

April 1, 1998

Stan Holland
Red Mountain Engineers
1216 Parkway Drive
Santa Fe, NM 87505

RE: CELLULAR ONE PARKING LOT IMPROVEMENTS. (K17-D81). GRADING AND DRAINAGE PLAN FOR PAVING AND SO #19 PERMIT APPROVALS. ENGINEER'S STAMP DATED FEBRUARY 2, 1998.

Dear Mr. Holland:

Based on the information provided on your March 10, 1998 submittal, the above referenced project is approved for Grading, Paving, and SO #19 Permits.

Please attach a copy of this approved plan to the construction sets prior to sign-off by Hydrology.

A separate permit is required for construction within City right-of-way. A copy of this approval letter must be on hand when applying for the excavation permit.

An Engineer's Certification will be required prior to Engineer's Certification.

If I can be of further assistance, please feel free to contact me at 924-3984.

Sincerely,

Lisa Ann Manwill, P.E.
Hydrology

c: Arlene Portillo
Andrew Garcia
File

Good for You, Albuquerque!



DRAINAGE INFORMATION SHEET

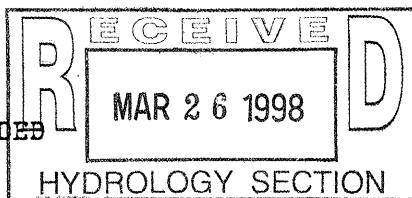
PROJECT TITLE: CELLULAR ONE - PARKING LOT IMPROVEMENTS ZONE ATLAS/DRNG. FILE #: K17-D81
 DRB #: NONE EPC #: NONE WORK ORDER #: _____
 LEGAL DESCRIPTION: LOTS 1, 2, 3 & 4 BLOCK 41 VALLEY VIEW ADDITION
 CITY ADDRESS: 145 QUINCY ST. N.E. ALB. NM
 ENGINEERING FIRM: RED MOUNTAIN ENGINEERS CONTACT: STAN HOLLAND
 ADDRESS: 1216 PARKWAY DR PHONE: 343-1517-DIRECT
SANTA FE NM 87505
 OWNER: CELLULAR ONE CONTACT: ERIC PETERSON / CONST. MAN.
 ADDRESS: 2125 E. ADAMS ST PHONE: 602-302-9876
PHOENIX, ARIZONA 85034
 ARCHITECT: NONE CONTACT: _____
 ADDRESS: _____ PHONE: _____
 SURVEYOR: RED MOUNTAIN ENGINEERS CONTACT: GREG STEINER
 ADDRESS: 4600-C MONTGOMERY BLVD NE SUITE 101 PHONE: 889-3004
 CONTRACTOR: _____ CONTACT: _____
 ADDRESS: _____ PHONE: _____

TYPE OF SUBMITTAL:

☐ DRAINAGE REPORT
☒ DRAINAGE PLAN
☐ CONCEPTUAL GRADING & DRAINAGE PLAN
☒ GRADING PLAN
☐ EROSION CONTROL PLAN
☐ ENGINEER'S CERTIFICATION
☐ OTHER _____

PRE-DESIGN MEETING:

☐ YES
☒ NO
☐ COPY PROVIDED



CHECK TYPE OF APPROVAL SOUGHT:

☐ SKETCH PLAT APPROVAL
☐ PRELIMINARY PLAT APPROVAL
☐ S. DEV. PLAN FOR SUB'D. APPROVAL
☐ S. DEV. PLAN FOR BLDG. PERMIT APPROVAL
☐ SECTOR PLAN APPROVAL
☐ FINAL PLAT APPROVAL
☐ FOUNDATION PERMIT APPROVAL
☐ BUILDING PERMIT APPROVAL
☐ CERTIFICATE OF OCCUPANCY APPROVAL
☒ GRADING PERMIT APPROVAL
☒ PAVING PERMIT APPROVAL
☐ S.A.D. DRAINAGE REPORT
☐ DRAINAGE REQUIREMENTS
☐ SUBDIVISION CERTIFICATION
☒ OTHER SO #19 (SPECIFY)

Approved & stamped 2-2-98

DATE SUBMITTED: _____

BY: _____



Drainage Analysis Report for
Existing Cellular One Facility
At 145 Quincy Street NE
Albuquerque, New Mexico

I Background

Red Mountain Engineers, Inc. was contracted by Cellular One to evaluate their existing facility at 145 Quincy Street NE, Albuquerque, N.M. for repairs to the existing parking lot with an entrance from Quincy Street. Due to expansion and relocation of equipment, numerous cuts and repairs have been made to the existing parking lot. As a result, the asphalt is failing in many areas and positive drainage from the lot is impaired.

After evaluation, it was decided to remove the existing asphalt, increase drainage slopes and install new base coarse and 3" of new asphalt. In conjunction with the new asphalt, the decision was made to use an underground detention system for storm water runoff control.

II Drainage Analysis

The overall site is 18,756 s.f. and was treated as all impervious area even though there are landscaped corridors on the site. The drainage area for the parking lot being replaced is:

8250 SF - Parking - Asphalt/Concrete Surface
1940 SF - Roof Area Drainage to Lot
10,190 SF Total Area

The 1994 version of AHYMO was used for the drainage analysis of the site. It was selected for its area specific application and ability to perform reservoir routing calculations. Two storm events were evaluated with input data as follows:

Input Data:

100 yr. - 24 hour storm event
P = 3.8 in
1 Hr. = 2.16 in
6 Hr. = 2.90 in
24 Hr. = 3.80 in
Area = 10,190 SF = 0.000365 sq. mi
Land Treatment = 100% Type D

Overhead?
25 yr. - 24 hour storm event
P = 3.2 in
1 Hr. = 1.4 in
6 Hr. = 2.3 in
24 Hr. = 3.20 in
Area = 10,190 SF = 0.000365 sq. mi
Land Treatment = 100% Type D

Analysis was performed with several combinations of pump sizes and storage volumes. Design criteria called for a storage volume and pump capacity large enough to handle the 100 year storm event without flooding the parking lot, and be able to handle smaller rain events and delay discharge for up to several hours. Print outs for these events are contained within this report as an Appendix.

III Final Design

A 60" diameter by 32 foot long RCP (Reinforced Concrete Pipe) was chosen with a total storage volume of 700 c.f. In addition to the tank storage, additional storage within the parking lot is available as "Ponding" before all storage capacity is exceeded and direct uncontrolled flow to the streets occurs. The amount of ponding within the parking lot is in excess of 2,000 c.f.

How much?

Two 1/2 H.P. sump pumps rated at approximately 90 gal./min (0.2 cfs) each will be used. In addition, float sensor controls will regulate on-off functions of the pumps individually. This means that there can be a lag time before discharging any storm water to the street. The two pumps will start alternately at the beginning of each start up cycle. This will extend the life of both pumps and in case one pump fails, there is still an operating pump available. All power to the pumps, etc. will be connected to the existing on-site generator.

Pumps will be located in a 48" dia. sump pit to minimize the amount of standing water. There will also be a manual control switch for the pumps to evacuate water from small rain events that do not activate the float sensors.

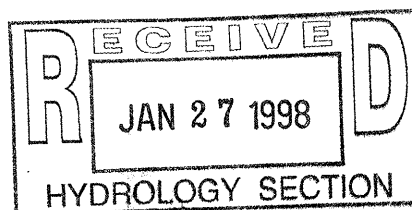
Analysis indicates that with both pumps running under the 100 yr. - 24 hour rainfall conditions and both starting with 12" of depth in the reservoir, reservoir capacity is not exceeded. Under conditions from the 25 yr. - 24 rainfall event and with one pump starting at 2.5 feet of depth and the second at 4.5 feet of depth, reservoir capacity is not exceeded. Also, there is a lag time of approximately 60 minutes from the beginning of the rain event until the first pump starts.

Ponding within the parking lot is anticipated under 100 yr. - 24 hour conditions with the pump activating floats set to accommodate the 25 yr. - 24 hour event. This condition would only exist for a short time.

Sincerely,



Stan Holland
Project Engineer



OUT

AHYMO PROGRAM (AHYMO194) - AMAFCA Hydrologic Model - January, 1994
RUN DATE (MON/DAY/YR) = 01/19/1998
START TIME (HR:MIN:SEC) = 07:57:44 USER NO.= RED_MTNM.194
INPUT FILE = CELL1-5.DAT

*CELLULAR ONE UNDERGROUND DETENTION SYSTEM

*25 YR. 24HR. EVENT P=3.2IN

*5' DIA. R.C.P. 32' LONG

*PUMP = 2-90GPM PUMPS

START TIME=0.0 CODE=0

RAINFALL TYPE=2 RAIN QT=0 RAIN ONE=1.4

RAIN SIX=2.3 RAIN DAY=3.2 DT=0.05

COMPUTED 24-HOUR RAINFALL DISTRIBUTION BASED ON NOAA ATLAS
2 - PEAK AT 1.40 HR.

DT =	.050000 HOURS	END TIME =	24.000000 HOURS			
.0000	.0220	.0445	.0676	.0914	.1158	.1409
.1668	.1935	.2212	.2497	.2794	.3102	.3422
.3757	.4107	.4474	.4861	.5270	.5704	.6168
.6227	.6292	.6481	.6928	.7695	.8883	1.0592
1.2930	1.4774	1.5607	1.6303	1.6912	1.7457	1.7950
1.8400	1.8813	1.9193	1.9544	1.9868	2.0168	2.0214
2.0259	2.0303	2.0347	2.0390	2.0433	2.0475	2.0516
2.0557	2.0598	2.0638	2.0678	2.0718	2.0757	2.0796
2.0835	2.0873	2.0911	2.0949	2.0987	2.1024	2.1062
2.1099	2.1135	2.1172	2.1209	2.1245	2.1281	2.1317
2.1353	2.1388	2.1424	2.1459	2.1494	2.1529	2.1564
2.1599	2.1633	2.1668	2.1702	2.1737	2.1771	2.1805
2.1839	2.1872	2.1906	2.1940	2.1973	2.2006	2.2040
2.2073	2.2106	2.2139	2.2172	2.2205	2.2237	2.2270
2.2303	2.2335	2.2368	2.2400	2.2432	2.2464	2.2496
2.2528	2.2560	2.2592	2.2624	2.2656	2.2687	2.2719
2.2750	2.2782	2.2813	2.2844	2.2876	2.2907	2.2938
2.2969	2.3000	2.3037	2.3073	2.3109	2.3146	2.3182

