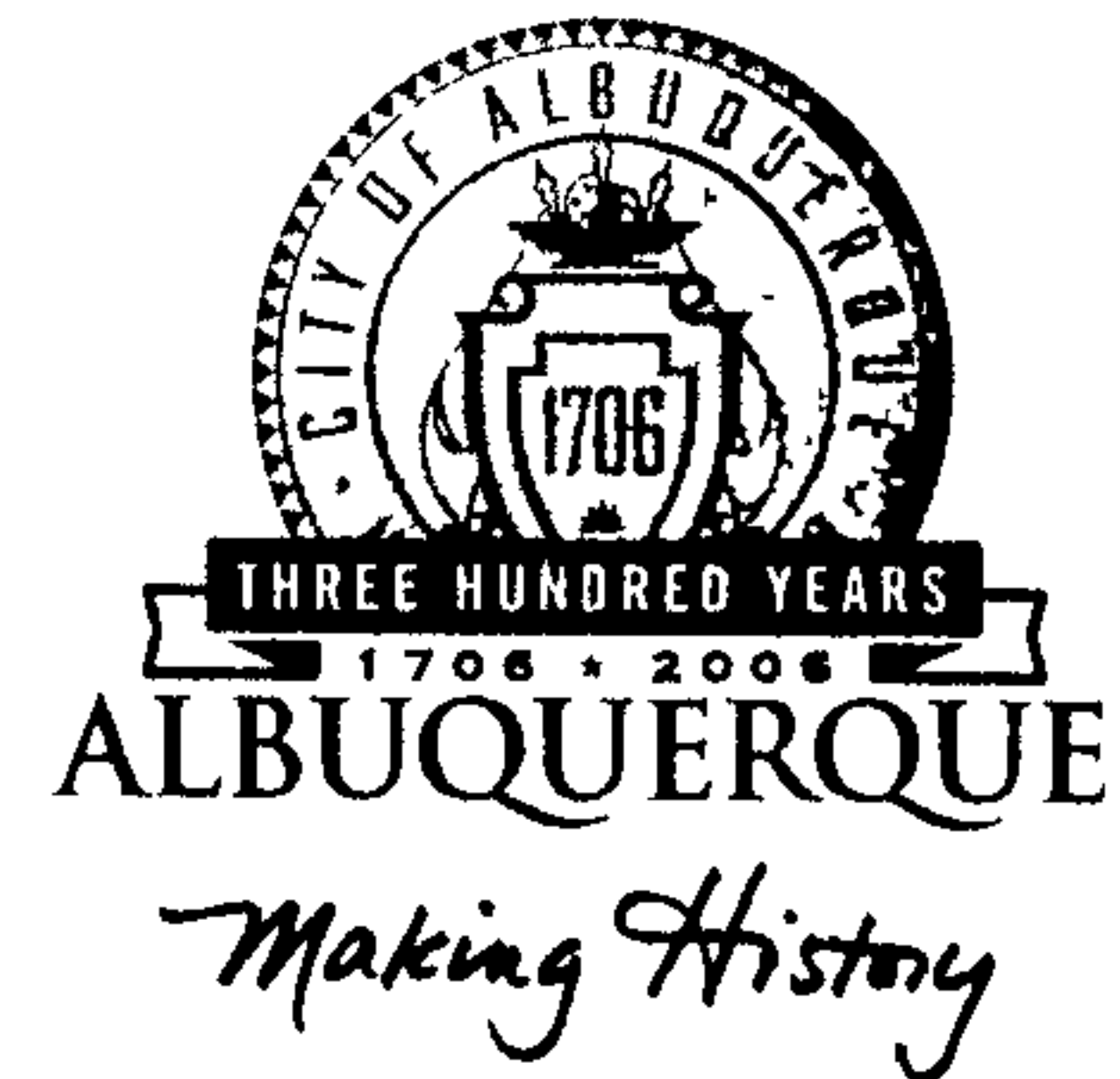


CITY OF ALBUQUERQUE



August 22, 2005

Mr. Ronald R. Bohannon, PE
TIERRA WEST, LLC
8509 Jefferson St. NE
Albuquerque, NM 87113

RE: VISTA PACIFIC SUBDIVISION, PHASE 2 (K-11/D69)
Engineers Certification for Release of Financial Guaranty
Engineers Stamp dated 08/04/2004
Engineers Certification dated 08/22/2005

Dear Ron:

Based upon the information provided in your Engineer's Certification Submittal dated 08/22/2005, the above referenced plan is adequate to satisfy the Grading and Drainage Certification for Release of Financial Guaranty.

P.O. Box 1293

If you have any questions, you can contact me at 924-3982

Albuquerque

Sincerely,

Arlene V. Portillo
Plan Checker, Planning Dept.- Hydrology
Development and Building Services

New Mexico 87103

www.cabq.gov

C: Marilyn Maldonado, COA# 695082
File

DRAINAGE AND TRANSPORTATION SHEET

(REV. 1/28/2003rd)

PROJECT TITLE: Vista Pacifica Phase 2
DRB: 1003280 EPC #: _____

ZONE MAP/DRG. FILE #: K-11 / D69
WORK ORDER #: 695082

LEGAL DESCRIPTION Tract 168, Town of Atrisco Grant, Unit 6
CITY ADDRESS: SW Corner of Corregidor Drive & Gonzales Road

ENGINEERING FIRM: Tierra West, LLC
ADDRESS: 8509 Jefferson NE
CITY, STATE: Albuquerque, NM

CONTACT: Ron Bohannon
PHONE: (505) 858-3100
ZIP CODE: 87113

OWNER: AMC Development
ADDRESS: 4407 Lomas Blvd. NE
CITY, STATE: Albuquerque, NM

CONTACT: Jeff Jesionowski
PHONE: 505-255-0249
ZIP CODE: 87110

ARCHITECT: _____
ADDRESS: _____
CITY, STATE: _____

CONTACT: _____
PHONE: _____
ZIP CODE: _____

SURVEYOR: Surv-Tek
ADDRESS: 9834 Valley View Dr. NW, Suite 100
CITY, STATE: Albuquerque, NM

CONTACT: Russ Hugg
PHONE: 505-897-3366
ZIP CODE: 87114

CONTRACTOR: Sundance Mechanical
ADDRESS: 4400 Alameda Blvd NE, Suite E
CITY, STATE: Albuquerque, NM

CONTACT: Carlos Speiss
PHONE: 505-345-2694
ZIP CODE: 87113

CHECK TYPE OF SUBMITTAL:

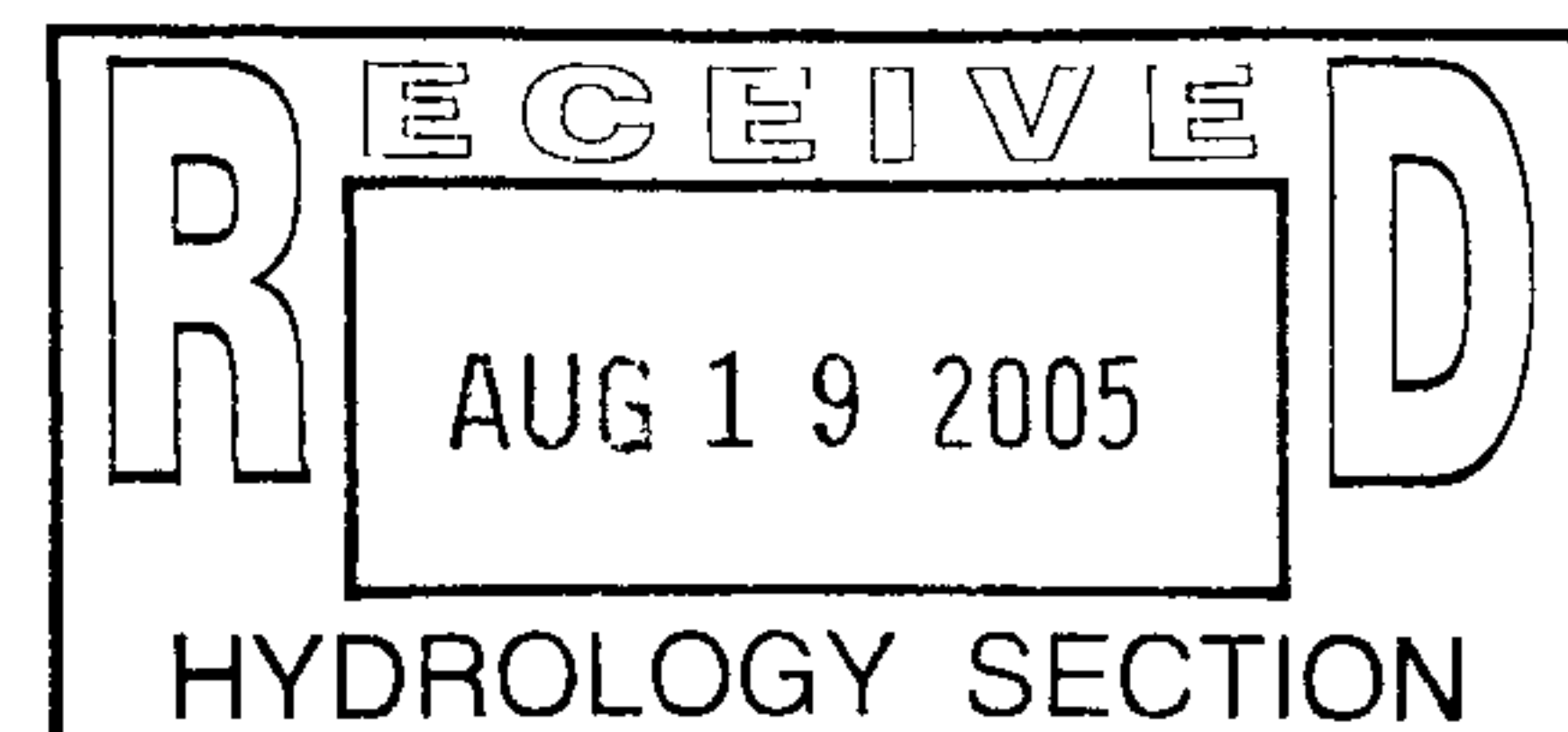
- ☐ DRAINAGE REPORT
☐ DRAINAGE PLAN 1st SUBMITTAL, **REQUIRES TCL or equal**
☐ DRAINAGE PLAN RESUBMITTAL
☐ CONCEPTUAL GRADING & DRAINAGE PLAN
☐ GRADING PLAN
☐ EROSION CONTROL PLAN
☒ ENGINEER'S CERTIFICATION (HYDROLOGY)
☐ CLOMR/LOMR
☐ TRAFFIC CIRCULATION LAYOUT (TCL)
☐ ENGINEERS CERTIFICATION (TCL)
☐ ENGINEERS CERTIFICATION (DRB APPR. SITE PLAN)
☐ OTHER

CHECK TYPE OF APPROVAL SOUGHT:

- ☒ SIA / FINANACIAL GUARANTEE RELEASE
☐ PRELIMINARY PLAT APPROVAL
☐ S. DEV. PLAN FOR SUB'D. APPROVAL
☐ S. DEV. PLAN FOR BLDG. PERMIT APPROVAL
☐ SECTOR PLAN APPROVAL
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☐ FOUNDATION PERMIT APPROVAL
☐ BUILDING PERMIT APPROVAL
☐ CERTIFICATE OF OCCUPANCY (PERM.)
☐ CERTIFICATE OF OCCUPANCY (TEMP.)
☐ GRADING PERMIT APPROVAL
☐ PAVING PERMIT APPROVAL
☐ WORK ORDER APPROVAL
☐ SO-19

WAS A PRE-DESIGN CONFERENCE ATTENDED:

- ☐ YES
☐ NO
☐ COPY PROVIDED

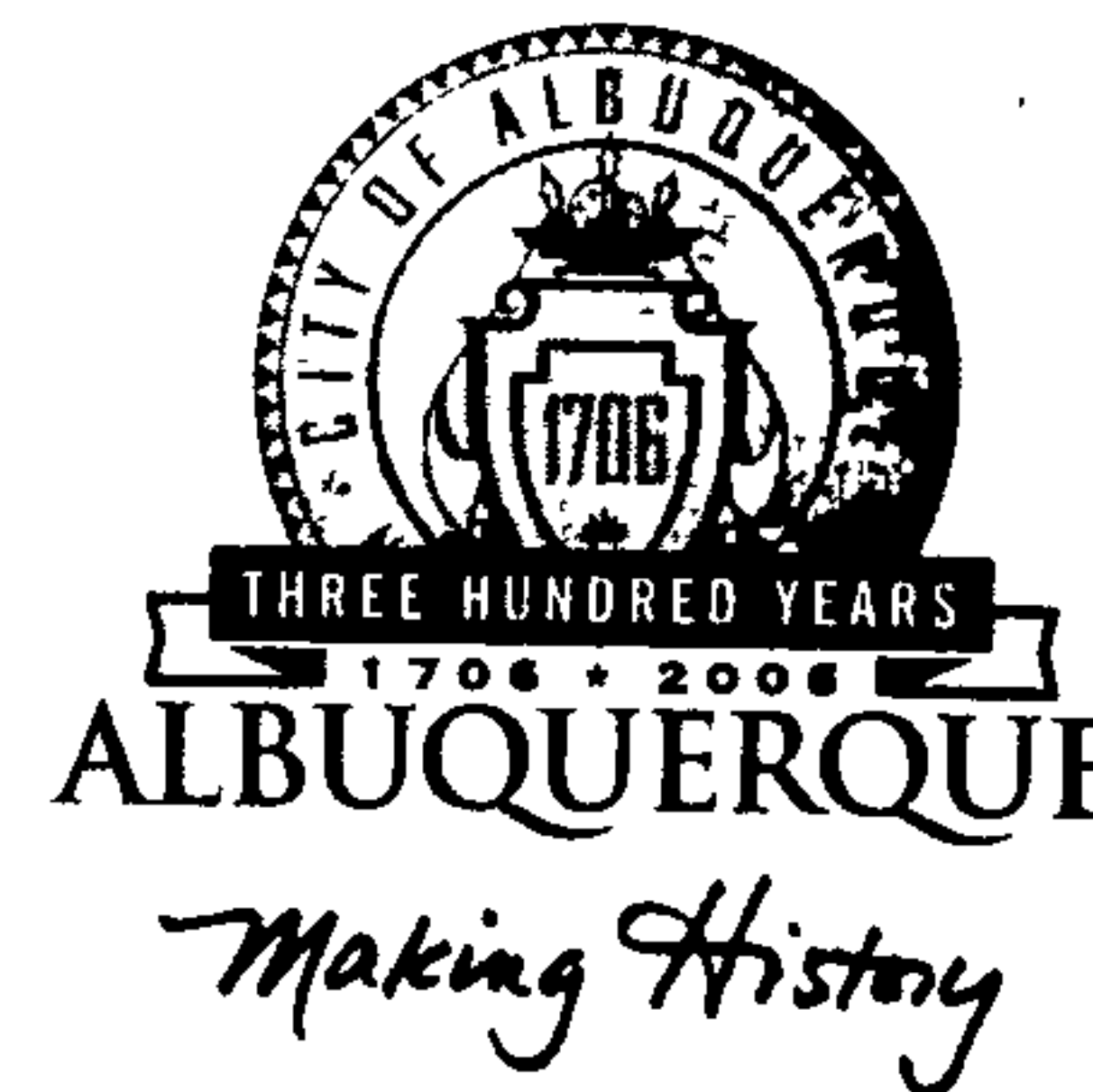


DATE SUBMITTED: 8/22/2005 BY: Ronald R. Bohannon, PE

Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a drainage submittal. The particular nature, location and scope of the proposed development defines the degree of drainage detail. One or more of the following levels of submittal may be required based on the following:

1. **Conceptual Grading and Drainage Plans** Required for approval of Site Development Plans greater than five (5) acres and Sector Plans.
2. **Drainage Plans** Required for building permits, grading permits, paving permits and site plans less than five (5) acres.
3. **Drainage Report** Required for subdivisions containing more than ten (10) lots or constituting five (5) acres or more.

