = 522.5 UF 9+1156 (20) = 105,6 CF 23+7.15 (30) = 452,2 CF 5+3.1(25) = 101.25CF 3437.55CF = .07891AF

ORIFICE CALCS. Q= C.a. Vzgh 2.05cfs=0,6(a) [2(32.2)1.25 (Area = 0.38085q.ft.

0.76 × 0.5 OPENING

Circle 2 10 This

28'-28.5 6+16.5 (20) = 225 16.5+27(20) = 435 11.5(40)=460 12+16 (40) = 560 32+61,5 (23)=1075,2 11.5(20)=230 45+28 (20) = 730 24+7.5 (20) = 315 22 (30) 5 = 330 4360.2 CF 8.75+17.5 (42) = 551.2 17.5+15 (40)= 650 16.7+7.5 (22) = 266.2 1820.4 CF

8(20) = 160

6180.6CF =141887AF

TOTAL =. 220797

<u>M</u>

D. Mark Goodwin & Associates, P.A. Consulting Engineers

P.O. BOX 90606, ALBUQUERQUE, NM 87199 (505) 828-2200 FAX 797-9539 e-mail: dmg@swcp.com

PROJECT (11)	Ford West Church
SUBJECT HY	1000y Calcs
BY	MOlogy Calcs DCH DATE Z-9-01
CHECKED	DATE
	SHFFT 5 OF 5