		OUT					
2.3218	2.3254	2.3289	2.3325	2.3361	2.3396	2.3431	
2.3467	2.3502	2.3537	2.3572	2.3606	2.3641	2.3676	
2.3710	2.3745	2.3779	2.3813	2.3847	2.3881	2.3915	
2.3949	2.3983	2.4016	2.4050	2.4083	2.4117	2.4150	
2.4183	2.4216	2.4249	2.4282	2.4315	2.4347	2.4380	
2.4413	2.4445	2.4477	2.4510	2.4542	2.4574	2.4606	
2.4638	2.4670	2.4701	2.4733	2.4765	2.4796	2.4828	
2.4859	2.4890	2.4921	2.4952	2.4983	2.5014	2.5045	
2.5076	2.5107	2.5137	2.5168	2.5198	2.5229	2.5259	
2.5289	2.5319	2.5350	2.5380	2.5410	2.5439	2.5469	
2.5499	2.5529	2.5558	2.5588	2.5617	2.5647	2.5676	
2.5705	2.5734	2.5763	2.5792	2.5821	2.5850	2.5879	
2.5908	2.5937	2.5965	2.5994	2.6022	2.6051	2.6079	
2.6107	2.6136	2.6164	2.6192	2.6220	2.6248	2.6276	
2.6304	2.6332	2.6359	2.6387	2.6415	2.6442	2.6470	
2.6497	2.6525	2.6552	2.6579	2.6606	2.6633	2.6661	
2.6688	2.6715	2.6741	2.6768	2.6795	2.6822	2.6849	
2.6875	2.6902	2.6928	2.6955	2.6981	2.7008	2.7034	
2.7060	2.7086	2.7113	2.7139	2.7165	2.7191	2.7217	
2.7242	2.7268	2.7294	2.7320	2.7345	2.7371	2.7397	
2.7422	2.7448	2.7473	2.7498	2.7524	2.7549	2.7574	
2.7599	2.7625	2.7650	2.7675	2.7700	2.7725	2.7750	
2.7774	2.7799	2.7824	2.7849	2.7873	2.7898	2.7922	
2.7947	2.7971	2.7996	2.8020	2.8045	2.8069	2.8093	
2.8117	2.8141	2.8165	2.8190	2.8214	2.8238	2.8261	
2.8285	2.8309	2.8333	2.8357	2.8381	2.8404	2.8428	
2.8451	2.8475	2.8498	2.8522	2.8545	2.8569	2.8592	
2.8615	2.8639	2.8662	2.8685	2.8708	2.8731	2.8754	
2.8777	2.8800	2.8823	2.8846	2.8869	2.8892	2.8915	
2.8937	2.8960	2.8983	2.9005	2.9028	2.9051	2.9073	
2.9096	2.9118	2.9140	2.9163	2.9185	2.9207	2.9230	
2.9252	2.9274	2.9296	2.9318	2.9340	2.9362	2.9384	
2.9406	2.9428	2.9450	2.9472	2.9494	2.9516	2.9537	
2.9559	2.9581	2.9602	2.9624	2.9646	2.9667	2.9689	
2.9710	2.9732	2.9753	2.9774	2.9796	2.9817	2.9838	
2.9860	2.9881	2.9902	2.9923	2.9944	2.9965	2.9986	
3.0007	3.0028	3.0049	3.0070	3.0091	3.0112	3.0133	
3.0154	3.0174	3.0195	3.0216	3.0236	3.0257	3.0278	
3.0298	3.0319	3.0339	3.0360	3.0380	3.0401	3.0421	
3.0441	3.0462	3.0482	3.0502	3.0523	3.0543	3.0563	
3.0583	3.0603	3.0623	3.0644	3.0664	3.0684	3.0704	
3.0724	3.0743	3.0763	3.0783	3.0803	3.0823	3.0843	
3.0862	3.0882	3.0902	3.0922	3.0941	3.0961	3.0980	
3.1000	3.1019	3.1039	3.1058	3.1078	3.1097	3.1117	
3.1136	3.1156	3.1175	3.1194	3.1213	3.1233	3.1252	

		OUT					
3.1271	3.1290	3.1309	3.1328	3.1348	3.1367	3.1386	
3.1405	3.1424	3.1443	3.1462	3.1480	3.1499	3.1518	
3.1537	3.1556	3.1575	3.1593	3.1612	3.1631	3.1650	
3.1668	3.1687	3.1705	3.1724	3.1743	3.1761	3.1780	
3.1798	3.1817	3.1835	3.1853	3.1872	3.1890	3.1909	
3.1927	3.1945	3.1964	3.1982	3.2000			

COMPUTE NM HYD ID=1 HYD NO=101 AREA=0.00036551596

PER A=0 PER B=0 PER C=0 PER D=100

TP=-0.133333 MASS RAIN=-1

K = .072666HR TP = .133333HR K/TP RATIO = .545000 SHA
 PE CONSTANT, N = 7.106420
 UNIT PEAK = 1.4427 CFS UNIT VOLUME = .9911 B = 526.
 28 P60 = 1.4000
 AREA = .000366 SQ MI IA = .10000 INCHES INF = .04000
 INCHES PER HOUR
 RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMBER METHOD - D
 T = .050000

ROUTE RESERVOIR ID=2 HYD NO=102 INFLOW=1 CODE=1

OUTFLOW	STORAGE	ELEV
0.0	0	0
0.01	0.00075	0.5
0.011	0.00231	1.0
0.012	0.00364	1.5
0.013	0.00539	2.0
0.20004	0.00721	2.5
0.20005	0.00904	3.0
0.20006	0.01078	3.5
0.20007	0.01237	4.0

	OUT	
0.40103	0.01367	4.5
0.40104	0.01442	5.0
0.40104	0.01499	5.5
0.20011	0.01510	6.0

* * * * *

TIME (HRS)	INFLOW (CFS)	ELEV (FEET)	VOLUME (AC-FT)	OUTFLOW (CFS)
.00	.00	.00	.000	.00
.05	.00	.00	.000	.00
.10	.00	.00	.000	.00
.15	.00	.00	.000	.00
.20	.00	.00	.000	.00
.25	.00	.00	.000	.00
.30	.02	.03	.000	.00
.35	.05	.13	.000	.00
.40	.08	.29	.000	.01
.45	.09	.50	.001	.01
.50	.10	.61	.001	.01
.55	.11	.74	.001	.01
.60	.11	.87	.002	.01
.65	.12	1.01	.002	.01
.70	.13	1.19	.003	.01
.75	.13	1.37	.003	.01
.80	.14	1.55	.004	.01
.85	.15	1.71	.004	.01
.90	.16	1.87	.005	.01
.95	.17	2.04	.006	.03
1.00	.18	2.17	.006	.08
1.05	.18	2.27	.006	.11
1.10	.14	2.31	.007	.13

			OUT	
1.15	.09	2.30	.006	.13
1.20	.08	2.26	.006	.11
1.25	.12	2.25	.006	.11
1.30	.20	2.30	.006	.13
1.35	.33	2.43	.007	.18
1.40	.49	2.68	.008	.20
1.45	.68	3.13	.009	.20
1.50	.77	3.77	.012	.20
1.55	.66	4.41	.013	.37
1.60	.51	4.91	.014	.40
1.65	.41	5.50	.015	.40
1.70	.34	5.00	.014	.40
1.75	.30	4.78	.014	.40
1.80	.27	4.48	.014	.39
1.85	.25	4.32	.013	.33
1.90	.22	4.21	.013	.28
1.95	.20	4.12	.013	.25
2.00	.19	4.06	.013	.22
2.05	.17	4.00	.012	.20
2.10	.13	3.93	.012	.20
2.15	.08	3.81	.012	.20
2.20	.06	3.64	.011	.20
2.25	.04	3.45	.011	.20
2.30	.03	3.25	.010	.20
2.35	.03	3.05	.009	.20
2.40	.02	2.85	.008	.20
2.45	.02	2.65	.008	.20
2.50	.02	2.45	.007	.18
2.55	.01	2.30	.006	.12
2.60	.01	2.19	.006	.09
2.65	.01	2.13	.006	.06
2.70	.01	2.08	.006	.04
2.75	.01	2.05	.006	.03
TIME (HRS)	INFLOW (CFS)	ELEV (FEET)	VOLUME (AC-FT)	OUTFLOW (CFS)
2.80	.01	2.03	.005	.02
2.85	.01	2.02	.005	.02
2.90	.01	2.01	.005	.02
2.95	.01	2.00	.005	.01
3.00	.01	2.00	.005	.01
3.05	.01	1.99	.005	.01
3.10	.01	1.99	.005	.01
3.15	.01	1.98	.005	.01

			OUT	
3.20	.01	1.98	.005	.01
3.25	.01	1.97	.005	.01
3.30	.01	1.97	.005	.01
3.35	.01	1.96	.005	.01
3.40	.01	1.96	.005	.01
3.45	.01	1.95	.005	.01
3.50	.01	1.95	.005	.01
3.55	.01	1.94	.005	.01
3.60	.01	1.94	.005	.01
3.65	.01	1.93	.005	.01
3.70	.01	1.93	.005	.01
3.75	.01	1.92	.005	.01
3.80	.01	1.92	.005	.01
3.85	.01	1.92	.005	.01
3.90	.01	1.91	.005	.01
3.95	.01	1.91	.005	.01
4.00	.01	1.90	.005	.01
4.05	.01	1.90	.005	.01
4.10	.01	1.90	.005	.01
4.15	.01	1.89	.005	.01
4.20	.01	1.89	.005	.01
4.25	.01	1.89	.005	.01
4.30	.01	1.89	.005	.01
4.35	.01	1.88	.005	.01
4.40	.01	1.88	.005	.01
4.45	.01	1.88	.005	.01
4.50	.01	1.88	.005	.01
4.55	.01	1.87	.005	.01
4.60	.01	1.87	.005	.01
4.65	.01	1.87	.005	.01
4.70	.01	1.87	.005	.01
4.75	.01	1.87	.005	.01
4.80	.01	1.86	.005	.01
4.85	.01	1.86	.005	.01
4.90	.01	1.86	.005	.01
4.95	.01	1.86	.005	.01
5.00	.01	1.86	.005	.01
5.05	.01	1.86	.005	.01
5.10	.01	1.86	.005	.01
5.15	.01	1.86	.005	.01
5.20	.01	1.86	.005	.01
5.25	.01	1.86	.005	.01
5.30	.01	1.86	.005	.01
5.35	.01	1.86	.005	.01
5.40	.01	1.86	.005	.01

			OUT	
26.35	.00	.66	.001	.01
26.40	.00	.65	.001	.01
26.45	.00	.63	.001	.01
26.50	.00	.62	.001	.01
26.55	.00	.60	.001	.01
26.60	.00	.59	.001	.01
26.65	.00	.58	.001	.01
26.70	.00	.56	.001	.01
26.75	.00	.55	.001	.01
26.80	.00	.54	.001	.01
26.85	.00	.52	.001	.01
26.90	.00	.51	.001	.01
26.95	.00	.50	.001	.01
27.00	.00	.47	.001	.01
27.05	.00	.44	.001	.01
27.10	.00	.42	.001	.01
27.15	.00	.40	.001	.01
27.20	.00	.38	.001	.01
27.25	.00	.36	.001	.01
27.30	.00	.34	.001	.01
27.35	.00	.32	.000	.01
27.40	.00	.30	.000	.01
27.45	.00	.29	.000	.01
27.50	.00	.27	.000	.01
27.55	.00	.26	.000	.01
27.60	.00	.24	.000	.00

WARNING - OUTFLOW EXCEEDS RESERVOIR CAPACITY

PEAK DISCHARGE = .401 CFS - PEAK OCCURS AT HOUR 1.65

MAXIMUM WATER SURFACE ELEVATION = 5.500

MAXIMUM STORAGE = .0150 AC-FT INCREMENTAL TIME= .050000

HRS

PRINT HYD

ID=1 CODE=1

PARTIAL HYDROGRAPH 101.00

RUNOFF VOLUME = 2.93134 INCHES = .0571 ACRE-FEET

PEAK DISCHARGE RATE = .77 CFS AT 1.500 HOURS BASIN AREA =
.0004 SQ. MI.