CITY OF ALBUQUERQUE



September 22, 2004

Ronald Bohannon, P.E.
Tierra West, LLC
8509 Jefferson NE
Albuquerque, NM 87113

**Re: Vista Pacifica 2, Grading and Drainage Plan
Engineer's Stamp dated 8-04-04 (K11-D69)**

Dear Mr. Bohannon,

Based upon the information provided in your submittal received 8-05-04, the above referenced grading plan is approved as amended. This is the plan that must be certified and submitted for release of financial guarantee.

P.O. Box 1293

This project requires a National Pollutant Discharge Elimination System (NPDES) permit. If you have any questions regarding this permit please feel free to call the DMD Storm Drainage Design section at 768-3654 (Charles Caruso).

Albuquerque

If you have any questions, you can contact me at 924-3981.

New Mexico 87103

Sincerely,

Kristal D. Metro
Engineering Associate, Planning Dept.
Development and Building Services

www.cabq.gov

C: Charles Caruso, DMD Storm Drainage Design
file

DRAINAGE AND TRANSPORTATION SHEET

(REV. 1/28/2003rd)

PROJECT TITLE: Vista Pacifica
DRB: 1003280 EPC #: Lot 168, Town of Atrisco, Grant Unit 6
ZONE MAP/DRG. FILE #: K-11/D69
WORK ORDER #: _____

LEGAL DESCRIPTION: Lot 168 Town of Atrisco, Grant Unit 6
CITY ADDRESS: Southwest corner of Gonzales Road & Corregidor Drive

ENGINEERING FIRM: Tierra West, LLC CONTACT: Ronald R. Bohannon, P.E.
ADDRESS: 8509 Jefferson NE PHONE: (505) 858-3100
CITY, STATE: Albuquerque, NM ZIP CODE: 87113

OWNER: AMC Development Services CONTACT: Jeff Jesionowski
ADDRESS: 400 Gold SW, Suite 750 PHONE: (505) 338-2285
CITY, STATE: ALBUQUERQUE, NM ZIP CODE: 87102

ARCHITECT: _____ CONTACT: _____
ADDRESS: _____ PHONE: _____
CITY, STATE: _____ ZIP CODE: _____

SURVEYOR: Surv -Tek CONTACT: Russ Hugg
ADDRESS: 5643 Paradise Blvd. NW PHONE: (505) 897-3366
CITY, STATE: ALBUQUERQUE, NM ZIP CODE: 87114

CONTRACTOR: _____ CONTACT: _____
ADDRESS: _____ PHONE: _____
CITY, STATE: _____ ZIP CODE: _____

CHECK TYPE OF SUBMITTAL:

- ☐ DRAINAGE REPORT
☐ DRAINAGE PLAN 1st SUBMITTAL, **REQUIRES TCL or equal**
☒ DRAINAGE PLAN RESUBMITTAL
☐ CONCEPTUAL GRADING & DRAINAGE PLAN
☐ GRADING PLAN
☐ EROSION CONTROL PLAN
☐ ENGINEER'S CERTIFICATION (HYDROLOGY)
☐ CLOMR/LOMR
☐ TRAFFIC CIRCULATION LAYOUT (TCL)
☐ ENGINEERS CERTIFICATION (TCL)
☐ ENGINEERS CERTIFICATION (DRB APPR. SITE PLAN)
☐ OTHER

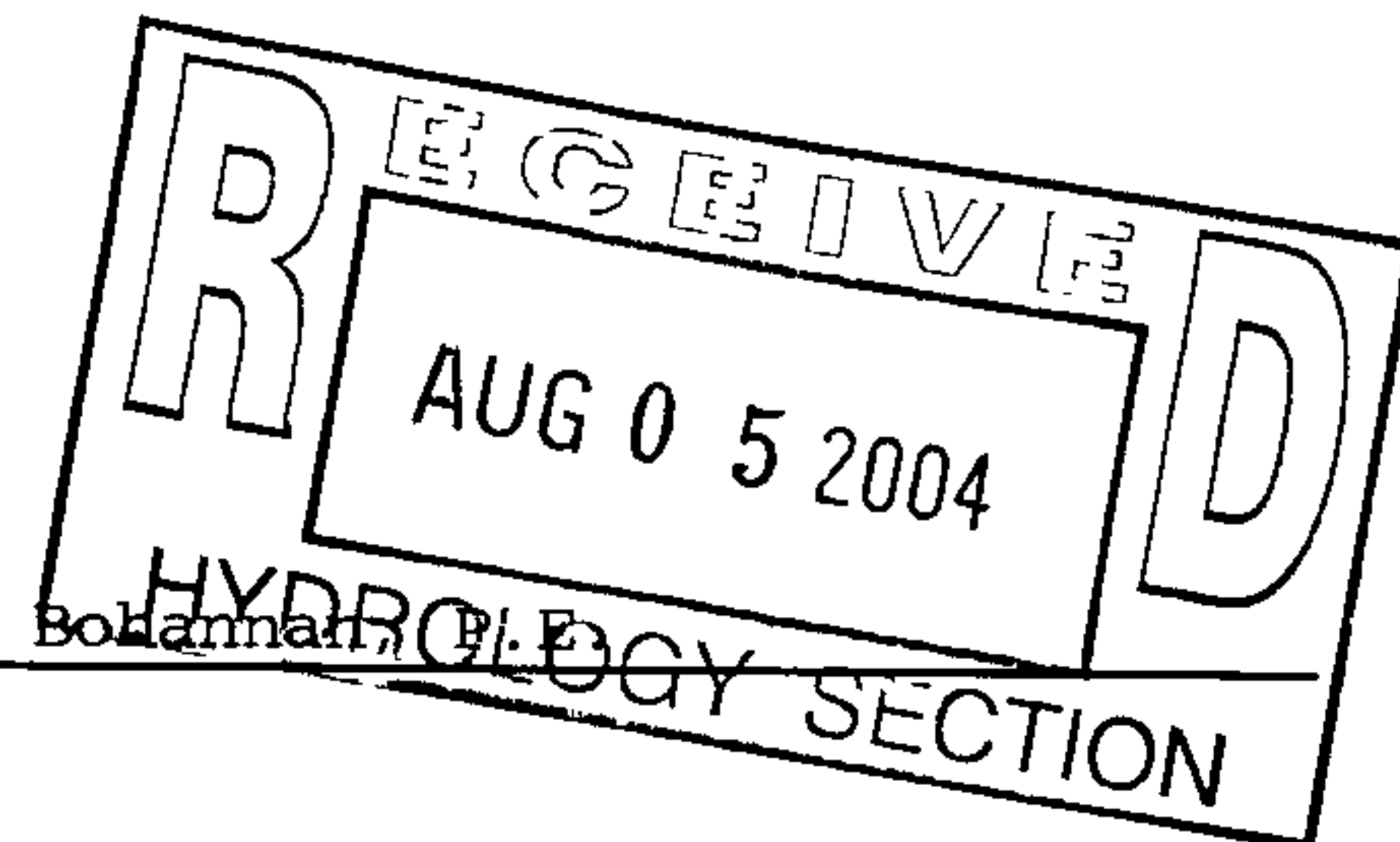
CHECK TYPE OF APPROVAL SOUGHT:

- ☐ SIA / FINANACIAL GUARANTEE RELEASE
☐ PRELIMINARY PLAT APPROVAL
☐ S. DEV. PLAN FOR SUB'D. APPROVAL
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☐ CERTIFICATE OF OCCUPANCY (PERM.)
☐ CERTIFICATE OF OCCUPANCY (TEMP.)
☒ GRADING PERMIT APPROVAL
☐ PAVING PERMIT APPROVAL
☐ WORK ORDER APPROVAL
☐ SO-19

WAS A PRE-DESIGN CONFERENCE ATTENDED:

- ☐ YES
☐ NO
☐ COPY PROVIDED

DATE SUBMITTED: 8/5/2004 BY: Ronald R. Bohannon, P.E.



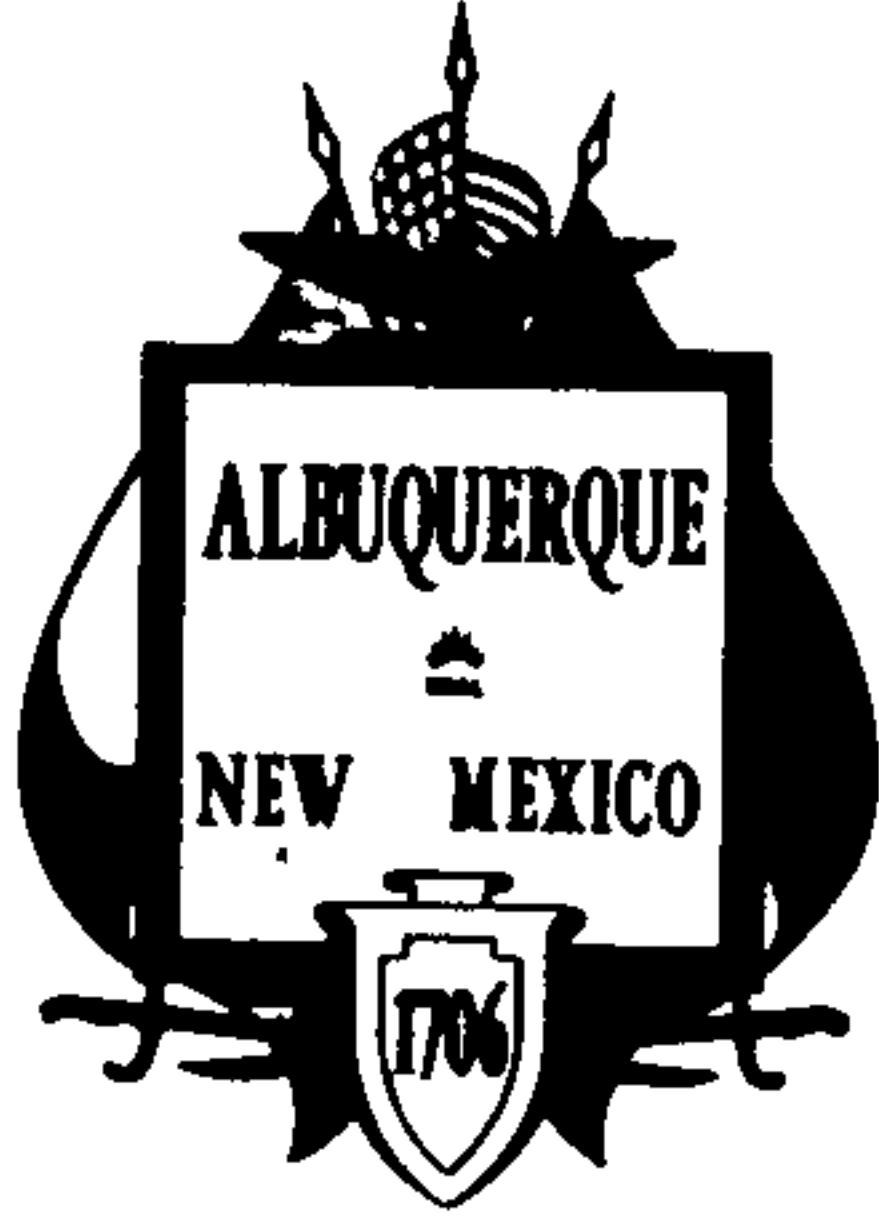
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3. **Drainage Report:** Required for subdivisions containing more than ten (10) lots or constituting five (5) acres or more.

Flow conveyed down

White Reserve Ave }
Vierra Ave } to
Red Mesa Ave } Corregidor Dr.

then down Corregidor to
exist drop inlet



City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

June 24, 2004

Ron Bohannon, PE
Tierra West LLC
8509 Jefferson NE
Albuquerque, NM 87113

Re: Vista Pacifica 2 Drainage Report
Engineer Stamp date 5-28-04 (K11/D69)

Dear Mr. Bohannon,

Based on information provided in your submittal dated 5-28-04, the above referenced report is approved for Preliminary Plat action by the DRB. Once that board has approved the plan, please submit a mylar copy of the grading plan for my signature in order to obtain a Grading Permit.

This project requires a National Pollutant Discharge Elimination System (NPDES) permit. Refer to the attachment that is provided with this letter for details. If you have any questions please feel free to call the Public Works Hydrology section at 768-3654 (Charles Caruso) or 768-3645 (Brian Wolfe).

If you have any questions, you can contact me at 924-3986.

