

CITY OF ALBUQUERQUE

Planning Department
Alan Varela, Director



Mayor Timothy M. Keller

March 1, 2024

Shawn Biazar
SBS Construction and Engineering, LLC
10209 Snowflake Ct. NW
Albuquerque, NM 87114

RE: D&B U-Sell-it
9531 Bluewater Rd NW
Grading & Drainage Plans
Engineer's Stamp Date: 02/22/24
Hydrology File: K10D066

Dear Mr. Biazar:

Based upon the information provided in your submittal received 01/08/2024, the Grading & Drainage Plans are approved for Building Permit, Grading Permit and SO-19 Permit. Please attach a copy of this approved plan in the construction sets for Building Permit processing along with a copy of this letter.

PRIOR TO CERTIFICATE OF OCCUPANCY:

1. Engineer's Certification, per the DPM Part 6-14 (F): *Engineer's Certification Checklist For Non-Subdivision* is required.
2. Please provide the executed paper Drainage Covenant (latest revision) printed on one-side only with Exhibit A and a check for **\$25.00** made out to "**Bernalillo County**" for the detention ponds per Article 6-15(C) of the DPM to Hydrology for review at Plaza de Sol.

As a reminder, if the project total area of disturbance (including the staging area and any work within the adjacent Right-of-Way) is 1 acre or more, then an Erosion and Sediment Control (ESC) Plan and Owner's certified Notice of Intent (NOI) is required to be submitted to the Stormwater Quality Engineer (Doug Hughes, PE, jhughes@cabq.gov, 924-3420) 14 days prior to any earth disturbance.

If you have any questions, please contact me at 924-3995 or rbrissette@cabq.gov.

Sincerely,

Renée C. Brissette, P.E. CFM
Senior Engineer, Hydrology
Planning Department



City of Albuquerque

Planning Department
Development & Building Services Division

DRAINAGE AND TRANSPORTATION INFORMATION SHEET (REV 6/2018)

Project Title: D& B U-SELL-IT **Building Permit #:** _____ **Hydrology File #:** _____
DRB#: _____ **EPC#:** _____ **Work Order#:** _____
Legal Description: TRACT B AND LOT 5, BLOCK 2, CLIFFORD WEST BUSINESS PARK, UNIT 1
City Address: 8531 BLUEWATER RD., NW

Applicant: SBS CONSTRUCTION AND ENGINEERING, LLC **Contact:** SHAWN BIAZAR
Address: 10209 SNOWFLAKE CT., NW, ALBUQUERQUE, NM 87114
Phone#: (505) 804-5013 **Fax#:** (505) 897-4996 **E-mail:** AECLLC@AOL.COM

Other Contact: _____ **Contact:** _____
Address: _____
Phone#: _____ **Fax#:** _____ **E-mail:** _____

TYPE OF DEVELOPMENT: _____ PLAT (# of lots) _____ RESIDENCE _____ DRB SITE ☒ ADMIN SITE

IS THIS A RESUBMITTAL? _____ Yes ☒ No

DEPARTMENT _____ TRANSPORTATION ☒ HYDROLOGY/DRAINAGE

Check all that Apply:

TYPE OF SUBMITTAL:

_____ ENGINEER/ARCHITECT CERTIFICATION
_____ PAD CERTIFICATION
_____ CONCEPTUAL G & D PLAN
☒ GRADING PLAN
☒ DRAINAGE REPORT
_____ DRAINAGE MASTER PLAN
_____ FLOODPLAIN DEVELOPMENT PERMIT APPLIC
_____ ELEVATION CERTIFICATE
_____ CLOMR/LOMR
_____ TRAFFIC CIRCULATION LAYOUT (TCL)
_____ TRAFFIC IMPACT STUDY (TIS)
_____ STREET LIGHT LAYOUT
_____ OTHER (SPECIFY) _____
_____ PRE-DESIGN MEETING?

TYPE OF APPROVAL/ACCEPTANCE SOUGHT:

☒ BUILDING PERMIT APPROVAL
_____ CERTIFICATE OF OCCUPANCY
_____ PRELIMINARY PLAT APPROVAL
_____ SITE PLAN FOR SUB'D APPROVAL
_____ SITE PLAN FOR BLDG. PERMIT APPROVAL
_____ FINAL PLAT APPROVAL
_____ SIA/ RELEASE OF FINANCIAL GUARANTEE
_____ FOUNDATION PERMIT APPROVAL
☒ GRADING PERMIT APPROVAL
_____ SO-19 APPROVAL
_____ PAVING PERMIT APPROVAL
_____ GRADING/ PAD CERTIFICATION
_____ WORK ORDER APPROVAL
_____ CLOMR/LOMR
_____ FLOODPLAIN DEVELOPMENT PERMIT
_____ OTHER (SPECIFY) _____

DATE SUBMITTED: 12-08-2024 **By:** SHAWN BIAZAR

COA STAFF:

ELECTRONIC SUBMITTAL RECEIVED: _____

FEE PAID: _____

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*
* ZONE 1
*
*****
* 100-YEAR,6-HR STORM ON-SITE BASIN (UNDER PROPOSED CONDITIONS) *
*****
START          TIME=0.0
RAINFALL       TYPE=1 RAIN QUARTER=0.0 IN
               RAIN ONE=1.69 IN RAIN SIX=2.17 IN
               RAIN DAY=2.49 IN DT=0.03333 HR