PRINT HYD

ID=2 CODE=1

OUT

PARTIAL HYDROGRAPH 102.00

RUNOFF VOLUME = 2.92932 INCHES = .0571 ACRE-FeET
PEAK DISCHARGE RATE = .40 CFS AT 1.650 HOURS BASIN AREA =
.0004 SQ. MI.

FINISH

NORMAL PROGRAM FINISH

END TIME (HR:MIN:SEC) = 07:57:44

OUT

AHYMO PROGRAM (AHYMO194) - AMAFCA Hydrologic Model - January, 1994

RUN DATE (MON/DAY/YR) = 01/26/1998

START TIME (HR:MIN:SEC) = 15:18:55

USER NO.= RE

D_MTNM.194

INPUT FILE = CELL1-4.DAT

*CELLULAR ONE UNDERGROUND DETENTION SYSTEM

*100 YR. 24HR. EVENT P=3.8IN

*5' DIA. R.C.P. 32' LONG

*PUMP = 2-90GPM PUMPS

START TIME=0.0 CODE=0

RAINFALL TYPE=2 RAIN QT=0 RAIN ONE=2.16

RAIN SIX=2.9 RAIN DAY=3.8 DT=0.05

COMPUTED 24-HOUR RAINFALL DISTRIBUTION BASED
ON NOAA ATLAS 2 - PEAK AT 1.40 HR.

DT = .050000 HOURS		END TIME = 24.					
000000 HOURS		.0000	.0103	.0208	.0317	.0429	.0
545	.0665	.0789	.0918	.1052	.1191	.1336	.1
487	.1646	.1813	.1989	.2174	.2372	.2582	.2
808	.3051	.3142	.3243	.3535	.4223	.5408	.7
240	.9877	1.3484	1.6329	1.7614	1.8687	1.9627	2.0
468	2.1229	2.1923	2.2561	2.3147	2.3688	2.4188	2.4
651	2.4766	2.4875	2.4978	2.5076	2.5171	2.5262	2.5
350	2.5434	2.5516	2.5596	2.5674	2.5749	2.5822	2.5

		OUT						
894	2.5964							
		2.6033	2.6100	2.6166	2.6231	2.6294	2.6	
356	2.6417							
		2.6478	2.6537	2.6595	2.6652	2.6709	2.6	
764	2.6819							
		2.6873	2.6927	2.6980	2.7032	2.7083	2.7	
134	2.7184							
		2.7234	2.7283	2.7332	2.7380	2.7427	2.7	
474	2.7521							
		2.7567	2.7613	2.7658	2.7703	2.7747	2.7	
791	2.7835							
		2.7878	2.7921	2.7963	2.8005	2.8047	2.8	
089	2.8130							
		2.8170	2.8211	2.8251	2.8291	2.8331	2.8	
370	2.8409							
		2.8448	2.8486	2.8525	2.8563	2.8600	2.8	
638	2.8675							
		2.8712	2.8749	2.8785	2.8822	2.8858	2.8	
894	2.8929							
		2.8965	2.9000	2.9037	2.9075	2.9112	2.9	
149	2.9185							
		2.9222	2.9259	2.9295	2.9331	2.9368	2.9	
404	2.9440							
		2.9476	2.9511	2.9547	2.9583	2.9618	2.9	
653	2.9689							
		2.9724	2.9759	2.9794	2.9828	2.9863	2.9	
898	2.9932							
		2.9966	3.0001	3.0035	3.0069	3.0103	3.0	
137	3.0170							
		3.0204	3.0238	3.0271	3.0304	3.0338	3.0	
371	3.0404							
		3.0437	3.0470	3.0502	3.0535	3.0568	3.0	
600	3.0633							
		3.0665	3.0697	3.0729	3.0761	3.0793	3.0	
825	3.0857							
		3.0889	3.0920	3.0952	3.0983	3.1015	3.1	
046	3.1077							
		3.1108	3.1139	3.1170	3.1201	3.1232	3.1	
263	3.1293							
		3.1324	3.1354	3.1385	3.1415	3.1445	3.1	
475	3.1505							
		3.1535	3.1565	3.1595	3.1625	3.1655	3.1	
684	3.1714							
		3.1743	3.1773	3.1802	3.1831	3.1860	3.1	
889	3.1918							

			OUT				
091	3.2120	3.1947	3.1976	3.2005	3.2034	3.2063	3.2
		3.2148	3.2177	3.2205	3.2233	3.2261	3.2
290	3.2318	3.2346	3.2374	3.2401	3.2429	3.2457	3.2
485	3.2512	3.2540	3.2567	3.2595	3.2622	3.2649	3.2
677	3.2704	3.2731	3.2758	3.2785	3.2812	3.2839	3.2
866	3.2892	3.2919	3.2946	3.2972	3.2999	3.3025	3.3
052	3.3078	3.3104	3.3131	3.3157	3.3183	3.3209	3.3
235	3.3261	3.3287	3.3313	3.3339	3.3364	3.3390	3.3
416	3.3441	3.3467	3.3492	3.3518	3.3543	3.3568	3.3
594	3.3619	3.3644	3.3669	3.3694	3.3719	3.3744	3.3
769	3.3794	3.3819	3.3843	3.3868	3.3893	3.3917	3.3
942	3.3966	3.3991	3.4015	3.4040	3.4064	3.4088	3.4
112	3.4137	3.4161	3.4185	3.4209	3.4233	3.4257	3.4
281	3.4304	3.4328	3.4352	3.4376	3.4399	3.4423	3.4
447	3.4470	3.4494	3.4517	3.4540	3.4564	3.4587	3.4
610	3.4634	3.4657	3.4680	3.4703	3.4726	3.4749	3.4
772	3.4795	3.4818	3.4841	3.4863	3.4886	3.4909	3.4
932	3.4954	3.4977	3.4999	3.5022	3.5044	3.5067	3.5
089	3.5112	3.5134	3.5156	3.5178	3.5201	3.5223	3.5
245	3.5267	3.5289	3.5311	3.5333	3.5355	3.5377	3.5
399	3.5420	3.5442	3.5464	3.5486	3.5507	3.5529	3.5
551	3.5572	3.5594	3.5615	3.5637	3.5658	3.5679	3.5
701	3.5722	3.5743	3.5764	3.5786	3.5807	3.5828	3.5

		OUT					
849	3.5870						
		3.5891	3.5912	3.5933	3.5954	3.5975	3.5
996	3.6017						
		3.6037	3.6058	3.6079	3.6100	3.6120	3.6
141	3.6161						
		3.6182	3.6202	3.6223	3.6243	3.6264	3.6
284	3.6305						
		3.6325	3.6345	3.6365	3.6386	3.6406	3.6
426	3.6446						
		3.6466	3.6486	3.6506	3.6526	3.6546	3.6
566	3.6586						
		3.6606	3.6626	3.6646	3.6666	3.6685	3.6
705	3.6725						
		3.6745	3.6764	3.6784	3.6803	3.6823	3.6
842	3.6862						
		3.6881	3.6901	3.6920	3.6940	3.6959	3.6
978	3.6998						
		3.7017	3.7036	3.7055	3.7075	3.7094	3.7
113	3.7132						
		3.7151	3.7170	3.7189	3.7208	3.7227	3.7
246	3.7265						
		3.7284	3.7303	3.7322	3.7340	3.7359	3.7
378	3.7397						
		3.7415	3.7434	3.7453	3.7471	3.7490	3.7
508	3.7527						
		3.7545	3.7564	3.7582	3.7601	3.7619	3.7
638	3.7656						
		3.7674	3.7693	3.7711	3.7729	3.7747	3.7
766	3.7784						
		3.7802	3.7820	3.7838	3.7856	3.7874	3.7
892	3.7910						
		3.7928	3.7946	3.7964	3.7982	3.8000	

COMPUTE NM HYD

ID=1 HYD NO=101 AREA=0.00036551596

PER A=0 PER B=0 PER C=0 PER D=100

TP=-0.133333 MASS RAIN=-1

	K =	.072666HR	TP =	.133333HR	K/TP RATIO =	.5
45000	SHAPE CONSTANT, N =	7.106420				
	UNIT PEAK =	1.4427	CFS	UNIT VOLUME =	.9911	
B =	526.28	P60 =	2.1600			
	AREA =	.000366	SQ MI	IA =	.10000	INCHES IN

OUT
 F = .04000 INCHES PER HOUR
 RUNOFF COMPUTED BY INITIAL ABSTRACTION/INFILTRATION NUMB
 ER METHOD - DT = .050000

ROUTE RESERVOIR ID=2 HYD NO=102 INFLOW=1 CODE=1

OUTFLOW	STORAGE	ELEV
0	0	0
0.401040	0.00075	0.5
0.401041	0.00231	1.0
0.401042	0.00364	1.5
0.401043	0.00539	2.0
0.401044	0.00721	2.5
0.401045	0.00904	3.0
0.401046	0.01078	3.5
0.401047	0.01237	4.0
0.401048	0.01367	4.5
0.401049	0.01442	5.0

* * * * *

TIME (HRS)	INFLOW (CFS)	ELEV (FEET)	VOLUME (AC-FT)	OUTFLOW (CFS)
.00	.00	.00	.000	.00
.05	.00	.00	.000	.00
.10	.00	.00	.000	.00
.15	.00	.00	.000	.00
.20	.00	.00	.000	.00