Sincerely,

Bradley L. Bingham, PE
Principal Engineer, Planning Dept
Development and Building Services

C: Chuck Caruso, CoA,
file

DRAINAGE AND TRANSPORTATION INFORMATION SHEET

(REV. 01/28/2003rd)

PROJECT TITLE: Lot 168, Town of Atrisco, Grant Unit 6
DRB #: _____ EPC #: _____

ZONE MAP/DRG. FILE #: K-11 / D069
WORK ORDER #: _____

LEGAL DESCRIPTION: Lot 168 Town of Atrisco, Grant Unit 6
CITY ADDRESS: Southwest corner of Gonzales Road & Corregidor Drive

ENGINEERING FIRM: TIERRA WEST, LLC
ADDRESS: 8509 JEFFERSON NE
CITY, STATE: ALBUQUERQUE, NM

CONTACT: RONALD R. BOHANNAN, P.E.
PHONE: (505) 858-3100
ZIP CODE: 87113

OWNER: AMC Development Services
ADDRESS: 400 Gold SW, Suite 750
CITY, STATE: Albuquerque, NM

CONTACT: Jeff Jesionowski
PHONE: (505) 858-2285
ZIP CODE: 87102

ARCHITECT: _____
ADDRESS: _____
CITY, STATE: _____

CONTACT: _____
PHONE: _____
ZIP CODE: _____

SURVEYOR: Surv-Tek
ADDRESS: 5643 Paradise Blvd. NW
CITY, STATE: Albuquerque, NM

CONTACT: Russ Hugg
PHONE: 897-3366
ZIP CODE: 87114

CONTRACTOR: _____
ADDRESS: _____
CITY, STATE: _____

CONTACT: _____
PHONE: _____
ZIP CODE: _____

CHECK TYPE OF SUBMITTAL:

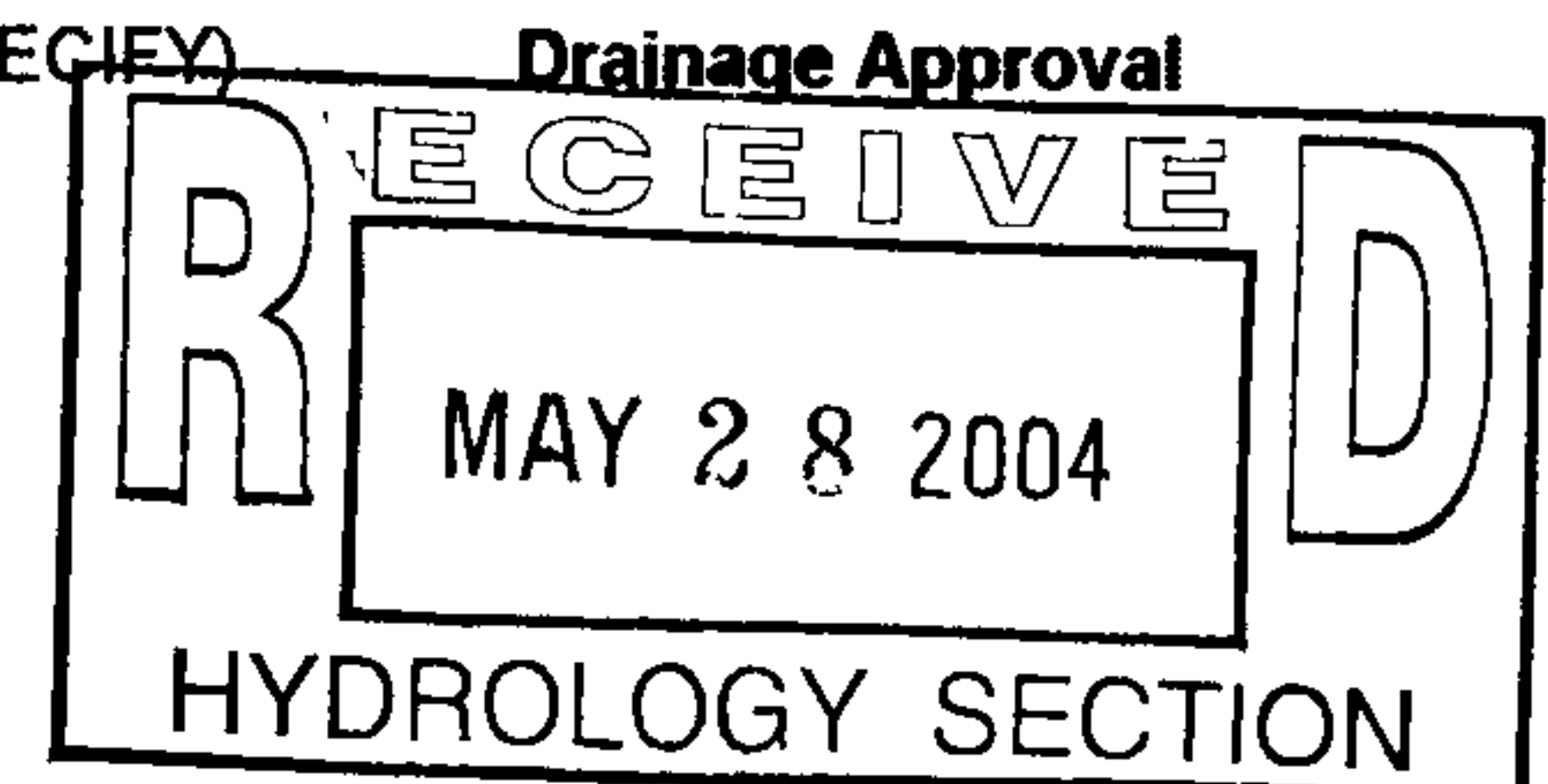
- ☒ DRAINAGE REPORT
- ☐ DRAINAGE PLAN 1st SUBMITTAL, **REQUIRES TCL or equal**
- ☐ DRAINAGE PLAN RESUBMITTAL
- ☐ CONCEPTUAL GRADING & DRAINAGE PLAN
- ☒ GRADING PLAN
- ☐ EROSION CONTROL PLAN
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- ☐ TRAFFIC CIRCULATION LAYOUT (TCL)
- ☐ ENGINEERS CERTIFICATION (TCL)
- ☐ ENGINEERS CERTIFICATION (DRB APPR. SITE PLAN)
- ☐ OTHER

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- ☐ CERTIFICATE OF OCCUPANCY (TEMP.)
- ☒ GRADING PERMIT APPROVAL
- ☐ PAVING PERMIT APPROVAL
- ☐ WORK ORDER APPROVAL
- ☐ OTHER (SPECIFY) _____

WAS A PRE-DESIGN CONFERENCE ATTENDED:

- ☐ YES
- ☒ NO
- ☐ COPY PROVIDED



DATE SUBMITTED: 5/28/2004 BY: Ronald R. Bohannan, P.E.

Requests for approvals of Site Development Plans and/or Subdivision Plats shall be accompanied by a drainage submittal.

The particular nature, location and scope of the proposed development defines the degree of drainage detail.

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3. **Drainage Report:** Required for subdivisions containing more than ten (10) lots or constituting five (5) acres or more.

**DRAINAGE REPORT
FOR**

***LOT 168
TOWN OF ATRISCO, GRANT UNIT 6***

VISTA PACIFICA PHASE 2

Prepared by:

**Tierra West, LLC
8509 Jefferson NE
Albuquerque, New Mexico 87113**

**Prepared for:
AMC Development Services**

May 2004

I certify that this report was prepared under my supervision, and I am a registered professional engineer in the State of New Mexico in good standing.


Ronald R. Bohannon
PE NO. 7868



Job No 240025

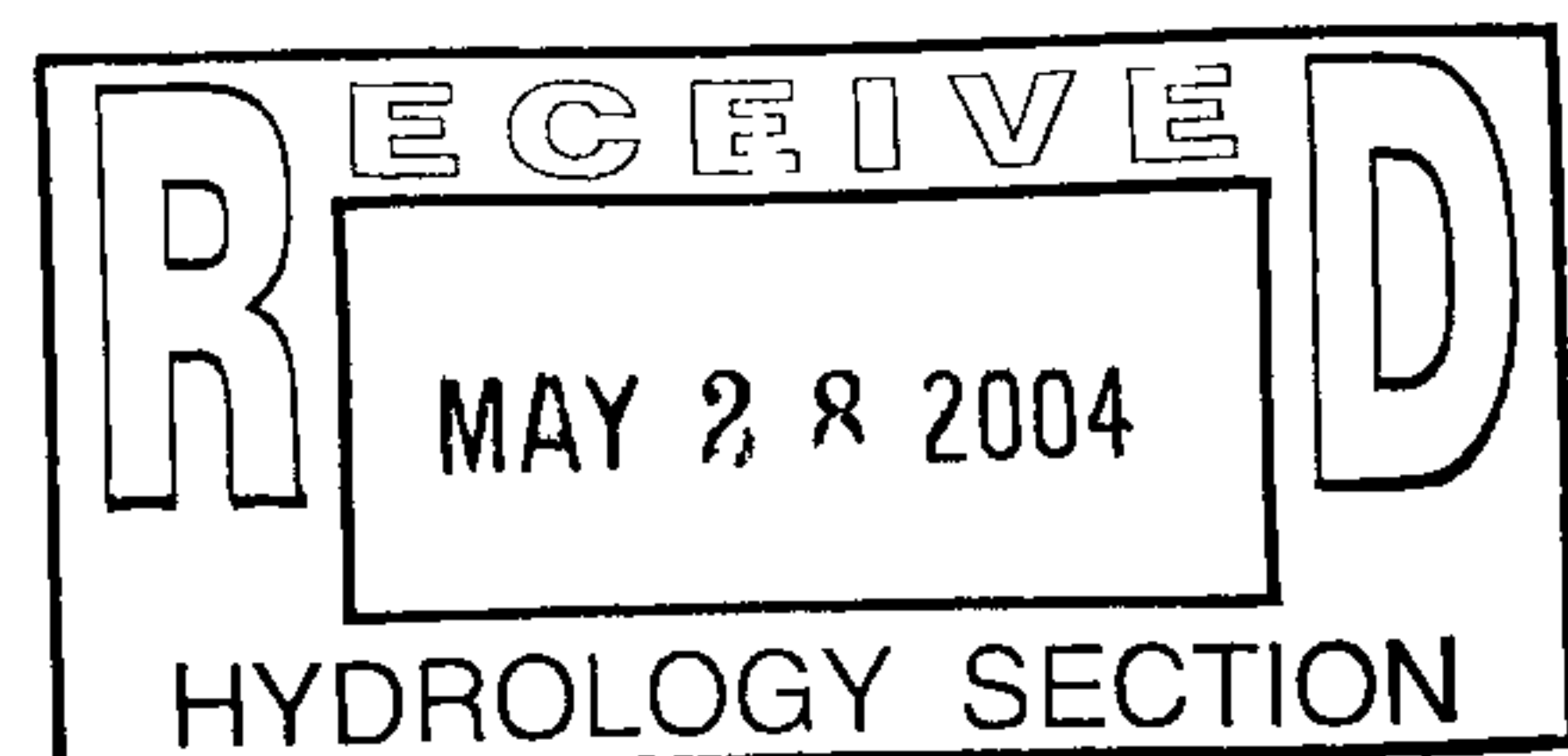


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APPENDIX A

Infrastructure List

MAP POCKET

Grading and Drainage Plan

Section I

Report

Prelude

This report is being prepared at the request of AMC Development Services, who is developing the second phase of Vista Pacifica, a new 36 lot of single family subdivision, entitled Vista Pacifica Phase 2.