SB 2 continued

2.05 cfs=0.60(a) [2(32.2) 1.14875

$$area = 0.3844 \text{ SF} = 77R^2$$

 $Radios = 4.275''$
 $Dlameta = 8.55''$

INV = 27.0

TTR = .3987 2TTR = 2,23838

Q = 1.486 (.3987) (.3166) .01/2

```
U.S. ARMY CORPS OF ENGINEERS
   WATER SURFACE PROFILES
                                                                                                 THE HYDROLOGIC ENGINEERING CENTER
   VERSION OF SEPTEMBER 1988
                                                                                                 609 SECOND STREET, SUITE D
   ERROR: 01,02
                                                                                                DAVIS, CALIFORNIA 95616-4687
                        1989
   UPDATED: 4 APRIL
                                                                                                (916) 756-1104, (916) 551-1748
                3/ 5/ 1
                                 14:56:51
                          TIME
 * RUN DATE
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                                                  XXXXXXX
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                                                                                  XXXXXXX
                             14:56:51
THIS RUN EXECUTED
  HEC2 RELEASE DATED SEP 88 UPDATED APR 1989
  ERROR CORR - 01,02
  MODIFICATION -
T1
            CLIFFORD WEST BUSINESS PARK
T2
            CHURCH CHANNEL OUTFALL
т3
            100-YEAR / 6-HOUR DESIGN STORM
               INQ
                        NINV
                                   IDIR
                                             STRT
                                                      METRIC
                                                                 HVINS
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    VARIABLE CODES FOR SUMMARY PRINTOUT
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                   2.05
X1
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GR
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                      CWSEL
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2.

.017 . 000 .000 4.77 1.15 101.18 .019828

14:56:51 THIS RUN EXECUTED 3/5/1

HEC2 RELEASE DATED SEP 88 UPDATED APR 1989

ERROR CORR - 01,02

MODIFICATION -

NOTE- ASTERISK (*) AT LEFT OF CROSS-SECTION NUMBER INDICATES MESSAGE IN SUMMARY OF ERRORS LIST

100-YEAR / 6-HOUR DE

SUMMARY PRINTOUT

	SECNO	Q	CWSEL	CRIWS	VCH	TOPWID	FRCH	EG
	1.000	2.05	. 42	. 51	4.77	1.15	1.38	. 77
1				•	•	•	-	

•

3/ 5/ 1 14:56:51

INIEL

OFFICE

MEMO

To:

File

From:

Richard Dourte

Subject: Mike Zamora issued perm TCL CO's following projects.

Date:

November 4, 2002

Drainage		Date	Date
file no.	Description	Received	Approved
C12/D5A	Quanz Auto Car Care	10-28-02	10-31-02
C13/D12	NM Sports and Wellness	11-01-02	11-01-02
C17/D113	Mechenbier Office/Warehouse	10-29-02	11-04-02
C19/D11D6	Wells Fargo Bank	10-29-02	10-29-02
E12/D2	Texaco Xpress Lube	10-31-02	- 10-31-02
E12/D14	Riverside Plaza, Tract 6, Bld L-1	10-24-02	10-25-02
E-12/D15A	Bosque School Gymnasium	10-31-02	11-01-02
F13/D14	Shephard of the Valley Pres. Church Renov. & Addn.	10-24-02	10-31-02
F17/D78	Lexus of Albuquerque	10-24-02	10-28-02
F21/D43	Casa Pacifica Condominiums	10-15-02	10-20-02
H11/D66	Sonic Drive In	10-23-02	10-22-02
J19/D39	Garcia Honda Renovation and Addition	10_12 02	10 10 00
K10/D37-	Fellowship Missionary Baptist Church		10-10-02 00-25 02-
L14/D1C	Roses Southwest Paper	10-23-02	10 20 02
L18/D66	Obee's Soup Salad Subs	10-23-02	10-28-02
M15/D24	Hilton Garden Inn		10-31-02
_ •		10-17-02	10-17-02