* TRACT 5
COMPUTE NM HYD ID=10 HYD NO=101.0 AREA=0.002041 SQ MI
               PER A=0.00 PER B=5.00 PER C=5.00 PER D=90.00
               TP=0.1333 HR MASS RAINFALL=-1

* TRACT B
COMPUTE NM HYD ID=20 HYD NO=102.0 AREA=0.007383 SQ MI
               PER A=0.00 PER B=5.00 PER C=5.00 PER D=90.00
               TP=0.1333 HR MASS RAINFALL=-1
*****
*          PONDING CONDITION-TRACT 5          *
*****
ROUTE RESERVOIR  ID=100 HYD NO=510.0 INFLOW ID=10 CODE=24
                  OUTFLOW(CFS)      STORAGE(AC-FT)  ELEVATION(FT)
                  0.00              0.00000        5123.65
                  0.01              0.02095        5124.33
                  0.01              0.04190        5125.00
                  0.39              0.04656        5125.10
                  0.62              0.05431        5125.20
                  0.86              0.06903        5125.33
                  0.89              0.13962        5125.70
                  0.95              0.16599        5125.80
                  1.01              0.19545        5125.90
*****
*          PONDING CONDITION-TRACT B          *
*****
ROUTE RESERVOIR  ID=200 HYD NO=520.0 INFLOW ID=20 CODE=24
                  OUTFLOW(CFS)      STORAGE(AC-FT)  ELEVATION(FT)
                  0.00              0.00000        5118.20
                  0.01              0.07497        5119.40
                  0.01              0.14993        5120.60
                  0.81              0.16646        5120.80
                  1.29              0.19104        5121.00
                  1.50              0.20635        5121.10
                  1.89              0.26439        5121.40
                  2.12              0.31315        5121.60
                  2.32              0.36997        5121.80
                  2.51              0.43485        5122.00
                  2.68              0.50779        5122.20
                  2.84              0.58879        5122.40
                  2.92              0.63231        5122.50
*****
FINISH

```

RUN DATE (MON/DAY/YR) =02/22/2024
USER NO.= AHYMO-I-9702c01000R31-AH

[illegible]

AHYMO PROGRAM (AHYMO_97) -

- Version: 1997.02d

RUN DATE (MON/DAY/YR) = 02/22/2024

