	WORK ORDER #:
	16 Town OF ATRISCO Grant
TY ADDRESS:	CONTACT: Frank D. Lovelady
IGINEERING FIRM: Lovelady & Associates	545-2267 :
ADDRESS: 300 ALAMOSA HW	
MER: Don Keith & ASSOC	contact: Don Keith
ADDRESS: 7116 Edwing CT NE	
RCHITECT: <u>Pick Bennett Architect</u>	CONTACT: Rick Bennott
ADDRESS: 1118 PARK AUE SW 87/	102 PHONE: 242-1859
URVEYOR: SOUTHWEST SURJEY CO 1	IC CONTACT: DAN GRAHEY
ADDRESS: 333 LOMAS BUD. NES	PTIONE: 247-4444
CONTRACTOR: - RAM CONSTRUCTION	
	PHONE: 452-126.6
DRAINAGE REPORT DRAINAGE PLAN	CK TYPE OF APPROVAL SOUGHT: SKETCH PLAT APPROVAL PRELIMINARY PLAT APPROVAL S. DEV. PLAN FOR SUB'D. APPROVAL S. DEV. PLAN FOR BLDG. PERMIT APPROVAL SECTOR PLAN APPROVAL FINAL PLAT APPROVAL FOUNDATION PERMIT APPROVAL BUILDING PERMIT APPROVAL CERTIFICATE OF OCCUPANCY APPROVAL GRADING PERMIT APPROVAL PAVING PERMIT APPROVAL S.A.D. DRAINAGE REPORT DRAINAGE REQUIREMENTS OTHER (SPECIFY)
DATE SUBMITTED: April 18,1997 BY: Frank D. Jovelody	APR 2 2 1997 HYDROLOGY SECTION

‡



Martin J. Chávez, Mayor

Robert E. Gurulé, Director

May 1,1997

Frank Lovelady Lovelady and Associates 300 Alamosa NW Albuquerque, New Mexico 87107

RE: REVISED DRAINAGE PLAN FOR DON KEITH MINI STORAGE (K11-D54) REVISION DATED 4/16/97

Dear Mr. Lovelady:

Based on the information provided on your April 22,1997 resubmittal, the above referenced site is approved for Building Permit. Please be advised that if the Building Permit has already been obtained, it is your responsibility to assure that the contractor is given a copy of this revised approved Drainage Plan.

Also, all the other requirements found on the October 9,1936 letter are still valid.

If I can be of further assistance, please feel free to contact me at 924-3986.

C: Andrew Garcia
File

Sincerely

Bernie J. Montoya CE Associate Engineer



TOTAL MATTERIANALIT GORITHVA

)JECT TITLE: DONKEITH MINI STORAG	E ZONE ATLAS/DRNG. FILE #: K-11/D-54
- N	WORK ORDER #:
SAL DESCRIPTION: POR TRACT 76	UITHIH TOWN OF ATRISCO GRANT
ry address:	
GINEERING FIRM: Lovelady & Associates	CONTACT: Frank D. Lovelady
ADDRESS: 300 ALAMOSA	HW PHONE: 345-2267
	CONTACT: DON KEITH
: ADDRESS: THE EDWINA CT	NE PHONE: 884-4070
CHITECT: RICK BENNETT, ARC	HITECT CONTACT: RICE-BENNETT.
ADDRESS: INS PARK AVE S	
RVEYOR: SOUTHWEST SURVEY	
ADDRESS: 333 LOMAS	NE PHONE: 247-4444
MIRACTOR: RAM CONSTRUCTO	ON CONTACT: WAYNE LUJAH
ADDRESS: 1321 TOBACCO	ROAD 5W PHONE: 883-2593
ADDRESS: (1987)	87105
•	CHECK TYPE OF APPROVAL SOUGHT:
PE OF SUBMITTAL:	SKETCH PLAT APPROVAL
DRAINAGE REPORT	PRELIMINARY PLAT APPROVAL
DRAINAGE PLAN	S. DEV. PLAN FOR SUB D. APPROVAL
CONCEPTUAL GRADING & DRAINAGE PLAN	S. DEV. PLAN FOR BLDG. PERMIT APPROVAL
GRADING PLAN	SECTOR PLAN APPROVAL
EROSION CONTROL PLAN	FINAL PLAT APPROVAL
ENGINEER'S CERTIFICATION	
OTHER	BUILDING PERMIT APPROVAL
	CERTIFICATE OF OCCUPANCY APPROVAL
'E-DESIGN MEETING:	GRADING PERMIT APPROVAL
YES	PAVING PERMIT APPROVAL
ИО	S.A.D. DRAINAGE REPORT
COPY PROVIDED	DRAINAGE REQUIREMENTS
	OTHER (SPECIFY) .
	SEP 2 2 1998
ITE SUBMITTED: SEPTEMBER 22	1998
BY: Frank O Fovelo	HYDROLOGY SECTION