DRAMAGE AND THANSPORTATION INFORMATION SHEET (FIEW. 1/11/2002)

選集: FALOUSA 選集:	これが	COUNTY OF THE	EMAPO	AG FILE	77 110
			FK OFFE	子婆:	
GAL DESCRIPTION: CLIFFO Y ADDRESS. 28550 SA	PAJ VKST BUISUESS	PARK LOTS	10 A.	Block 2	2
· ·		1 4 4			
GINEEHINGFIRE FINKL	SOUDWIN + ASSOLI	+135		EST. D'ALS	HOELZER
ALAXIESS: J.C.DOK	70101		Partie		828 2200
GITT, STATE ALBOOK			ZP CX	The second secon	1/9_
ADERESS 8550 S	ONARY BAPTIST CA	work			1 R.
ADERESS: 8350 S	ALL BELL RO. NIL	1.	Carrent Carrent	505/2	BURDELL
ERIY, STATE ALBUQ	WERDUR NIM		ZIP EX	DE: 87/	12-8725
CHITECT: RALPA L	111.0271			**************************************	
ADDRESS: 160 LAN	IGE ON IN	- 	COM		VILORD
GETT, STATE: BLIZZ	1000 SC			بالروسية المراوع السكان سياسي	33-9610
			ZP CC	<u> </u>	106
AVEYOR SURVEYS SO	ULL WEST LTD.			DAN	
ALKIRESS 337 LO	MAS NE.		PACE	= (505) 5	98-0303
CITY, STATE ALGOR		**	ZEP CO	DE 377	02
NTRACTOR MUL + 115500	intes INC.			EE MIKK	TANSK!
ALAXIESS:	LO BULLARD RO.		P+10)		1-295
CHY, STATE TYLER	- 775XAS		ZIP CO		
DRAINTAGE REPORT DRAINTAGE REAM COMCEPTUAL GRADING & GRADING PLAN ERCSION CONTROL PLAN ENGINEER'S CERTIFICATION LATER ENGINEER'S CERTIFICATION ENGINEER'S CERTIFICATION OTHER	M (HYDROLOGY). YOUT (TCL) M (TCL)		FINANCE PLAN EV. PLAN CTOR PLAN AMATION LUNG PER ANG P	PEFECUAL PEFECUAL OF OCCUPATIONS OF OCCUPATIONS OF APPROVAL TAPPROVAL	EFECTAL THOUGH COVEL CV (FERM) CV (FERM)
A PRE-DESIGN CONFERENCE YES WO COPY PROVIDED	ATTENDED		SEP 2 ROLOG	5 2002 Y SECTIO	
ESUBATTED: 9/25/		سر مرد اوران	TINSK.	P	

Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a desirage submittal. The particular nature, location and scope of the proposed development defines the degree of drawinge detail. One or more of the following levels of submitted may be required based on the following,

1. Conceptual Grading and Drainage Flan: Required for approval of Site Development Plans greater than five 2. Drainage Plans: Required for building permits, grading permits, paving permits and site plans less than the (5)

3. Draining Report: Required for subdivisions containing more than (10) to be or constituting fire (5) acres or

9/25/02 - Cld with to Phyllis. - Sent letter dated 9/25-

903-581-8322

5620 OLD BULLARD RD., STE. 128 • TYLER, TEXAS 75703

September 20, 2002

Mr. Mike Zomorra
City of Albuquerque
P. O. Box 1293
Albuquerque, New Mexico 87103

Re: Fellowship Missionary Baptist Church

Dear Mr. Zomorra:

This letter is to certify the site improvements indicated on our permit drawing C 1.01 6/28/01 have been carried out in the field in a manner consistent with this drawing. Therefore, please accept this letter as a certification of these improvements.

Thank you for your attention to this matter.

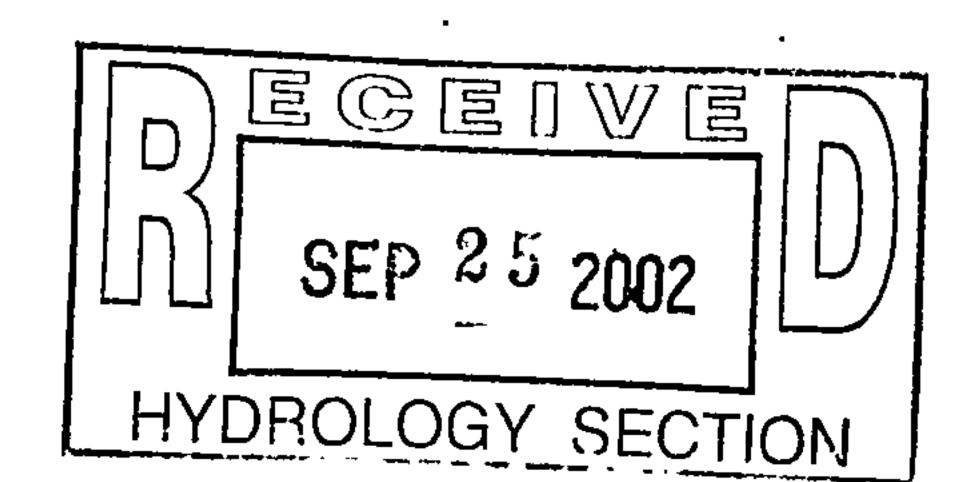
Sincerely,

E. Ralph Walden

Director of Design

cc: Mike Tanski

ERW/bg





P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

September 26, 2002

Diane Hoelzer, P.E.
Mark Goodwin & Assoc.
P.O. Box 90606
Albuquerque, New Mexico 87199

RE: CLIFFORD WEST CHURCH

(K-10/D37)

(8570 Saul Bell Rd) (AKA: Fellowship Missionary Baptist Church)
CERTIFICATE OF OCCUPANCY APPROVAL-Temporary Extension
ENCINEERS CERTIFICATION DATES OF 1999

ENGINEERS CERTIFICATION DATED 8/28/2002

Dear Ms. Hoelzer:

Based on the information provided in your submittal dated September 26, 2002, the above referenced project is approved for an Extension of TEMPORARY Certificate of Occupancy.

An Extension of the Temporary Certificate of Occupancy has been issued for another 30 days, allowing the following outstanding drainage issues to be completed within this time scope.