START TIME (HR:MIN:SEC) = 22:26:50

USER NO.= AHYMO-I-9702c01000R31-AH

INPUT FILE = BLUEB.TXT

*

* ZONE 1

*

* 100-YEAR,6-HR STORM ON-SITE BASIN (UNDER PROPOSED CONDITIONS) *

START TIME=0.0

RAINFALL TYPE=1 RAIN QUARTER=0.0 IN

RAIN ONE=1.69 IN RAIN SIX=2.17 IN

RAIN DAY=2.49 IN DT=0.03333 HR

COMPUTED 6-HOUR RAINFALL DISTRIBUTION BASED ON NOAA ATLAS 2 - PEAK AT 1.40 HR.

DT = .033330 HOURS END TIME = 5.999400 HOURS

.0000	.0037	.0075	.0114	.0153	.0193	.0234
.0276	.0319	.0363	.0408	.0455	.0502	.0551
.0601	.0652	.0705	.0759	.0815	.0873	.0933
.0995	.1060	.1126	.1196	.1268	.1344	.1423
.1505	.1593	.1685	.1731	.1781	.1835	.1949
.2206	.2600	.3167	.3941	.4958	.6254	.7868
.9839	1.1668	1.2432	1.3076	1.3650	1.4172	1.4652
1.5098	1.5515	1.5905	1.6273	1.6620	1.6947	1.7258
1.7551	1.7830	1.8094	1.8345	1.8583	1.8646	1.8705
1.8761	1.8815	1.8867	1.8918	1.8967	1.9014	1.9060
1.9105	1.9149	1.9191	1.9233	1.9273	1.9313	1.9352
1.9390	1.9427	1.9464	1.9500	1.9535	1.9569	1.9603
1.9637	1.9670	1.9702	1.9734	1.9766	1.9797	1.9827
1.9857	1.9887	1.9916	1.9945	1.9974	2.0002	2.0030
2.0058	2.0085	2.0112	2.0139	2.0165	2.0191	2.0217
2.0242	2.0268	2.0293	2.0317	2.0342	2.0366	2.0390
2.0414	2.0438	2.0461	2.0485	2.0508	2.0530	2.0553
2.0575	2.0598	2.0620	2.0642	2.0664	2.0685	2.0707
2.0728	2.0749	2.0770	2.0791	2.0811	2.0832	2.0852
2.0872	2.0892	2.0912	2.0932	2.0952	2.0971	2.0991
2.1010	2.1029	2.1048	2.1067	2.1086	2.1105	2.1123
2.1142	2.1160	2.1178	2.1197	2.1215	2.1233	2.1251
2.1268	2.1286	2.1304	2.1321	2.1338	2.1356	2.1373
2.1390	2.1407	2.1424	2.1441	2.1458	2.1474	2.1491
2.1507	2.1524	2.1540	2.1556	2.1573	2.1589	2.1605
2.1621	2.1637	2.1653	2.1668	2.1684	2.1700	

* TRACT 5

COMPUTE NM HYD

ID=10 HYD NO=101.0 AREA=0.002041 SQ MI

PER A=0.00 PER B=5.00 PER C=5.00 PER D=90.00

TP=0.1333 HR MASS RAINFALL=-1

K = .072649HR TP = .133300HR K/TP RATIO = .545000 SHAPE CONSTANT, N = 7.106420

UNIT PEAK = 7.2522 CFS UNIT VOLUME = .9978 B = 526.28 P60 = 1.6900

AREA = .001837 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR

RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

K = .118998HR TP = .133300HR K/TP RATIO = .892706 SHAPE CONSTANT, N = 3.972111
