DRAINAGE INFORMA	TION SHEET
PROJECT TITLE: ME TUEL APTS ZONE AT	LAS/DRAINAGE FILE #K-//DY
LEGAL DESCRIPTION: LOT C-10-B, ATRPO	
CITY ADDRESS: 5901 Central N.W	
ENGINEERING FIRM: Weiss-Hines Engineering, Inc. CONTACT: STEVE CLARK	
ADDRESS: 1100 Alvarado N.E.	
OWNER: DR. WM. MCTVER	
ADDRESS:	PHONE:
ARCHITECT: BILL BURK	CONTACT:
ADDRESS:	PHONE:
SURVEYOR: GORDON DOUGLAS	CONTACT:
ADDRESS:	PHONE:
CONTRACTOR:	CONTACT:
ADDRESS:	PHONE:
PRE-DESIGN MEETING: THE PRE-DESIGN MEETING MEE	
YES     ( APR 0 6 1988 )	
× NO	
COPY OF CONFERENCE ROLOGY SECTION PROJECT NO.	
TYPE OF SUBMITTAL:	CHECK TYPE OF APPROVAL SOUGHT:
DRAINAGE REPORT	SECTOR PLAN APPROVAL
DRAINAGE PLAN	SKETCH PLAT APPROVAL
CONCEPTUAL GRADING & DRAIN PLAN	PRELIMINARY PLAT APPROVAL
GRADING PLAN	SITE DEVELOPMENT PLAN APPROVAL
EROSION CONTROL PLAN	FINAL PLAT APPROVAL
ENGINEER'S CERTIFICATION	BUILDING PERMIT APPROVAL
Resubmittal	FOUNDATION PERMIT APPROVAL
	CERTIFICATE OF OCCUPANCY APPROVAL
	ROUGH GRADING PERMIT APPROVAL
DATE SUBMITTED: $4-5-88$	GRADING/PAVING PERMIT APPROVAL
BY: Weiss-Hines Eng. Inc.	OTHER(SPECIFY).
REV. 10/85	•
DATE RECEIVED	·



# City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

MAYOR

CHIEF ADMINISTRATIVE OFFICER DEPUTY CAO PUBLIC SERVICES DEPUTY CAO PLANNING/DEVELOPMENT

KEN SCHULTZ

GENE ROMO

FRANK MARTINEZ

**BILL MUELLER** 

April 28, 1988

Steve Clark, P.E. Weiss-Hines Engineering, Inc. 1100 Alvarado, NE Albuquerque, New Mexico 87110

RE: GRADING & DRAINAGE REPORT FOR THE MCIVER APARTMENTS SUBMITTED APRIL 6, 1988, FOR BUILDING PERMIT APPROVAL (K11/D46)

Dear Mr. Clark:

Your submittal, referred to above, with your engineer's stamp dated April 6, 1988, is approved for Building Permit sign-off by the Hydrology Section. This approved drawing must be included with the construction sets routed for permit sign-off.

If you have any questions, please call me at 768-2650.

Cordially,

G. Stuart Reeder, P.E. C.E./Hydrology Section

GSR



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SCOPE:

The proposed improvements compris a slab-on-grade tri-plex apartment building (2500 SF total), a 1750 SF Turfstone parking area/retention pond, associated walks, and landscaped areas.

The present site is surrounded on all four sides by a 6'-high cinder block wall. An existing 1100 SF adobe house will remain. The lot is mostly unvegetated dirt and nearly level in grade, with slight depressions existing near the east and west walls.

The intent of this plan is to show:

- a) Grading relationships between the existing ground elevations and proposed finished elevations in order to facilitate positive drainage to designated discharge points.
- b) The extent of proposed site improvements, including buildings, walks and
- c) The flow rate/volume of rainfall runoff across or around these improvements and methods of handling these flows to meet City requirements for drainage management.
- d) The relationship of onsite improvements with existing neighboring property to insure an orderly transition between proposed and surrounding grades.

