PROJECT TITLE: Yale heservoir zon	NE ATLAS/DRNG. FILE #: K-14028
CITY ADDRESS: not available 2201	Chingo S
	antral NE
ENGINEERING FIRM: Tom Mann & Asse Inc	CONTACT: Tom Mann
	PHONE: 265-5611
OHNER: City of Albuqueique	CONTACT: Art Stewart.
ADDRESS: Po. Box 1293	PHONE: 168 2772
HRCHITECT: CH2M H11	CONTACT: Pete Jacobson
ADDRESS: 6121 Indian School WE	PHONE: 884-5600
JRVEYOR: Jom Mann & Assoc Inc	CONTACT:
ADDRESS:	PHONE:
CONTRACTOR:	CONTACT:
ADDRESS:	PHONE:
- ND HYDROLOGY SECTION EL	RB ND. PC ND. RDJ. ND.
TYPE OF SUBMITTAL:	
DRAINAGE REPORT	TYPE OF APPROVAL SOUGHT:
DEUTNOCE DI ON	PRELIMINARY PLAT APPROVAL
(/ GOLDENTLINE	SITE DEVELOPMENT PLAN APPROVAL
GRADING PLAN	FINAL PLAT APPROVAL
EROSION CONTROL PLAN	BUILDING PERMIT APPROVAL
ENGINEER'S CERTIFICATION	FOUNDATION PERMIT APPROVAL
	CERTIFICATE OF OCCUPANCY APPROVAL
	ROUGH GRADING PERMIT APPROVAL
***************************************	GRADING/PAVING PERMIT APPROVAL OTHER (SPECIEVAL
-	UTHER (SPECIFY)
DATE SUBMITTED: 4-/24-/87	
BY: Jon Wan	

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City of Albuquerque

P.O. BOX 1293 ALBUQUERQUE, NEW MEXICO 87103

Ken Schultz Mayor UTILITY DEVELOPMENT DIVISION HYDROLOGY SECTION (505) 768-2650

May 4, 1987

Tom Mann, P.E. Tom Mann & Associates, Inc. 811 Dallas, NE Albuquerque, New Mexico 87110

RE: CONCEPTUAL GRADING & DRAINAGE PLAN FOR YALE

RESERVOIR (K-16/D28) ENGINEER'S STAMP DATED APRIL 24, 1987

Dear Tom;

Based on the information provided on your submittal of April 24, 1987, the above referenced conceptual drainage plan is approved for Site Development.

Please be advised that prior to Building Permit release, a submittal indicating Building Permit approval will be required.

If I can be of further assistance, please feel free to call me at 768-2650.

Cordially,

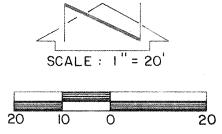
Bernie J. Mortoya, C.E. Engineering Assistant