		OUT		
.25	.00	.00	.000	.00
.30	.00	.00	.000	.00
.35	.00	.00	.000	.00
.40	.00	.00	.000	.00
.45	.00	.00	.000	.00
.50	.01	.01	.000	.00
.55	.02	.02	.000	.02
.60	.04	.04	.000	.03
.65	.05	.05	.000	.04
.70	.05	.06	.000	.05
.75	.06	.07	.000	.06
.80	.06	.08	.000	.06
.85	.07	.08	.000	.07
.90	.07	.09	.000	.07
.95	.08	.09	.000	.08
1.00	.08	.10	.000	.08
1.05	.09	.11	.000	.09
1.10	.08	.10	.000	.08
1.15	.06	.09	.000	.07
1.20	.08	.09	.000	.07
1.25	.16	.15	.000	.12
1.30	.30	.29	.000	.23
1.35	.49	.50	.001	.40
1.40	.75	.79	.002	.40
1.45	1.05	1.53	.004	.40
1.50	1.18	2.35	.007	.40
1.55	1.02	3.15	.010	.40
1.60	.79	3.77	.012	.40
1.65	.63	4.21	.013	.40
1.70	.53	4.50	.014	.40
1.75	.47	4.76	.014	.40
1.80	.42	4.88	.014	.40
1.85	.38	4.88	.014	.40
1.90	.35	4.79	.014	.40
1.95	.32	4.62	.014	.40
2.00	.29	4.41	.013	.40
2.05	.26	4.22	.013	.40
2.10	.21	3.97	.012	.40
2.15	.14	3.67	.011	.40
2.20	.11	3.32	.010	.40
2.25	.08	2.97	.009	.40
2.30	.07	2.61	.008	.40
2.35	.06	2.22	.006	.40
2.40	.05	1.82	.005	.40
2.45	.05	1.37	.003	.40

		OUT		
2.50	.04	.85	.002	.40
2.55	.04	.36	.001	.29
2.60	.03	.03	.000	.02
2.65	.03	.04	.000	.03
2.70	.03	.04	.000	.03
2.75	.03	.04	.000	.03

TIME (HRS)	INFLOW (CFS)	ELEV (FEET)	VOLUME (AC-FT)	OUTFLOW (CFS)
2.80	.03	.03	.000	.03
2.85	.03	.03	.000	.03
2.90	.02	.03	.000	.02
2.95	.02	.03	.000	.02
3.00	.02	.03	.000	.02
3.05	.02	.03	.000	.02
3.10	.02	.03	.000	.02
3.15	.02	.03	.000	.02
3.20	.02	.03	.000	.02
3.25	.02	.03	.000	.02
3.30	.02	.02	.000	.02
3.35	.02	.02	.000	.02
3.40	.02	.02	.000	.02
3.45	.02	.02	.000	.02
3.50	.02	.02	.000	.02
3.55	.02	.02	.000	.02
3.60	.02	.02	.000	.02
3.65	.02	.02	.000	.02
3.70	.02	.02	.000	.02
3.75	.02	.02	.000	.02
3.80	.02	.02	.000	.02
3.85	.02	.02	.000	.02
3.90	.02	.02	.000	.02
3.95	.02	.02	.000	.02
4.00	.02	.02	.000	.02
4.05	.02	.02	.000	.02
4.10	.02	.02	.000	.02
4.15	.02	.02	.000	.02
4.20	.02	.02	.000	.02
4.25	.02	.02	.000	.02
4.30	.02	.02	.000	.02
4.35	.02	.02	.000	.02
4.40	.02	.02	.000	.02
4.45	.02	.02	.000	.02
4.50	.02	.02	.000	.02

		OUT		
23.40	.01	.01	.000	.01
23.45	.01	.01	.000	.01
23.50	.01	.01	.000	.01
23.55	.01	.01	.000	.01
23.60	.01	.01	.000	.01
23.65	.01	.01	.000	.01
23.70	.01	.01	.000	.01
23.75	.01	.01	.000	.01
23.80	.01	.01	.000	.01
23.85	.01	.01	.000	.01
23.90	.01	.01	.000	.01
23.95	.01	.01	.000	.01
24.00	.01	.01	.000	.01
24.05	.01	.01	.000	.01
24.10	.01	.01	.000	.01
24.15	.00	.01	.000	.00

PEAK DISCHARGE = .401 CFS - PEAK OCCURS AT HOUR
 1.80
 MAXIMUM WATER SURFACE ELEVATION = 4.882
 MAXIMUM STORAGE = .0142 AC-FT INCREMENTAL TIME
 = .050000HRS

PRINT HYD ID=1 CODE=1

PARTIAL HYDROGRAPH 101
 .00

RUNOFF VOLUME = 3.53986 INCHES = .0690 AC
 RE-FEET
 PEAK DISCHARGE RATE = 1.18 CFS AT 1.500 HOURS
 BASIN AREA = .0004 SQ. MI.

PRINT HYD ID=2 CODE=1

PARTIAL HYDROGRAPH 102
 .00

RUNOFF VOLUME = 3.53921 INCHES = .0690 AC
 RE-FEET

PEAK DISCHARGE RATE = OUT .40 CFS AT 1.800 HOURS
BASIN AREA = .0004 SQ. MI.

FINISH

5:18:56 NORMAL PROGRAM FINISH END TIME (HR:MIN:SEC) = 1

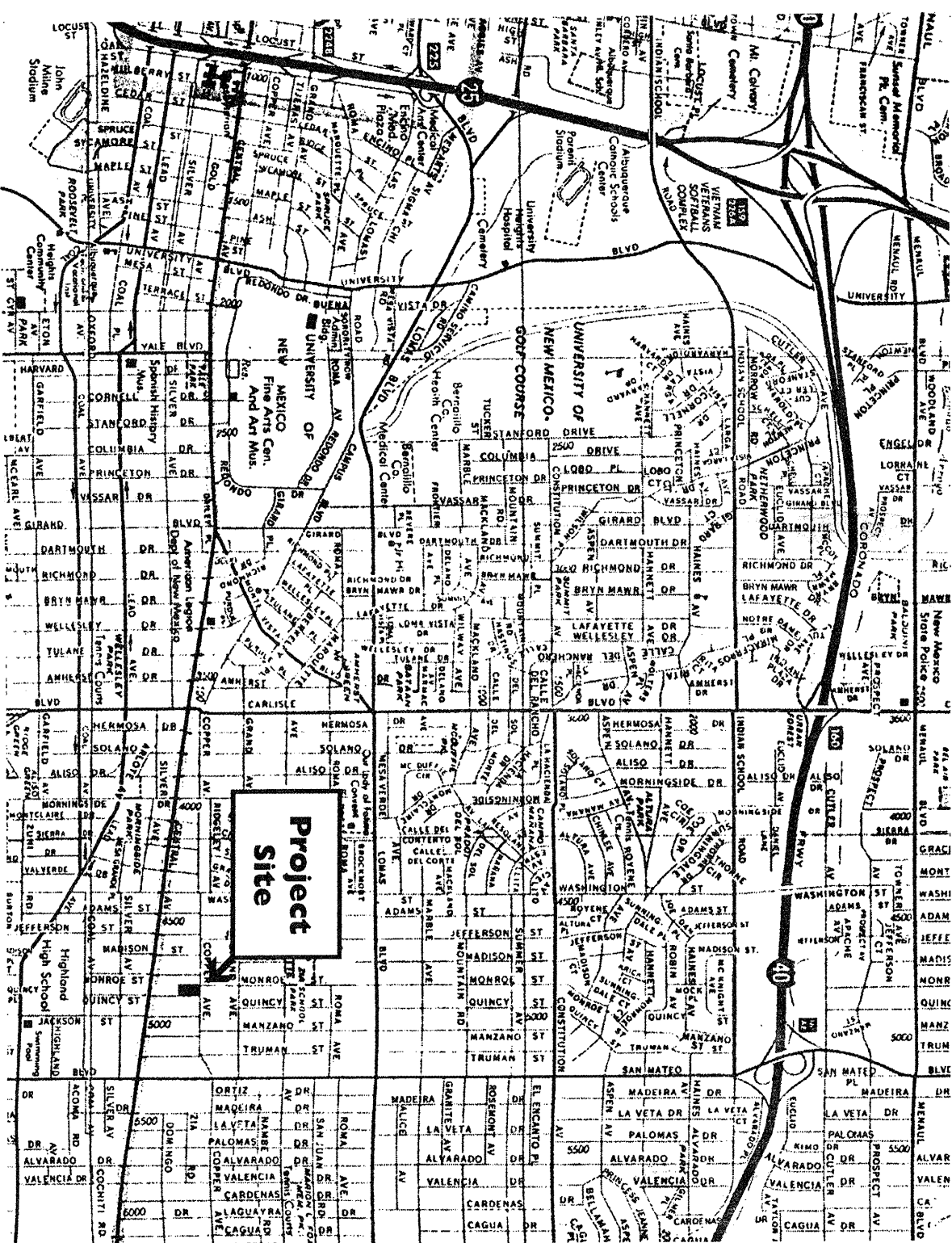
CONSTRUCTION PLANS

for

CELLULARONE OFFICE

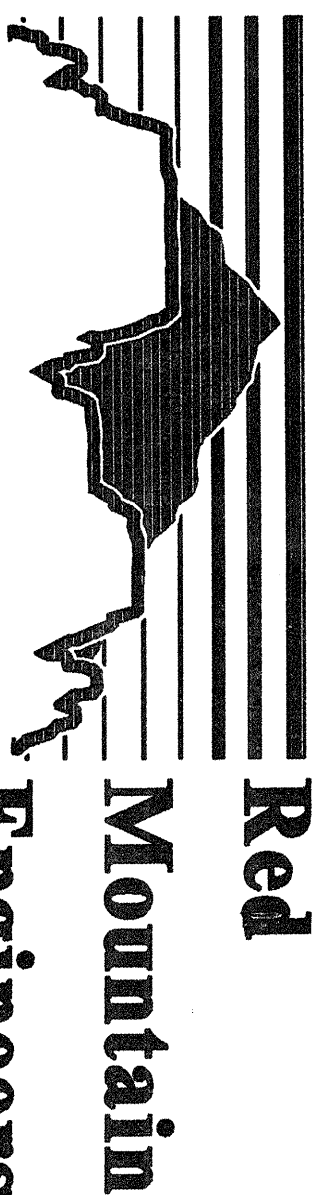
PARKING AREA IMPROVEMENTS

Albuquerque, New Mexico



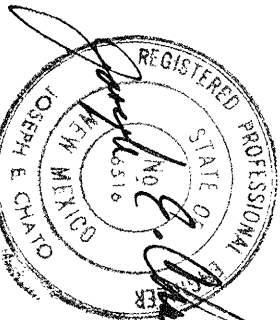
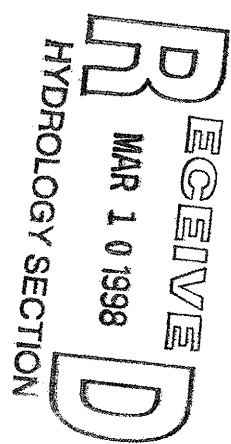
VICINITY MAP

SHEET	DESCRIPTION
C-1	COVER SHEET
C-2	TOPOGRAPHIC SURVEY
C-3	GRADING and DRAINAGE PLAN / CIVIL DETAILS
C-4	DETENTION SYSTEM DETAILS, STRIPING PLAN and GENERAL/CONSTRUCTION NOTES
E-1	ELECTRICAL PLAN



Engineers • Surveyors
1216 Parkway Drive
P.O. Box 16115 87506-6115
Santa Fe, NM 473-7373
Phone: (505) 473-7373

4600 Montgomery Blvd., N.E.
Building C, Suite 7101
Albuquerque, NM 87109
Phone: (505) 888-5004



SHEET No.

