

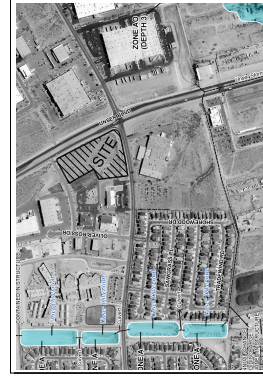
EROSION CONTROL PLAN

POST SITE SIGN AND RAIN GAUGE

SILT FENCE SHOWN IN RED
INSTALL 4-8 INCH RIPRAP AT
CONSTRUCTION ENTRANCE



VOCALITY MAP: K-10-Z



FIRM MAP: 3500100328J

LEGAL DESCRIPTION:
TRACTS 5/BLK 2 AND TRACT B
CLIFFORD WEST BUSINESS PARK, UNIT 1
ALBUQUERQUE, BERNALILLO COUNTY, NEW MEXICO

LEGEND	
— 503.00 —	EXISTING CONTOUR (MAJOR)
— 502.00 —	EXISTING CONTOUR (MINOR)
— 501.00 —	BOUNDARY LINE LIMIT OF DISTURBANCE
X 5242.70	PROPOSED SPOT ELEVATION
X 5009.16	EXISTING GRADE
X FL=5075.65	EXISTING FLOWLINE ELEVATION
— 1 —	PROPOSED RETAINING WALL
BC=41.30	BOTTOM OF CHANAL
TF=42.00	TOP OF FOOTING
TRW=45.12	TOP OF RETAINING WALL
HP	HIGH POINT
AS-BUILT GRADES	AS-BUILT GRADES
AS-BUILT SPOT ELEVATIONS	AS-BUILT SPOT ELEVATIONS



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RUNOFF AND PONDING CALCULATIONS
TRACTS 5/BLK 2 AND TRACT B
CLIFFORD WEST BUSINESS PARK, UNIT 1

DRAWING: SHEET # 1 OF 2
DATE: 1/1/2024
DRAWN BY: SHS

Location

Tract 5/Block 2 and Tract B, Clifford West Business, Unit 1 is located at the northwest corner of Unser Blvd. NW and Bluewater Rd. NW. See attached portion of Vicinity Map K-10-Z for exact location.

Purpose

The purpose of this drainage report is to present a grading and drainage solution for the proposed development.

Existing Drainage Conditions

This falls within the Master Drainage Plan K09/D023 prepared by Mark Goodwin and Associates. The site drains from north to south and west to Bluewater. There are two existing inlets on Blue Water Rd. close to the Unser Blvd. The site does not fall within a designated floodplain.

Proposed Conditions and On-Site Drainage Management Plan

Based on the Master Drainage Plan K09/D023 prepared by Mark Goodwin and Associates the total allowable discharge for Tract 5 is 1.01 cfs and Tract B is 2.91 cfs. Therefore, the runoff is being detained on site and discharge at the allowable rates.

Tract B drains to a ponding area to the south and discharges to back of sidewalk curb at a flow rate of 0.85 cfs (which is less than the allowable discharge rate of 2.91 cfs) and 1.88 cfs (which is less than the allowable discharge rate of 2.91 cfs) into the parking lot during the detention period. Part of the runoff backs up into the parking lot during the detention period. Part of the pond is retention to meet the first flush volume ponding requirement.

Tract 5 drains to a ponding area to the south as well and discharges towards the entrance and out to Blue Water Rd. at a flow rate of 0.85 cfs (which is less than the allowable discharge rate of 1.01 cfs) through a 0.67'x0.33' opening. The runoff backs up into the parking lot during the detention period. Part of the pond is retention to meet the first flush volume ponding requirement. See Sheet 2 for ponding and runoff calculation using AHJ MO. All the runoff which discharges onto Blue Water Rd. from this project will drain west to two existing inlets.

First flush ponding volume provided for Tract 5 is 1,825.20 cf which more that required volume of 1,792.36 cf. First flush ponding volume provided for Tract B is 6,531.07 cf which more that required volume of 6,483.25 cf.

FIRST FLUSH VOLUME CALCULATION

Tract 5 First Volume requirement = $(0.42/12) * (85,235.70) = 6,486.25$ CF

Tract B First Volume requirement = $(0.42/12) * (1,221,031) = 1,792.74$ CF

VOLUME CALCULATIONS FOR STORM WATER QUALITY PONDS

TRACT 5
BOTTOM AREA = 1,352.00 SF
TOP AREA = 1,352.00 SF
DEPTH = 1.35'
POND VOLUME = 1,352.00 X 1.35
POND VOLUME = 1,825.20 CF

TRACT B
BOTTOM AREA = 2,721.28 SF
TOP AREA = 2,721.28 SF
DEPTH = 2.40'
POND VOLUME = 2,721.28 X 2.40
POND VOLUME = 6,531.07 CF