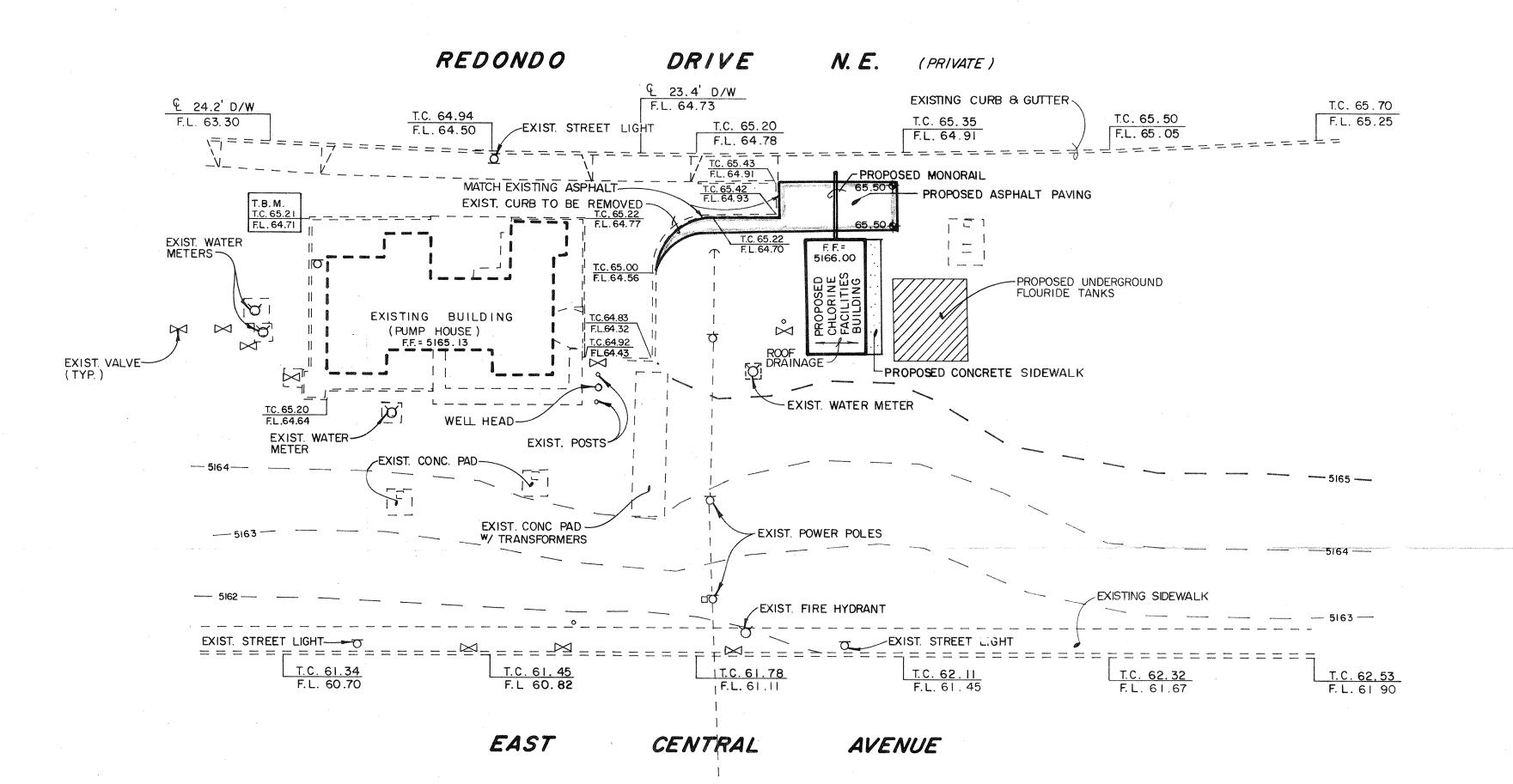
BJM/bsj

PUBLIC WORKS DEPARTMENT

ENGINEERING GROUP

Telephone (505) 768-2500





DRAINAGE PLAN

The following items concerning the Yale Well No. 3 Drainage Plan are contained hereon:

Vicinity Map
 Grading Plan
 Calculations

The proposed improvements, as shown by the Vicinity Map, are located on the south side of Redondo Drive N.E. and east of Yale Drive N.E. At present, the site is fully developed as a park and a pumping station.

As shown by Plate K-16 of the Albuquerque Master Drainage Study, the site does not lie within a designated Flood Hazard Zone. Further study of this plate reveals that downstream flooding does not exist. As shown by the grading plan, the site is surrounded by streets and is higher than the streets. Therefore, offsite flows are minimal.

The Grading Plan shows 1) existing and proposed grades indicated by spot elevations and contours at 1'0" intervals, 2) continuity between existing and proposed elevations, 3) the limit and character of the existing improvements, and 4) the limit and character of the proposed improvements. As shown by this plan, the proposed improvements consist of the construction of a chlorination building and underground fluoride tank system and additional asphalt paving. Stormwaters from this project will mainly be from the park paving area and the roof of the building. The waters will be allowed to drain through the park in a southerly manner. Flows will not be concentrated. The area of landscaping as compared to area of construction is quite large, therefore, additional runoff can be handled quite easily by the landscaped areas.

The Calculations which appear hereon analyze both the existing and developed conditions for the 100-year, 6-hour rainfall event. The Rational Method has been used to quantify the peak rate of discharge and the SCS Method has been used to quantify the volume of runoff. Both Methods have been used in accordance with the City of Albuquerque Development Process Manual, Volume II, and the Mayor's Emergency Rule adopted January 14, 1986. As shown by these calculations, the proposed improvements will result in a minimal increase in runoff discharged from the site. This pattern of runoff is consistent with the predesign conference recap which accompanies this submittal.

CALCULATIONS

Ground Cover Information

From SCS Bernalillo County Soil Survey, Plate 31: Cut and Fill - CU Hydrologic Soil Group: A

Existing Pervious CN = 39 (DPM Plate 22.2 C-2
Open spaces, park: good condition)
Developed Pervious CN = 39 (DPM Plate 22.2 C-2
Open spaces, park: good condition)

Time of Concentration/Time to Peak

 $T_C = 0.0078 L^{0.77}/S^{0.385}$ (Kirpich Equation) $T_D = T_C = 10 min.$

Point Rainfall

P₆ = 2.2 in. (DPM Plate 22.2 D-1)

Rational Method

Discharge: Q = CiA

where C varies $i = P_6 (6.84) T_C^{-0.51} = 4.65 in/hr$ $P_6 = 2.2 in (DPM Plate 22.2D-1)$ $T_C = 10 min (minimum)$ A = area, acres