#### DRAINAGE PLAN CONCEPT:

This site cannot discharge storm flows into the storm sewer in Central Avenue (300 feet to the south) because of an absence of positive slope in that direction. Additionally, the site is totally enclosed by an existing cinder block wall making it necessary to retain storm runoff on-site. In order to facilitate absorption of runoff volumes into the ground within a 24-hour period, a Turfstone parking area underlaid with sand and gravel will serve as a retention pond. It is sized to accept the 1124 CF volume of water that would be generated by a 100-year storm on the developed site. Thus, as shown on the plans and in the calculations, all runoff shall be retained on-site and shall be absorbed within 24 hours.

In order to protect the foundations of existing and proposed structures on the site from water infiltration from the retention ponds (proposed at east side of site and existing on property adjacent to the north), a 36"-deep concrete curtain wall will be constructed on the western perimeter of the ponding area and just south of the block wall on the north property line, as shown on the plans.

### GENERAL NOTES:

construction remain on-site.

LEGAL: Tract C-10-B of the Airport Unit in the Town of Atrisco Grant, Albuquerque, NM.

Gordon Douglas & Associates, 126 Washington S.E., Albuquerque, NM. February,

B.M.: NM 448-C1, a NMSHC brass cap set in the top of a concrete post flush with the ground in the west median at the intersection of Central Avenue SW and 59th Street SW. Elevation = 5,074.073'.

T.B.M.: Double spike in a light pole 50' northeast of northeast corner of the site. Elevation = 5,077.18'.

SOILS: According to the SCS Soil Survey for Bernalillo County (Plate 30), the soil type is MWA (Madwez-Wink Association), a fine sandy loam of Hydrologic Group B.

within an established flood zone. OFF-SITE DRAINAGE: There are no significant off-site flows that affect the site. An

FLOOD HAZARD: According to FEMA Flood Boundary Map (Panel 27), the site does not lie

existing storm retention pond on the property to the north (Tract C-10-A) is adjacent to the block wall on the site's north property line. EROSION CONTROL: No flows will leave the site and any sediment generated during

CALCULATIONS: Calculations are based on the City of Albuquerque D.P.M. Manual, Vol. II for the 100-year, 6-hour storm, using the Rational Formula to compare the existing and proposed runoff rates.

### RATIONAL METHOD- Q = CIA

Area of site: 9,390 SF = 0.327 AC

Run-off Coefficient:

Existing site: Undeveloped Area = 8,324 SF Roof Area = 3,568 SF Paved Area = 506 SF Landscaped Area = 3,445 SF Roofed Area = 1,060 SF Paved Area = 1,128 SF Turfstone Area = 1,750 SF  $C_{ij} = (8324)(0.40) = 0.34$  $C_i = (3568)(0.90) = 0.32$ 

 $C_P = (506)(0.95) = 0.05$  $C_1 = (3445)(0.25) = 0.09$  $C_{1} = (1060)(0.90) = 0.10$  $C_P = (1127)(0.95) = 0.11$ 

 $C_1 = (1750)(0.55) = 0.10$ 

Composite C = 0.49Composite C = 0.62

## Rainfall Intensity:

 $I = P_6 (6.84) T_6^{-6.51} = 4.65$ " per hour where  $P_6 = 2.2^{\circ} (DPM 22.2 D-1)$ 

 $T_c = 10 \text{ minutes}$ 

Existing Condition: Developed Condition:  $Q_{100} = (0.49)(4.65)(0.227)$  $Q_{100} = (0.62)(4.65)(0.227)$ = 0.5 cfs = 0.7 cfs

 $V_{100} = (0.49)(P_6)(9,390)/12$  $V_{100} = (0.62)(2.2)(9890)/12$ = 1124 CF

 $Q_{100} = (0.7) - (0.5) = 0.2 \text{ cfs (increase)}$  $V_{100} = (1124) - (838) = 236 \text{ CF (increase)}$ 

PERCOLATION TEST RESULTS (P.S.I., INC., ALBUQUERQUE, HM, 1/26/38):