UNIT PEAK = .54096 CFS UNIT VOLUME = .9749 B = 353.31 P60 = 1.6900
AREA = .000204 SQ MI IA = .42500 INCHES INF = 1.04000 INCHES PER HOUR
RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

* TRACT B

COMPUTE NM HYD ID=20 HYD NO=102.0 AREA=0.007383 SQ MI
PER A=0.00 PER B=5.00 PER C=5.00 PER D=90.00
TP=0.1333 HR MASS RAINFALL=-1

K = .072649HR TP = .133300HR K/TP RATIO = .545000 SHAPE CONSTANT, N = 7.106420
UNIT PEAK = 26.234 CFS UNIT VOLUME = .9989 B = 526.28 P60 = 1.6900
AREA = .006645 SQ MI IA = .10000 INCHES INF = .04000 INCHES PER HOUR
RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

K = .118998HR TP = .133300HR K/TP RATIO = .892706 SHAPE CONSTANT, N = 3.972111
UNIT PEAK = 1.9568 CFS UNIT VOLUME = .9935 B = 353.31 P60 = 1.6900
AREA = .000738 SQ MI IA = .42500 INCHES INF = 1.04000 INCHES PER HOUR
RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - DT = .033330

* PONDING CONDITION-TRACT 5 *

ROUTE RESERVOIR	ID=100 HYD NO=510.0 INFLOW ID=10 CODE=24	OUTFLOW(CFS)	STORAGE(AC-FT)	ELEVATION(FT)
		0.00	0.00000	5123.65
		0.01	0.02095	5124.33
		0.01	0.04190	5125.00
		0.39	0.04656	5125.10
		0.62	0.05431	5125.20
		0.86	0.06903	5125.33
		0.89	0.13962	5125.70
		0.95	0.16599	5125.80
		1.01	0.19545	5125.90

* * * * *

TIME (HRS)	INFLOW (CFS)	ELEV (FEET)	VOLUME (AC-FT)	OUTFLOW (CFS)
.00	.00	5123.65	.000	.00
.80	.04	5123.65	.000	.00
1.60	3.41	5125.42	.086	.87
2.40	.22	5125.54	.109	.88
3.20	.06	5125.25	.060	.72
4.00	.05	5125.01	.043	.06
4.80	.05	5125.01	.042	.05
5.60	.05	5125.01	.042	.05
6.40	.00	5125.00	.042	.02
7.20	.00	5125.00	.042	.01
8.00	.00	5125.00	.042	.01

8.80	.00	5125.00	.042	.01
9.60	.00	5125.00	.042	.01
10.40	.00	5125.00	.042	.01
11.20	.00	5125.00	.042	.01
12.00	.00	5125.00	.042	.01
12.80	.00	5125.00	.042	.01
13.60	.00	5125.00	.042	.01
14.40	.00	5125.00	.042	.01
15.20	.00	5125.00	.042	.01
16.00	.00	5125.00	.042	.01
16.80	.00	5125.00	.042	.01
17.60	.00	5125.00	.042	.01
18.40	.00	5125.00	.042	.01
19.20	.00	5125.00	.042	.01

PEAK DISCHARGE = .882 CFS - PEAK OCCURS AT HOUR 2.10
 MAXIMUM WATER SURFACE ELEVATION = 5125.602
 MAXIMUM STORAGE = .1208 AC-FT INCREMENTAL TIME= .033330HRS

 * PONDING CONDITION-TRACT B *

ROUTE RESERVOIR	ID=200	HYD NO=520.0	INFLOW ID=20	CODE=24