Location

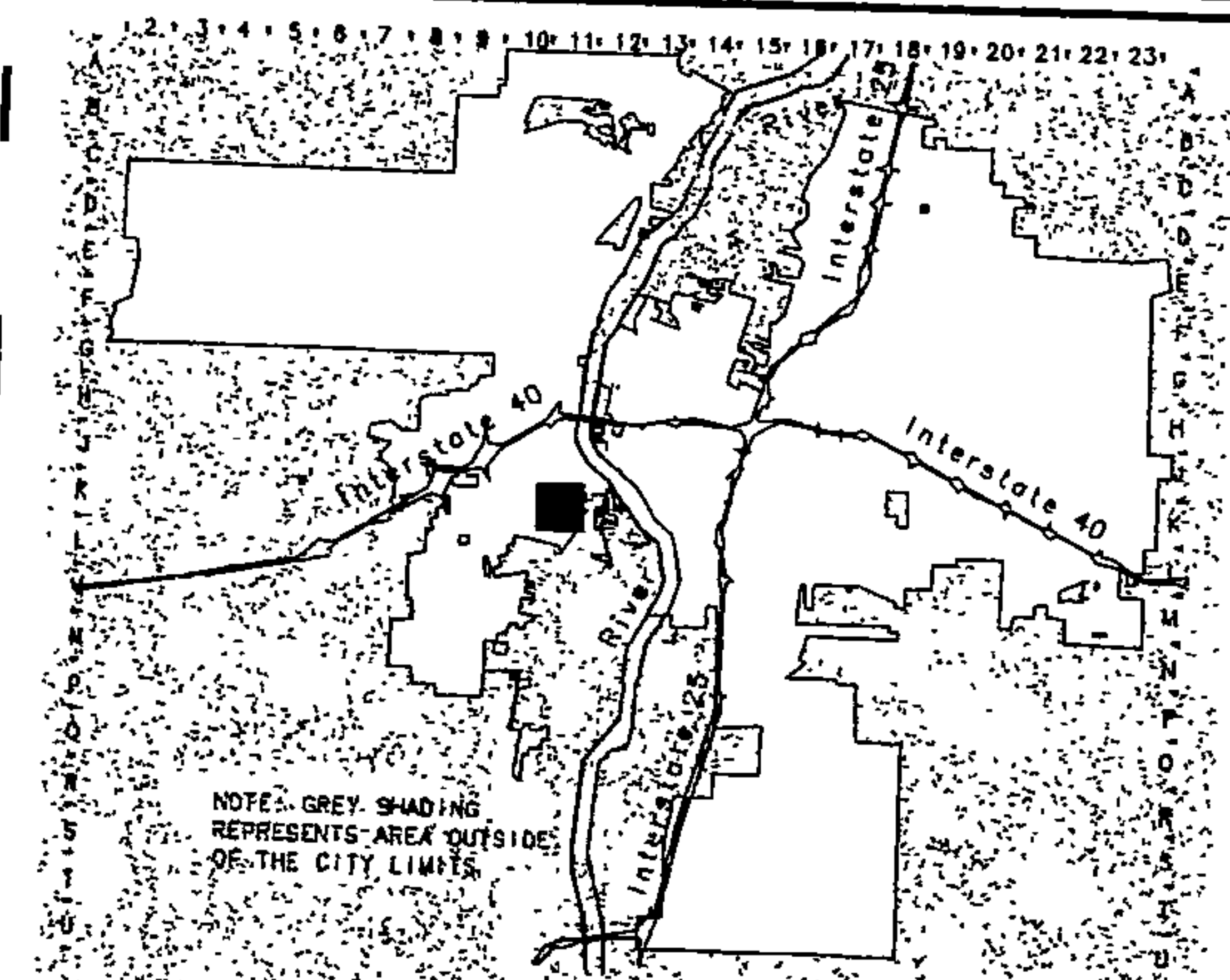
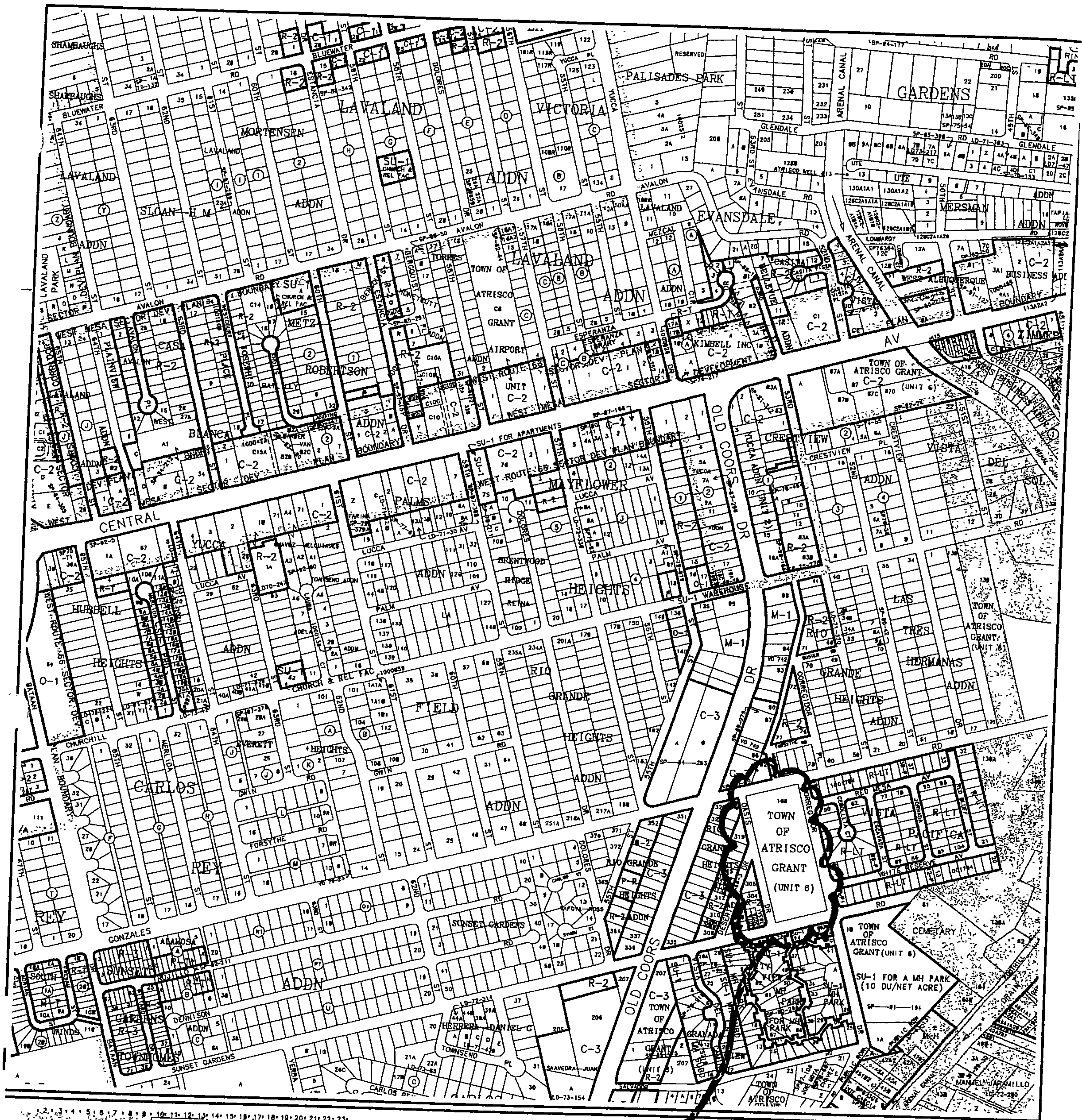
The subject site is located on the west side of Albuquerque near South Coors. Specifically, the site is located at the southwest corner of Gonzales Road and Corregidor Drive. The site is highlighted and shown on the enclosed Zone Atlas page number K-11. The site will be built in one phase and contains 4.59 acres, more or less.

Purpose

The purpose of this drainage report is to present a grading and drainage solution for the proposed subdivision. We are requesting preliminary plat approval, SIA infrastructure, and grading permit approval at this time.

Existing Drainage Conditions

The site is currently undeveloped and lies adjacent to the Gonzales Road Pond installed by the City. The Pond is situated north of the site and takes the flow from the site and discharges the pipe in a pressure siphon system to the Rio Grande River. The undeveloped flow drains naturally east toward Corregidor Drive. We intercept a small portion of the flows west of Oasis Drive. To the east lies Vista Pacifica Phase I, which drains to a storm sewer in Corregidor Drive; this area south drains south and does not impact the site.

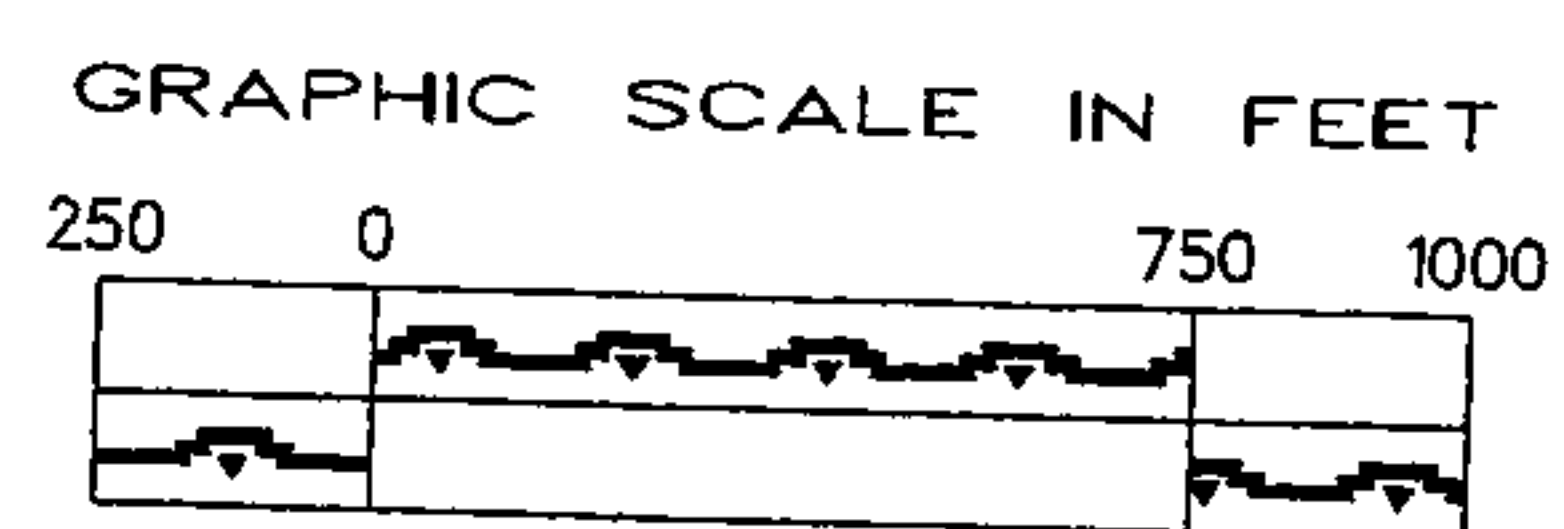


CITY OF
Albuquerque

Albuquerque Geographic Information System
PLANNING DEPARTMENT

© Copyright 2003

SITE



Zone Atlas Page

K-11-Z

Map Amended through July 09, 2003

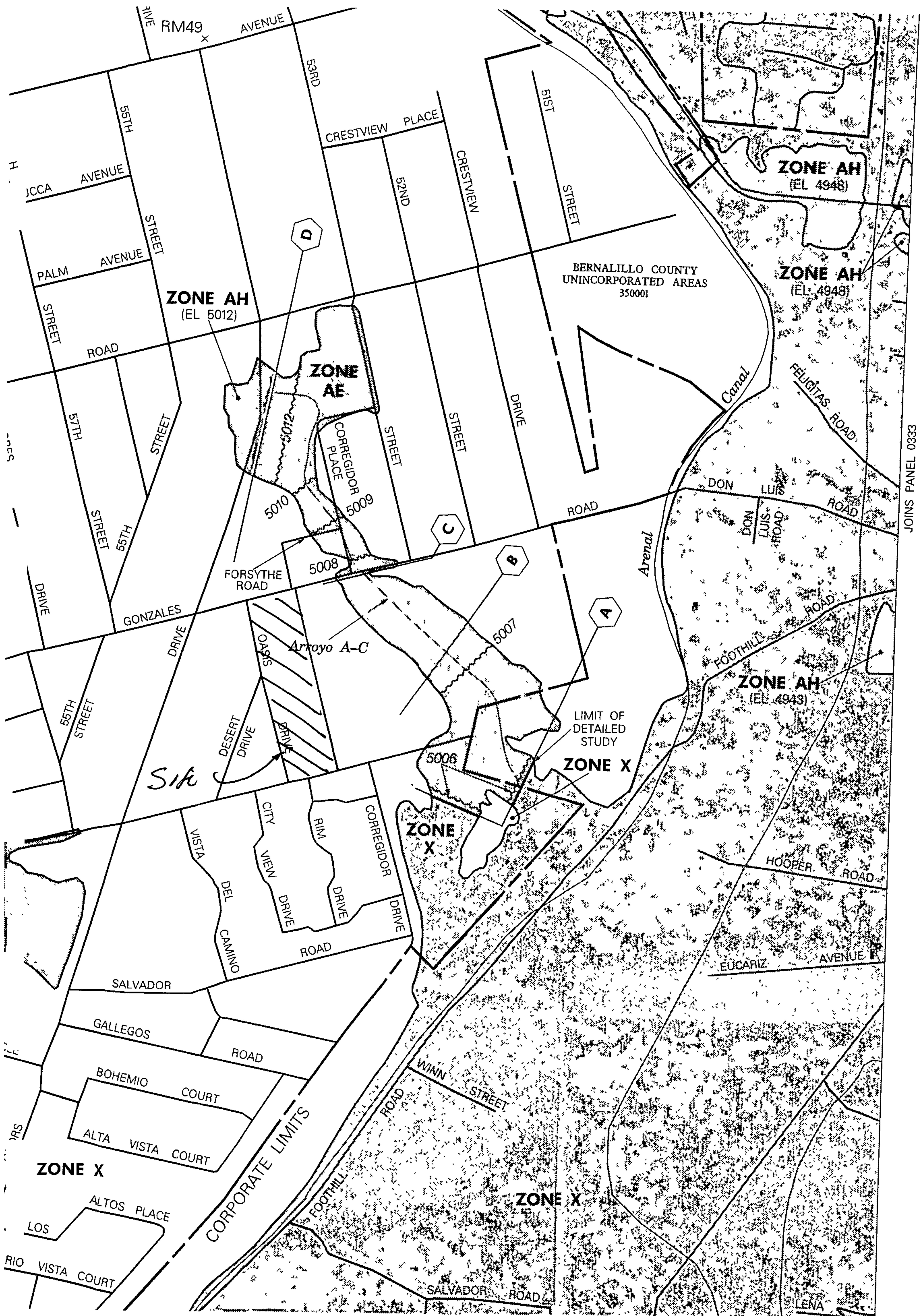
Gonzales Road drains west to the northeast corner of the site and then further east to the bluff. The two streets north (52nd Street and Crestview Drive) drain south to Gonzales Road that consists of a temporary pavement section and is not improved.

The City designed the Gonzales Pond and sized the facility to handle the developed flows from the site and surrounding streets. An existing 54-inch storm drain traverses Corregidor Road and drains through a flap gate into the pond. The pond is drained east down Gonzales Road to Crestview Drive north. Conversations with BHI indicated the water surface elevation is between 4998.9 and 5000.95. They are resolving the difference based upon previous studies. All of the finish pads are greater than these elevations in the event the pond does not drain. The outfall has a siphon at the MRGCD drain which creates a very sensitive outfall subject to high maintenance potential if sediment and debris can enter the system. Therefore, all developed flow for the project and the two streets flow either to the pond or north to an alternate drainage collection system.