C-1

TOPOGRAPHIC SURVEY
LOTS 1, 2, 3 AND 4, BLOCK 41
VALLEY VIEW ADDITION
CITY OF ALBUQUERQUE
BERNALILLO COUNTY
NEW MEXICO

NOTES

- 1) BASIS OF BEARINGS IS FROM A SITE PLAN PREPARED BY LEEDSHILL-HENKINOFF INC., AND DATED OCTOBER 1994.
- 2) ELEVATION DATUM IS BASED UPON ALBUQUERQUE CITY SURVEY MONUMENT "G-K18A" ELEVATION (FEET/MSL) = 5247.33
- 3) IMPROVEMENTS SHOWN ARE CURRENT AS OF NOV. 17, 1997, DATE OF SURVEY.
- 4) LOCATION OF UNDERGROUND UTILITIES AND EASEMENTS NOT VERIFIED BY THIS PLAN. UNDERGROUND UTILITIES MAY EXIST WHERE NONE ARE SHOWN. BY OTHERS AND MAY BE INCOMPLETE OR OBSOLETE AT THE TIME OF SURVEY. THIS PLAN IS BASED UPON INFORMATION PROVIDED BY THE OWNER. THE SURVEYOR HAS NO CONTROL OVER THE LOCATION OF ANY UTILITY LINE OR PIPELINE, OR UNDERGROUND UTILITY LINE IN OR NEAR THE AREA OF THE WORK. IN ADVANCE OF AND DURING EXCAVATION WORK, THE CONTRACTOR IS TO LOCATE, RESPOND, AND PRESERVE ANY AND ALL EXISTING UTILITIES, PIPELINES AND UNDERGROUND UTILITY LINES. THE CONTRACTOR IS URGED TO COMPLY WITH STATE STATUTES, MUNICIPAL AND LOCAL ORDINANCES, AND ANY OTHER APPLICABLE REGULATIONS CONCERNING THE LOCATION OF THESE LINES, IN PLANNING AND CONDUCTING EXCAVATION.

LEGEND

- DENOTES FOUND POINT (AS NOTED)
- DENOTES PROPERTY CORNER (POINT NOT FOUND)
- FO— DENOTES UNDERGROUND FIBER OPTICS LINE
- E— DENOTES UTILITY POLE, OVERHEAD LINES AND ANCHOR GUY
- E— DENOTES UNDERGROUND ELECTRIC LINE
- X— DENOTES EXISTING FENCE
- S— DENOTES SANITARY SINKER LINE
- G— DENOTES UNDERGROUND GAS LINE
- T— DENOTES UNDERGROUND TELEPHONE LINE
- DENOTES TELEPHONE PEDESTAL
- ⊕ DENOTES PARKING LOT LIGHT W/ 18" DIA. CONCRETE BASE
- ⊙ DENOTES VEGETATION TYPE GROUND COVER
- ⊙ DENOTES JUNIPER SHRUB
- ⊙ DENOTES DECIDUOUS TREE W/APPROXIMATE DAPLINE
- ⊙ DENOTES PINE TREE W/APPROXIMATE DAPLINE
- DENOTES CONCRETE SLAB/SIDEWALK/CURB AND GUTTER
- DENOTES EDGE OF ASPHALT PAVING
- ⊕ DENOTES SANITARY SEWER CLEANOUT
- DENOTES SPOT ELEVATIONS FROM MARCH 1996 SURVEY SEE ABBREVIATIONS BELOW
- DENOTES SPOT ELEVATIONS FROM NOV. 1997 SURVEY SEE ABBREVIATIONS BELOW

ABBREVIATIONS

- FL FLOW LINE
- TTA TOP OF BUILDING
- TTA TOP OF CURB
- TSM TOP OF SLAB
- TML TOP OF WALL

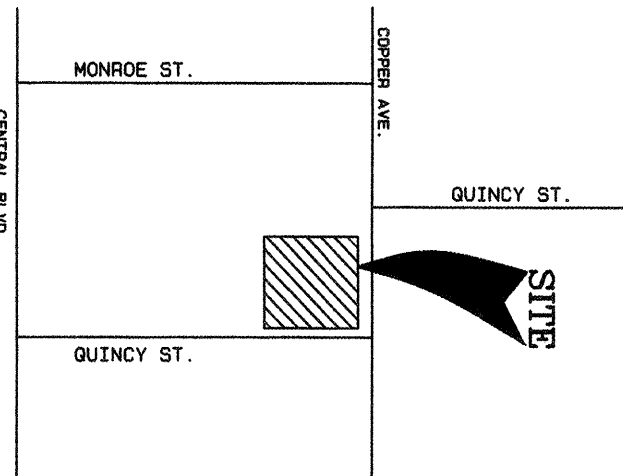
SHEET
C-2

THIS IS NOT A BOUNDARY SURVEY
APPARENT LOT LINES AND PROPERTY CORNERS
ARE SHOWN FOR ORIENTATION ONLY

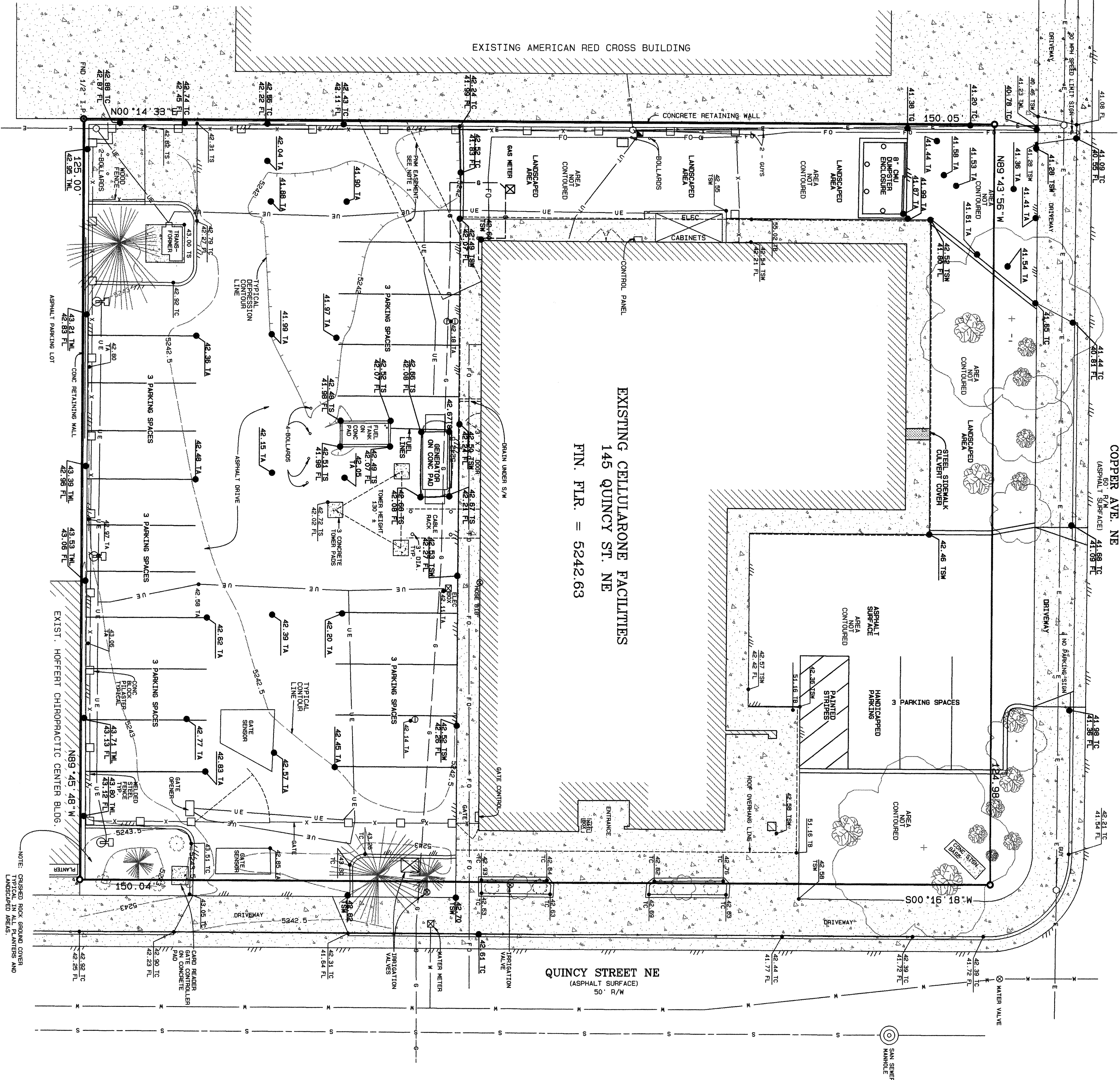
Red Mountain
Engineers, Inc.