	OUTFLOW(CFS)		STORAGE(AC-FT)	ELEVATION(FT)
	0.00		0.00000	5118.20
	0.01		0.07497	5119.40
	0.01		0.14993	5120.60
	0.81		0.16646	5120.80
	1.29		0.19104	5121.00
	1.50		0.20635	5121.10
	1.89		0.26439	5121.40
	2.12		0.31315	5121.60
	2.32		0.36997	5121.80
	2.51		0.43485	5122.00
	2.68		0.50779	5122.20
	2.84		0.58879	5122.40
	2.92		0.63231	5122.50

* * * * *

TIME (HRS)	INFLOW (CFS)	ELEV (FEET)	VOLUME (AC-FT)	OUTFLOW (CFS)
.00	.00	5118.20	.000	.00
.80	.15	5118.21	.000	.00
1.60	12.31	5121.63	.322	2.15
2.40	.82	5122.04	.450	2.54
3.20	.23	5121.63	.321	2.15
4.00	.18	5121.13	.211	1.53
4.80	.17	5120.70	.158	.40
5.60	.19	5120.65	.154	.19
6.40	.02	5120.62	.151	.09
7.20	.00	5120.60	.150	.01
8.00	.00	5120.60	.150	.01
8.80	.00	5120.60	.150	.01
9.60	.00	5120.60	.150	.01

PEAK DISCHARGE = 2.608 CFS - PEAK OCCURS AT HOUR 2.13
 MAXIMUM WATER SURFACE ELEVATION = 5122.116
 MAXIMUM STORAGE = .4770 AC-FT INCREMENTAL TIME=.033330HRS

FINISH

END TIME (HR:MIN:SEC) = 22:26:50

VOLUME/DISCHARGE CALCULATIONS

DETENTION/RETENTION POND-TRACT 5

Ab - Bottom Of The Pond Surface Area
 At - Top Of The Pond Surface Area
 D - Water Depth
 Dt - Total Pond Depth
 C - Change In Surface Area / Water Depth

$$\text{Volume} = \text{Ab} * \text{D} + 0.5 * \text{C} * \text{D}^2$$

$$\text{C} = (\text{At} - \text{Ab}) / \text{Dt}$$

Ab = 1,352.00
 At = 13,511.91
 Dt = 0.90
 C = 13511.01

From Elev. 5123.65 to 5125.00 (Retention Vol.)

ACTUAL ELEV.	DISCHARGE DEPTH (FT)	VOLUME (AC-FT)	Q (CFS)
5123.65	0.00	0.00000	0.00
5124.33	0.00	0.02095	0.00
5125.00	0.00	0.04190	0.00
5125.10	0.10	0.04656	0.39
5125.20	0.20	0.05431	0.62
5125.33	0.33	0.06903	0.86
5125.70	0.70	0.13962	0.89
5125.80	0.80	0.16599	0.95
5125.90	0.90	0.19545	1.01

Orifice Equation

Used from top of opening @ 5125.33 to top of wall @ 5125.90

Q = $\text{CA} (2gh)^{1/2}$
 Opening in the wall 0.67' x 0.33' width x height
 C = 0.6
 h = 0.90 ft
 g = 32.2
 A = 0.2211 sf
 Q = 1.01 cfs

Weir Equation

Used from bottom of opening @ 5125.00 to top of opening 5125.33

Q = $\text{CLH}^{2/3}$
 Opening in the wall 0.67' x 0.33' width x height
 C = 2.70
 L = 0.67
 H = 0.33
 Q = 0.86 cfs

VOLUME/DISCHARGE CALCULATIONS

DETENTION/RETENTION POND-TRACT B