Upon completion of all of the above outstanding drainage issues, final certification will need to be resubmitted for issuance of a Permanent Certificate of Occupancy.

If I can be of further assistance, please feel free to contact me at 924-3981.

Sincerely,

Teresa A. Martin

Hydrology Plan Checker

Town A. Martin

Development and Building Services Division

BUB

c: Certificate of Occupancy Clerk, COA
Drainage file
Approval file



D. Mark Goodwin & Associates, P.A. Consulting Engineers

P.O. BOX 90606, ALBUQUERQUE, NM 87199 (505) 828-2200 FAX 797-9539 e-mail: dmg@swcp.com

September 25, 2002

Ms. Teresa Martin Hydrology Plan Checker Development and Building Services Division City of Albuquerque PO Box 1293 Albuquerque, NM 87103

Re:

Clifford West Church

Engineers Stamp 9-25-02 (K10/D37)

Dear Mr. Martin

In response to your letter dated August 30, 2002 the swale in the access road has been graded but the improvements associated with the property to the south is still pending. We are requesting extension of a temporary C.O. until such time as these drainage improvements are complete as I or my clients have no control over the adjacent property owners time to complete. I have also discuss this with Brad Bingham in City Hydrology.

Please call me if you have any questions.

Sincerely,

MARK GOODWIN & ASSOCIATES, P.A.

Diane Hoelzer, PE Senior Engineer

DLH/dlh f:\\clifford.chu\hyd_2.ltr SEP 25 2002

HYDROLOGY SECTION

DRAINAGE AND TRANSPORTATION INFORMATION SHEET

(REV.1/11/2002)

PROJECT TITLE:	Clifford West Church		ZONE ATLAS	S#:	K10 / D37	
DRB#:	1000918 EPC#	`	_ WORKORDE	R#:		
LEGAL DESCRIPTION:	Lots 10, 11, 12, Block 2, Clifford West E	Business	Park, Unit 1		.•	
CITY ADDRESS:						
ENGINEERING FIRM:	Mark Goodwin & Associates, PA		CONTACT:	Diane Ho	elzer, PE	
ADDRESS:	PO Box 90606, ABQ, NM 87199		PHONE:	828-2200		
<u>OWNER</u> :	Fellowship Missionary Baptist Church		CONTACT:	Burrell		
ADDRESS:	8570 Saul Bell Road, ABQ, NM 87121		PHONE:	321-8315		
ARCHITECT:	Paul & Associates		CONTACT:	Chad Young		
ADDRESS:	5620 Bullard # 128, Tyler, Texas 75703		PHONE:	903-581-8322 x 102		
SURVEYOR:	Southwest Surveying Co. Inc.		CONTACT:			
ADDRESS:	333 Lomas Blvd. NE, ABQ, NM, 87102	· .	PHONE:	998-0303	}	
CONTRACTOR:	Custom Grading & Cambro Construction	Inc.	CONTACT:			
ADDRESS:			PHONE:		· · · · · · · · · · · · · · · · · · ·	
YPE OF SUBMITTAL: DRAINAGE REPO		CHEC	K TYPE OF APP S!A / FINANCIA		NTY RELEASE	
DRAINAGE PLAN	•		PRELIMINARY	•	•	
DRAINAGE PLAN CONCEPTUAL GRADING & DRAINAGE PLAN			- S. DEV. PLAN FOR SUB'D APPROVAL			
GRADING PLAN			- S. DEV. PLAN FOR BLDG PERMIT APPROVAL			
EROSION CONTROL PLAN			– SECTOR PLAN APPROVAL			
X ENGINEER'S CERTIFICATION (HYDROLOGY)			— FINAL PLAT APPROVAL			
CLOMR/LOMR			— FOUNDATION PERMIT APPROVAL			
TRAFFIC CIRCULATION LAYOUT (TCL)			BUILDING PERMIT APPROVAL			
ENGINEER'S CERTIFICATION (TCL)			CERTIFICATE OF OCCUPANCY (PERM)			
ENGINEER'S CERTIFICATION (DRB APPR. SITE PLAN)		X	CERTIFICATE OF OCCUPANCY (TEMP)			
OTHER			GRADING PERMIT APPROVAL			
			PAVING PERMIT APPROVAL			
WAS A PRE-DESIGN CO	ONFERENCE ATTENDED:		WORK ORDER	RAPPROV	'AL	
YES			OTHER (SPEC	IFY)		
NO COPY PROVIDED				国 G 国 2	5 2002 J	
				DROLO	AND THE PARTY OF T	
DATE SUBMITTED:	September 25, 2002	·.	LHY.			
BY: WOLLA THO			•			

Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a drainage submittal. The particular nature, location and scope of the proposed development defines the degree of drainage detail. One or more of to the following levels of submittal may be required based on the following:

- 1. Conceptual Grading and Drainage Plan: Required for approval of Site Development Plans greater than five (5).