October 5,1998

Frank Lovelady Lovelady & Associates 300 Alamosa NW Albuquerque, New Mexico 87107

RE: REVISED ENGINEER CERTIFICATION FOR DON KEITH MINI STORAGE UNITS (K11-D54) REVISED CERTIFICATION STATEMENT DATED 9/22/98

Dear Mr. Lovelady:

Based on the information provided on your September 22,1998 resubmittal, Engineer Certification for the above referenced site is acceptable.

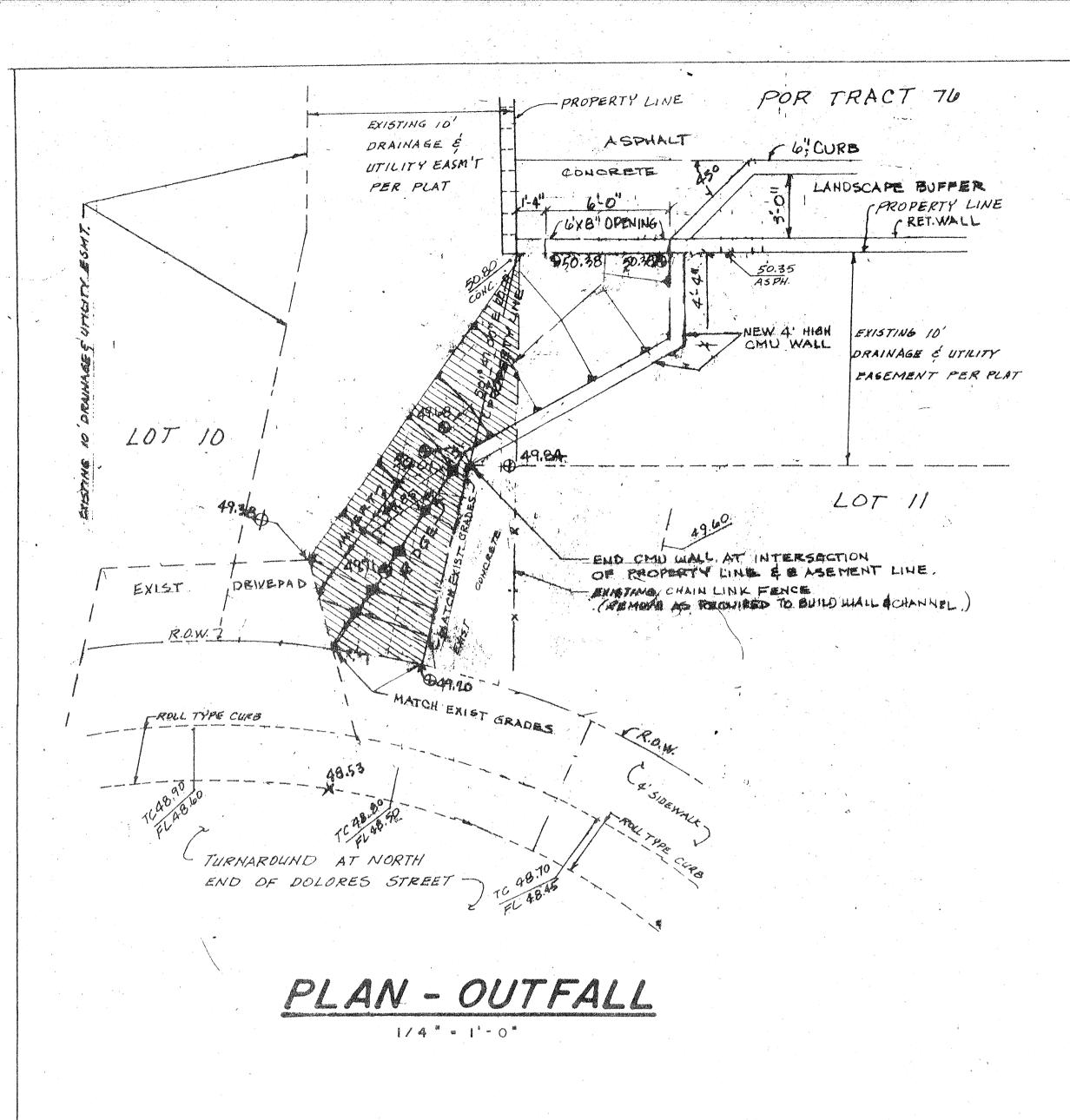
If I can be of further assistance, please feel free to contact me at 924-3986.