SCS Method

Volume: V = 3630(DRO) A

Where DRO = Direct runoff in inches A = area, acres

Existing Condition

Atotal = 80,500 sf = 1.85 Ac
Roof area = 1540 sf (0.02)
Paved area = 2285 sf (0.03)
Landscaped area = 76,675 sf (0.95)
C = 0.29 (Weighted average per Emergency Rule, 1/14/86)
Qloo = CiA = 0.29(4.65)(1.85) = 2.5 cfs
Aimp = 3825 sf; % impervious = 05 %
Composite CN = 42 (DPM Plate 22.2 C-3)

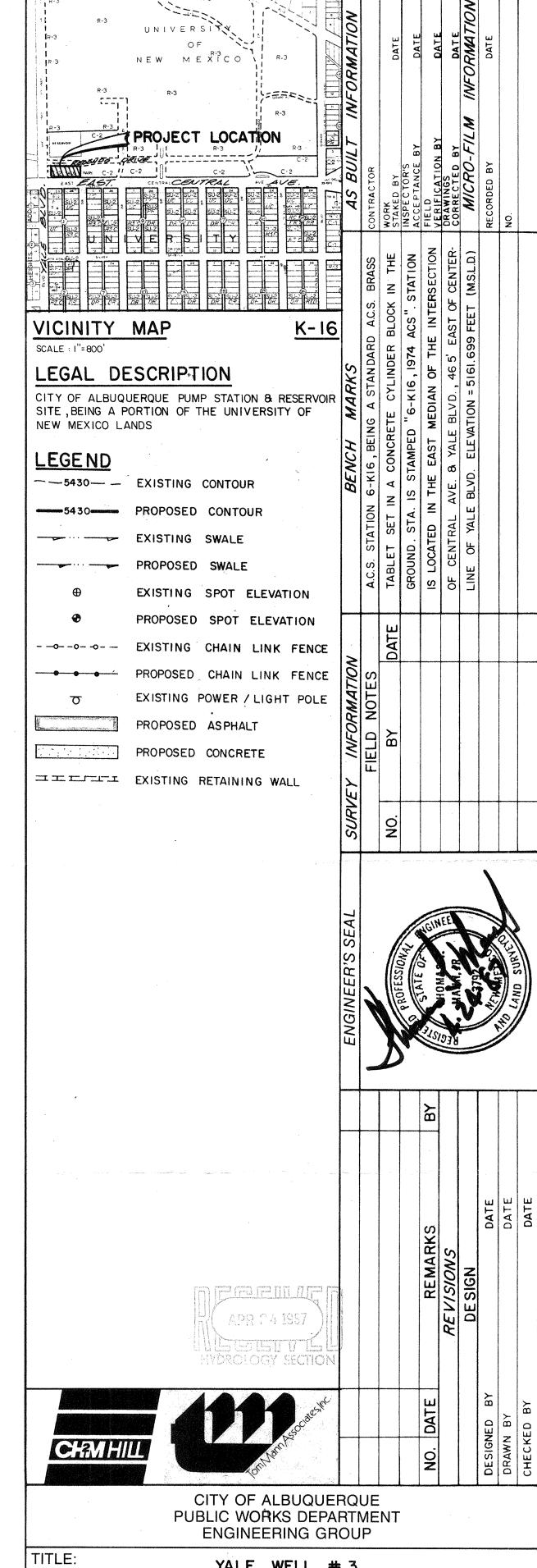
DRO = -0- in (DPM Plate 22.2 C-4) $V_{100} = 3630$ (DRO)A = -0- cf Developed Condition

Atotal = 80,500 sf = 1.85 Ac
Roof area = 2140 sf (0.03)
Paved area = 2785 sf (0.03)
Landscaped area = 75,575 sf (0.94)
C = 0.29 (Weighted average per Emergency Rule, 1/14/86)
Q100 = CiA = 0.29(4.65)(1.85) = 2.5 cfs
Aimp = 4925 sf; % impervious = 06 %
Composite CN = 43 (DPM Plate 22.2 C-3)
DRO = -0- in (DPM Plate 22.2 C-4)

Comparison

 $Q_{100} = 2.5 - 2.5 = 0$ cfs $V_{100} = 0 - 0 = 0$ cf

 $V_{100} = 3630$ (DRO) A = -0- cf



CONCEPTUAL GRADING AND DRAINAGE PLAN

APPROVALS ENGINEER DATE APPROVALS ENGINEER DATE

D.R.C. Chair

Trans. Dev.

Utility Dev.

DRAWING

MAP NO. SHEET OF

K-16

T.M.A. # 61333