Flood Plain and Soil Condition

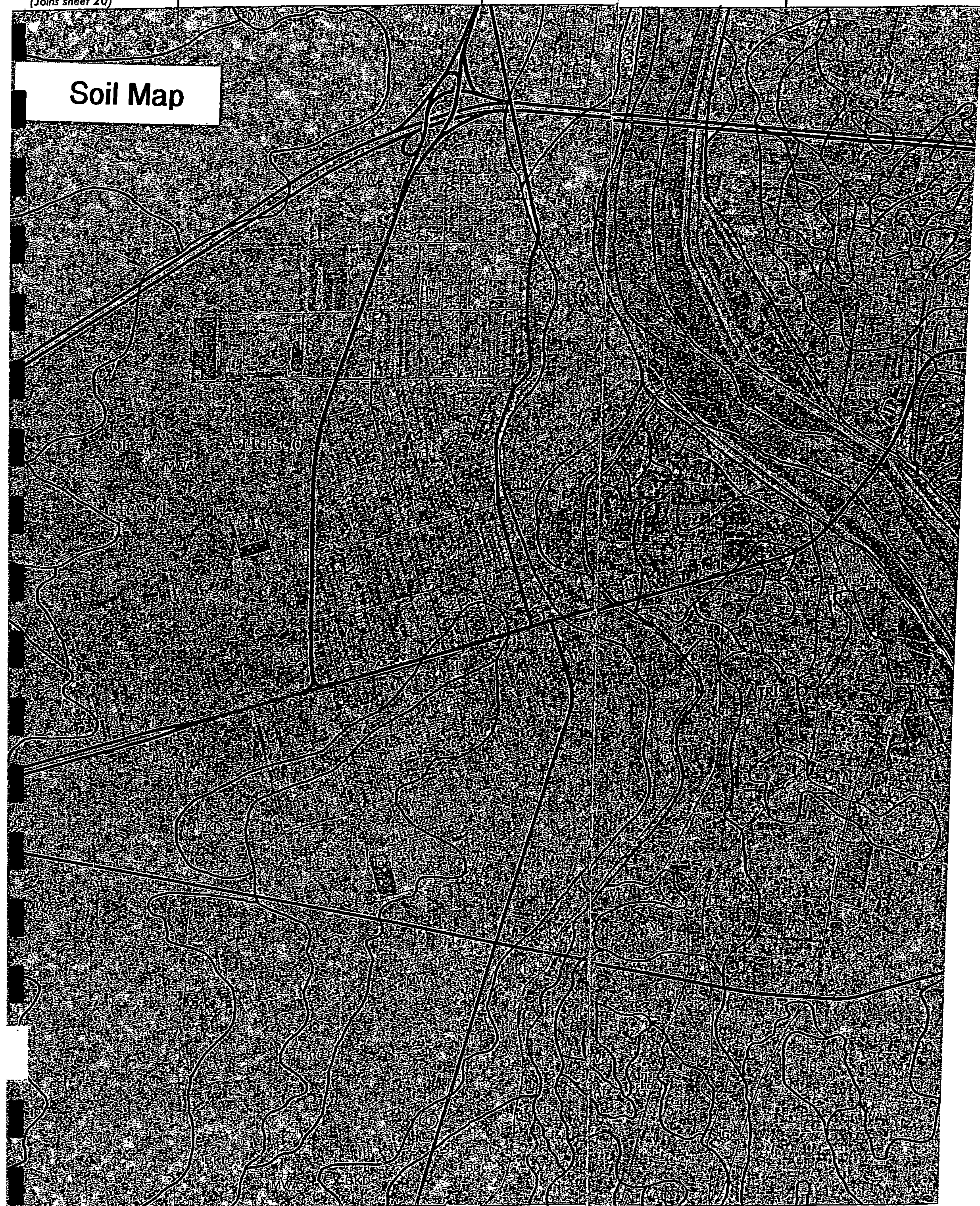
The site is located on FIRM Map. 35001C0329 D as shown on the attached excerpt. The map shows that the site does not lie within any 100-year flood plains. The map has been changed but has not been reissued by FEMA. This site lies west of the old flood plain in either case. The new flood plain map limits the flood plain to the Gonzales Pond.

The site contains two soil types: Madurez Wink (MWA) and Tijeras (TgB) as indicated on the Soil Map (sheet #42) from the Soil Conservation Service Survey of Bernalillo County. The Tijeras is gentle sloping old alluvial fan material with moderate runoff and soil erosion. The Madurez Wink series is a sandy loam with slow runoff and moderate to severe soil erosion. (See attached soil map for site location).



JOINS PANEL 0333

Soil Map

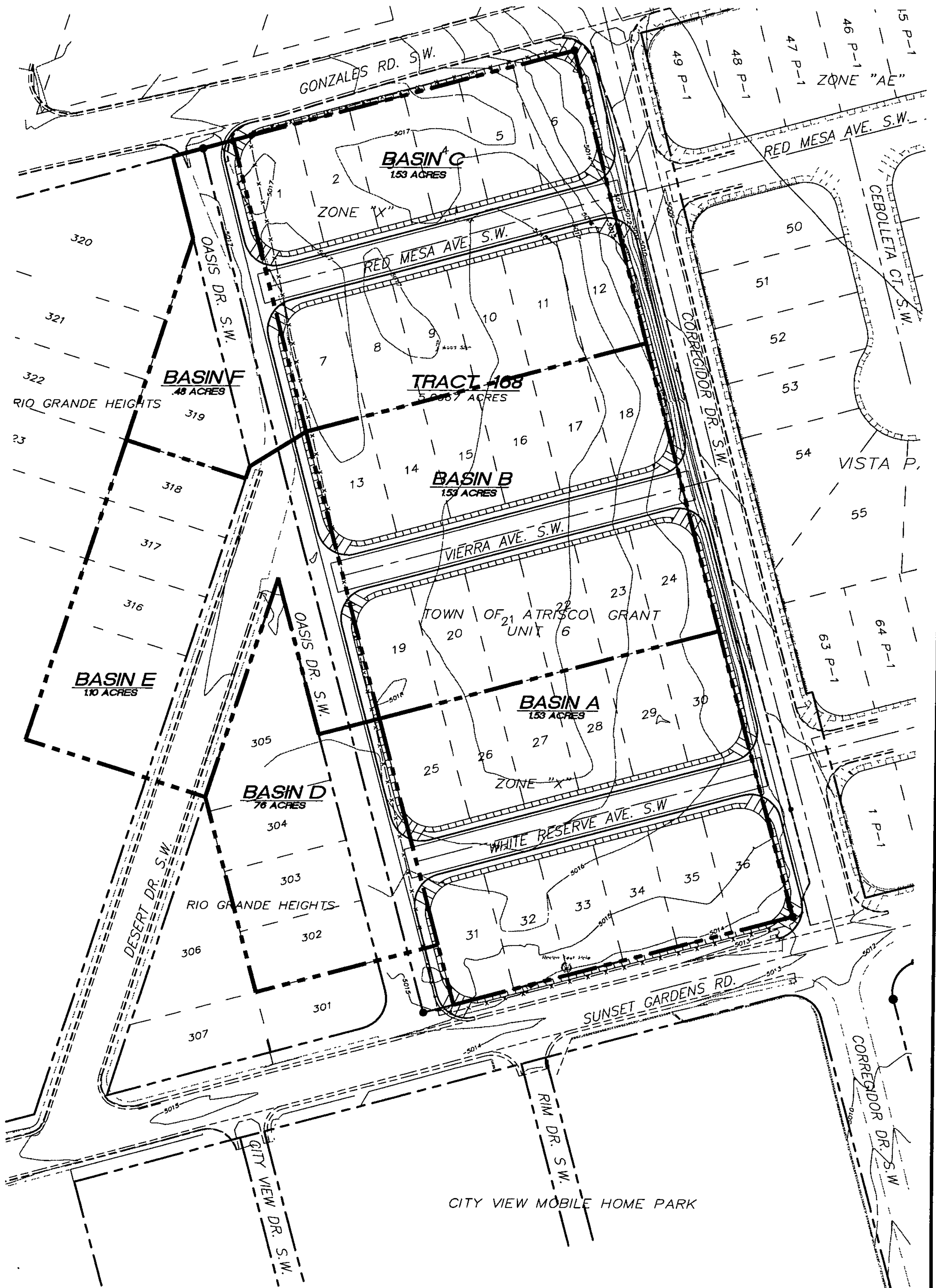


Proposed Drainage Management Plan

The entire site will be graded and all of the surface improvements will be built out in their entirety. The enclosed grading plan (in the map pocket) shows the grades for the entire project.

The development was divided into six basins to analyze the proposed conditions. Basins A, B, and C are on site for Vista Pacifica Phase 2. Basins C, D, and E are planned basins west of the site and contribute to the amount of discharge which will drain east to Corregidor Drive. These basins are designated as off-site basins. Basin A, B and C discharge a total of 16.47 cfs, the off-site basins, D, E, and F contribute 8.96 cfs for a total discharge of 25.43 cfs.

The flows are conveyed down White Reserve Avenue, Vierra Avenue, and Red Mesa Avenue to Corregidor Drive. These streets have capacity to convey the developed flows with a 4" curb. The flows are then conveyed down Corregidor to an existing Single "C" drop inlet, then carried, via storm sewer, to an existing 54" RCP. This 54" RCP drains to Gonzales Pond. The amount of discharge the Single "C" drop inlet can carry is 24.69 cfs. The actual amount of discharge from the six basins was 25.43 cfs, not significantly greater and the existing inlet is adequate. A 50% clogging factor was used in the grate which, if reduced, accepts the slightly larger flow. Corregidor Drive can support the flow from the basins with the existing 8" curb.



PROPOSED BASIN MAP

Infrastructure List

Shown in Appendix A is the proposed list for the project.

Calculations

The weighted E method from the “City of Albuquerque Development Process Manual Volume 11 – Design Criteria, 1997 Revision” was used to calculate the runoff and volume for the site.

Summary

The site will discharge 25.43 cfs, flowing from White Reserve Avenue, Vierra Avenue, and Red Mesa Avenue to Corregidor Drive via 4” curbs. The flow is then conveyed down Corregidor to an existing Single “C” drop inlet and then carried, via storm sewer, to an existing 54” RCP. This 54” RCP drains to Gonzales Pond.

Section II

Runoff Calculations

Weighted E Method

Developed On-Site Basins

Basin	Area (sf)	Area (acres)	Treatment A		Treatment B		Treatment C		Treatment D		100-Year			10-Year		
			%	(acres)	%	(acres)	%	(acres)	%	(acres)	Weighted E (ac-ft)	Volume (ac-ft)	Flow cfs	Weighted E (ac-ft)	Volume (ac-ft)	Flow cfs
A	66,360	1.52	0%	0	20%	0.30	20%	0.30	60%	0.91	1.514	0.192	5.49	0.876	0.111	3.33
B	66,360	1.52	0%	0	20%	0.30	20%	0.30	60%	0.91	1.514	0.192	5.49	0.876	0.111	3.33
C	66,360	1.52	0%	0	20%	0.30	20%	0.30	60%	0.91	1.514	0.192	5.49	0.876	0.111	3.33

Developed Off-Site Basins

Basin	Area (sf)	Area (acres)	Treatment A		Treatment B		Treatment C		Treatment D		100-Year			10-Year		
			%	(acres)	%	(acres)	%	(acres)	%	(acres)	Weighted E (ac-ft)	Volume (ac-ft)	Flow cfs	Weighted E (ac-ft)	Volume (ac-ft)	Flow cfs
D	33,427	0.77	17%	0.130454	0%	0.00	28%	0.21	45%	0.35	1.239	0.079	2.29	0.695	0.044	1.35
E	48,255	1.11	0%	0	0%	0.00	19%	0.21	81%	0.90	1.784	0.165	4.53	1.088	0.100	2.91
F	21,285	0.49	0%	0	0%	0.00	0%	0.00	100%	0.49	1.970	0.080	2.14	1.240	0.050	1.41

Equations:

$$\text{Weighted E} = E_a \cdot A_a + E_b \cdot A_b + E_c \cdot A_c + E_d \cdot A_d / (\text{Total Area})$$

$$\text{Volume} = \text{Weighted D} \cdot \text{Total Area}$$

$$\text{Flow} = Q_a \cdot A_a + Q_b \cdot A_b + Q_c \cdot A_c + Q_d \cdot A_d$$

Excess Precipitation, E (inches)		
Zone 1	100-Year	10 - Year
E _a	0.44	0.08
E _b	0.67	0.22
E _c	0.99	0.44
E _d	1.97	1.24

Peak Discharge (cfs/acre)		
Zone 1	100-Year	10 - Year
Q _a	1.29	0.24
Q _b	2.03	0.76
Q _c	2.87	1.49
Q _d	4.37	2.89

Weighted E Method

Undeveloped On-Site Basins

Basin	Area (sf)	Area (acres)	Treatment A		Treatment B		Treatment C		Treatment D		100-Year			10-Year		
			%	(acres)	%	(acres)	%	(acres)	%	(acres)	Weighted E (ac-ft)	Volume (ac-ft)	Flow cfs	Weighted E (ac-ft)	Volume (ac-ft)	Flow cfs
A	66,360	1.52	100%	1.52	0%	0.00	0%	0	0%	0.00	0.440	0.056	1.97	0.080	0.010	0.37
B	66,360	1.52	100%	1.52	0%	0.00	0%	0	0%	0.00	0.440	0.056	1.97	0.080	0.010	0.37
C	66,360	1.52	100%	1.52	0%	0.00	0%	0	0%	0.00	0.440	0.056	1.97	0.080	0.010	0.37
Total	199,080	4.57										0.168	5.90		0.030	1.10

Undeveloped Off-Site Basins

Basin	Area (sf)	Area (acres)	Treatment A		Treatment B		Treatment C		Treatment D		100-Year			10-Year		
			%	(acres)	%	(acres)	%	(acres)	%	(acres)	Weighted E (ac-ft)	Volume (ac-ft)	Flow cfs	Weighted E (ac-ft)	Volume (ac-ft)	Flow cfs
D	35,000	0.80	0%	0.00	0%	0.00	65%	0.5222681	35%	0.28	1.333	0.089	2.73	0.720	0.048	1.59
E	51,675	1.19	0%	0.00	20%	0.24	0%	0	80%	0.95	1.710	0.169	4.63	1.036	0.102	2.92
F	24,625	0.57	0%	0.00	0%	0.00	0%	0	100%	0.57	1.970	0.093	2.47	1.240	0.058	1.63
Total	111,300	2.56										0.351	9.83		0.209	6.15

Equations:

Weighted E = $E_a \cdot A_a + E_b \cdot A_b + E_c \cdot A_c + E_d \cdot A_d$ / (Total Area)

Volume = Weighted D * Total Area

Flow = $Q_a \cdot A_a + Q_b \cdot A_b + Q_c \cdot A_c + Q_d \cdot A_d$

Excess Precipitation, E (inches)		
Zone 1	100-Year	10 - Year
E_a	0.44	0.08
E_b	0.67	0.22
E_c	0.99	0.44
E_d	1.97	1.24

Peak Discharge (cfs/acre)		
Zone 1	100-Year	10 - Year
Q_a	1.29	0.24
Q_b	2.03	0.76
Q_c	2.87	1.49
Q_d	4.37	2.89

Street Capacity Calculations

White Reserve Avenue
28' F-F Street Section with 4" curb

Slope= 0.017

Q = 7.78 cfs

For water depths less than 0.0625 feet

Y= Water depth
Area = $16 \cdot Y^2$
P= $\text{SQRT}(1025 \cdot Y^2) + Y$
n= 0.017