4600-C Montgomery Blvd. NE
Albuquerque, NM 87109
Phone: (505) 889-3004

NAME: CELLARONE, ALBUQUERQUE OFFICE
SCALE: 1" = 10'
CHECKED BY: G.L.S.
PROJECT NO.: 97813



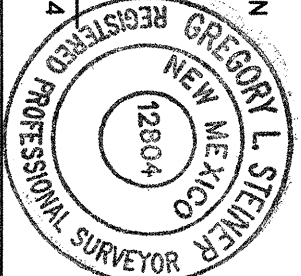
EXISTING CELLARONE FACILITIES
145 QUINCY ST. NE
FIN. FLR. = 5242.63

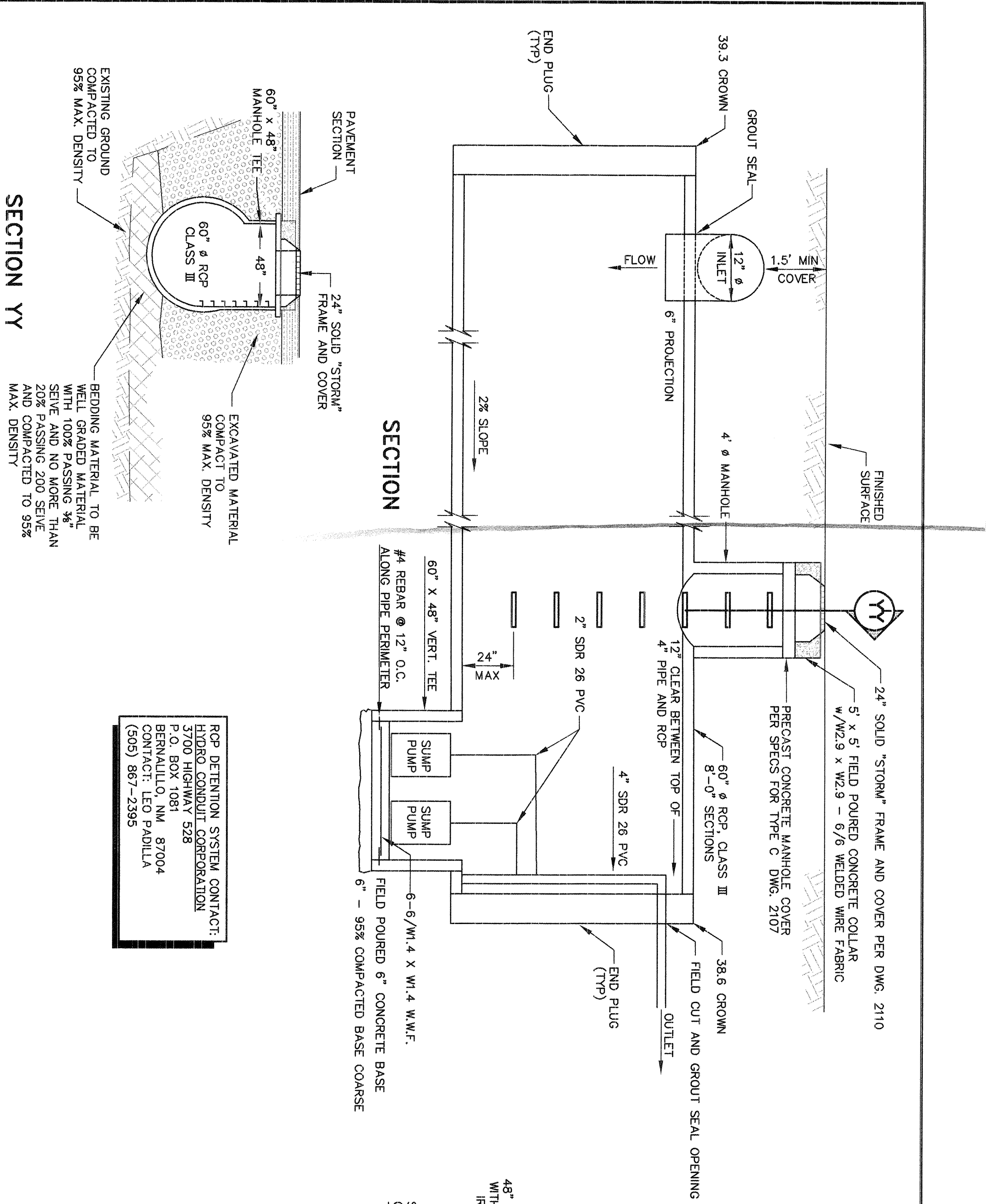


SURVEYORS CERTIFICATE

I HEREBY CERTIFY THAT THIS TOPOGRAPHIC SURVEY AND THE NOTES SHOWN HEREON WERE PREPARED UNDER MY DIRECTION FROM A SURVEY PERFORMED IN ACCORDANCE WITH THE RULES AND REGULATIONS OF THE BOARD OF SURVEYORS OF THE STATE OF NEW MEXICO. THIS TOPOGRAPHIC SURVEY MEETS THE REQUIREMENTS OF THE "MINIMUM STANDARDS FOR LAND SURVEYING IN NEW MEXICO."

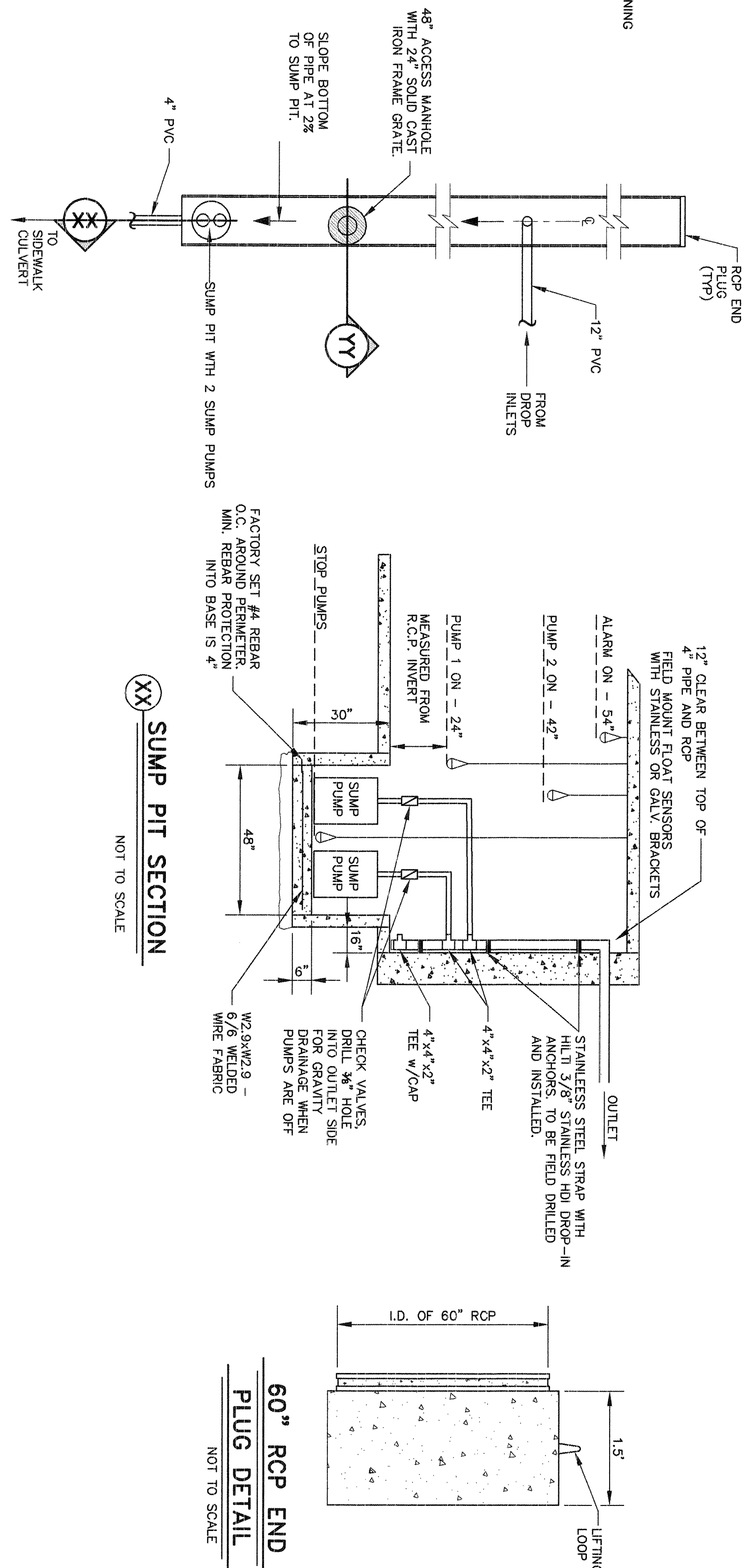
DATE 3-6-98
REG. PROF. SURVEYOR NEW MEXICO NO. 12804
GREGORY L. STEINER





RCP DETENTION SYSTEM CONTRACT:
 SUDAS CONSULT CORPORATION
 3700 HWY 528
 P.O. BOX 1081
 BERNALILLO, NM 87004
 CONTACT: LEO PADILLA
 (505) 867-2395

60" RCP DETENTION SYSTEM
 NOT TO SCALE



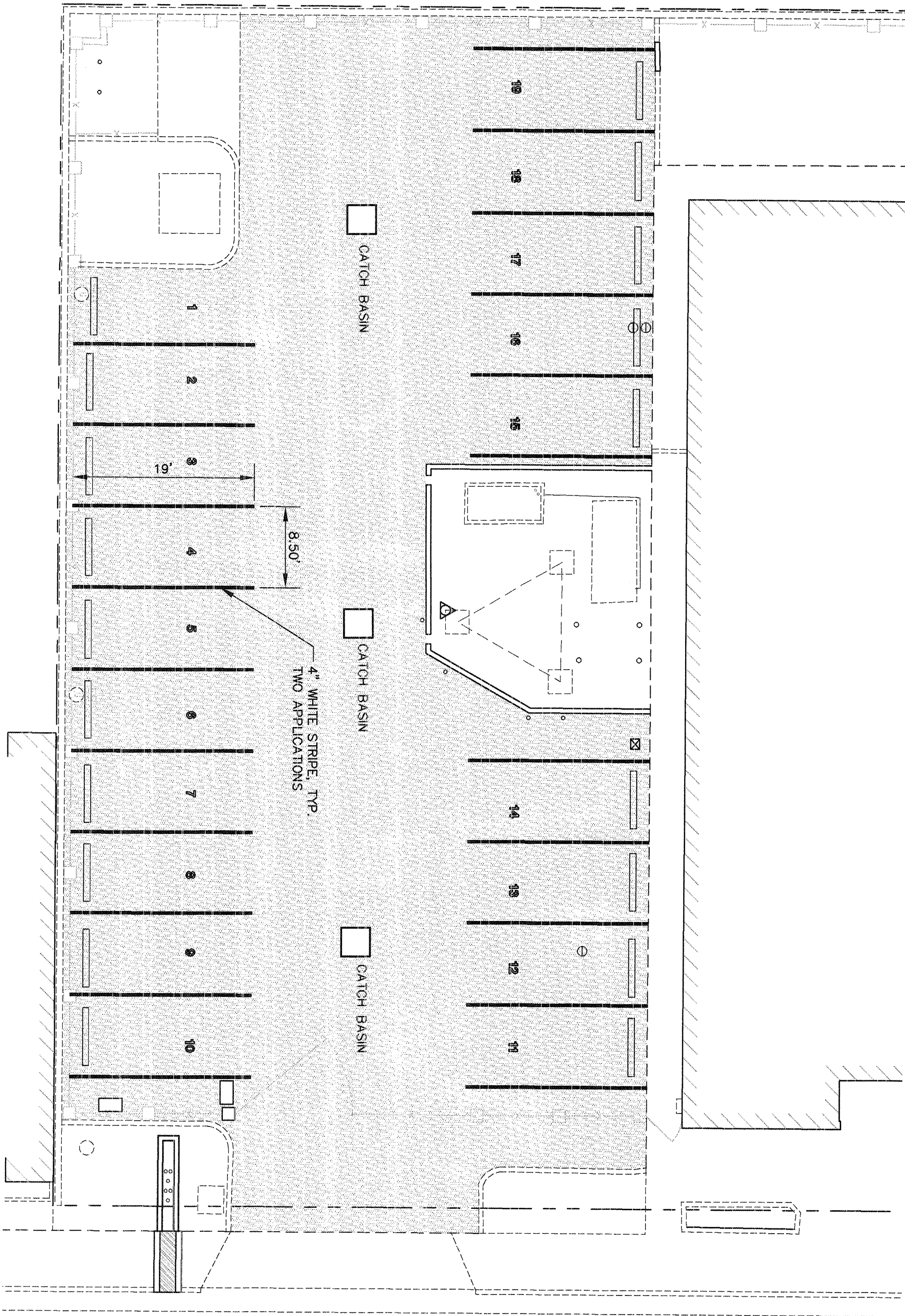
60" RCP END PLUG DETAIL
 NOT TO SCALE

GENERAL NOTES:

- CITY OF ALBUQUERQUE SPECIFICATIONS FOR PUBLIC WORKS CONSTRUCTION, LATEST EDITION, SHALL BE THE STANDARD SPECIFICATION REFERRED TO HEREIN AS THE "STANDARD SPECIFICATION".
- WATER SHUTOFF REQUIREMENTS: THE CONTRACTOR SHALL BE RESPONSIBLE FOR TIMING AND COORDINATION OF WATER SHUTOFF AT LEAST FIVE (5) WORKING DAYS PRIOR TO CONSTRUCTION.
- TWO (2) WORKING DAYS PRIOR TO ANY EXCAVATION, CONTRACTOR MUST CONTACT THE PUBLIC UTILITY COMPANIES SERVING THIS SITE SO THAT THE EXISTING FACILITIES CAN BE LOCATED BY THOSE ENTITIES.
- SHOULD A CONFLICT EXIST BETWEEN THESE PLANS AND ACTUAL FIELD CONDITIONS, THE CONTRACTOR SHALL PROMPTLY NOTIFY THE ENGINEER IN WRITING SO THAT THE CONFLICT CAN BE RESOLVED WITH A MINIMUM AMOUNT OF DELAY FOR ALL PARTIES.
- THE CONTRACTOR SHALL MAINTAIN ACCESS TO ALL BUILDINGS AND ADJACENT PROPERTIES DURING CONSTRUCTION.
- ALL WORK ON THIS PROJECT SHALL BE PERFORMED IN ACCORDANCE WITH APPLICABLE FEDERAL, STATE AND LOCAL LAWS, RULES AND REGULATIONS CONCERNING SAFETY AND HEALTH.
- IF THE REMOVAL OF EXISTING CURB AND GUTTER, SIDEWALK AND/OR PAVING IS REQUIRED, THE CONTRACTOR SHALL SAWCUT AND/OR REMOVE TO THE NEAREST JOINT. WHEN ADJOINING NEW PAVEMENT TO EXISTING, THE CONTRACTOR SHALL CUT BACK THE EXISTING PAVEMENT TO A STABLE, UNDISTURBED SUBGRADE. ANY EXISTING PAVEMENT OPENED TO PAVEMENT MARKING, EXISTING CURB AND GUTTER, SIDEWALK, PAVEMENT PAVEMENT MARKING, AND ANY OTHER CITY FACILITY NOT TO BE REMOVED UNDER THIS CONTRACT WHICH IS DAMAGED OR DISPLACED BY THE CONTRACTOR SHALL BE REMOVED AND REPLACED BY THE CONTRACTOR AT THE CONTRACTOR'S EXPENSE AND PER CITY STANDARDS.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR SAFETY OBTAINING THE REQUIRED COMPACTON. THE CONTRACTOR SHALL SELECT AND USE METHODS WHICH SHALL NOT BE INJURIOUS OR DAMAGING TO THE EXISTING FACILITIES AND STRUCTURES WHICH SURROUND THE WORK AREAS.
- THE CONTRACTOR SHALL NOTIFY THE CONSTRUCTION MANAGEMENT DIVISION OF THE PROPOSED COMMENCEMENT OF CONSTRUCTION AND PROPOSED WORK SCHEDULE FOR WORK WITHIN THE CITY R.O.W. AT LEAST 24 HOURS PRIOR TO THE ACTUAL COMMENCEMENT OF WORK. THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING ACCESS TO THE CONSTRUCTION SITE AT ALL TIMES DURING WORKING HOURS.
- THE OWNER SHALL BE RESPONSIBLE, THROUGH HIS ENGINEER, FOR MAKING ALL ENGINEERING PLAN CHANGES AND REVISIONS TO THE APPROVED AND SEALED ENGINEERING DRAWINGS. ALL CHANGES SHALL BE APPROVED BY THE CITY ENGINEER. THE CONTRACTOR SHALL MAINTAIN RECORDS.
- THE CONTRACTOR SHALL PROVIDE AN AREA TO STORE CONSTRUCTION MATERIALS AND EQUIPMENT TO BE LOCATED IN AN APPROPRIATE MANNER THAT WILL PREVENT SCATTERING. ALL DEBRIS INCLUDING TREES AND UNDERGROWTH SHALL BE DISPOSED OF PROPERLY WITHIN THE CITY LANDFILL. ALL DEBRIS SHALL BE REMOVED FROM THE SITE PRIOR TO FINAL SITE INSPECTION.
- THE CONTRACTOR SHALL CONFINED HIS OPERATIONS TO THE CONSTRUCTION LIMITS OF THE PROJECT AND IN NO WAY SHALL ENCROACH ONTO ADJACENT PROPERTIES UNLESS LEGAL EASEMENTS OR RIGHTS OF WAY HAVE BEEN OBTAINED. ANY ENCROACHMENT SHALL BE REMOVED AND THE PROPERTY LINE IN ACCORDANCE WITH CHAPTER 70 U.B.C. THE CONTRACTOR WILL BE HELD RESPONSIBLE FOR ANY AGREEMENT NECESSARY OR DAMAGE CAUSED BY CONSTRUCTION ACTIVITIES TO PUBLIC OR PRIVATE PROPERTY INCLUDING UTILITIES.
- THREE (3) WORKING DAYS PRIOR TO BEGINNING CONSTRUCTION THE CONTRACTOR SHALL SUBMIT TO THE CONSTRUCTION COORDINATION DIVISION A DETAILED CONSTRUCTION SCHEDULE. TWO (2) WORKING DAYS PRIOR TO CONSTRUCTION THE CONTRACTOR SHALL OBTAIN A DRAINAGING PERMIT FROM THE CONSTRUCTION COORDINATION DIVISION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS PRIOR TO OCCUPANCY AN INTERSECTION. SEE SECTION 29 OF THE SPECIFICATIONS.
- ALL STREET STRIPING, ALTERED OR DESTROYED SHALL BE REPLACED WITHIN 24 HOURS OF THE DATE OF REMOVAL. THE CONTRACTOR SHALL REPAIR AND RELOCATE PLASTIC PAVEMENT MARKINGS BY CONTRACTOR WITH RELOCATED TO LOCATION AS EXISTING OR AS INDICATED BY THIS PLAN SET.
- ANY WORK AFFECTING AN ARTERIAL ROADWAY REQUIRES 24 HOUR CONSTRUCTION.
- REMOVE EXISTING ASPHALT WITHIN DEFINED BOUNDARIES.
- R.C.P. TO BE 60" DIA. I.D. CLASS III
- SUMP PUMPS TO BE HYDROMATIC, MODEL SP505H, 1/4 H.P. (OR APPROVED EQUAL)
- REUSE EXISTING BOLLARDS
- REUSE EXISTING CURB STOPS AND INSTALL NEW AS NEEDED
- RESET LOWER AND RESET ALL UTILITY BOXES, S.A.S, CLEANOUTS, ELECTRIC BOXES, ETC.
- CONTRACTOR TO COORDINATE WITH CELLULARONE TO MAINTAIN ACCESS TO LOT DURING CONSTRUCTION
- GATE ON DRAINS TO BE MEFNAH CONVEX GRATE, R-3210 OR APPROVED EQUAL.
- BENCHMARK ON EXISTING TOWER CONCRETE PAD (SEE SHIT. C-3). ELEV. = 5242.72

CONSTRUCTION NOTES:

- REMOVE EXISTING ASPHALT WITHIN DEFINED BOUNDARIES.
- R.C.P. TO BE 60" DIA. I.D. CLASS III
- SUMP PUMPS TO BE HYDROMATIC, MODEL SP505H, 1/4 H.P. (OR APPROVED EQUAL)
- REUSE EXISTING BOLLARDS
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- BENCHMARK ON EXISTING TOWER CONCRETE PAD (SEE SHIT. C-3). ELEV. = 5242.72



STRIPING PLAN FOR PARKING AREA

SCALE: 1" = 10'

Red Mountain Engineers, Inc.
 Engineers • Surveyors

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PROJECT: **CELLULARONE OFFICE - Albuquerque, NM**