Ab - Bottom Of The Pond Surface Area
 At - Top Of The Pond Surface Area
 D - Water Depth
 Dt - Total Pond Depth
 C - Change In Surface Area / Water Depth

$$\text{Volume} = Ab * D + 0.5 * C * D^2$$

$$C = (At - Ab) / Dt$$

Ab = 2,721.28
 At = 19,397.21
 Dt = 1.90
 C = 8776.81

From Elev. 5118.20 to 5120.60 (Retention Vol.)

ACTUAL ELEV.	DISCHARGE DEPTH (FT)	VOLUME (AC-FT)	Q (CFS)
5118.20	0.00	0.00000	0.00
5119.40	0.00	0.07497	0.00
5120.60	0.00	0.14993	0.00
5120.80	0.20	0.16646	0.81
5121.00	0.40	0.19104	1.29
5121.10	0.50	0.20635	1.50
5121.40	0.80	0.26439	1.89
5121.60	1.00	0.31315	2.12
5121.80	1.20	0.36997	2.32
5122.00	1.40	0.43485	2.51
5122.20	1.60	0.50779	2.68
5122.40	1.80	0.58879	2.84
5122.50	1.90	0.63231	2.92

Orifice Equation Used from top of opening @ 5121.10 to top of wall @ 5122.50

$$Q = CA (2gh)^{0.50}$$

Opening in the wall 0.88' x 0.50' width x height

C = 0.6

h = 1.90 ft

g = 32.2

A = 0.4400 sf

Q = 2.92 cfs

Weir Equation Used from bottom of opening @ 5120.60 to top of opening 5121.10

$$Q = CLH^{2/3}$$

Opening in the wall 0.88' x 0.50' width x height

C = 2.70

L = 0.88

H = 0.50

Q = 1.50 cfs

Location

Tract 5/Block 2 and Tract B, Clifford West Business, Unit 1 is located at the northwest corner of Unser Blvd. NW and Bluewater Rd. NW. See attached portion of Vicinity Map K-10-Z for exact location.

Purpose

The purpose of this drainage report is to present a grading and drainage solution for the proposed development.

Existing Drainage Conditions

This falls within the Master Drainage Plan K09/D023 prepared by Mark Goodwin and Associates. The site drains from north to south and west to Bluewater. There are two existing inlets on Blue Water Rd. close to the Unser Blvd. The site does not fall within a designated floodplain.

Proposed Conditions and On-Site Drainage Management Plan

Based on the Master Drainage Plan K09/D023 prepared by Mark Goodwin and Associates the total allowable discharge for Tract 5 is 1.01 cfs and Tract B is 2.91 cfs . Therefore, the runoff is being detained on site and discharge at the allowable rates.