Depth (ft)	Area (ft^2)	P (ft)	R (A/P)	Q (cfs)	2Q (cfs)	Vel (ft/s)	D*V	Fr	D2 (ft)
0.01	0.0016	0.33	0.00	0.00	0.00	0.33	0.00	0.58	0.00455
0.02	0.0064	0.66	0.01	0.00	0.01	0.52	0.01	0.65	0.01082
0.025	0.01	0.83	0.01	0.01	0.01	0.60	0.02	0.67	0.01429
0.035	0.0196	1.16	0.02	0.01	0.03	0.75	0.03	0.71	0.0217
0.045	0.0324	1.49	0.02	0.03	0.06	0.89	0.04	0.74	0.02964
0.052	0.043264	1.72	0.03	0.04	0.08	0.98	0.05	0.76	0.03545
0.06	0.0576	1.98	0.03	0.06	0.12	1.08	0.06	0.78	0.04231
0.0625	0.0625	2.06	0.03	0.07	0.14	1.11	0.07	0.78	0.0445

For water depths greater than 0.0625 ft but less than 0.3025 ft

Y1= Y-0.0625
A2= $A1 + 2 \cdot Y1 + 25 \cdot Y1^2$
P2= $P1 + \text{SQRT}(2501 \cdot Y1^2) + Y1$

Depth (ft)	Area (ft^2)	P (ft)	R (A/P)	Q (cfs)	2Q (cfs)	Vel (ft/s)	D*V	Fr	D2 (ft)
0.063	0.063506	2.09	0.03	0.07	0.14	1.11	0.07	0.78	0.04476
0.1	0.172656	3.98	0.04	0.24	0.49	1.41	0.14	0.78	0.07171
0.13	0.311406	5.51	0.06	0.52	1.05	1.68	0.22	0.82	0.09929
0.16	0.495156	7.04	0.07	0.96	1.92	1.94	0.31	0.86	0.12952
0.2	0.810156	9.08	0.09	1.84	3.69	2.28	0.46	0.90	0.17269
0.207	0.873506	9.43	0.09	2.04	4.07	2.33	0.48	0.90	0.18051
0.2612	1.446942	12.20	0.12	3.98	7.96	2.75	0.72	0.95	0.24339
0.3025	1.9825	14.31	0.14	6.05	12.10	3.05	0.92	0.98	0.29361

For water depths greater than 0.3025 ft but less than 0.333 ft

Y2= Y - 0.3025
A3= $A2 + Y2^2 \cdot 14$
P3= $P2 + Y2$

Depth (ft)	Area (ft^2)	P (ft)	R (A/P)	Q (cfs)	2Q (cfs)	Vel (ft/s)	D*V	Fr	D2 (ft)
0.303	1.9895	14.31	0.14	6.09	12.17	3.06	0.93	0.98	0.29468
0.3039	2.0021	14.31	0.14	6.15	12.30	3.07	0.93	0.98	0.29662
0.3062	2.0343	14.31	0.14	6.32	12.63	3.10	0.95	0.99	0.30158
0.31	2.0875	14.31	0.15	6.59	13.18	3.16	0.98	1.00	0.30979
0.3125	2.1225	14.32	0.15	6.78	13.55	3.19	1.00	1.01	0.31519
0.32	2.2275	14.32	0.16	7.34	14.68	3.30	1.05	1.03	0.33144
0.3317	2.3913	14.34	0.17	8.26	16.52	3.45	1.15	1.06	0.35689
0.333	2.4095	14.34	0.17	8.36	16.73	3.47	1.16	1.06	0.35972

For water depths greater than 0.333 ft but less than 0.513 ft

Y3= Y - 0.333
A4= $A3 + 14 \cdot Y3 + 25 \cdot Y3^2$
P4= $P3 + \text{SQRT}(2501 \cdot Y3^2)$

Depth (ft)	Area (ft^2)	P (ft)	R (A/P)	Q (cfs)	2Q (cfs)	Vel (ft/s)	D*V	Fr	D2 (ft)
0.335	2.4376	14.44	0.17	8.49	16.97	3.48	1.17	1.06	0.36193
0.3601	2.80726	15.69	0.18	10.16	20.32	3.62	1.30	1.06	0.39029
0.38	3.122725	16.69	0.19	11.64	23.29	3.73	1.42	1.07	0.41354
0.4196	3.809389	18.67	0.20	15.05	30.10	3.95	1.66	1.07	0.46157
0.4603	4.596832	20.70	0.22	19.21	38.42	4.18	1.92	1.09	0.51301
0.504	5.534525	22.89	0.24	24.48	48.96	4.42	2.23	1.10	0.57024
0.513	5.7395	23.34	0.25	25.68	51.35	4.47	2.30	1.10	0.58225

Street Capacity Calculations

Vierra Avenue

28' F-F Street Section with 4" curb

Slope= 0.0315

Q = 10.02 cfs

For water depths less than 0.0625 feet

Y= Water depth
Area = $16 \cdot Y^2$
P= $\text{SQRT}(1025 \cdot Y^2) + Y$
 $\gamma = 0.017$

Depth (ft)	Area (ft^2)	P (ft)	R (A/P)	Q (cfs)	2Q (cfs)	Vel (ft/s)	D*V	Fr	D2 (ft)
0.01	0.0016	0.33	0.00	0.00	0.00	0.44	0.00	0.78	0.00715
0.02	0.0064	0.66	0.01	0.00	0.01	0.71	0.01	0.88	0.01679
0.025	0.01	0.83	0.01	0.01	0.02	0.82	0.02	0.91	0.02209
0.035	0.0196	1.16	0.02	0.02	0.04	1.02	0.04	0.96	0.03336
0.045	0.0324	1.49	0.02	0.04	0.08	1.21	0.05	1.01	0.04536
0.052	0.043264	1.72	0.03	0.06	0.12	1.33	0.07	1.03	0.05412
0.06	0.0576	1.98	0.03	0.08	0.17	1.47	0.09	1.06	0.06445
0.0625	0.0625	2.06	0.03	0.09	0.19	1.51	0.09	1.06	0.06774

For water depths greater than 0.0625 ft but less than 0.3025 ft

Y1= Y-0.0625
A2= $A1 + 2 \cdot Y1 + 25 \cdot Y1^2$
P2= $P1 + \text{SQRT}(2501 \cdot Y1^2) + Y1$

Depth (ft)	Area (ft^2)	P (ft)	R (A/P)	Q (cfs)	2Q (cfs)	Vel (ft/s)	D*V	Fr	D2 (ft)
0.063	0.063506	2.09	0.03	0.10	0.19	1.51	0.10	1.06	0.06814
0.1	0.172656	3.98	0.04	0.33	0.66	1.92	0.19	1.07	0.10911
0.13	0.311406	5.51	0.06	0.71	1.42	2.29	0.30	1.12	0.15043
0.16	0.495156	7.04	0.07	1.31	2.62	2.64	0.42	1.16	0.19547
0.2	0.810156	9.08	0.09	2.51	5.02	3.10	0.62	1.22	0.25952
0.207	0.873506	9.43	0.09	2.77	5.55	3.18	0.66	1.23	0.27111
0.2612	1.446942	12.20	0.12	5.42	10.84	3.75	0.98	1.29	0.36399
0.3025	1.9825	14.31	0.14	8.24	16.47	4.15	1.26	1.33	0.43797

For water depths greater than 0.3025 ft but less than 0.333 ft

Y2= Y - 0.3025
A3= $A2 + Y2^2 \cdot 14$
P3= $P2 + Y2$

Depth (ft)	Area (ft^2)	P (ft)	R (A/P)	Q (cfs)	2Q (cfs)	Vel (ft/s)	D*V	Fr	D2 (ft)
0.303	1.9895	14.31	0.14	8.28	16.57	4.16	1.26	1.33	0.43952
0.3039	2.0021	14.31	0.14	8.37	16.74	4.18	1.27	1.34	0.44231
0.3062	2.0343	14.31	0.14	8.60	17.19	4.23	1.29	1.35	0.44946
0.31	2.0875	14.31	0.15	8.97	17.95	4.30	1.33	1.36	0.46127
0.3125	2.1225	14.32	0.15	9.22	18.45	4.35	1.36	1.37	0.46905
0.32	2.2275	14.32	0.16	9.99	19.99	4.49	1.44	1.40	0.49243
0.3317	2.3913	14.34	0.17	11.24	22.48	4.70	1.56	1.44	0.52903
0.333	2.4095	14.34	0.17	11.38	22.77	4.72	1.57	1.44	0.53311

For water depths greater than 0.333 ft but less than 0.513 ft

Y3= Y - 0.333
A4= $A3 + 14 \cdot Y3 + 25 \cdot Y3^2$
P4= $P3 + \text{SQRT}(2501 \cdot Y3^2)$

Depth (ft)	Area (ft^2)	P (ft)	R (A/P)	Q (cfs)	2Q (cfs)	Vel (ft/s)	D*V	Fr	D2 (ft)
0.335	2.4376	14.44	0.17	11.55	23.11	4.74	1.59	1.44	0.53638
0.3601	2.80726	15.69	0.18	13.83	27.66	4.93	1.77	1.45	0.57829
0.38	3.122725	16.69	0.19	15.85	31.70	5.08	1.93	1.45	0.6126
0.4196	3.809389	18.67	0.20	20.48	40.97	5.38	2.26	1.46	0.68331
0.4603	4.596832	20.70	0.22	26.15	52.30	5.69	2.62	1.48	0.75888
0.504	5.534525	22.89	0.24	33.33	66.65	6.02	3.03	1.49	0.84279
0.513	5.7395	23.34	0.25	34.95	69.90	6.09	3.12	1.50	0.86038

Street Capacity Calculations

Red Mesa Avenue
28' F-F Street Section with 4" curb

Slope= 0.0287

Q = 7.63 cfs

For water depths less than 0.0625 feet

Y= Water depth
 Area = $16 \cdot Y^2$
 P= $\text{SQRT}(1025 \cdot Y^2) + Y$
 n= 0.017

Depth (ft)	Area (ft^2)	P (ft)	R (A/P)	Q (cfs)	2Q (cfs)	Vel (ft/s)	D*V	Fr	D2 (ft)
0.01	0.0016	0.33	0.00	0.00	0.00	0.42	0.00	0.75	0.00669
0.02	0.0064	0.66	0.01	0.00	0.01	0.67	0.01	0.84	0.01575
0.025	0.01	0.83	0.01	0.01	0.02	0.78	0.02	0.87	0.02072
0.035	0.0196	1.16	0.02	0.02	0.04	0.98	0.03	0.92	0.03132
0.045	0.0324	1.49	0.02	0.04	0.07	1.16	0.05	0.96	0.04262
0.052	0.043264	1.72	0.03	0.06	0.11	1.27	0.07	0.98	0.05087
0.06	0.0576	1.98	0.03	0.08	0.16	1.40	0.08	1.01	0.06059
0.0625	0.0625	2.06	0.03	0.09	0.18	1.44	0.09	1.01	0.06369

For water depths greater than 0.0625 ft but less than 0.3025 ft

Y1= Y-0.0625
 A2= $A1 + 2 \cdot Y1 + 25 \cdot Y1^2$
 P2= $P1 + \text{SQRT}(2501 \cdot Y1^2) + Y1$

Depth (ft)	Area (ft^2)	P (ft)	R (A/P)	Q (cfs)	2Q (cfs)	Vel (ft/s)	D*V	Fr	D2 (ft)
0.063	0.063506	2.09	0.03	0.09	0.18	1.44	0.09	1.01	0.06407
0.1	0.172656	3.98	0.04	0.32	0.63	1.83	0.18	1.02	0.1026
0.13	0.311406	5.51	0.06	0.68	1.36	2.18	0.28	1.07	0.14154
0.16	0.495156	7.04	0.07	1.25	2.50	2.52	0.40	1.11	0.18402
0.2	0.810156	9.08	0.09	2.40	4.79	2.96	0.59	1.17	0.24446
0.207	0.873506	9.43	0.09	2.65	5.29	3.03	0.63	1.17	0.2554
0.2612	1.446942	12.20	0.12	5.17	10.35	3.57	0.93	1.23	0.3431
0.3025	1.9825	14.31	0.14	7.86	15.72	3.97	1.20	1.27	0.41298

For water depths greater than 0.3025 ft but less than 0.333 ft

Y2= Y - 0.3025
 A3= $A2 + Y2 \cdot 14$
 P3= $P2 + Y2$

Depth (ft)	Area (ft^2)	P (ft)	R (A/P)	Q (cfs)	2Q (cfs)	Vel (ft/s)	D*V	Fr	D2 (ft)
0.303	1.9895	14.31	0.14	7.91	15.82	3.97	1.20	1.27	0.41445
0.3039	2.0021	14.31	0.14	7.99	15.98	3.99	1.21	1.28	0.41709
0.3062	2.0343	14.31	0.14	8.21	16.41	4.03	1.24	1.28	0.42386
0.31	2.0875	14.31	0.15	8.56	17.13	4.10	1.27	1.30	0.43506
0.3125	2.1225	14.32	0.15	8.80	17.61	4.15	1.30	1.31	0.44243
0.32	2.2275	14.32	0.16	9.54	19.08	4.28	1.37	1.33	0.46459
0.3317	2.3913	14.34	0.17	10.73	21.46	4.49	1.49	1.37	0.49927
0.333	2.4095	14.34	0.17	10.87	21.73	4.51	1.50	1.38	0.50313

For water depths greater than 0.333 ft but less than 0.513 ft

Y3= Y - 0.333
 A4= $A3 + 14 \cdot Y3 + 25 \cdot Y3^2$
 P4= $P3 + \text{SQRT}(2501 \cdot Y3^2)$

Depth (ft)	Area (ft^2)	P (ft)	R (A/P)	Q (cfs)	2Q (cfs)	Vel (ft/s)	D*V	Fr	D2 (ft)
0.335	2.4376	14.44	0.17	11.03	22.05	4.52	1.52	1.38	0.50622
0.3601	2.80726	15.69	0.18	13.20	26.40	4.70	1.69	1.38	0.54579
0.38	3.122725	16.69	0.19	15.13	30.26	4.84	1.84	1.39	0.57819
0.4196	3.809389	18.67	0.20	19.55	39.11	5.13	2.15	1.40	0.64499
0.4603	4.596832	20.70	0.22	24.96	49.92	5.43	2.50	1.41	0.71639
0.504	5.534525	22.89	0.24	31.81	63.62	5.75	2.90	1.43	0.79569
0.513	5.7395	23.34	0.25	33.36	66.72	5.81	2.98	1.43	0.81233

Street Capacity Calculations

Corregidor - AP #1
32' F-F Street Section with 8" curb
 Slope= 0.009

Q = 8.76 cfs

For water depths less than 0.125 feet

Y= Water depth
 Area = $8 \cdot Y^2$
 P= $\text{SQRT}(257 \cdot Y^2) + Y$
 n= 0.017