SHEET TITLE: **DETENTION SYSTEM DETAILS, STRIPING PLAN and GENERAL/CONSTRUCTION NOTES**

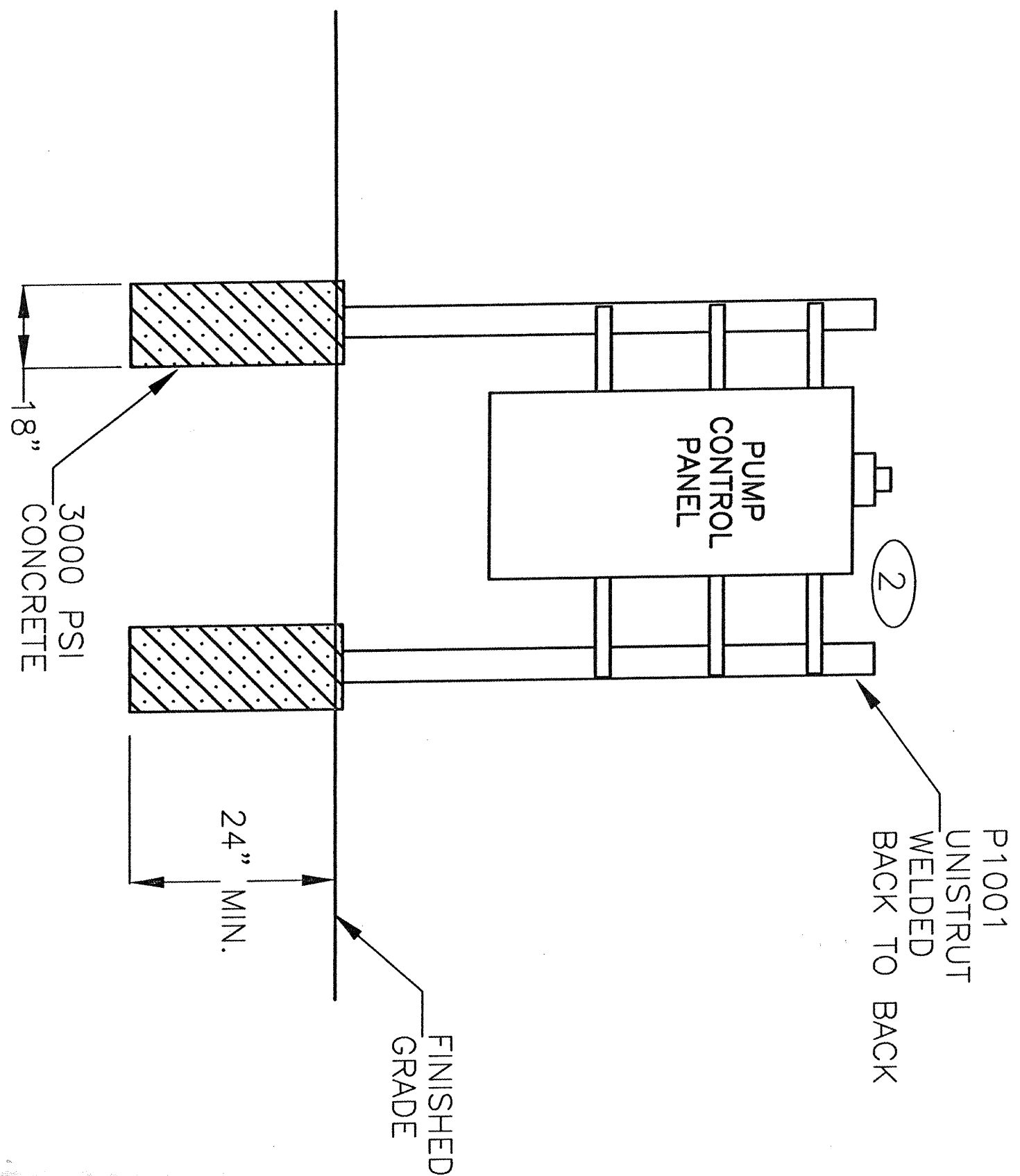


No.	REVISION	BY	APPVD.	DATE

PROJECT:	97813	DESIGN:	TSH
FILE NAME:	97813.dwg	CHECKED:	JEC
DATE:	2/2/98	DRAWN:	TSH,BRC
SCALE:	AS NOTED		

SHEET No. **C-4**

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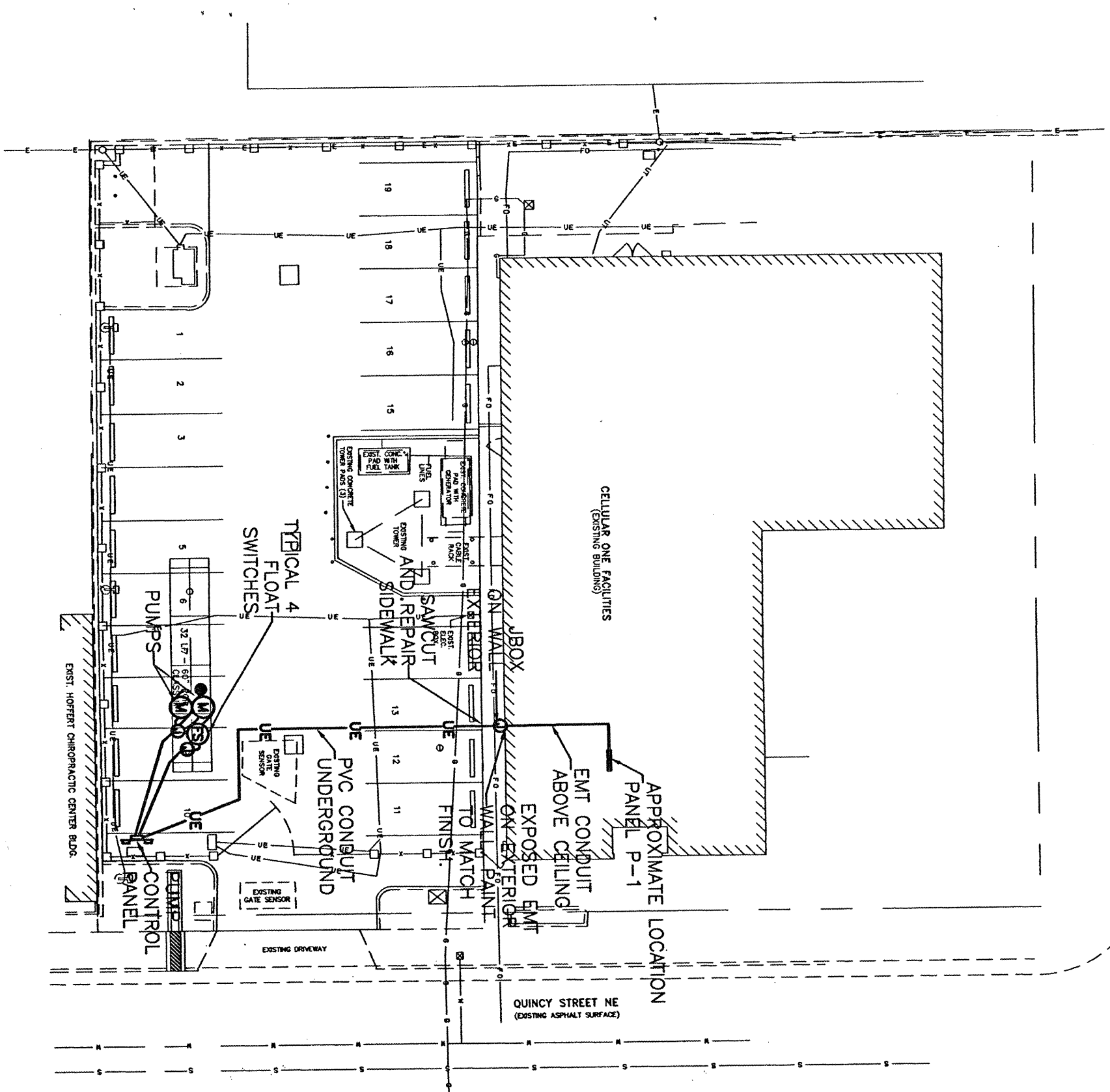


MOUNTING DETAIL

SCALE: NONE

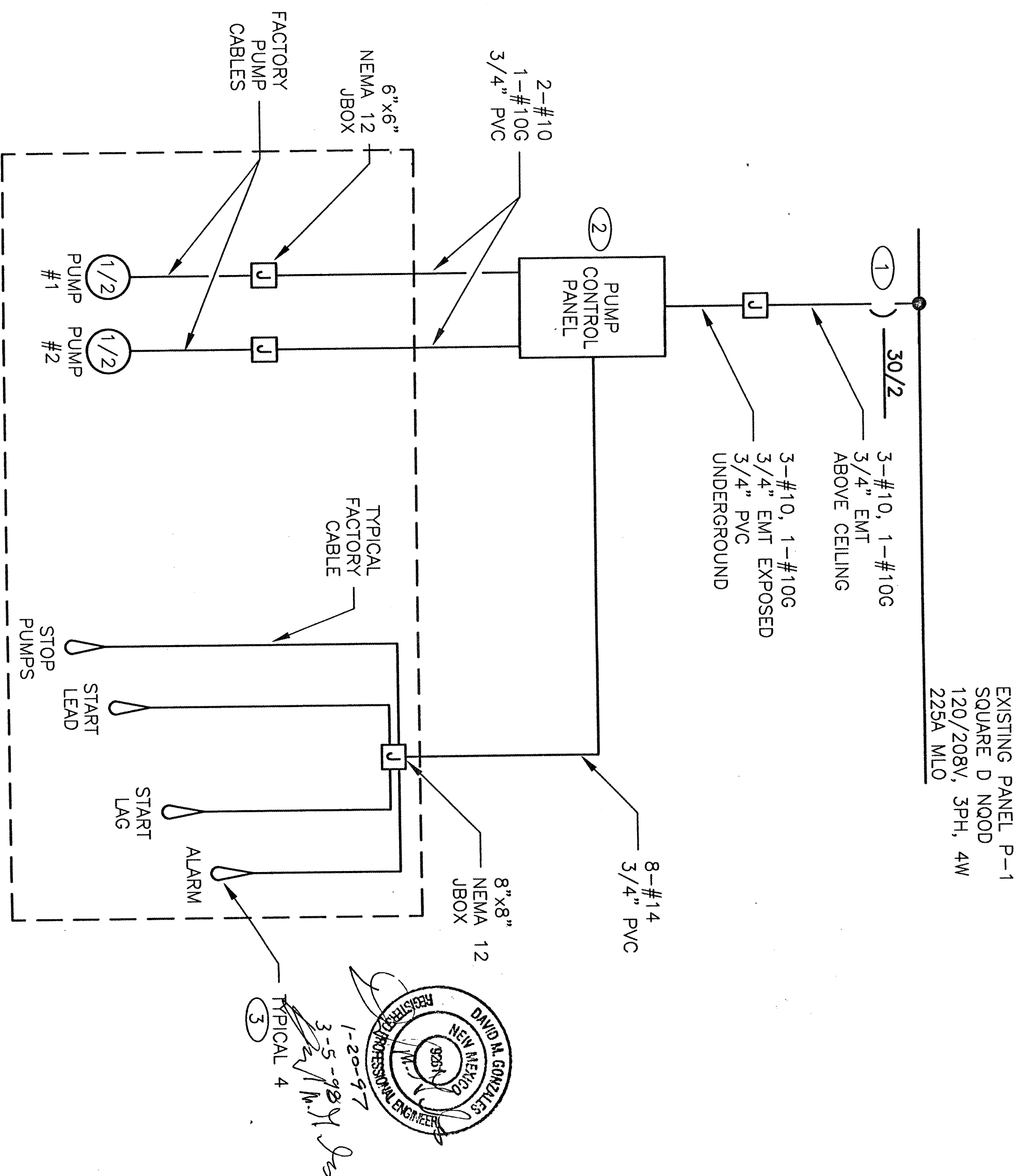
EQUIPMENT SCHEDULE

- ① FURNISH AND INSTALL NEW BOLT-ON CIRCUIT BREAKER IN EXISTING PANELBOARD.
- ② PUMP CONTROL PANEL: CONTROL OF TWO SUMP PUMPS FOR DUPLEX OPERATION. INCLUDE LEAD/LAG SELECTOR AND ALTERNATOR RELAY. PROVIDE MOTOR CONTACTORS, OVERLOADS, AND CONTROL RELAYS FOR AUTOMATIC OPERATION FROM LEVEL FLOATS. PROVIDE ALARM RELAY AND RED FLASHING BEACON. AUDIBLE ANNUNCIATOR NOT REQUIRED OR SHALL BE DISABLED. ALL COMPONENTS TO BE INSTALLED IN NEMA 3R ENCLOSURE. PANEL TO BE PROVIDED BY PUMP SUPPLIER. INCLUDE MEANS FOR MANUAL START/STOP OF EACH PUMP.
- ③ LEVEL FLOATS: FLYGT MERCURY SWITCH LEVEL FLOATS.



ELECTRICAL SITE PLAN

SCALE: 1" = 20'-0"



EXISTING PANEL P-
SQUARE D NQOD
120/208V, 3PH, 4W
225A MLO

CELLULAR ONE

145 QUINCY N.E.
ALBUQUERQUE, NEW MEXICO 87108

POWER ENGINEERING

1309 AGUA FRIA
SANTA FE, NEW MEXICO 87501

TEL (505) 982-7071
FAX (505) 982-2274

DATE:	1/
DRAWN BY:	
DESIGN BY:	
REVIEWED BY:	