Tract B drains to a ponding area to the south and discharges to back of sidewalk culvert at a flow rate of 2.61 cfs (witch is less than the allowable discharge rate of 2.91 cfs) through a 0.88'x0.50' opening in the wall to Blue Water Rd. The runoff backs up into the parking lot during the detention period. Part of the pond is retention to meet the first flush volume ponding requirement.

Tract 5 drains to a ponding area to the south as well and discharges towards the entrance and out to Blue Water Rd. at a flow rate of 0.88 cfs (witch is less than the allowable discharge rate of 1.01 cfs) through a 0.67'x0.33' opening. The runoff backs up into the parking lot during the detention period. Part of the pond is retention to meet the first flush volume ponding requirement. See Sheet 2 for ponding and runoff calculation using AHYMO. All the runoff which discharges onto Blue Water Rd. from this project will drain west to two existing inlets.

First flush ponding volume provided for Tract 5 is 1,825.20 cf which more that required volume of 1,792.36 cf. First flush ponding volume provided for Tract B is 6,531.07 cf which more that required volume of 6,483.25 cf.

FIRST FLUSH VOLUME CALCULATION

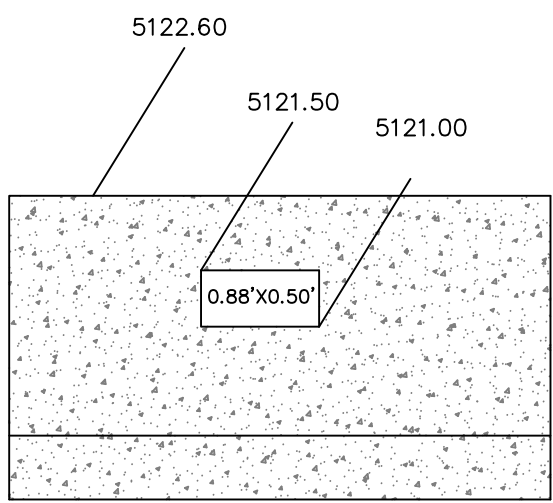
Tract 5 First Volume requirement = $(0.42/12 \times 185,235.70) = 6,486.25$ CF

Tract B First Volume requirement = $(0.42/12 \times 51,221.03) = 1,792.74$ CF

VOLUME CALCULATIONS FOR STORM WATER QUALITY PONDS

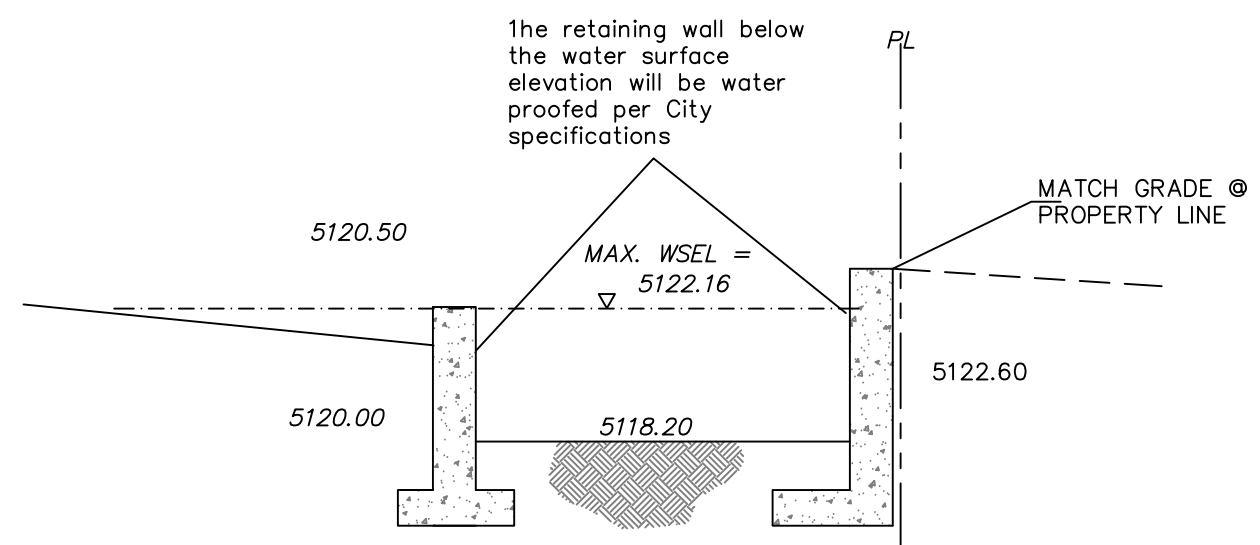
TRACT 5
BOTTOM AREA = 1,352.00 SF
TOP AREA = 1,352.00 SF
DEPTH = 1.35'
POND VOLUME=1,352.00 X 1.35
POND VOLUME=1,825.20 CF

TRACT B
BOTTOM AREA = 2,721.28 SF
TOP AREA = 2,721.28 SF
DEPTH = 2.40'
POND VOLUME=2,721.28 X 2.40
POND VOLUME=6,531.07 CF

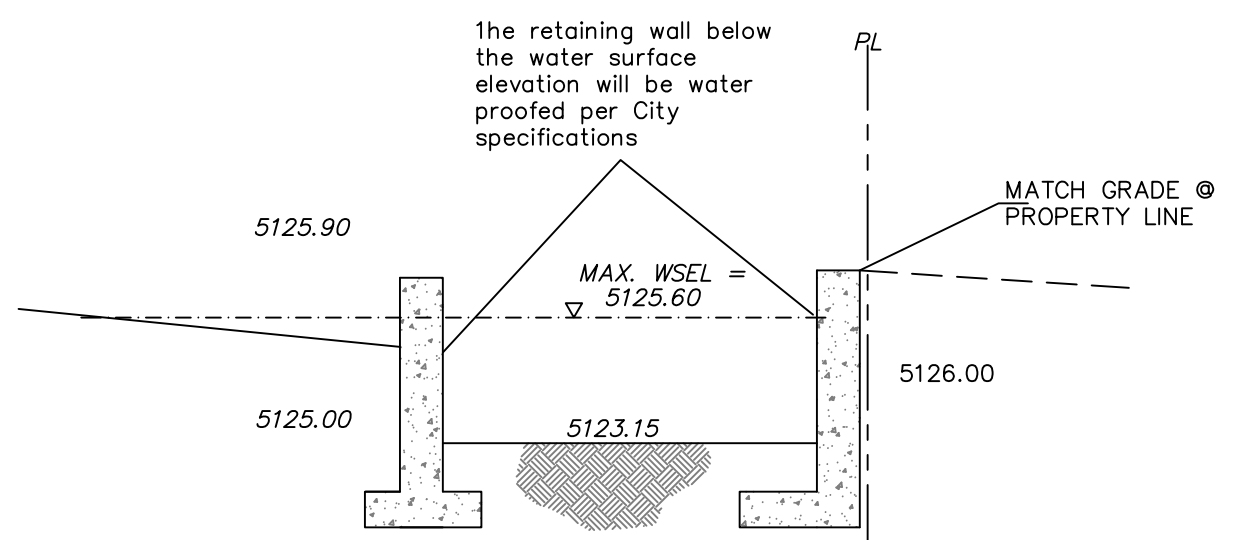


TYPICAL WALL OPENING

More details will be provided at the time of wall permitting

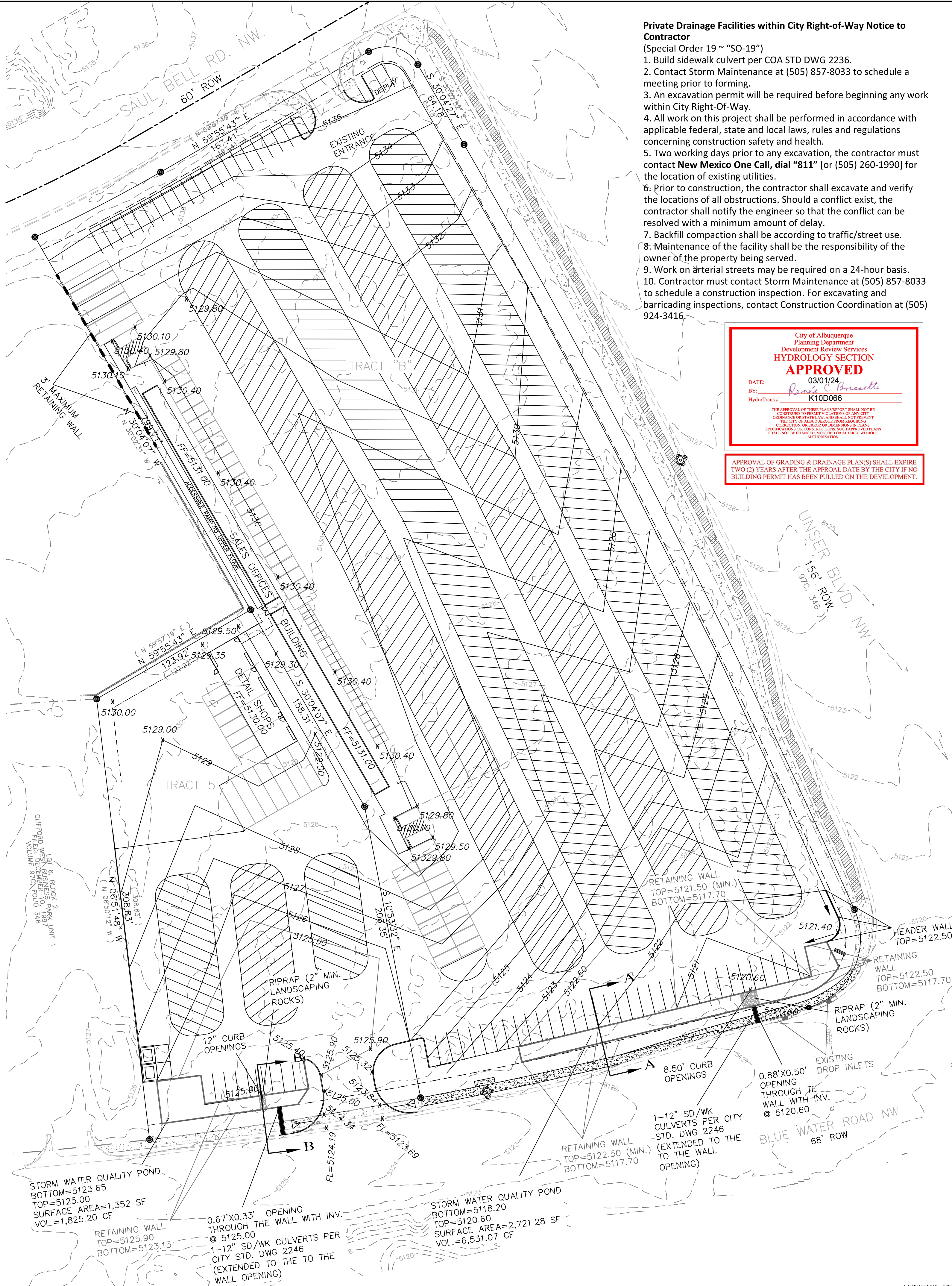
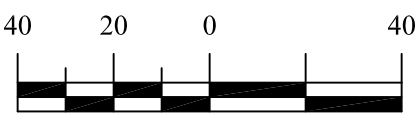


SECTION A-A



SECTION B-B

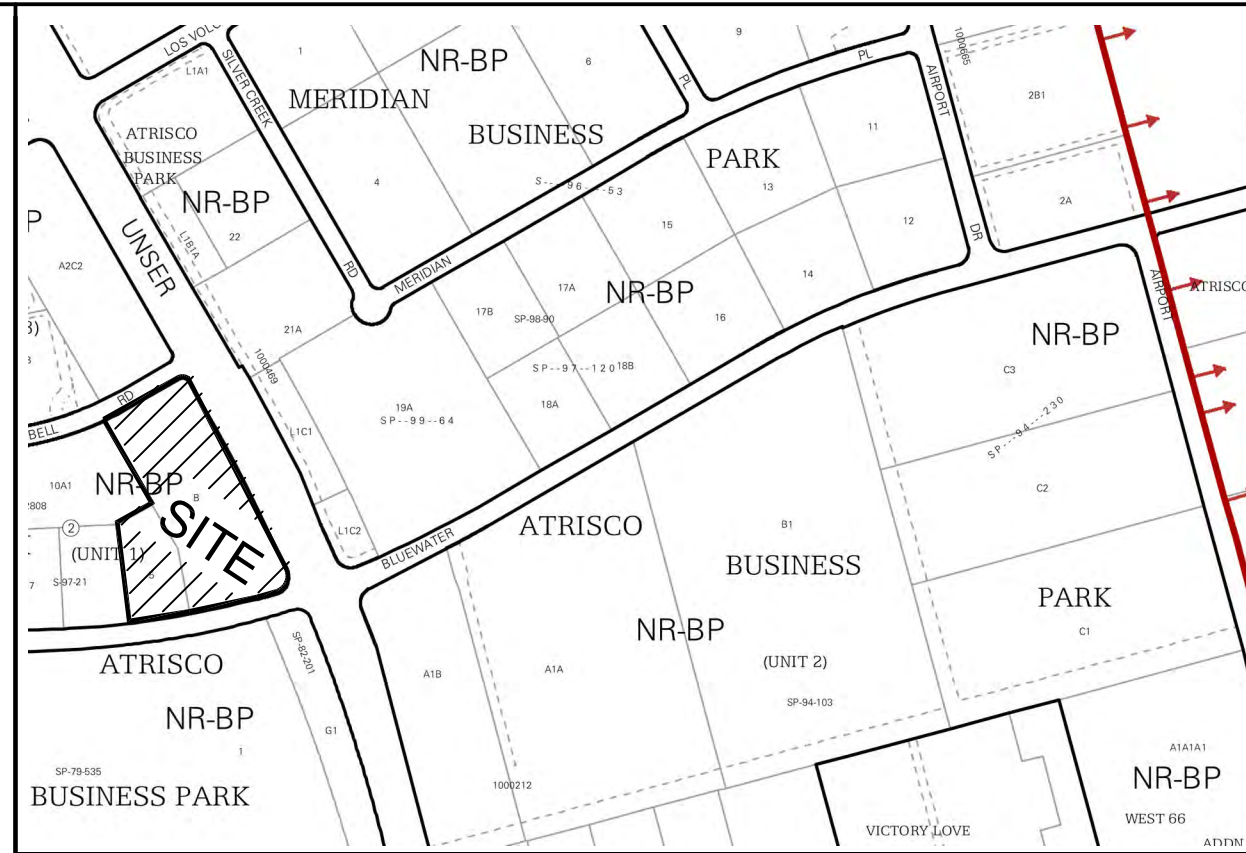
GRAPHIC SCALE



- Private Drainage Facilities within City Right-of-Way Notice to Contractor**
(Special Order 19 ~ "SO-19")
1. Build sidewalk culvert per COA STD DWG 2236.
 2. Contact Storm Maintenance at (505) 857-8033 to schedule a meeting prior to forming.