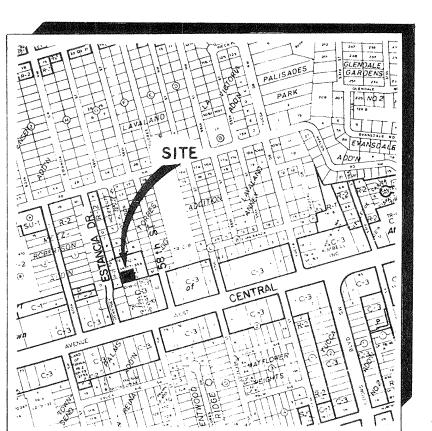
Hole Volume =  $\pi r^2 h = (3.14)(0.25')^2(6') = 1.2 \text{ CF}$ Percolation Rate = 6"/hour = 6"/12 hours Area of Hole =  $2\pi rh$  = (2)(3.14)(0.25')(6') = 9.4 SF Percolation Rate = 1.2 CF/12 hours/9.4 SF = 0.01 CF/hour/SF

#### RETENTION FOND SIZING:

Area of Retention Pond = 1,750 SF Runoff volume to be stored/absorbed = 1,104 CF

- 1. Infiltration Volume = (1750 SF)(0.01 CF/hr/SF)(24 hours) = 420 CF absorbed in
- 2. Turfstone void volume = (24")(13")(3-5/3")(0.67) = 0.6 CF/block 1750 SF (0.6 CF/block)(0.3 efficiency) = 105 CF 3 SF/block
- 3. Turfstone gravel bedding = (1750 SF)(2" bedding)(0.3 efficiency) = 88 CF
- 4. Turfstone gravel base = (1750 SF)(1 ft gravel)(0.3 efficiency) = 525 CF SUMMARY: 1 + 2 + 3 + 4 = 420 + 105 + 88 + 525 = 1138 CF storage

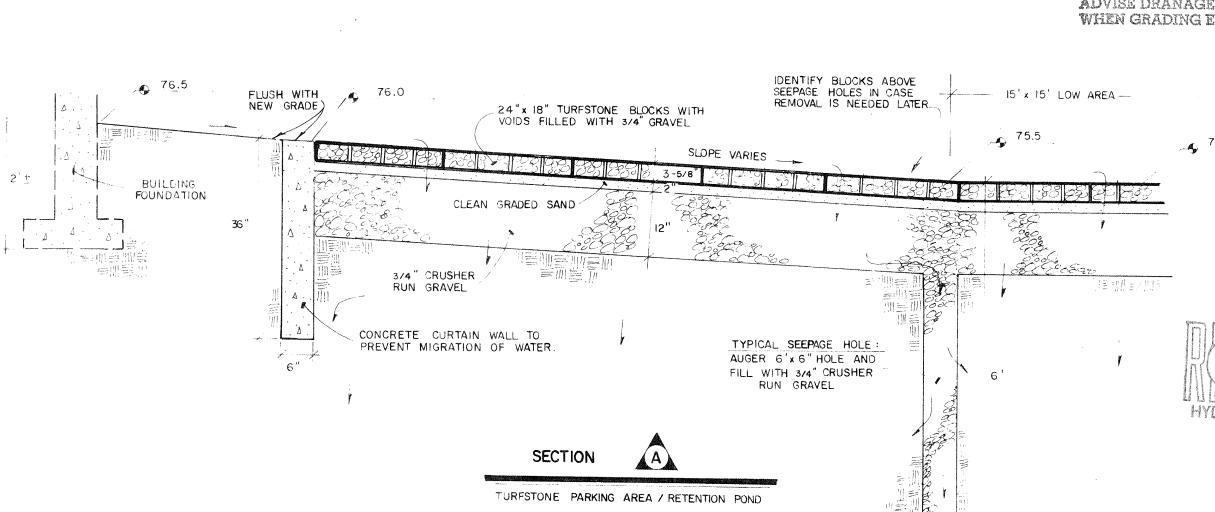
APPROVED FOR DRAINAGE ADVISE DRANAGE INSPECTOR WHEN GRADING EXECUTED



VICINITY MAP K-II

FLOOD HAZARD MAP

BUILDING (EXISTING, PROPOSED) EXISTING SPOT ELEVATION EXISTING CONTOUR PROPOSED SPOT ELEVATION PROPOSED CONTOUR SURFACE FLOW DIRECTION (EXISTING, PROPOSED) TOP OF GRADE WALL (LESS THAN 18" HIGH) TOP OF RETAINING WALL (MORE THAN 18" HIGH) TOP OF ASPHALT TOP OF CURB



HYDROLOGY SECTION

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