C: Andrew Garcia
File

Sincerely

Bernie J. Montoya ČE Associate Engineer





EXISTING CONDITIONS:

The site is located on the south side of west Central Avenue between 57th Street and 59th Street. 58th Street intersects Central Avenue on the north side only and the intersection is across the street from the site. Central Avenue is paved with standard. curb and gutter. The site is presently partially occupied by a motel which is used as apartments. The motel covers the westerly portion of the site. There is a very significant drop in elevation from the northwest corner of the site, elevation 5068.0, to the southeast corner of the site, 5049.0. The property west, of the site is vacant between the site and 59th Street. The property east of the site is developed with an office building and asphalt parking lot. There is a concrete wall along the northern two-thirds of the east property line that is part of the office building development. The site is surrounded on the west, north and east sides by a six-foot-high wrought iron fence. Along the south side there is a concrete block walk on Lot 10, Bentwood Ridge Subdivision. There is no wall between the site and Lot 11, Bentwood Ridge Subdivision. (The Zone Atlas map shows the subdivision name as Brentwood Ridge Subdivision which is apparently erroneous since the plat is filed under the name Bentwood Ridge Subdivision). The street serving Bentwood Ridge Subdivision is Dolores Drive which is paved with roll-type curb and gutter. There is a 30" RCP storm sewer line in Churchill Road with inlets at the NW and SW curb returns of the intersection of Dolores Drive and Churchill Road.

PROPOSED CONDITIONS:

It is proposed to develop a portion of the site as a mini-storage facility as shown on the plan. The development shown is Phase One. No design has been prepared for Phase Two but it is assumed that it will be entirely pavement or roof surface and will occupy the entire area shown as phase two on the drainage plan. Calculations for Phase Two are also shown so that the drainage outfall structure can be designed for the entire development. It is proposed to be discharged through existing platted drainage easements into Dolores Road. The alternative would be to pump all runoff into Central Avenue which is to be avoided, if possible.

DRAINAGE CRITERIA:

The calculations shown on this plan were prepared in accordance with Section 22.2, Hydrology, of the Development Process Manual, Volume 2, Design Criteria, for the City of Albuquerque in cooperation with Bernalillo County, New Mexico, and the Albuquerque Metropolitan Arroyo Flood Control Authority, January 1993.

PRECIPITATION ZONE:

The site is west of the Rio Grande River and is, therefore, in Precipitation Zone 1.

LAND TREATMENTS, EXCESS PRECIPITATION AND UNIT PEAK DISCH.:

The peak discharge per acre and excess precipitation are shown for the four land treatments in Zone 1 in the table below, the values contained in which are from the City of Albuquerque, D.P.M.

Land		IN (E) AND UNIT (acre)		EAK DISCHARGE (q) E(Inches)			
Treatment	100-year	10-year	100-year.	10 year			
A	1.29	0.24	0.44	0.08			
В	2.03	0.76	0.67	0.22			
C	2.87	1.49	0.99	0.44			
D	4.37	2.89	1.97	1.24			

EXISTING AND DEVELOPED LAND TREATMENT PERCENTAGES:

PHASE ONE

Land	Percent	and Treat. Areas Area of Site Sq.Ft. Acres	Developed Land Treat. Area Percent Area of Site of Site Sq.Ft. Acres				
Treatments A	of Site	og.rr. acres	<u> </u>	bd2\fa.ku.bu			
${f B}$	••• :		3.5	2,032	0.047		
· C	90.3	52,069 1.195	61.1	35,217	0.808		
D	9.7	5,566 0.128	35.4	20386	0.468		
TOTALS	100.0	57,635 1.323	100.0	57,635	1.323		
Land Treatments A							
В		3.5 2,032	0.047				
С		33,8 19,468	0.447	•			
D		62.7 36,135	0.829				
TOTALS		100.0 57,635	1.323				