- 2. Drainage Plans: Required for building permits, grading permits, paving permits and site plans less than five (5).
- 3. Drainage Report: Required for subdivisions containing more than ten (10) lots or constituting five (5) acres or more.

Diane Hoelzef, PE



P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

August 30, 2002

Diane Hoelzer, P.E.
Mark Goodwin & Assoc.
P.O. Box 90606
Albuquerque, New Mexico 87199

RE: CLIFFORD WEST CHURCH

(K-10/D37)

(8570 Saul Bell Rd) (AKA: Fellowship Missionary Baptist Church)

CERTIFICATE OF OCCUPANCY APPROVAL-Temporary

ENGINEERS CERTIFICATION DATED 8/28/2002

Dear Ms. Hoelzer:

Based on the information provided in your submittal dated August 29, 2002, the above referenced project is approved for a **TEMPORARY** Certificate of Occupancy.

A Temporary Certificate of Occupancy has been issued for 30 days, allowing the following outstanding drainage issues to be completed within this time scope.

Upon a site inspection it was noted that the channel and outlet on the adjacent property to the South must be constructed. Also, the swale in the emergency access road must be completed.

Upon completion of all of the above outstanding drainage issues, final certification will need to be resubmitted for issuance of a Permanent Certificate of Occupancy.

If I can be of further assistance, please feel free to contact me at 924-3981.

Sincerely,

Triese A. Martin

Hydrology Plan Checker

Development and Building Services Division

BUB

c: Certificate of Occupancy Clerk, COA

Drainage file

Approval file

DRAINAGE AND TRANSPORTATION INFORMATION SHEET

(REV.1/11/2002)

PROJECT TITLE:	Clifford West Church	, , <u>.</u> ,	ZONE ATLAS	S#: K10 / D37 .				
DRB#:	1000918 EPC#		WORKORDE	R#:				
LEGAL DESCRIPTION:	Lots 10, 11, 12, Block 2, Clifford West	Business P	ark, Unit 1					
CITY ADDRESS:	8590 SAUL Benc TU	>						
		-						
ENGINEERING FIRM:	Mark Goodwin & Associates, PA		CONTACT:	Diane Hoelzer, PE				
ADDRESS:	PO Box 90606, ABQ, NM 87199	<i></i>	PHONE:	828-2200				
<u>OWNER</u> :	Fellowship Missionary Baptist Church	·	CONTACT:	Burrell				
ADDRESS:	8570 Saul Bell Road, ABQ, NM 87121		PHONE:	321-8315				
ARCHITECT:	Paul & Associates	·	CONTACT:	Chad Young				
ADDRESS:	5620 Bullard # 128, Tyler, Texas 75703		PHONE:	903-581-8322 x 102				
SURVEYOR:	Southwest Surveying Co. Inc.	· 	CONTACT:	· · · · · · · · · · · · · · · · · · ·				
ADDRESS:	333 Lomas Blvd. NE, ABQ, NM, 87102		PHONE:	998-0303				
CONTRACTOR:	Custom Grading & Cambro Constructio	n Inc.	CONTACT:					
ADDRESS:		<u>, </u>	PHONE:					
YPE OF SUBMITTAL: DRAINAGE REPORT			•	AL GUARANTY RELEASE ' PLAT APPROVAL				
CONCEPTUAL GRADING & DRAINAGE PLAN			S. DEV. PLAN FOR SUB'D APPROVAL					
GRADING PLAN		,	S. DEV. PLAN FOR BLDG PERMIT APPROVAL					
EROSION CONTROL PLAN			SECTOR PLAN APPROVAL					
X ENGINEER'S CERTIFICATION (HYDROLOGY)			FINAL PLAT APPROVAL					
CLOMR/LOMR			FOUNDATION PERMIT APPROVAL					
TRAFFIC CIRCULATION LAYOUT (TCL)		·	BUILDING PERMIT APPROVAL					
	RTIFICATION (TCL)	X	CERTIFICATE OF OCCUPANCY (PERM)					
ENGINEER'S CER	RTIFICATION (DRB APPR. SITE PLAN)	X	X CERTIFICATE OF OCCUPANCY (TEMP)					
OTHER			GRADING PERMIT APPROVAL					
 			PAVING PERM	IIT APPROVAL				
WAS A PRE-DESIGN CONFERENCE ATTENDED:			WORK ORDER APPROVAL					
YES		<u> </u>	OTHER (SPEC	CIFY)				
NO								
DATE SUBMITTED: BY: Many	August 28, 2002		国(G)[AUG] HYDROLC	宝ワン国 292002 GY SECTION				

Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a drainage submittal. The particular nature, location and scope of the proposed development defines the degree of drainage detail. One or more of to the following levels of submittal may be required based on the following:

- 1. Conceptual Grading and Drainage Plan: Required for approval of Site Development Plans greater than five (5).
- 2. Drainage Plans: Required for building permits, grading permits, paving permits and site plans less than five (5).
- 3. Drainage Report: Required for subdivisions containing more than ten (10) lots or constituting five (5) acres or more.

Diane Hoelzer, PE



City of Albuquerque P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

August 15, 2002

David Gatterman

BJM Development Consultant
4409 Karrol Road SW

Albuquerque New Mexico 87119

(AKA: CCEPFOND WEST CHUKCH)

RE: Grading and Drainage Plan for Fellowship Missionary Baptist Church Parking Lot (K10-D37) Dated August 5, 2002

Dear Mr. Gatterman:

The above referenced drainage plan received August 7, 2002 is approve for grading and paving. Upon completion of the project please certify the project per the DPM. If you have any questions please call me at 924-3982.

Sincerely

Carlos A. Montoya

City Floodplain Administrator

DRAINAGE INFORMATION SHEET

PROJECT TITLE: tellowship 11115510navy pa	ptist Churchone MAP/DRG. FILE #: K10-D37 WORK ORDER#:
DRB #:EPC#:	- C - U + D
LEGAL DESCRIPTION: LOT 10-A Block 2 C	1 ittord west business tark unit
CITY ADDRESS: 6550 5 au Bell Rel N.W.	
Day Day Conc	contact: Bernie J. Montaga
ENGINEERING FIRM: BJM Development Cons	PHONE: 877-494
ADDRESS: 4409 Kawol Rd SW	CG 87121 ZIP CODE: 87121
CITY, STATE: A lbuguerjur, New Mexi	
OWNER: Fellowship Missionary Baptist (ADDRESS: 915 515 ASh 5.E.	Church Contact: Pastor Dunn PHONE: 242-8428
OWNER: TOTOWSTIP FILISSIONIUM JUDITS.	PHONE: 242-8428
CITY, STATE:	ZIP CODE: 8 1106
ARCHITECT:	CONTACT:
ADDRESS:	PHONE:ZIP CODE:
CITY, STATE:	
#1 (PA (FT)/AD)	CONTACT:
SURVEYOR:ADDRESS	PHONE:
CITY, STATE:	
	•
CONTRACTOR:	CONTACT:
ADDRESS:	PHONE:
CITY, STATE:	ZIP CODE:
CONCEPTUAL GRADING & DRAINAGE PLAN GRADING PLAN EROSION CONTROL PLAN ENGINEER'S CERTIFICATION CLOMPLOMP OTHER WAS A PRE-DESIGN CONFERENCE ATTENDED: YES Verbal with Carlos Montoya NO COPY PROVIDED	PRELIMINARY PLAT APPROVAL S. DEV. PLAN FOR SUB'D. APPROVAL S. DEV. PLAN FOR BLDG. PERMIT APPROVAL SECTOR PLAN APPROVAL FINAL PLAT APPROVAL FOUNDATION PERMIT APPROVAL BUILDING PERMIT APPROVAL CERTIFICATE OF OCCUPANCY APPROVAL VING PERMIT APPROVAL PAVING PERMIT APPROVAL WORK ORDER APPROVAL OTHER (SPECIFY) BY: BY:
DATE SUBMITTED: 8/7/2002	BY: Denie / VI Longo
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The particular nature, location and scope of the propose more of the following levels of submittal may be required 1. Conceptual Grading and Drainage Plan: Fig. (5) acres and Sector Plans. 2. Drainage Plans: Required for building permacres. 3. Drainage Report: Required for subdivisions	sed development defines the degree of drainage detail. One ed based on the following: Required for approval of Site Development Plans greater than hits, grading permits, paving permits and site plans less than the containing more than ten (10) lots or constituting five (5) acres and a site plans less than the containing more than ten (10) lots or constituting five (5) acres a UG 0 ? 2002