Depth (ft)	Area (ft^2)	P (ft)	R (A/P)	Q (cfs)	2Q (cfs)	Vel (ft/s)	D*V	Fr	D2 (ft)
0.01	0.0008	0.1703	0.0047	0.0002	0.0004	0.2326	0.0023	0.4099	0.0027
0.02	0.0032	0.3406	0.0094	0.0012	0.0024	0.3692	0.0074	0.4601	0.0064
0.04	0.0128	0.6812	0.0188	0.0075	0.0150	0.5861	0.0234	0.5164	0.0154
0.06	0.0288	1.0219	0.0282	0.0221	0.0442	0.7680	0.0461	0.5525	0.0257
0.08	0.0512	1.3625	0.0376	0.0476	0.0953	0.9304	0.0744	0.5797	0.0368
0.1	0.0800	1.7031	0.0470	0.0864	0.1727	1.0796	0.1080	0.6016	0.0487
0.12	0.1152	2.0437	0.0564	0.1404	0.2809	1.2191	0.1463	0.6202	0.0612
0.125	0.1250	2.1289	0.0587	0.1566	0.3132	1.2527	0.1566	0.6244	0.0643

For water depths greater than 0.125 ft but less than 0.405 ft

Y1= Y - 0.125
 A2= $A1 + 2 \cdot Y1 + 25 \cdot Y1^2$
 P2= $P1 + \text{SQRT}(2501 \cdot Y1^2)$

Depth (ft)	Area (ft^2)	P (ft)	R (A/P)	Q (cfs)	2Q (cfs)	Vel (ft/s)	D*V	Fr	D2 (ft)
0.13	0.1356	2.3840	0.0569	0.1664	0.3327	1.2266	0.1595	0.5995	0.0630
0.16	0.2256	3.9143	0.0576	0.2792	0.5584	1.2374	0.1980	0.5452	0.0670
0.2	0.4156	5.9547	0.0698	0.5843	1.1686	1.4058	0.2812	0.5540	0.0859
0.24	0.6856	7.9951	0.0858	1.1057	2.2113	1.6126	0.3870	0.5801	0.1106
0.28	1.0356	10.0355	0.1032	1.8895	3.7789	1.8245	0.5108	0.6076	0.1384
0.32	1.4656	12.0759	0.1214	2.9793	5.9586	2.0328	0.6505	0.6333	0.1682
0.3464	1.7932	13.4225	0.1336	3.8863	7.7726	2.1672	0.7507	0.6489	0.1888
0.39	2.4106	15.6466	0.1541	5.7451	11.4901	2.3832	0.9295	0.6725	0.2241
0.405	2.6450	16.4117	0.1612	6.4958	12.9916	2.4559	0.9946	0.6801	0.2365

For water depths greater than 0.405 ft but less than 0.667 ft

Y2= Y - 0.405
 A3= $A2 + Y2^2 \cdot 16$
 P3= $P2 + Y2$

Depth (ft)	Area (ft^2)	P (ft)	R (A/P)	Q (cfs)	2Q (cfs)	Vel (ft/s)	D*V	Fr	D2 (ft)
0.41	2.7250	16.4167	0.1660	6.8251	13.6503	2.5046	1.0269	0.6893	0.2442
0.44	3.2050	16.4467	0.1949	8.9334	17.8669	2.7873	1.2264	0.7405	0.2906
0.47	3.6850	16.4767	0.2236	11.2592	22.5184	3.0554	1.4360	0.7854	0.3375
0.5	4.1650	16.5067	0.2523	13.7914	27.5829	3.3113	1.6556	0.8252	0.3848
0.55	4.9650	16.5567	0.2999	18.4462	36.8924	3.7153	2.0434	0.8828	0.4647
0.5945	5.6770	16.6012	0.3420	23.0213	46.0426	4.0552	2.4108	0.9268	0.5368
0.63	6.2450	16.6367	0.3754	26.9484	53.8969	4.3152	2.7186	0.9581	0.5949
0.667	6.8370	16.6737	0.4100	31.2929	62.5858	4.5770	3.0529	0.9876	0.6560

For water depths greater than 0.667 ft but less than 0.847 ft

Y3= Y - 0.667

A4= A3 + 16 * Y3 + 25 * Y3^2
P4= P3 + SQRT(2501 * Y3^2)

Depth (ft)	Area (ft^2)	P (ft)	R (A/P)	Q (cfs)	2Q (cfs)	Vel (ft/s)	D*V	Fr	D2 (ft)
0.7	7.3922	18.3240	0.4034	33.4685	66.9369	4.5275	3.1693	0.9536	0.6568
0.73	7.9442	19.8243	0.4007	35.8078	71.6157	4.5074	3.2904	0.9297	0.6618
0.75	8.3372	20.8245	0.4004	37.5558	75.1116	4.5046	3.3784	0.9166	0.6671
0.77	8.7502	21.8247	0.4009	39.4540	78.9079	4.5089	3.4719	0.9055	0.6736
0.8	9.4072	23.3250	0.4033	42.5839	85.1678	4.5267	3.6214	0.8919	0.6854
0.82	9.8702	24.3252	0.4058	44.8606	89.7212	4.5450	3.7269	0.8845	0.6946
0.85	10.6022	25.8255	0.4105	48.5646	97.1292	4.5806	3.8935	0.8756	0.7101

Street Capacity Calculations

Corregidor - AP #2
32' F-F Street Section with 8" curb
Slope= 0.009

Q = 20.21 cfs

For water depths less than 0.125 feet

$$r = \text{Water depth}$$
$$\text{Area} = 8 \cdot Y^2$$
$$D = \sqrt{257 \cdot Y^2} + Y$$
$$= 0.017$$

Depth (ft)	Area (ft^2)	P (ft)	R (A/P)	Q (cfs)	2Q (cfs)	Vel (ft/s)	D*V	Fr	D2 (ft)
0.01	0.0008	0.1703	0.0047	0.0002	0.0004	0.2326	0.0023	0.4099	0.0027
0.02	0.0032	0.3406	0.0094	0.0012	0.0024	0.3692	0.0074	0.4601	0.0064
0.04	0.0128	0.6812	0.0188	0.0075	0.0150	0.5861	0.0234	0.5164	0.0154
0.06	0.0288	1.0219	0.0282	0.0221	0.0442	0.7680	0.0461	0.5525	0.0257
0.08	0.0512	1.3625	0.0376	0.0476	0.0953	0.9304	0.0744	0.5797	0.0368
0.1	0.0800	1.7031	0.0470	0.0864	0.1727	1.0796	0.1080	0.6016	0.0487
0.12	0.1152	2.0437	0.0564	0.1404	0.2809	1.2191	0.1463	0.6202	0.0612
0.125	0.1250	2.1289	0.0587	0.1566	0.3132	1.2527	0.1566	0.6244	0.0643

For water depths greater than 0.125 ft but less than 0.405 ft

$$Y1 = Y - 0.125$$
$$A2 = A1 + 2 \cdot Y1 + 25 \cdot Y1^2$$
$$P2 = P1 + \sqrt{2501 \cdot Y1^2}$$

Depth (ft)	Area (ft^2)	P (ft)	R (A/P)	Q (cfs)	2Q (cfs)	Vel (ft/s)	D*V	Fr	D2 (ft)
0.13	0.1356	2.3840	0.0569	0.1664	0.3327	1.2266	0.1595	0.5995	0.0630
0.16	0.2256	3.9143	0.0576	0.2792	0.5584	1.2374	0.1980	0.5452	0.0670
0.2	0.4156	5.9547	0.0698	0.5843	1.1686	1.4058	0.2812	0.5540	0.0859
0.24	0.6856	7.9951	0.0858	1.1057	2.2113	1.6126	0.3870	0.5801	0.1106
0.28	1.0356	10.0355	0.1032	1.8895	3.7789	1.8245	0.5108	0.6076	0.1384
0.32	1.4656	12.0759	0.1214	2.9793	5.9586	2.0328	0.6505	0.6333	0.1682
0.3464	1.7932	13.4225	0.1336	3.8863	7.7726	2.1672	0.7507	0.6489	0.1888
0.39	2.4106	15.6466	0.1541	5.7451	11.4901	2.3832	0.9295	0.6725	0.2241
0.405	2.6450	16.4117	0.1612	6.4958	12.9916	2.4559	0.9946	0.6801	0.2365

For water depths greater than 0.405 ft but less than 0.667 ft

$$Y2 = Y - 0.405$$
$$A3 = A2 + Y2 \cdot 16$$
$$P3 = P2 + Y2$$

Depth (ft)	Area (ft^2)	P (ft)	R (A/P)	Q (cfs)	2Q (cfs)	Vel (ft/s)	D*V	Fr	D2 (ft)
0.41	2.7250	16.4167	0.1660	6.8251	13.6503	2.5046	1.0269	0.6893	0.2442
0.44	3.2050	16.4467	0.1949	8.9334	17.8669	2.7873	1.2264	0.7405	0.2906
0.47	3.6850	16.4767	0.2236	11.2592	22.5184	3.0554	1.4360	0.7854	0.3375
0.5	4.1650	16.5067	0.2523	13.7914	27.5829	3.3113	1.6556	0.8252	0.3848
0.55	4.9650	16.5567	0.2999	18.4462	36.8924	3.7153	2.0434	0.8828	0.4647
0.5945	5.6770	16.6012	0.3420	23.0213	46.0426	4.0552	2.4108	0.9268	0.5368
0.63	6.2450	16.6367	0.3754	26.9484	53.8969	4.3152	2.7186	0.9581	0.5949
0.667	6.8370	16.6737	0.4100	31.2929	62.5858	4.5770	3.0529	0.9876	0.6560

For water depths greater than 0.667 ft but less than 0.847 ft

$$Y3 = Y - 0.667$$

A4= A3 + 16 * Y3 + 25 * Y3^2
P4= P3 + SQRT(2501 * Y3^2)

Depth (ft)	Area (ft^2)	P (ft)	R (A/P)	Q (cfs)	2Q (cfs)	Vel (ft/s)	D*V	Fr	D2 (ft)
0.7	7.3922	18.3240	0.4034	33.4685	66.9369	4.5275	3.1693	0.9536	0.6568
0.73	7.9442	19.8243	0.4007	35.8078	71.6157	4.5074	3.2904	0.9297	0.6618
0.75	8.3372	20.8245	0.4004	37.5558	75.1116	4.5046	3.3784	0.9166	0.6671
0.77	8.7502	21.8247	0.4009	39.4540	78.9079	4.5089	3.4719	0.9055	0.6736
0.8	9.4072	23.3250	0.4033	42.5839	85.1678	4.5267	3.6214	0.8919	0.6854
0.82	9.8702	24.3252	0.4058	44.8606	89.7212	4.5450	3.7269	0.8845	0.6946
0.85	10.6022	25.8255	0.4105	48.5646	97.1292	4.5806	3.8935	0.8756	0.7101

Street Capacity Calculations

Corregidor - AP #3
32' F-F Street Section with 8" curb
 Slope= 0.009

Q = 20.92 cfs

For water depths less than 0.125 feet

Y= Water depth
 Area = $8 \cdot Y^2$
 P= $\text{SQRT}(257 \cdot Y^2) + Y$
 r= 0.017

Depth (ft)	Area (ft^2)	P (ft)	R (A/P)	Q (cfs)	2Q (cfs)	Vel (ft/s)	D*V	Fr	D2 (ft)
0.01	0.0008	0.1703	0.0047	0.0002	0.0004	0.2326	0.0023	0.4099	0.0027
0.02	0.0032	0.3406	0.0094	0.0012	0.0024	0.3692	0.0074	0.4601	0.0064
0.04	0.0128	0.6812	0.0188	0.0075	0.0150	0.5861	0.0234	0.5164	0.0154
0.06	0.0288	1.0219	0.0282	0.0221	0.0442	0.7680	0.0461	0.5525	0.0257
0.08	0.0512	1.3625	0.0376	0.0476	0.0953	0.9304	0.0744	0.5797	0.0368
0.1	0.0800	1.7031	0.0470	0.0864	0.1727	1.0796	0.1080	0.6016	0.0487
0.12	0.1152	2.0437	0.0564	0.1404	0.2809	1.2191	0.1463	0.6202	0.0612
0.125	0.1250	2.1289	0.0587	0.1566	0.3132	1.2527	0.1566	0.6244	0.0643

For water depths greater than 0.125 ft but less than 0.405 ft

Y1= Y - 0.125
 A2= $A1 + 2 \cdot Y1 + 25 \cdot Y1^2$
 P2= $P1 + \text{SQRT}(2501 \cdot Y1^2)$

Depth (ft)	Area (ft^2)	P (ft)	R (A/P)	Q (cfs)	2Q (cfs)	Vel (ft/s)	D*V	Fr	D2 (ft)
0.13	0.1356	2.3840	0.0569	0.1664	0.3327	1.2266	0.1595	0.5995	0.0630
0.16	0.2256	3.9143	0.0576	0.2792	0.5584	1.2374	0.1980	0.5452	0.0670
0.2	0.4156	5.9547	0.0698	0.5843	1.1686	1.4058	0.2812	0.5540	0.0859
0.24	0.6856	7.9951	0.0858	1.1057	2.2113	1.6126	0.3870	0.5801	0.1106
0.28	1.0356	10.0355	0.1032	1.8895	3.7789	1.8245	0.5108	0.6076	0.1384
0.32	1.4656	12.0759	0.1214	2.9793	5.9586	2.0328	0.6505	0.6333	0.1682
0.3464	1.7932	13.4225	0.1336	3.8863	7.7726	2.1672	0.7507	0.6489	0.1888
0.39	2.4106	15.6466	0.1541	5.7451	11.4901	2.3832	0.9295	0.6725	0.2241
0.405	2.6450	16.4117	0.1612	6.4958	12.9916	2.4559	0.9946	0.6801	0.2365