WEIGHTED UNIT PEAK DISCHARGE VALUES:

Existing	q _{w100}	$= 0.903 \times 2.87 + 0.097 \times 4.37 = 3.02$
,	Quero	$= 0.903 \times 1.49 + 0.097 \times 2.98 = 1.63$
Phase One	9w100	$\approx 0.035 \times 2.03 + 0.611 \times 2.87 + 0.354 \times 4.37 = 3.37$
•	.qwio	$= 0.035 \times 0.76 + 0.611 \times 1.49 + 0.354 \times 2.89 = 1.96$
Phase Two	Qw100	$= 0.035 \times 2.03 + 0.338 \times 2.87 + 0.627 \times 4.37 = 3.78$
•	$q_{w_{10}}$	$= 0.035 \times 0.76 + 0.338 \times 1.49 + 0.627 \times 2.89 = 2.34$

WEIGHTED EXCESS PRECIPITATION:

Bxisting	Ew100	= .2	0.903	X 0.99	+	0.097	Х	1.97	-	1.09			
	Euro	Amplified	0.903	X = 0.44	+-+	0.097	X	1.24	April 1986	0.52			
Phase One	E _{w100}	22	0.035	X 0.67	* +	0.611	X	0.99	-+-	0.354	X 1.9	7 ==	: 1
	Ewio		0.035	$\times 0.22$! +	0.611	-X	0.44	X	0.3540	X 1.24		0.73
Phase Two	E _{W100}	-	0.035	$\times 0.67$	1 +	0.338	X	0.99	+	0.627	X 1.9	/ ==	1.5
	E _{w10}	e vice	0.035	X 0.22	2. +	0.338	X	0.44	+	0.627	X 1.2	1 ===	0.9

SUMMARY OF VOLUMES AND PEAK DISCHARGE RATES

	<u>Y100</u>	V10	Q100	Q1Q
EXISTING	5235	2498	4.00	2.16
PHASE ONE	6388	3458	4.46	2.59
PHASE TWO	7637	4467	5.00	3.10

ANALYSIS OF DOWNSTREAM CONDITIONS:

It is proposed to discharge runoff from the site into the north end of Dolores Drive by means of sidewalk culverts extending from the south edge of the site to the end of the turnaround and occupying the extreme westerly end of Lot 11, Bentwood Ridge Subdivision. The plat for Bentwood Ridge Subdivision shows a 10' drainage and utility easement along the north side of Lot 11 and along the east side of Lot 10. Crossing Lot 10 is not practical since there is a concrete block wall between the two lots and Lot 10 is paved with concrete. It is more direct to take the drainage straight out across Lot IT even though an easement must be obtained for the short distance between the existing drainage easement and the turnaround.

The capacity of Dolores Drive will be analyzed below. There are inlets at the NW and SW corners of Churchill Avenue and Dolores Drive.

There are two projects scheduled for 1996 downstream from the site, the Osage/La Media storm drain and the detention pond east of Coors between Churchill and Gonzales. The site is an infill site and the increase in runoff resulting from development is not large. Therefore, detention ponding would not appear to be a requirement.

OFF-SITE FLOW:

The site has minimal off-site flow from the vacant lot to the west. It appears that about half of that lot could possibly drain onto the site under the wrought iron security fence. The area is covered with native grasses and weeds and is considered to be Land Treatment A. The area is a triangular area approximately 95' X 250' = 11,875 SF or 0.27 acres. $q_{100} = 1.29 \text{ cfs} / \text{ acre}$ $Q_{100} = 0.27 \text{ X } 1.29 = 0.35 \text{ cfs}$

The off-site flow from this area is accepted by means of a single "D" inlet placed at the SW corner of the site as shown. The infet is drained through the site by an 8" PVC pipe to another single "D" inlet and then to a double "D" inlet which collects all of the site runoff. The grate elevation of the off-site flow inlet is 54.30; the invert is 52.30. Use Orifice Equation, $Q = CA(2GH)^{1/2}$, C = 0.6 A = 0.3494 H = 2.0 - 0.33 = 1.67 $Q = 0.6 \times 0.3494 (2 \times 32.2 \times 1.67)^{1/2} = 2.17 \text{ cfs} > 0.35 \text{ cfs}$. The larger diameter pipe will give some protection against clogging.