The particular nature, location and scope of the propose more of the following levels of submittal may be required. 1. Conceptual Grading and Drainage Plan: Following Conceptual Grading and Drainage Plan: Following Plans: Required for building permacres. 2. Drainage Plans: Required for subdivisions. 3. Drainage Report: Required for subdivisions.	sed development defines the degree of drainage detail. One ed based on the following: Required for approval of Site Development Plans greater than hits, grading permits, paving permits and site plans less than the containing more than ten (10) lots or constituting five (5) acres to the containing more than ten (10) lots or constituting five (5) acres to the containing more than ten (10) lots or constituting five (5) acres to the containing more than ten (10) lots or constituting five (5) acres to the containing more than ten (10) lots or constituting five (5) acres to the containing more than ten (10) lots or constituting five (5) acres to the containing more than ten (10) lots or constituting five (5) acres to the containing more than ten (10) lots or constituting five (5) acres to the containing more than ten (10) lots or constituting five (5) acres to the containing more than ten (10) lots or constituting five (5) acres to the containing more than ten (10) lots or constituting five (5) acres to the containing five (5) acres to the containing more than ten (10) lots or constituting five (5) acres to the containing more than ten (10) lots or constituting five (5) acres to the containing more than ten (10) lots or constituting five (5) acres to the containing more than ten (10) lots or constituting five (5) acres to the containing more than ten (10) lots or constituting five (5) acres to the containing more than ten (10) lots or constituting five (5) acres to the containing more than ten (10) lots or constituting five (5) acres to the containing more than ten (10) lots or constituting five (5) acres to the containing more than ten (10) lots or constituting five (5) acres to the containing more than ten (10) lots or constituting five (10) lots or containing more than ten (10) lots or cont



P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

August 10, 2001

Diane Hoelzer, P.E.
Mark Goodwin & Assoc.
P.O. Box 90606
Albuquerque, NM 87199

RE: CLIFFORD WEST CHURCH, Clifford West Business Park Unit 1 (K10-D37). Revised DRAINAGE REPORT, Revised GRADING AND DRAINAGE PLAN FOR SITE DEVELOPMENT PLAN FOR BUILDING PERMIT, GRADING PERMIT, AND BUILDING PERMIT APPROVALS. ENGINEER'S STAMP DATED JULY 12, 2001. G & D PLAN Restamped JULY 25. 2001 FOR FIRE MARSHALL'S REQUIREMENTS.

Dear Ms. Hoelzer:

Based on the information provided on your July 27, 2001 submittal, the above referenced project is approved for Site Development Plan for Building Permit, and for Grading and Building Permits.

Please attach a copy of this approved plan - with July 25, 2001 Stamp Date - to the construction sets prior to sign-off by Hydrology.

Prior to Certificate of Occupancy approval, an Engineer's Certification per the DPM will be required.

If I can be of further assistance, please feel free to contact me at 924-3984.

Sincerely,

John P. Mufray.

Hydrology

C

Perri Martin



D. Mark Goodwin & Associates, P.A. Consulting Engineers

P.O. BOX 90606, ALBUQUERQUE, NM 87199 (505) 828-2200 FAX 797-9539

e-mail: dmg@swcp.com

July 26, 2001

Mr. John Murray, PE Hydrology Reviewer City of Albuquerque PO Box 1293 Albuquerque, NM 87103

Re: Request Site Development Plan for Building Permit Approval, Building Permit Approval, Grading Permit Approval for Clifford West Church Revised Grading and Drainage Plan, Engineers Stamp 7-25-01 (K10/D37)

Dear Mr. Murray;

In response to comments received from the Fire Marshall on Monday I made some minor changes to the emergency access road along the east side of the property. I revised the turning radius's and the width changed from an 18' width to a 20' width. The grading and drainage plan concept remains the same.

