

DRAINAGE PLAN

The following items concerning the High Street Apartments Drainage Plan are contained hereon:

1. Vicinity Map

2. Grading Plan
3. Calculations

The proposed improvements, as shown by the Vicinity Map, are located on the southwest quadrant of the intersection of Lead Avenue S.E. and High Street S.E. At present, the site is undeveloped. Much of the surrounding area is currently developed, thereby making this an infill site. As shown by Plate K-14 of the Albuquerque Master Drainage Study, this site does not lie within a designated Flood Hazard Zone. Downstream flooding is not apparent, and therefore does not appear to be a problem. At present, the site flows from east to west onto the public alley located west of the project site. From that point, runoff spills onto the adjoining lots located west of the public alley. No offsite flows enter the site along the north and east property lines since the existing streets route runoff away from the project site. No offsite flows enter the site along the south property lines since the existing wall routes runoff away from the project site. No offsite flows enter the site along the west property line since the existing public alley is graded in a manner which will route runoff away from the project site.

The Grading Plan shows 1) existing and proposed grades indicated by contours at 1'0" intervals, 2) continuity between existing and proposed grades, and 3) the limit and character of the proposed improvements. As shown by this plan, the proposed improvements consist of the construction of new apartment buildings along with adjacent paving and landscaping. Flows generated by the proposed improvements will be routed from east to west onto the public alley located west of the project site. The public alley will be improved to accept runoff from the project site. The proposed improvements consist of the repaving of the public alley and the construction of a single 'D' storm drain inlet which will connect to the existing 15" RCP storm drain system located in Lead Avenue S.E. which is the outfall for this site and the proposed public alley improvements (City Project No. 3171). The Developer is providing cross lot drainage for the proposed drainage pattern. Based upon the fact that this site is an infill site and the elimination of runoff discharged onto the adjoining lots located west of the public alley and the proximity of downstream improvements, the free discharge of runoff from this site is appropriate.

The Calculations which appear hereon analyze both the existing and developed conditions for the 100-year, 6-hour rainfall event. The Rational Method has been used to quantify the peak rate of discharge and the SCS Method has been used to quantify the volume of runoff. Both methods have been used in accordance with the City of Albuquerque Development Process Manual, Volume II and the Mayor's Emergency Rule adopted January 14, 1986. As shown by these calculations, the proposed improvements will increase the total discharge from this site by approximately 0.5 cfs.

CALCULATIONS

Ground Cover Information

From SCS Bernalillo County Soil Survey,
Plate: 30, Cu - Cut and fill land
Hydrologic Soil Group: A
Existing Pervious CN = 54 (DPM Plate 22.2 C-2
Pasture or Range Land: fair condition)
Developed Pervious CN = 39 (DPM Plate 22.2 C-2)

Time of Concentration/Time to Peak

 $T_{C} = 0.0078 L^{0.77}/S^{0.385}$ (Kirpich Equation)

 $T_D = T_C = 10 \text{ min.}$

Point Rainfall

 $P_6 = 2.26$ in. (DPM Plate 22.2 D-1)

Rational Method

Discharge: Q = Cil

where C varies $i = P_6$ (6.84) T_C -0.51 = 4.78 in/hr P_6 = 2.26 in (DPM Plate 22.2D-1) T_C = 10 min (minimum) A = area, acres

SCS Method

Volume: V = 3630(DRO) A

Where DRO = Direct runoff in inches
A = area, acres

Existing Condition

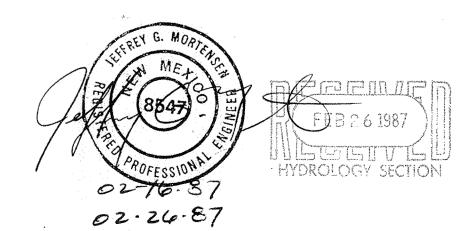
Atotal = 21,250 sf = 0.49 Ac C = 0.40 (Weighted average per Emergency Rule, 1/14/86) $Q_{100} = CiA = (0.40)(4.78)(0.49) = 1.0 cfs$ $A_{imp} = -0 - sf$; % impervious = -0 - % Composite CN = 54 (DPM Plate 22.2 C-3) DRO = 0.1 in (DPM Plate 22.2 C-4) $V_{100} = 3630$ (DRO) A = 180 cf

Developed Condition

Atotal = 21,250 sf = 0.49 Ac Roof area = 4560 sf (0.21) Paved area = 7050 sf (0.34) Landscaped area = 9640 sf (0.45) $C = \underbrace{0.62}_{\text{Ql00}} \text{ (Weighted average per Emergency Rule, 1/14/86)} \\ Q_{100} = \text{CiA} = (0.62)(4.78)(0.49) = 1.5 \text{ cfs} \\ A_{\text{imp}} = 11,610 \text{ sf; % impervious = 55 % } \\ \text{Composite CN} = 72 \text{ (DPM Plate 22.2 C-3)} \\ \text{DRO} = 0.5 \text{ in (DPM Plate 22.2 C-4)} \\ V_{100} = 3630 \text{ (DRO) A} = 890 \text{ cf}$

Comparison

 $\Delta Q_{100} = 1.5 - 1.0 = 0.5$ cfs (increase) $\Delta V_{100} = 890 - 180 = 710$ cf (increase)



GRADING AND DRAINAGE PLAN
HIGH STREET APARTMENTS

FILE NO.