For water depths greater than 0.405 ft but less than 0.667 ft

Y2= Y - 0.405
 A3= $A2 + Y2^2 \cdot 16$
 P3= $P2 + Y2$

Depth (ft)	Area (ft^2)	P (ft)	R (A/P)	Q (cfs)	2Q (cfs)	Vel (ft/s)	D*V	Fr	D2 (ft)
0.41	2.7250	16.4167	0.1660	6.8251	13.6503	2.5046	1.0269	0.6893	0.2442
0.44	3.2050	16.4467	0.1949	8.9334	17.8669	2.7873	1.2264	0.7405	0.2906
0.47	3.6850	16.4767	0.2236	11.2592	22.5184	3.0554	1.4360	0.7854	0.3375
0.5	4.1650	16.5067	0.2523	13.7914	27.5829	3.3113	1.6556	0.8252	0.3848
0.55	4.9650	16.5567	0.2999	18.4462	36.8924	3.7153	2.0434	0.8828	0.4647
0.5945	5.6770	16.6012	0.3420	23.0213	46.0426	4.0552	2.4108	0.9268	0.5368
0.63	6.2450	16.6367	0.3754	26.9484	53.8969	4.3152	2.7186	0.9581	0.5949
0.667	6.8370	16.6737	0.4100	31.2929	62.5858	4.5770	3.0529	0.9876	0.6560

For water depths greater than 0.667 ft but less than 0.847 ft

Y3= Y - 0.667

A4= A3 + 16 * Y3 + 25 * Y3^2
P4= P3 + SQRT(2501 * Y3^2)

Depth (ft)	Area (ft^2)	P (ft)	R (A/P)	Q (cfs)	2Q (cfs)	Vel (ft/s)	D*V	Fr	D2 (ft)
0.7	7.3922	18.3240	0.4034	33.4685	66.9369	4.5275	3.1693	0.9536	0.6568
0.73	7.9442	19.8243	0.4007	35.8078	71.6157	4.5074	3.2904	0.9297	0.6618
0.75	8.3372	20.8245	0.4004	37.5558	75.1116	4.5046	3.3784	0.9166	0.6671
0.77	8.7502	21.8247	0.4009	39.4540	78.9079	4.5089	3.4719	0.9055	0.6736
0.8	9.4072	23.3250	0.4033	42.5839	85.1678	4.5267	3.6214	0.8919	0.6854
0.82	9.8702	24.3252	0.4058	44.8606	89.7212	4.5450	3.7269	0.8845	0.6946
0.85	10.6022	25.8255	0.4105	48.5646	97.1292	4.5806	3.8935	0.8756	0.7101

Street Capacity Calculations

Corregidor - AP #4
32' F-F Street Section with 8" curb
 Slope= 0.009

Q = 30.64 cfs

For water depths less than 0.125 feet

Y= Water depth
 Area = $8 \cdot Y^2$
 P= $\text{SQRT}(257 \cdot Y^2) + Y$
 n= 0.017

Depth (ft)	Area (ft^2)	P (ft)	R (A/P)	Q (cfs)	2Q (cfs)	Vel (ft/s)	D*V	Fr	D2 (ft)
0.01	0.0008	0.1703	0.0047	0.0002	0.0004	0.2326	0.0023	0.4099	0.0027
0.02	0.0032	0.3406	0.0094	0.0012	0.0024	0.3692	0.0074	0.4601	0.0064
0.04	0.0128	0.6812	0.0188	0.0075	0.0150	0.5861	0.0234	0.5164	0.0154
0.06	0.0288	1.0219	0.0282	0.0221	0.0442	0.7680	0.0461	0.5525	0.0257
0.08	0.0512	1.3625	0.0376	0.0476	0.0953	0.9304	0.0744	0.5797	0.0368
0.1	0.0800	1.7031	0.0470	0.0864	0.1727	1.0796	0.1080	0.6016	0.0487
0.12	0.1152	2.0437	0.0564	0.1404	0.2809	1.2191	0.1463	0.6202	0.0612
0.125	0.1250	2.1289	0.0587	0.1566	0.3132	1.2527	0.1566	0.6244	0.0643

For water depths greater than 0.125 ft but less than 0.405 ft

Y1= Y-0.125
 A2= $A1 + 2 \cdot Y1 + 25 \cdot Y1^2$
 P2= $P1 + \text{SQRT}(2501 \cdot Y1^2)$

Depth (ft)	Area (ft^2)	P (ft)	R (A/P)	Q (cfs)	2Q (cfs)	Vel (ft/s)	D*V	Fr	D2 (ft)
0.13	0.1356	2.3840	0.0569	0.1664	0.3327	1.2266	0.1595	0.5995	0.0630
0.16	0.2256	3.9143	0.0576	0.2792	0.5584	1.2374	0.1980	0.5452	0.0670
0.2	0.4156	5.9547	0.0698	0.5843	1.1686	1.4058	0.2812	0.5540	0.0859
0.24	0.6856	7.9951	0.0858	1.1057	2.2113	1.6126	0.3870	0.5801	0.1106
0.28	1.0356	10.0355	0.1032	1.8895	3.7789	1.8245	0.5108	0.6076	0.1384
0.32	1.4656	12.0759	0.1214	2.9793	5.9586	2.0328	0.6505	0.6333	0.1682
0.3464	1.7932	13.4225	0.1336	3.8863	7.7726	2.1672	0.7507	0.6489	0.1888
0.39	2.4106	15.6466	0.1541	5.7451	11.4901	2.3832	0.9295	0.6725	0.2241
0.405	2.6450	16.4117	0.1612	6.4958	12.9916	2.4559	0.9946	0.6801	0.2365

For water depths greater than 0.405 ft but less than 0.667 ft

Y2= Y - 0.405
 A3= $A2 + Y2 \cdot 16$
 P3= $P2 + Y2$

Depth (ft)	Area (ft^2)	P (ft)	R (A/P)	Q (cfs)	2Q (cfs)	Vel (ft/s)	D*V	Fr	D2 (ft)
0.41	2.7250	16.4167	0.1660	6.8251	13.6503	2.5046	1.0269	0.6893	0.2442
0.44	3.2050	16.4467	0.1949	8.9334	17.8669	2.7873	1.2264	0.7405	0.2906
0.47	3.6850	16.4767	0.2236	11.2592	22.5184	3.0554	1.4360	0.7854	0.3375
0.5	4.1650	16.5067	0.2523	13.7914	27.5829	3.3113	1.6556	0.8252	0.3848
0.55	4.9650	16.5567	0.2999	18.4462	36.8924	3.7153	2.0434	0.8828	0.4647
0.5945	5.6770	16.6012	0.3420	23.0213	46.0426	4.0552	2.4108	0.9268	0.5368
0.63	6.2450	16.6367	0.3754	26.9484	53.8969	4.3152	2.7186	0.9581	0.5949
0.667	6.8370	16.6737	0.4100	31.2929	62.5858	4.5770	3.0529	0.9876	0.6560

For water depths greater than 0.667 ft but less than 0.847 ft

Y3= Y - 0.667

A4= A3 + 16 * Y3 + 25 * Y3^2
P4= P3 + SQRT(2501 * Y3^2)

Depth (ft)	Area (ft^2)	P (ft)	R (A/P)	Q (cfs)	2Q (cfs)	Vel (ft/s)	D*V	Fr	D2 (ft)
0.7	7.3922	18.3240	0.4034	33.4685	66.9369	4.5275	3.1693	0.9536	0.6568
0.73	7.9442	19.8243	0.4007	35.8078	71.6157	4.5074	3.2904	0.9297	0.6618
0.75	8.3372	20.8245	0.4004	37.5558	75.1116	4.5046	3.3784	0.9166	0.6671
0.77	8.7502	21.8247	0.4009	39.4540	78.9079	4.5089	3.4719	0.9055	0.6736
0.8	9.4072	23.3250	0.4033	42.5839	85.1678	4.5267	3.6214	0.8919	0.6854
0.82	9.8702	24.3252	0.4058	44.8606	89.7212	4.5450	3.7269	0.8845	0.6946
0.85	10.6022	25.8255	0.4105	48.5646	97.1292	4.5806	3.8935	0.8756	0.7101

Single 'C' Drop Inlet EFFECTIVE AREA

Area at the grate:

$$\begin{aligned} L &= 38.375'' - 7(2@ \text{ middle bars}) \\ &= 34.875'' \\ &= 2.906' \end{aligned}$$

$$\begin{aligned} W &= 25.5'' - 13(2@ \text{ middle bars}) \\ &= 19'' \\ &= 1.583' \end{aligned}$$

$$\begin{aligned} \text{Area} &= 1.583' \times 2.906' \\ &= 4.601 \text{ ft}^2 \end{aligned}$$

$$\begin{aligned} \text{Effective Area} &= 4.601 - 4.601 (0.5 \text{ clogging factor}) \\ &= 2.30 \text{ ft}^2 \text{ at the grate} \end{aligned}$$

Orifice Equation

$$Q = CA \sqrt{2gH}$$

$$Q = 0.6 \times 4.601 \times \sqrt{2 \times 32.2 \times 0.67}$$

$$Q = 18.13 \text{ cfs}$$

Area at the throat:

$$\begin{aligned} L &= 47.375@ \\ &= 3.95' \end{aligned}$$

$$\begin{aligned} H &= 10:@ - 42@ \\ &= 63@ \\ &= 0.5208' \end{aligned}$$

$$\begin{aligned} \text{Area} &= 3.95' \times 0.5208' \\ &= 2.06 \text{ ft}^2 \text{ at the throat} \end{aligned}$$

Weir Equation

$$Q = CLH^{(3/2)}$$

$$Q = 2.95 \times 3.95 \times 0.67^{(3/2)}$$

$$Q = 6.39 \text{ cfs}$$

Total Capacity

$$Q = 18.13\text{grate} + 6.39\text{throat}$$

$$Q = 24.69 \text{ cfs}$$

Flow required = 25.43 cfs > 24.69 cfs; Computed flow not significantly greater, therefore Single C inlet has capacity

Pipe Capacity

Pipe	D	Slope	Area	R	Q Provided	Q Required	Velocity
	(in)	(%)	(ft^2)		(cfs)	(cfs)	(ft/s)
1	24	2	3.14	0.500	32.08	25.43	8.09

Manning's Equation:

$Q = 1.49/n * A * R^{(2/3)} * S^{(1/2)}$

A = Area

R = D/4

S = Slope

n = 0.013

Appendix A

Infrastructure List

Project Number: _____

Date Submitted: _____

INFRASTRUCTURE LIST

Date Site Plan Approved: _____

Date Preliminary Plat Approved: _____

Date Preliminary Plat Expires: _____

DRB Project No.: _____

DRB Application No.: _____

EXHIBIT "A"

**TO SUBDIVISION IMPROVEMENTS AGREEMENT
DEVELOPMENT REVIEW BOARD (D.R.B.) REQUIRED INFRASTRUCTURE LIST**

Vista Pacifica Subdivision Phase 2

PROPOSED NAME OF PLAT AND/OR SITE DEVELOPMENT PLAN

Tract 168, Town of Atrisco Grant Unit 6

EXISTING LEGAL DESCRIPTION PRIOR TO PLATTING ACTION

Following is a summary of PUBLIC/PRIVATE Infrastructure required to be constructed or financially guaranteed for the above development. This Listing is not necessarily a complete listing. During the SIA process and/or in the review of the construction drawings, if the DRC Chair determines that appurtenant items and/or unforeseen items have not been included in the infrastructure listing, the DRC Chair may include those items in the listing and related financial guarantee. Likewise, if the DRC Chair determines that appurtenant or non-essential items can be deleted from the listing, those items may be deleted as well as the related portions of the financial guarantees. All such revisions require approval by the DRC Chair, the User Department and agent/owner. If such approvals are obtained, these revisions to the listing will be incorporated administratively. In addition, any unforeseen items which arise during construction which are necessary to complete the project and which normally are the Subdivider's responsibility will be required as a condition of project acceptance and close out by the City.

SIA Sequence #	COA DRC Project #	Size	Type of Improvement	Location	From	To	Private Inspector	City Inspector	City Cnst Engineer
		16' F-E	Residential Paving Curb and Gutter 4' Sidewalk (West Side Only)	Corregidor Drive	Sunset Gardens Rd.	Gonzales Rd.	/	/	/
		16' F-E	Residential Paving Curb and Gutter 4' Sidewalk (East Side Only)	Oasis Drive	Sunset Gardens Rd.	Gonzales Rd.	/	/	/
		28' F-F	Residential Paving Curb and Gutter 4' Sidewalk (Both Sides)*	Red Mesa Avenue	Corregidor Drive	Oasis Drive	/	/	/
		28' F-F	Residential Paving Curb and Gutter 4' Sidewalk (Both Sides)*	Vierra Avenue	Corregidor Drive	Oasis Drive	/	/	/
		28' F-F	Residential Paving Curb and Gutter 4' Sidewalk (Both Sides)*	White Reserve Avenue	Corregidor Drive	Oasis Drive	/	/	/
		8"	SAS Graviity Line	Red Mesa Avenue	Corregidor Drive	Oasis Drive	/	/	/
		8"	SAS Graviity Line	Vierra Avenue	Corregidor Drive	Oasis Drive	/	/	/
		8"	SAS Graviity Line	White Reserve Avenue	Corregidor Drive	Oasis Drive	/	/	/
		8"	Water PVC Line	Red Mesa Avenue	Corregidor Drive	Oasis Drive	/	/	/

SIA Sequence #	COA DRC Project #

Size	Type of Improvement	Location	From	To
8"	Water PVC Line	Vierra Avenue	Corregidor Drive	Oasis Drive
8"	Water PVC Line	White Reserve Avenue	Corregidor Drive	Oasis Drive

Private Inspector	City Inspector	City Cnst Engineer
/	/	/
/	/	/
/	/	/
/	/	/
/	/	/
/	/	/
/	/	/

NAME OF PLAT AND/OR SITE PLAN__Vista Pacifica Unit 2_____

NOTES

- 1

*All internal sidewalk deferred.
- 2
- 3

AGENT / OWNER

Ronald R. Bohannon
NAME (print)

DEVELOPMENT REVIEW BOARD MEMBER APPROVALS

DRB CHAIR - date

PARKS & GENERAL SERVICES - date

TRANSPORTATION DEVELOPMENT - date

AMAFCA - date

UTILITY DEVELOPMENT - date

- date

CITY ENGINEER - date

- date

FIRM
3/28/04
SIGNATURE - date

MAXIMUM TIME ALLOWED TO CONSTRUCT
THE IMPROVEMENTS WITHOUT A DRB
EXTENSION: _____

DESIGN REVIEW COMMITTEE REVISIONS

REVISION	DATE	DRC CHAIR	USER DEPARTMENT	AGENT / OWNER

Map Pocket

Grading and Drainage Plan