Please call me if you have any questions.

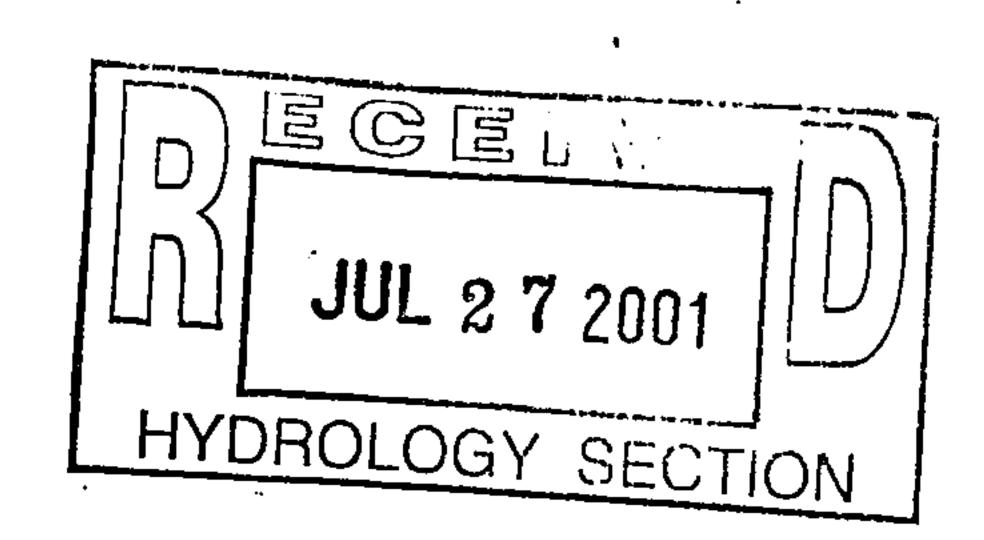
Sincerely,

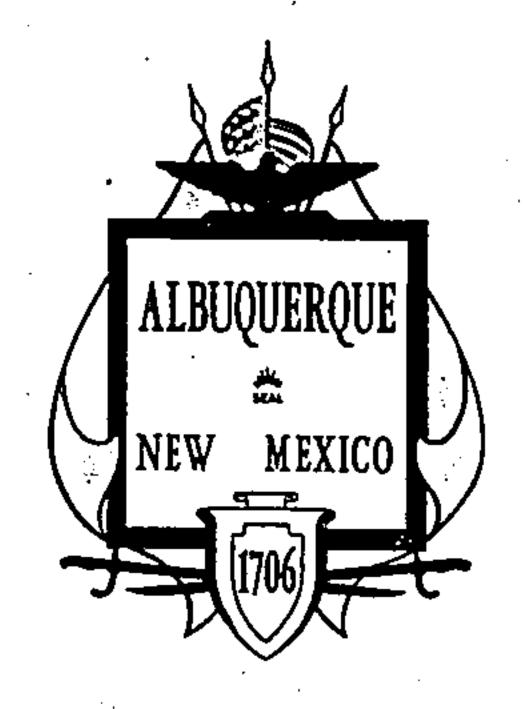
MARK GOODWIN & ASSOCIATES, P.A.

Diane Hoelzer, PE Senior Engineer

DLH/dlh

f:\\clifford.chu\hyd_2.ltr





P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

July 23, 2001

Diane Hoelzer, P.E.
Mark Goodwin & Assoc.
P.O. Box 90606
Albuquerque, NM 87199

RE: CLIFFORD WEST CHURCH, Clifford West Business Park Unit 1 (K10-D37). Revised DRAINAGE REPORT, Revised GRADING AND DRAINAGE PLAN FOR SITE DEVELOPMENT PLAN FOR BUILDING PERMIT, GRADING PERMIT, AND BUILDING PERMIT APPROVALS. ENGINEER'S STAMP DATED JULY 12, 2001.

Dear Ms. Hoelzer:

Based on the information provided on your July 12, 2001 submittal, the above referenced project is approved for Site Development Plan for Building Permit, and for Grading and Building Permits.

Please attach a copy of this approved plan to the construction sets prior to sign-off by Hydrology.

Prior to Certificate of Occupancy approval, an Engineer's Certification per the DPM will be required.

If I can be of further assistance, please feel free to contact me at 924-3984.

Sincerely,

John P. Murray, P.E.

Hydrology

c: Terri Martin File



P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

March 29, 2001

Diane Hoelzer, P.E.
Mark Goodwin & Assoc.
P.O. Box 90606
Albuquerque, NM 87199

RE: CLIFFORD WEST CHURCH, Clifford West Business Park Unit 1 (K10-D37).

GRADING AND DRAINAGE PLAN FOR SITE DEVELOPMENT PLAN FOR
BUILDING PERMIT APPROVAL. ENGINEER'S STAMP DATED MARCH 8, 2001.

Dear Ms, Hoelzer:

Based on the information provided on your March 8, 2001 submittal, the above referenced project is approved for SITE DEVELOPMENT PLAN for Building Permit (for DRB action). This G&D Plan should be labeled "Conceptual."

Prior to the issuance of the Building Permit, furnish the necessary data and details for the design of the proposed system, e.g., sizing of the ponds, etc.

If I can be of further assistance, please feel free to contact me at 924-3984.

Sincerely,

John P. Murray, P.E.

Hydrology

C

Terri Martin

DRAINAGE INFORMATION SHEET

PROJECT TITLE:	Clifford West Church		ZONE ATLA	S#:	K 10/D37	
DRB#:	EPC#		WORK	ORDER#:		
LEGAL DESCRIPTION:	Lot 10, 11, 12, Block 2, Clifford West E	Business	Park Unit 1			
CITY ADDRESS:						
			·			
ENGINEERING FIRM:	Mark Goodwin & Associates, PA		CONTACT:			
ADDRESS:	P.O. Box 90606, Albuquerque, NM 87199		PHONE:	828-2200		
OWNER:			CONTACT:			
ADDRESS:	· · · · · · · · · · · · · · · · · · ·		PHONE:		- -	
ARCHITECT:	Paul and Associates		CONTACT:	Chad Young	·	
ADDRESS:	5620 Bullard Rd.#128, Tyler, Texas 75703		PHONE:	903-581-8322	2 x 102	
SURVEYOR:	ALS		CONTACT:	TIM ALDRICH		
ADDRESS:	4109 MONTGOMERY BLVD.		PHONE:	884-1990		
CONTRACTOR:			CONTACT:			
ADDRESS:	······································		PHONE:	· · ·		
TYPE OF SUBMITTA	L:	CHECK 1	TYPE OF APP	ROVAL SOUGI	HT:	
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X GRADING PI	LAN 	<u>X</u> S	. DEV. PLAN	FOR BLDG PE	RMIT APPROVAL	
EROSION CO	ONTROL _	S	SECTOR PLAN APPROVAL			
ENGINEER'S	S CERTIFICATION	F	FINAL PLAT APPROVAL			
OTHER .		F	FOUNDATION PERMIT APPROVAL			
EASEMENT	VACATION	B	BUILDING PERMIT APPROVAL			
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PRE-DESIGN MEET	ING:	G	GRADING PERMIT APPROVAL			
YES		P	PAVING PERMIT APPROVAL			
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DATE SUBMITTED:	MARCH 8, 2001	•	TE (2 [3] [V] [当门	
BY: MA	0 45006		1 D) LET			
DIANE HOEL	ZER, PE		n M	AR 0 3 200		