 3. An excavation permit will be required before beginning any work within City Right-Of-Way.
 4. All work on this project shall be performed in accordance with applicable federal, state and local laws, rules and regulations concerning construction safety and health.
 5. Two working days prior to any excavation, the contractor must contact **New Mexico One Call, dial "811"** for (505) 260-1990 for the location of existing utilities.
 6. Prior to construction, the contractor shall excavate and verify the locations of all obstructions. Should a conflict exist, the contractor shall notify the engineer so that the conflict can be resolved with a minimum amount of delay.
 7. Backfill compaction shall be according to traffic/street use.
 8. Maintenance of the facility shall be the responsibility of the owner of the property being served.
 9. Work on arterial streets may be required on a 24-hour basis.
 10. Contractor must contact Storm Maintenance at (505) 857-8033 to schedule a construction inspection. For excavating and barricading inspections, contact Construction Coordination at (505) 924-3416.

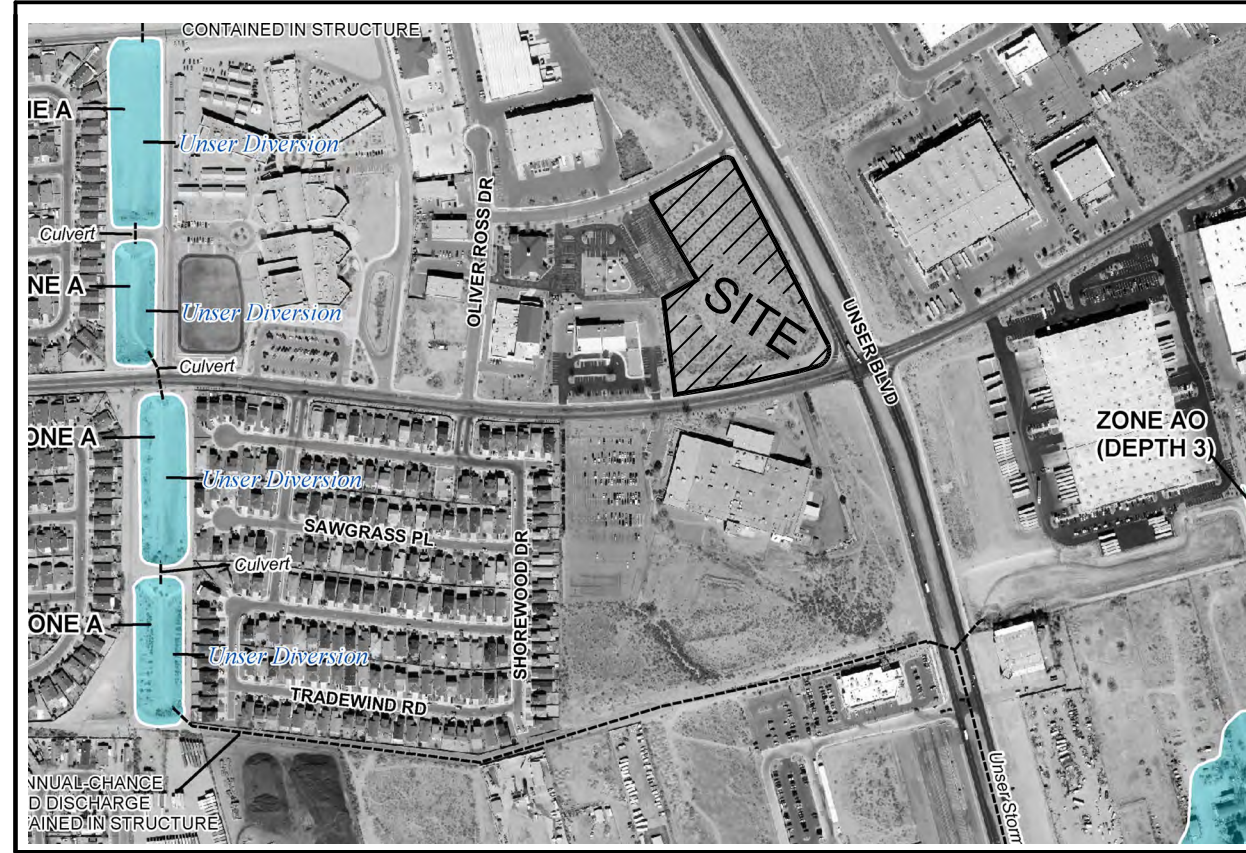


APPROVAL OF GRADING & DRAINAGE PLANS SHALL EXPIRE TWO (2) YEARS AFTER THE APPROVAL DATE BY THE CITY IF NO BUILDING PERMIT HAS BEEN PULLED ON THE DEVELOPMENT.



VICINITY MAP:

K-10-Z



FIRM MAP:

35001C0328J

LEGAL DESCRIPTION:

TRACTS 5/BLK 2 AND TRACT B
CLIFFORD WEST BUSINESS PARK, UNIT 1
ALBUQUERQUE, BERNALILLO COUNTY, NEW MEXICO

LEGEND

- 5030 — EXISTING CONTOUR (MAJOR)
- 5029 — EXISTING CONTOUR (MINOR)
- — BOUNDARY LINE
- ✕ 5242.70 PROPOSED SPOT ELEVATION
- ✕ 5029.16 EXISTING GRADE
- ✕ FL=5075.65 EXISTING FLOWLINE ELEVATION
- — PROPOSED RETAINING WALL
- BC=41.30 BOTTOM OF CHANEL
- TF=42.00 TOP OF FOOTING
- TRW=45.12 TOP OF RETAINING WALL
- HP HIGH POINT
- 42.40 AS-BUILT GRADES
- ✕ 42.40 AS-BUILT SPOT ELEVATIONS



SBS CONSTRUCTION AND ENGINEERING, LLC

7632 WILLIAM MOYERS AVE., NE
ALBUQUERQUE, NEW MEXICO 87122
(505) 804-5013
EMAIL: AECLLC@AOL.COM

RUNOFF AND PONDING CALCULATIONS
TRACTS 5/BLK 2 AND TRACT B
CLIFFORD WEST BUSINESS PARK, UNIT 1

DRAWING:	DRAWN BY:	DATE:	SHEET #
BLUEWATER.DWG	SH-B	2/22/2024	1 OF 2