DOUBLE "D" INLET - CAPACITY OF OUTLET PIPES:

It is proposed to use 4 each 6" PVC pipes throught the rear retaining wall so the the flow profile will be as low as possible as it enters the drainge channel that leads to the sidewalk culvert. Design Q = 5.35 cfs. Use Orifice Equation $Q = CA(2GH)^{1/2}$ C = 0.6 A = 0.1963 H = (53.0 - 49.7) - 0.25 = 3.05 $Q = 0.6 \times 0.1963 (2 \times 32.2 \times 3.05)^{1/2} = 1.65 \text{ cfs}' + 4 \times 1.65 = 6.60 \text{ cfs} + 5.35 \text{ cfs}$ The four 6" PVC pipes are adequate.

ANALYSIS OF DOWNSTREAM CAPACITY:

According to the drainage plan for Bentwood Ridge Subdivision by Community Sciences Corporation, November 1978, each lot is supposed to pond runoff on site The street right-of-way only was to generate runoff that would leave the subdivision. The total peak discharge, including off-site flow, that the street would have to carry was 7.8 CFS. The hydrology has since changed and it is not certain if all, or any, of the residences actually ponded on-site. Therefore, the total site area is assumed to discharge to Dolores Drive. Area = 3.666 Ac. Assume 20% Treatment B, 10% Treatment C and 70% Treatment D. $qW = 0.2 \times 2.03 + 0.1 \times 2.87 + 0.7 \times 4.37 = 3.75 \text{ cfs/ac}$

 $O = 3.66 \times 3.75 = 13.73 \text{ CFS}$ Off-site flow from the mini-storage site is 5.35 cfs Total Q = 13.73 + 5.35 cfs = 19.08 cfs

The street carrying capacity of Dolores Drive is calculated by Manning's Equation based on the Standard Drawing 2415 for roll type curb and assuming a cross slope of 2% from the lip of the curb. Total area = 2.36 SF P = 2.625 + 13.5 = 16.13 ft. R = A/P = 2.36 / 16.13 = 0.1463 S = 0.0414

V = (1.486 / 0.017)(0.1463)2/3(0.0414)1/2 = 4.93 fps $Q = AV = 2.36 \times 4.93 = 11.63$ cfs or 23.26 cfs for the total street cross section. There is adequate capacity in the street for the anticipated peak flow.

Having inspected the site represented by this plan and having taken spot elevations

of finish floors and at critical locations, I hereby certify that the as-constructed grading and drainage plan is in substantial compliance with the approved grading and drainage plan with engineer's stamp dated 4/14/95, and most recently revised

CHANNEL BETWEEN STORAGE UNITS (DETAIL "A"):

Area drained = $180' \times 130' = 23.400 \text{ sf} = 0.5372 \text{ ac}$. $qW = 0.05 \times 2.03 + 0.05 \times 2.87 + 0.9 \times 4.37 = 4.18 \text{ cfs}$ $Q100 = 0.5372 \times 4.18 = 2.25 \text{ cfs}$

Analyze inlet by Weir Equation, $Q = CLH^{3/2}$ C = 3.0 L = 6.0 H = 0.25 $Q = 3.0 \times 6.0 \times 0.25^{-3/2} = 2.25 \text{ cfs}$ (Adequate). Analyze Channel By RV dump using Manning's Equation: $Q = A(1.486/N) R^{2/3} S^{1/2}$ $A = 3 \times 0.25 = 0.75 cfs$ P = 3.5' R = 0.2143S = 0.0113 $Q = 0.75 (1.486 / 0.017)0.2143^{2/3} (0.0113)^{1/2} = 2.49 \text{ cfs} > 2.25 \text{ cfs} (Adequate).$

REVISED 9/22/98 REVISED 4/16/97 REVISED 2/27/97 REVISED 9/13/96 REVESED 11/7 95



FRANK D. LOVELADY P.E. 300 ALAMOSA ROAD N.W. ALBUQUERQUE, N.M. 87107

DON KEITH MINI - STORAGE GRADING & DRAINAGE PLAN

Of

Scale: See Details Date: 4/95

(505) 345-2267

ENGINEER'S CERTIFICATION:

D-2

Designed: FDL

Checked: FDL Drawn: STAFF

